

**California High-Speed Rail Authority**



**RFP No.: HSR 14-32**

**Request for Proposals for Design-Build  
Services for Construction Package 4**

**Book III, Part A.1  
Design Criteria Manual**



# California High-Speed Train Project



## Design Criteria

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	Mar 2012	Initial Release, R0
1	Dec 2013	Rolling Stock Procurement
2	Feb 2014	Revision 2
3	Feb 2016	Revision 3





# Table of Contents

## **1 GENERAL**

- 1.1 Design Criteria Overview
- 1.2 Basis of Design
- 1.3 Regulations, Codes, Standards, and Guidelines
- 1.4 General Design Parameters
- 1.5 Durability
- 1.6 Noise Limitation
- 1.7 Vibration Limitation
- 1.8 Sustainability
- 1.9 Climatic Conditions
- 1.10 Elevation and Air Pressure
- 1.11 Air Quality

## **2 DESIGN SURVEY AND MAPPING**

- 2.1 Scope
- 2.2 Regulations, Codes, and Standards
- 2.3 Quality Assurance/Quality Control
- 2.4 Survey and Mapping
- 2.5 Right-of-Way Mapping
- 2.6 Design Survey and Mapping Deliverables

## **3 TRACKWAY CLEARANCES**

- 3.1 Scope
- 3.2 Regulations, Codes, Standards, and Guidelines
- 3.3 Clearances to Overhead and Adjacent Structures and Facilities
- 3.4 Vehicle Clearance Envelopes
- 3.5 Track Center Spacing
- 3.6 Other Clearance Requirements

**4 TRACK GEOMETRY**

- 4.1 Scope
- 4.2 Regulations, Codes, Standards, and Guidelines
- 4.3 Types of Rail Corridors
- 4.4 Horizontal Alignment
- 4.5 Vertical Alignment
- 4.6 Combined Horizontal and Vertical Curves
- 4.7 High-Speed Turnouts (60 mph and faster)
- 4.8 Low and Medium Speed Turnouts (50 mph and slower)
- 4.9 Non-Standard Turnouts
- 4.10 High-Speed Crossovers
- 4.11 Low and Medium Speed Crossovers
- 4.12 Double Crossovers (Scissors Crossovers)
- 4.13 Track Layout along Station Platforms
- 4.14 Access Tracks to Yards and Maintenance Facilities
- 4.15 Yards Tracks

**5 TRACKWORK**

- 5.1 Scope
- 5.2 Regulations, Codes, Standards, and Guidelines
- 5.3 Track Components and Materials
- 5.4 Rail
- 5.5 Rail Welding and Joints
- 5.6 Rail Fastenings
- 5.7 Other Track Material
- 5.8 Track Support Systems
- 5.9 Non-Ballasted Track
- 5.10 Ballasted Track
- 5.11 Turnouts and Other Special Trackwork Components
- 5.12 Grade Crossings

**6 ROLLING STOCK AND VEHICLE INTRUSION PROTECTION**

- 6.1 Scope
- 6.2 Regulations, Codes, Standards, and Guidelines
- 6.3 Protection of HST Operating Infrastructure from Vehicle Intrusion
- 6.4 Containment of HST Rolling Stock

**7 CIVIL**

- 7.1 Scope
- 7.2 Regulations, Codes, Standards, and Guidelines
- 7.3 Grading, Side Slopes, and Retaining Walls for Roadway and Site Embankments
- 7.4 Construction Site Best Management Practices
- 7.5 Slope/Surface Protection Systems Best Management Practices
- 7.6 Site Work – Non-Authority Facilities
- 7.7 Site Work – Authority Facilities
- 7.8 Access Control Devices
- 7.9 Wildlife Crossing
- 7.10 Maintenance and Protection of Traffic during Construction

**8 DRAINAGE**

- 8.1 Scope
- 8.2 Regulations, Codes, Standards, and Guidelines
- 8.3 Policies
- 8.4 Hydrological Analysis
- 8.5 Hydraulic Design
- 8.6 Trackway and Facility Drainage Systems
- 8.7 Detention/Retention of Surface Water Runoff
- 8.8 Storm Water Quality Management
- 8.9 Application of Approved Software

**9 UTILITIES**

- 9.1 Scope
- 9.2 Regulations, Codes, Standards, and Guidelines
- 9.3 Policies and Agreements
- 9.4 CHST Utilities
- 9.5 Non-CHST Utilities

**10 GEOTECHNICAL**

- 10.1 Scope
- 10.2 Regulations, Codes, Standards, and Guidelines
- 10.3 General Requirements
- 10.4 Subsurface Investigation and Data Analysis
- 10.5 Geotechnical Reports
- 10.6 Aerial Structure / Bridge Foundations and Stations / Miscellaneous Structures
- 10.7 Retaining Walls and Trenches
- 10.8 Embankments for HST Trackway
- 10.9 Cut Slopes
- 10.10 Existing Slopes
- 10.11 Cut-and-Cover Underground Structures
- 10.12 Seismic Design
- 10.13 Formation Supporting Track Structures

**11 SEISMIC**

- 11.1 Scope
- 11.2 Regulations, Codes, Standards, and Guidelines
- 11.3 Seismic Design and Analysis Plan
- 11.4 Design Classifications
- 11.5 Seismic Design Approach
- 11.6 Seismic Design Requirements
- 11.7 Bridges, Aerial Structures, and Grade Separations
- 11.8 Passenger Stations and Building Structures

**12 STRUCTURES**

- 12.1 Scope
- 12.2 Regulations, Codes, Standards, and Guidelines
- 12.3 Types of Structures
- 12.4 Structural Design Requirements
- 12.5 Permanent and Transient Loads and Load Factors for Structures Supporting HST
- 12.6 Track-Structure Interaction
- 12.7 Structural Design of Surface Facilities and Buildings
- 12.8 Design Considerations for Bridges and Aerial Structures
- 12.9 Requirements for Bridges not Supporting HST
- 12.10 Earth Retaining Structures
- 12.11 Cut-and-Cover Structures

- 12.12 Support and Underpinning of Structures
- 12.13 Areas of Potential Explosion
- 12.14 Structure Interface Issues

### **13 TUNNELS**

- 13.1 Scope
- 13.2 Regulations, Codes, Standards, and Guidelines
- 13.3 Basic High-Speed Train Tunnel Configuration and Geometry
- 13.4 Tunnel Portals
- 13.5 Tunnel Ventilation Requirements and Configurations
- 13.6 Materials
- 13.7 Fire Protection
- 13.8 Design Loads
- 13.9 Structural Analysis
- 13.10 Ground Movement
- 13.11 Measurement and Monitoring
- 13.12 Waterproofing
- 13.13 Drainage
- 13.14 Special Ground Conditions
- 13.15 System-wide Interfaces
- 13.16 Tunnel Portal Facilities
- 13.17 Portal Infrastructure Requirements for Different Categories of Tunnel

### **14 STATIONS**

- 14.1 Scope
- 14.2 Regulations, Codes, Standards, and Guidelines
- 14.3 Stations
- 14.4 Station Site Design
- 14.5 Landscaping

### **15 SUPPORT FACILITIES**

- 15.1 Scope
- 15.2 Regulations, Codes, Standards, and Guidelines
- 15.3 Support Facilities
- 15.4 Support Facilities Site Design

**16 MECHANICAL (TBD)**

**17 FACILITY POWER AND LIGHTING SYSTEMS**

- 17.1 Scope
- 17.2 Regulations, Codes Standards, and Guidelines
- 17.3 Description of the Facility Power System
- 17.4 Classifications of Electrical Loads
- 17.5 General Requirements
- 17.6 Supply Voltage
- 17.7 New Electrical Utility Services
- 17.8 Emergency Power Supply Systems
- 17.9 Sustainable Power Sources
- 17.10 Facility Power System Equipment
- 17.11 Specialized Electrical Equipment and/or Loads
- 17.12 Lighting System Requirements
- 17.13 Specialized Lighting
- 17.14 Raceway Systems
- 17.15 Wiring/Cabling Systems
- 17.16 Identification Requirements (Equipment, Conductors, Conduit, Tray)
- 17.17 Electrical Discrete and Serial I/O Point Requirements
- 17.18 Temporary Power and Lighting
- 17.19 Grounding, Bonding and Lightning Protection
- 17.20 Fire Detection System
- 17.21 Vehicular Traffic Management System
- 17.22 Facility Communication Systems
- 17.23 Spares

**18 FIRE PROTECTION (TBD)**

**19 BUILDING AUTOMATION AND MANAGEMENT SYSTEMS (TBD)**

**20 TRACTION POWER SUPPLY SYSTEM**

- 20.1 Scope
- 20.2 Regulations, Codes, Standards, and Guidelines
- 20.3 Overview and General Design Criteria
- 20.4 Key Design Criteria
- 20.5 Related Documentation
- 20.6 System Voltage and Frequency
- 20.7 System Configuration
- 20.8 Traction Electrification System Interfaces
- 20.9 Traction Power Facilities and Associated Sites
- 20.10 Electrical Materials
- 20.11 Emergency Power
- 20.12 Wayside Equipment
- 20.13 Calculations
- 20.14 New Power Utility Services
- 20.15 Spares

**21 OVERHEAD CONTACT SYSTEM AND TRACTION POWER RETURN SYSTEM**

- 21.1 Scope
- 21.2 Regulations, Codes, Standards, and Guidelines
- 21.3 Definitions
- 21.4 Overhead Contact System Description and General Performance Requirements
- 21.5 Environmental Conditions and Climatic Loading Requirements
- 21.6 Overhead Contact Line Design
- 21.7 Conductor Tensions
- 21.8 Catenary Conductors
- 21.9 Other Overhead Conductors and Cables
- 21.10 Dynamic Behavior and Quality of Current Collection
- 21.11 Current Capacity of Overhead Contact System
- 21.12 Sectionalizing and Switching
- 21.13 Insulation Coordination Requirements for OCS Installations
- 21.14 OCS Clearances and Protection against Electric Shock
- 21.15 OCS Structural Requirements
- 21.16 Traction Power Return System
- 21.17 OCS Interfaces with Other Disciplines

**22      GROUNDING AND BONDING REQUIREMENTS**

- 22.1 Scope
- 22.2 Regulations, Codes and Standards
- 22.3 General Grounding and Bonding Requirements
- 22.4 Maximum Permissible Step and Touch Potential
- 22.5 Grounding and Bonding Requirements for Facilities/Buildings and Structures
- 22.6 Grounding and Bonding Systems for the Traction Electrification System Equipment and Structures
- 22.7 Grounding and Bonding Protection Systems for the Automatic Train Control System Equipment and Structures
- 22.8 Grounding and Bonding Protection Systems for the Communications System Equipment and Structures
- 22.9 Grounding and Bonding Requirements for Facility Power Systems and Lighting Systems
- 22.10 Grounding Requirements for Raceway, Cable Tray, Underground Ductbanks, and Structures
- 22.11 Cables
- 22.12 Grounding and Bonding Requirements for Utilities
- 22.13 Lightning Protection

**23      CORROSION CONTROL**

- 23.1 Scope
- 23.2 Regulations, Codes, Standards, and Guidelines
- 23.3 Soil and Water Corrosion Control
- 23.4 Atmospheric Corrosion Control
- 23.5 Stray Current Sources and Protection

**24      AUTOMATIC TRAIN CONTROL**

- 24.1 Scope
- 24.2 Regulations, Codes, Standards, and Guidelines
- 24.3 General Design Requirements
- 24.4 Related Documentation
- 24.5 Automatic Train Control Functional Requirements
- 24.6 Grounding, Bonding, and Lightning Protection System
- 24.7 Onboard Equipment
- 24.8 Wayside Equipment
- 24.9 System Time
- 24.10 Maintenance and Diagnostic Functions
- 24.11 Automatic Train Supervision Subsystem



## **25 YARD SIGNALING**

- 25.1 Scope
- 25.2 Regulations, Codes, Standards, and Guidelines
- 25.3 General Design Requirements
- 25.4 Design Life
- 25.5 Environmental
- 25.6 Onboard Equipment
- 25.7 Wayside Equipment
- 25.8 System Configuration
- 25.9 Functional Requirements
- 25.10 Yard Signal Control System
- 25.11 Yard Signal Control System Hardware Requirements
- 25.12 Electromagnetic Compatibility
- 25.13 Reliability, Availability, and Maintainability Requirements
- 25.14 Software Requirements
- 25.15 Yard Safety Critical System Requirements
- 25.16 Maintenance and Diagnostic Functions
- 25.17 Yard Signal Control System Interfaces with Other Systems
- 25.18 Transition Tracks between Main Tracks and Yard Tracks
- 25.19 Maintenance of Infrastructure Yards

## **26 ELECTROMAGNETIC COMPATIBILITY AND INTERFERENCE**

- 26.1 Scope
- 26.2 Regulations, Codes, Standards, and Guidelines
- 26.3 Description of Electromagnetic Compatibility Program and Electromagnetic Interference
- 26.4 General Design Requirements
- 26.5 Overview of EMC Design Criteria Categories
- 26.6 Communications EMC Design Criteria
- 26.7 Automatic Train Control EMC Design Criteria
- 26.8 Traction Electrification System EMC Design Criteria
- 26.9 Rolling Stock EMC Design Criteria
- 26.10 Station and Facility Equipment EMC Design Criteria

**27 SUPERVISORY CONTROL AND DATA ACQUISITIONS SUBSYSTEMS**

- 27.1 Scope
- 27.2 Regulations, Codes, Standards, and Guidelines
- 27.3 Related Documentation
- 27.4 Overview of the SCADA Subsystems and Related Communications Systems
- 27.5 SCADA Subsystems

**28 COMMUNICATIONS**

- 28.1 Scope
- 28.2 Regulations, Codes, Standards, and Guidelines
- 28.3 Related Documentation
- 28.4 Communications Systems
- 28.5 Communications Physical Support Rooms, Enclosures, Shelters and Equipment
- 28.6 External Services

**29 ROLLING STOCK – CORE SYSTEMS INTERFACES**

- 29.1 Scope
- 29.2 Regulations, Codes, Standards, and Guidelines
- 29.3 General Interface Design Requirements
- 29.4 Traction Power Interfaces
- 29.5 Overhead Contact System Interfaces
- 29.6 Automatic Train Control Interfaces
- 29.7 Communications Interfaces

**30 RELIABILITY, AVAILABILITY, MAINTAINABILITY, AND SAFETY (TBD)**

**31 SYSTEMS ASSURANCE (TBD)**

**32 SYSTEM SAFETY AND SECURITY**

- 32.1 Safety and Security Policy Statement
- 32.2 Scope
- 32.3 Regulations, Codes, Standards, and Guidelines
- 32.4 Goals
- 32.5 Hazard Management
- 32.6 Threat and Vulnerability Management
- 32.7 Safety and Security Certification

# Chapter 1

## General

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	13 Mar 12	Initial Release, R0
1	May 2013	Revision 1
2	Sept 2015	Revision 2
3	Feb 2016	Revision 3



# Table of Contents

<b>1</b>	<b>GENERAL .....</b>	<b>1-1</b>
1.1	Design Criteria Overview .....	1-1
1.1.1	Purpose and Extent.....	1-1
1.1.2	Design Standard Classifications .....	1-1
1.1.3	Revisions.....	1-1
1.1.4	Other California High-Speed Train Project Documents .....	1-2
1.2	Basis of Design.....	1-2
1.2.1	Infrastructure .....	1-2
1.2.2	Traction Power .....	1-3
1.2.3	Train Control and Communications.....	1-3
1.2.4	Rolling Stock .....	1-3
1.2.5	Design and Operating Speeds .....	1-3
1.2.6	Trip Travel Times .....	1-3
1.2.7	Comfort.....	1-3
1.2.8	Safety and Reliability.....	1-4
1.3	Regulations, Codes, Standards, and Guidelines .....	1-4
1.3.1	Regulations and Codes.....	1-4
1.3.2	Standards and Guidelines.....	1-5
1.3.3	Other Jurisdiction .....	1-7
1.3.4	Precedence of Conflicting Requirements.....	1-8
1.4	General Design Parameters .....	1-9
1.4.1	Units of Measurement .....	1-9
1.4.2	Milepost and Stationing.....	1-9
1.4.3	Design Variances .....	1-9
1.4.4	Professional Licensing Requirement.....	1-10
1.4.5	Third Party Facilities.....	1-10
1.4.6	Design Life .....	1-10
1.4.7	Standardization .....	1-12
1.5	Durability.....	1-12
1.6	Noise Limitation .....	1-12
1.7	Vibration Limitation .....	1-13
1.8	Sustainability .....	1-13
1.8.1	Demonstration of High Performance Design .....	1-13
1.8.2	Water Conservation .....	1-13
1.8.3	Energy Conservation.....	1-13
1.8.4	Green or Renewable Energy.....	1-14
1.9	Climatic Conditions .....	1-14
1.9.1	Temperature.....	1-14
1.9.2	Relative Humidity .....	1-14
1.9.3	Rainfall .....	1-14
1.9.4	Thunderstorms .....	1-14
1.9.5	Wind .....	1-17
1.9.6	Dust Storms.....	1-18
1.9.7	Fog .....	1-18
1.10	Elevation and Air Pressure .....	1-18

1.11	Air Quality .....	1-18
------	-------------------	------

## **Tables**

Table 1-1:	Document Revision History .....	1-2
Table 1-2:	Design Life .....	1-11
Table 1-3:	Weather Conditions by Segment.....	1-16
Table 1-4:	Maximum Wind Gust for Locations near Alignment .....	1-17

## Acronyms

Authority	California High-Speed Rail Authority
CHST	California High-Speed Train
CHSTP	California High-Speed Train Project
mph	miles per hour





# 1 General

## 1.1 Design Criteria Overview

### 1.1.1 Purpose and Extent

This document establishes criteria, guidelines and requirements for the design of Infrastructure and Systems elements of the California High-Speed Train Project (CHSTP). Additional guidelines are required for the design of facilities of other owners/operations.

These criteria include design survey and mapping, trackway clearances, track geometry, trackwork, rolling stock and vehicle intrusion protection, civil, drainage, utilities, geotechnical, seismic, structures, tunnels, stations, support facilities, facility power and lighting systems, traction power supply systems, overhead contact system and traction power return system, grounding and bonding requirements, corrosion control, automatic train control, yard signaling, electromagnetic compatibility and interface, supervisory control and data acquisition subsystems, communications, rolling stock-core systems interfaces and, safety and security.

The intended use of these criteria is for final design of the CHSTP, unless indicated otherwise.

### 1.1.2 Design Standard Classifications

The following terms are used to classify the criteria:

- **Recommended** – Standard to be equaled or exceeded where there are no major physical, cost or schedule constraints. Designers should use ‘Recommended’ values to the extent practical.
- **Minimum/Maximum** – Represent limits. Designers shall make every effort to avoid the use of minimum/maximum values. These values are acceptable where constraints make the use of ‘Recommended’ values impracticable.
- **“Shall”** – Indicates mandatory requirement that must be strictly implemented. Waiver is permissible only under approval of design variance.
- **“Should”** – Indicates preferred course of action. Design variance is not required if it is not exercised.
- **“May”** – Indicates permissible course of action within the limits of the standards. Design variance is not required if it is not exercised.

### 1.1.3 Revisions

Revisions of these criteria, in part or in whole, will be issued by the Authority or its representative. It is the responsibility of the user to base their design on the latest applicable version of these criteria. Refer to the General Provisions for related requirements. Table 1-1

- 1 illustrates a Design Criteria revision history. Proposed revisions to the CHSTP Design Criteria  
 2 may be submitted to the California High-Speed Rail Authority (Authority).

**Table 1-1: Document Revision History**

Revision Number	Revision Date (DD MMM YY)	Summary Description
0	DD MMM YY	Initial Release

3

#### 1.1.4 Other California High-Speed Train Project Documents

4 The following CHSTP documents may be considered with these Design Criteria:

- 5 • Standard Specifications
- 6 • Standard Drawings
- 7 • Directive Drawings
- 8 • CADD Manual
- 9 • Plan Preparation Manual
- 10 • Design Variance Request Process
- 11 • California High-Speed Rail Authority and Federal Railroad Administration. 2005. *Final*
- 12 *Program Environmental Impact Report/Environmental Impact Statement for the Proposed California*
- 13 *High-Speed Train System.*
- 14 • *Bay Area to Central Valley Program Environmental Impact Report/Environmental Impact Statement*

## 1.2 Basis of Design

15 The California High-Speed Train (CHST) system is an electrified steel-wheel-on-steel-rail  
 16 system with standard gauge tracks that shall meet the following physical, functional, and  
 17 performance requirements.

### 1.2.1 Infrastructure

- 18 • Fully grade-separated crossings
- 19 • Dual-track on the mainline along the entire length with separate station stopping tracks
- 20 • Fully access-controlled railway with intrusion detection monitoring systems and intrusion
- 21 protection systems as required

### 1.2.2 Traction Power

- Electric traction system – 2 x 25 kV, 60 Hz autotransformer power supply system
- Capable of accommodating a minimum of 12 double trainsets per hour per direction

### 1.2.3 Train Control and Communications

- Designed to support 3-minute headways
- An automatic train control system targeted to be equivalent to the European Railway Traffic Management System (ERTMS) standard with capability for operating at speeds of up to 220 mph, subject to FRA approval.
- Equipped with high-capacity and redundant communications systems capable of supporting fully automated train operations.

### 1.2.4 Rolling Stock

- Trainsets using a distributed traction power configuration, approximately 660 feet in length capable of coupling to provide 1,320-foot-long double trainsets
- Seating capacity for approximately 450 to 500 passengers per 660-foot trainset (900 to 1,000 passengers for a 1,320-foot trainset)

### 1.2.5 Design and Operating Speeds

A design speed of 250 mph is required where cost-effective and where topographic, geometric, operational, and environmental conditions permit. The design shall allow for sustained operating speed of 220 mph.

In areas where shared-use track is anticipated, such as San Francisco–San Jose and Los Angeles–Anaheim, the maximum design speed is 125 mph.

### 1.2.6 Trip Travel Times

Governing legislation and other legal documentation dictate characteristics of the CHST system. The requirements are identified in Division 3, Chapter 20 of the Streets and Highways Code.

### 1.2.7 Comfort

The CHST system shall provide passengers with a comfortable ride experience:

- Ride comfort (smoothness of ride) shall be achieved with lateral and vertical acceleration values equal to or less than 0.05g and 0.045g, respectively, for the maximum design speeds.
- Noise vibration and temperature are discussed in subsequent sections of these criteria.

### 1.2.8 Safety and Reliability

Trains, facilities, systems and operational processes shall be designed, constructed, and implemented in a manner that promotes the safety and security of persons and property. The design, construction, testing, and start-up of the CHSTP shall comply with applicable safety and security laws, regulations, requirements and railroad industry practices. Refer to the *System Safety and Security* chapter.

The CHST system shall:

- Conform to the requirements of applicable U.S. federal, state, and local laws, governing rules and regulations.
- Be fully grade separated at crossings and fully access-controlled.
- Incorporate supervisory control and data acquisition system.
- Incorporate climatic and seismic monitoring systems.

## 1.3 Regulations, Codes, Standards, and Guidelines

The following sections list the system-wide, international, federal, state, and industry-specific, regulations, codes, standards, and guidelines that have been used in development of these criteria. In addition, the regulations, codes, standards, and guidelines pertaining to each area of design are listed at the beginning of its corresponding chapter. These lists may not include all those applicable. It is the designer's responsibility to determine additional regulations, codes, and standards that are applicable.

Additional, applicable requirements by authorities having jurisdiction over the design, construction, and operation of the facility shall be identified and applied as required for approval.

See General Provisions for precedence of Regulations, Codes, Ordinances, and Standards.

Unless a specific publication edition is identified, the latest edition shall apply.

Deviations from the criteria stated herein require approval from the Authority.

### 1.3.1 Regulations and Codes

Federal and state regulations and codes govern passenger and freight rail systems in the U.S. These regulations are typically the basis of design and govern the operation of conventional rail networks and are neither specifically applicable to the basis of design nor do they govern the operation of HST systems with speeds over 150 mph. As such, international regulations and codes provide additional guidance. Other regulations and codes apply to the design of CHST buildings and facilities and are not specific to the CHST system.

### 1.3.1.1 International Regulations and Codes

- Technical Specifications for Interoperability (TSI) concerning Trans-European High-Speed Rail
- European Standards (EN for European Norms)
  - European Committee for Standardization (CEN)
  - European Committee for Electrotechnical Standardization (CENELEC)
  - European Telecommunications Standard Institute (ETSI)
- International Union of Railways (UIC) Code

### 1.3.1.2 Federal and National Regulations and Codes

- Americans with Disabilities Act (ADA)
- Americans with Disabilities Standards for Transportation Facilities
- Code of Federal Regulations (CFR), specifically Title 47, Title 49 Part 37 and Parts 200-299, and Title 28 Part 36.
- U.S. Environmental Protection Agency (EPA) Laws, Regulations, Guidance and Dockets, and Executive Orders
- National Electric Code (NEC)
- National Electrical Safety Code (NESC)
- National Fire Protection Association (NFPA) Codes and Standards

### 1.3.1.3 State Regulations and Codes

- California Building Standards Code (CBSC), Title 24 of CCR
- California Business and Professions Code
- California Code of Regulations (CCR)
- California Public Utilities Commission (CPUC) General Orders (GOs)
- California Streets and Highways Code

### 1.3.2 Standards and Guidelines

Standards have been developed by governments, industries, and operators for design and construction to ensure consistency and compatibility among various elements of a rail system. In some cases, fulfillment of standards may be required to secure regulatory approvals from the U.S. Army Corps of Engineers, Division of the State Architect, Office of the State Fire Marshall, California Coastal Commission, Caltrans, and other agencies and authorities.

### 1.3.2.1 International Standards and Guidelines

- 1 • International Electrotechnical Commission (IEC)

### 1.3.2.2 Federal Standards and Guidelines

- 2 • Federal Railroad Administration (FRA) Standards and Guidelines
- 3 • Federal Emergency Management Agency (FEMA) Guidelines
- 4 • Federal Highway Administration (FHWA) Guidelines
- 5 • National Earthquake Hazards Reduction Program (NEHRP)
- 6 • U.S. Army Corps of Engineers Guidelines
- 7 • U.S. Bureau of Land Management Surveying Manual
- 8 • United States Geological Survey (USGS) Standards

### 1.3.2.3 State Standards and Guidelines

- 9 • California Disabled Accessibility Guidebook (CalDAG)
- 10 • California Seismic and Safety Commission Standards and Guidelines
- 11 • California Occupational Safety and Health Administration (Cal/OSHA) Standards
- 12 • California Department of Transportation (Caltrans)
  - 13 – Caltrans Bridge Design Manuals, including Bridge Design Specification (CBDS), Bridge
  - 14 Design Practices Manual (CBPD), Bridge Design Aids Manual (CBDA), Bridge Design
  - 15 Details Manual (CBDD), Bridge Memo to Designers Manual (CMTD), Standard
  - 16 Specifications, Standard Plans, Seismic Design Memorandum, Caltrans Seismic Design
  - 17 Criteria (CSDC)
  - 18 – Caltrans Highway Design Manual
  - 19 – Caltrans Plans Preparation Manual and other guidelines for report preparation
  - 20 – Caltrans Project Development Procedures Manual
  - 21 – Caltrans Standard Plans
  - 22 – Caltrans Surveys Manual
  - 23 – Caltrans Transportation Management Planning Guidelines
  - 24 – Caltrans User's Guide to Photogrammetric Products and Services
  - 25 – Caltrans Right of Way Manual, and Forms and Exhibits
  - 26 – Caltrans Stormwater Quality Handbooks
- 27 • Other Right of Way Publications
- 28 • California Manual on Uniform Traffic Control Devices (MUTCD)

#### 1.3.2.4 Industry Standards and Guidelines

- 1 • American Association of State Highway and Transportation Officials (AASHTO) Guidance
- 2 • American Concrete Institute (ACI) Building Code Requirements
- 3 • American Institute of Steel Construction (AISC) Steel Construction Manual
- 4 • American Public Transit Association (APTA) Guidance
- 5 • American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for
- 6 Railway Engineering, Portfolio of Trackwork Plans, and Communications and Signals
- 7 Manual of Recommended Practices
- 8 • American Society for Photogrammetry and Remote Sensing (ASPRS) Manual
- 9 • ASTM International (ASTM) Standards
- 10 • American Society of Civil Engineers (ASCE) Guidelines
- 11 • American Welding Society (AWS) Codes
- 12 • American National Standards Institute (ANSI) Standards
- 13 • American Standard Code for Information Interchange (ASCII)
- 14 • American Society of Heating, Refrigerating and Air-Conditioning Engineers (ASHRAE)
- 15 Guidelines
- 16 • American Society of Mechanical Engineers (ASME) Standards and Guidelines
- 17 • Underwriters Laboratories (UL) Standards
- 18 • Institute of Electrical and Electronics Engineers (IEEE) Standards
- 19 • Association of American Railroads (AAR) Standards

#### 1.3.3 Other Jurisdiction

20 Additional regulations, codes, standards, and guidelines may need to be considered along with  
21 site-specific permit and operational requirements. The following are representative agencies,  
22 organizations and services that may have specific design standards and specifications,  
23 operational and facility requirement that may need to be considered in the design of CHST  
24 facilities.

- 25 • Air Quality Districts
- 26 • Bicycle Coalitions
- 27 • City, County, Municipal, Codes, and Ordinances
- 28 • City, County, Municipal Utilities Codes, and Standards
- 29 • Congestion Management Agencies
- 30 • County Transportation Authorities

- 1 • Departments of Public Works
- 2 • Fire Departments
- 3 • Police Departments
- 4 • Emergency Responders
- 5 • Freight Railroads, including:
  - 6 – BNSF Railway Engineering Standards
  - 7 – Union Pacific (UP) Railroad Engineering Standards
- 8 • Flood Control Districts
- 9 • Passenger Rail and local Transit Agencies, including:
  - 10 – Amtrak Standards and Guidelines
  - 11 – Peninsula Corridor Joint Powers Board (Caltrain) Design Criteria and Engineering
  - 12 Standards
  - 13 – Southern California Regional Rail Authority (SCRRA) Engineering Standards
- 14 • Parks and Recreation Departments
- 15 • Public Utilities Commission(s)
- 16 • Regional Comprehensive Planning Agencies
- 17 • Regional Council of Governments
- 18 • Regional Environment Agencies and Commissions
- 19 • Regional Water Quality Control Boards
- 20 • School Districts
- 21 • Waste Management entities

### **1.3.4 Precedence of Conflicting Requirements**

#### **1.3.4.1 Precedence by Jurisdiction**

- 22 CHSTP design criteria are specifically applicable to HST facilities within the Authority right-of-  
23 way and on tracks on which high-speed trains operate.
- 24 Where HSTs operate within another (Third Party) railroad's or transportation's right-of-way,  
25 the design requirements of the Authority and the Third Party shall apply.
- 26 The design of facilities owned by a Third Party located outside the Authority's right-of-way  
27 shall comply with the requirements prescribed by the Third Party.



Where Third Party infrastructure is located within the Authority’s right-of-way, the more stringent of the requirements shall apply as required to achieve concurrence of the Authority and Third Party.

#### **1.3.4.2 Precedence by Type of Requirement**

In general, applicable regulations and codes take precedence over standards and guidelines. In the case of differing values between the regulations, codes, standards, and guidelines the criterion followed shall be that which results in the satisfaction of all applicable requirements.

Refer to the specific design criteria for applicability of international regulations codes and standards.

#### **1.3.4.3 Precedence of CHSTP Requirements**

See General Provisions for precedence of documents for each Contract Package.

See General Provisions for precedence of design criteria and design standards for Shared Corridor configurations.

See General Provisions for resolution of conflicts for each Contract Package.

### **1.4 General Design Parameters**

#### **1.4.1 Units of Measurement**

The CHST system shall be based on U.S. Customary Units, defined by the National Institute of Standards and Technology (NIST). This is consistent with guidelines prepared by Caltrans.

Dual units have been used in these Design Criteria under the following condition: If the source document uses metric units, then the U.S. Customary Units are shown with the metric unit equivalent shown in parenthesis.

Design and construction drawings shall be developed in accordance with the CHSTP CADD Manual.

#### **1.4.2 Milepost and Stationing**

Milepost and stationing shall be as defined in the CHSTP Plan Preparation Manual.

#### **1.4.3 Design Variances**

Approved design variances are required for design elements that do not meet the limiting (maximum/minimum) design criteria. Requests for variances to design criteria shall follow the CHSTP Design Variance Request Process.

Design variances and exceptions to a Third Party’s design standards shall follow the Third Party’s design exception process.

- 1 Where a design variance is required for both the CHSTP and Third Party, the design exception  
2 process for both entities shall be followed.

#### **1.4.4 Professional Licensing Requirement**

- 3 Documents such as drawings, specifications, and calculations intended for the construction of  
4 CHST facilities shall be prepared under the supervision and approval of a licensed professional  
5 in accordance with the requirements of Title 16 of the CCR and shall be subject to the limitations  
6 of the licensing laws of the state of California.

#### **1.4.5 Third Party Facilities**

- 7 Unless otherwise stated in the contract documents, the design, construction, replacement or  
8 alteration of Third Party facilities shall be done in-kind and in conformance with the published  
9 standards of the authority having jurisdiction.

#### **1.4.6 Design Life**

- 10 Design life for the CHST system infrastructure and systems elements are presented in Table 1-2.  
11 These values are intended as baseline requirements for use in defining and assessing design and  
12 development standards and requirements, alternative materials and designs, and operational  
13 and maintenance activities.

**Table 1-2: Design Life**

Infrastructure	Design Life
Track and Civil Works, including: <ul style="list-style-type: none"> <li>• Site, earthwork, line layout, storm drainage</li> <li>• Concrete slab</li> </ul> with the exception of: <ul style="list-style-type: none"> <li>• Track, including rails, ties/clips and ballast</li> <li>• Roadway, pavement, parking facilities</li> <li>• Switches and Turnouts</li> </ul>	100 years   50 years 40 years 30 years
Structures, including: <ul style="list-style-type: none"> <li>• Underground structures</li> <li>• Above-ground facilities, including bridges, aerial structures, passenger stations, ventilation buildings</li> <li>• Retaining Walls</li> <li>• Components of the grounding, bonding, and lightning protection system embedded within concrete structures</li> </ul> with the exception of: <ul style="list-style-type: none"> <li>• Support Facilities</li> <li>• Movement Expansion joints, bearings</li> </ul>	100 years     50 years 50 years
Mechanical, Electrical, Plumbing, Ventilation and Fire Protection Systems	30 years
Systems	
Traction Power Systems, including: <ul style="list-style-type: none"> <li>• Traction power supply system (TPS)</li> <li>• Overhead contact system (OCS) support structures and conducts, with the exception of the contact wire, the life of which is dependent upon the number of pantograph passes.</li> <li>• Grounding, bonding, and lightning protection system</li> </ul>	50 years
Train Control and Communications System, including: <ul style="list-style-type: none"> <li>• ATC systems</li> <li>• Yard signal systems and their subsystems</li> <li>• Equipment and supporting cabling</li> <li>• Supervisory Control and Data Acquisition</li> <li>• Communications wired and wireless data transport systems</li> <li>• Communications administrative, control and timing systems</li> <li>• Communications safety, security and fire detection systems</li> <li>• Communications copper and fiber optic cable infrastructure and associated equipment</li> </ul>	30 years
Other technology-based systems: <ul style="list-style-type: none"> <li>• Equipment and non-safety critical, microcontrollers, computers, software and similar commercial off-the-shelf (COTS) equipment</li> </ul>	10 years

RFP No.: HSR 14-32 – EXECUTION VERSION

#### 1.4.7 Standardization

Design shall use standard materials and equipment where possible. Standardization ensures ease of procurement and inventory management, minimizes staff training, optimizes maintenance, and avoids long lead times for materials, equipment, and components.

Equipment and materials shall meet industry standards, be available off the shelf to the extent possible, and supplied by established manufacturers. Selection of equipment and materials shall consider long-term costs, ease of construction and maintenance, and readily available technical support.

#### 1.5 Durability

Design shall assess potential for deterioration of materials and assemblies, including deterioration specific to exposure to the environment. Materials and detail assemblies shall be durable with minimal maintenance and repairs throughout their design life. For surfaces and assemblies for which appearance is important, durability shall include maintenance required to preserve appearance. Design shall take into account the following aspects of durability:

- Control of moisture
- Control of corrosion (including material compatibility)
- Control of ultraviolet light exposure
- Control of exposure to industrial and vehicular pollution
- Minimize damage from wear and tear
- Ease of repair

#### 1.6 Noise Limitation

Noise from construction and operation of the CHST system and ancillary sources shall not exceed limits defined in the following:

- CHSTP Program and Project-specific Environmental Impact Report/Environmental Impact Statement
- FRA's High-Speed Ground Transportation Noise and Vibration Impact Assessment Guideline
- Applicable noise laws and regulations

## 1.7 Vibration Limitation

Vibration from construction and operation of HSTs and ancillary sources shall not exceed limits defined in the following:

- CHSTP Program and Project-specific Environmental Impact Report/Environmental Impact Statement
- FRA’s High-Speed Ground Transportation Noise and Vibration Impact Assessment Guideline
- Applicable vibration laws and regulations

## 1.8 Sustainability

### 1.8.1 Demonstration of High Performance Design

Stations and maintenance facility buildings shall be designed in accordance with CalGreen Code mandatory and voluntary measures, at minimum. “Voluntary Measures” shall be understood to be mandatory. The goal for station and maintenance facility buildings is to optimize the design in regard to site design, energy, water, materials use, indoor air quality, construction practices, and management. A means to demonstrate this optimization may be to use the Leadership in Energy and Environmental Design (LEED), the Living Buildings Challenge, or other appropriate assessment methodology. If such an assessment is used, the rating shall be the maximum feasible.

### 1.8.2 Water Conservation

The CHST system shall, at a minimum, use water conservation and efficiency guidelines in CalGreenCode mandatory and voluntary sections for all planning, procurement, design, construction, operations, and maintenance of facilities. The goal for facilities is, where appropriate to the climate, to work toward potable water self-sufficiency through consumption reduction, recycling, and on-site capture and storage. Stormwater should be either managed onsite to supply the facility’s internal water demands and landscaping, or released for management through acceptable natural time-scale surface flow, groundwater recharge, agricultural use or adjacent building needs.

### 1.8.3 Energy Conservation

The CHST system shall incorporate energy conservation and efficiency features and operating procedures into the facilities and operating systems to the maximum extent feasible. CHST stations and maintenance facility buildings shall be designed to achieve net-zero site energy, as measured over the course of a year.

#### 1.8.4 Green or Renewable Energy

In operation, the Authority will produce or procure enough renewable energy to feed into the California grid, equal to the amount facilities and train operation consume. To help meet this goal, design shall specify the use of building-based and on-site renewable technologies (such as solar, geothermal, wind, biomass and biogas) to the extent feasible.

### 1.9 Climatic Conditions

Climatic conditions necessary for design, including those that are site-specific, shall be researched and considered by the designer. Facility- and operations-specific temperatures or conditions are prescribed in the corresponding chapters. Some design elements will consider system-wide extremes and others will be based on the conditions in a specific location.

Table 1-3 presents a summary of corridor climatic conditions by segment. Analyses of climatic conditions shall consider the extreme conditions in the corridor as a whole and/or a particular section of a corridor, as applicable.

In addition to historical climate data, accommodate, to the extent practical, climate trends as they become apparent such as greater extremes in temperature and precipitation.

#### 1.9.1 Temperature

Areas closer to the Pacific Ocean generally experience more moderate temperatures and less variation in temperature from day to night and summer to winter.

#### 1.9.2 Relative Humidity

Relative humidity varies across the state and by time of year. Coastal areas generally have medium to high humidity year round. Inland, humidity is generally high during the winter and low during the summer. In general, areas further from the ocean have a lower relative humidity. The corridor near Palmdale is in the westernmost range of the Mojave Desert and has low humidity year-round. Weather patterns also influence humidity.

#### 1.9.3 Rainfall

Precipitation for a single day indicates the general rainfall intensity that infrastructure and rolling stock will encounter. The highest record maximum precipitation measured in a day along the alignment occurred in the San Gabriel Range (between Palmdale and Los Angeles). Additional guidance necessary for hydrologic design, such as rainfall intensity, duration and frequency is covered in the *Drainage* chapter.

#### 1.9.4 Thunderstorms

Thunderstorms are relatively uncommon within California but may occur at any time of year. The Bay Area, the Central Coast, and much of the Central Valley experience an average of less

1 than 5 days with thunderstorms each year. The rest of the state experiences 5 to 10.4 mean  
2 number of days with thunderstorms per year. Many thunderstorms produce little precipitation,  
3 and fires may result from lightning strikes. When heavy precipitation occurs, flash flooding  
4 may result. Severe hail events are uncommon although hail up to 1/2-inch diameter has been  
5 reported.

6

Table 1-3: Weather Conditions by Segment

	Record Extreme Maximum Temperature (°F)	Record Extreme Minimum Temperature (°F)	Mean Number of Days with Freezing Temperatures	Mean Maximum Daily Precipitation (inches)	Annual Record Total Snowfall (inches)	Mean Maximum Daily Snowfall (inches)	Annual Fastest Mile of Wind (mph)	Annual Mean Occurrence of Gust >50 mph	Annual Mean Number of Days with Heavy Fog
San Francisco – San Jose	106–110°	11–20°	0.5–30.4	2.01–2.50"	2.1–6.0"	0.1–3.0"	<b>41–45</b>	<b>2.5–3.4</b>	15.5–20.4
San Jose –Merced	111–115°	11–20°	30.5–60.4	2.01–2.50"	6.1–12.0"	3.1–6.0"	<b>41–45</b>	0.5–1.4	25.5–30.4
Merced – Fresno	<b>116–120°</b>	11–20°	30.5–60.4	1.00–1.50"	2.1–6.0"	0.1–3.0"	<b>41–45</b>	0.5–1.4	<b>30.5–35.4</b>
Fresno –Bakersfield	111–115°	11–20°	30.5–60.4	1.00–1.50"	0.1–2.0"	0.1–3.0"	<b>41–45</b>	0.5–1.4	<b>30.5–35.4</b>
Bakersfield –Palmdale	111–115°	<b>–9–0°</b>	<b>90.5–120.4</b>	1.51–2.00"	<b>48.1–72.0"</b>	<b>12.1–15.0"</b>	<b>41–45</b>	0.5–1.4	20.5–25.4
Palmdale – Los Angeles	111–115°	1–10°	30.5–60.4	<b>3.01–3.50"</b>	12.1–24.0"	6.1–9.0"	<b>41–45</b>	0.5–1.4	15.5–20.4
Los Angeles –Anaheim	111–115°	21–32°	0.5–30.4	2.01–2.50"	0.0"	0.0"	<b>41–45</b>	0.5–1.4	20.5–25.4
Los Angeles –San Diego	111–115°	11–20°	30.5–60.4	2.51–3.00"	0.1–2.0"	0.1–3.0"	<b>41–45</b>	0.5–1.4	<b>30.5–35.4</b>
Sacramento –Merced	111–115°	11–20°	30.5–60.4	1.51–2.00"	0.1–2.0"	0.1–3.0"	<b>41–45</b>	1.5–2.4	<b>30.5–35.4</b>
Altamont	111–115°	11–20°	30.5–60.4	1.51–2.00"	0.1–2.0"	0.1–3.0"	<b>41–45</b>	1.5–2.4	25.5–30.4

This data is included as general information and not for use in application of these design criteria.

**Source:** National Climatic Data Center (NCDC), National Oceanic and Atmospheric Administration (NOAA). Climate Atlas of the United States: Data Documentation. April 2010.  
<http://www.ncdc.noaa.gov/oa/about/cdrom/climatls2/datadoc.html>

**Weather Condition Definitions:**

Record Extreme Maximum Temperature – Highest temperature recorded in the segment

Record Extreme Minimum Temperature – Lowest temperature recorded in the segment

Mean Number of Days with Freezing Temperatures – Number of days per year on average that temperatures in the segment are below 32°F (maximum value for the segment)

Mean Maximum Daily Precipitation – Maximum precipitation in one day during an average year (maximum value for the segment)

Annual Record Total Snowfall – Maximum amount of snowfall recorded over one year in the segment (maximum value for the segment)

Mean Maximum Daily Snowfall – Maximum snowfall in one day during an average year (maximum value for the segment)

Annual Fastest Mile of Wind – Average speed obtained during the passage of one mile of wind (maximum value for the segment)

Annual Mean Occurrence of a Gust > 50 mph – Frequency of gusts of over 50 mph in 1 year during an average year(maximum value for the segment)

Annual Mean Number of Days with Heavy Fog – Frequency of days with fog resulting in visibility of less than 0.25 miles in an average year(maximum value for the segment)

**Notes:**

1. Data is provided in ranges consistent with the source data. Specific values will fall within the range provided by more discrete information is not provided.
2. Numbers in bold represent system-wide extreme (maximum/minimum)
3. NCDC archives weather data from the National Weather Service, Military Services, Federal Aviation Administration, the Coast Guard, and volunteer observers. NCDC has a database of U.S. climate data and maps that portray the climate of the U.S. by such elements as temperature, precipitation, snow, wind, and pressure. The period of record for most of this data is 1961 to 1990.  
National Climatic Data Center, National Oceanic and Atmospheric Administration. Climate Maps of the United States. <http://cdo.ncdc.noaa.gov/cgi-bin/climaps/climaps.pl>



### 1.9.5 Wind

California is within a zone of prevailing westerly wind and is east of the semi-permanent high pressure area of the northeast Pacific Ocean. The basic flow of air is from the west or northwest for most of the year. However, mountains and other local terrain deflect and funnel these winds and wind direction may reflect other local factors besides prevailing circulation. During certain times of year, strong, gusty, dry winds with speeds that may exceed 100 mph flow from the east into the Central Valley and southern California; this wind is commonly referred to as “Santa Ana Wind.” A similar condition leads to “northers” in the Sacramento and San Joaquin Valleys, which lead to warmer temperatures and a dry, persistent wind.

The fastest mile of wind, or average speed obtained during the passage of one mile of wind, is the metric for measuring constant wind speeds and normalizes the erratic nature of wind flow. For all segments, the annual fastest mile of wind is 41 to 45 mph. Maximum wind gusts at locations along the alignment are provided in Table 1-4.

Tornados have been reported in California but only about once or twice a year. They are rarely severe.

**Table 1-4: Maximum Wind Gust for Locations near Alignment**

Monitoring Location	Gust (mph)
San Francisco International Airport	74
Mountain View/ Moffett Naval Air Station	64
Fresno	54
Bakersfield	46
Edwards Air Force Base	75
Palmdale	63
Los Angeles City Hall	49
Santa Ana/El Toro Marine Corps Air Station	68
Riverside/March Air Force Base	56
Camp Pendleton Marine Corp Air Station	51
San Diego	64
Stockton	46
Sacramento	74

**Notes:** The climatic wind data contained in this summary was extracted from the NCDC’s Local Climatological Data publication, Navy and Air Force climatic briefs, and other sources. The total period of this summary is 1930-1996. The period of record (POR) for which wind data is summarized varies for individual sites and may begin and end at any time during the 1930-1996 period. All available wind data is provided regardless of POR or source.  
<http://www.ncdc.noaa.gov/oa/mpp/wind1996.pdf>

### 1.9.6 Dust Storms

Dust storms occur occasionally along the HST alignment. Dust storms require dry and windy conditions and are most common near dry lake beds and agricultural areas. They are least common in highly urban areas. Dust storms have historically been the most severe in the Central Valley. They also occur in conjunction with wildfires and agricultural burning. Wildfires and other large fires are relatively common and may result in smoky conditions far away from the location of the fire.

### 1.9.7 Fog

Heavy fog occurs throughout the state, resulting in visibility less than 0.25 miles.

### 1.10 Elevation and Air Pressure

Elevation along the alignment rises above sea level approximately 3,000 feet across Tehachapi Pass, 3,225 feet across the San Gabriel Mountains and 1,300 feet across Pacheco Pass. Air pressure varies widely based on elevation and weather conditions.

### 1.11 Air Quality

State air quality condition data is gathered and disseminated by the California Air Resources Board (CARB). Summary air quality conditions are provided by county and by air basin. The alignment includes the following California air basins:

- Sacramento Valley
- San Francisco Bay Area
- San Joaquin Valley
- Mojave Desert
- South Coast
- San Diego County

Emissions that are tracked by CARB include ozone, carbon monoxide, carbon dioxide, hydrogen sulfate, methane, NO<sub>x</sub>, PM<sub>2.5</sub>, PM<sub>10</sub>, sulfur dioxide, and lead. Historic air quality data is available through CARB's Air Quality Data Query Tool (<http://www.arb.ca.gov/aqmis2/aqdselect.php>).

## Chapter 2

### Design Survey and Mapping

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	07 Mar 12	Initial Release, R0
1	May 2013	Revision 1



## Table of Contents

<b>2</b>	<b>DESIGN SURVEY AND MAPPING .....</b>	<b>2-1</b>
2.1	Scope .....	2-1
2.2	Regulations, Codes, Standards, and Guidelines .....	2-1
2.3	Quality Assurance/Quality Control .....	2-1
2.4	Survey and Mapping .....	2-2
2.4.1	Horizontal Datum.....	2-2
2.4.2	Vertical Datum.....	2-2
2.4.3	California State Plane Coordinates and Combination Factors.....	2-2
2.4.4	Accuracy and Standards.....	2-3
2.4.5	Survey Control.....	2-3
2.4.6	Photogrammetric and Topographic Mapping Surveys .....	2-4
2.4.7	Digital Terrain Model.....	2-5
2.4.8	Third Party Topographic Mapping Requirements.....	2-5
2.5	Right-of-Way Mapping .....	2-6
2.5.1	Property Surveys and Right-of-Way Mapping.....	2-6
2.5.2	Right-of-Way Mapping Requirements.....	2-6
2.6	Design Survey and Mapping Deliverables.....	2-7

## Tables

Table 2-1:	Topographic Mapping; Preliminary and Final Design .....	2-5
------------	---	-----

## Acronyms

AGPS	Airborne Global Positioning System
Authority	California High-Speed Rail Authority
Caltrans	California Department of Transportation
CHSTP	California High-Speed Train Project
DTM	Digital Terrain Model
GPS	Global Positioning System
PLS	Professional Land Surveyor
QA/QC	Quality Assurance/Quality Control

## 2 Design Survey and Mapping

### 2.1 Scope

This chapter provides design survey and mapping criteria for the compilation of topographic mapping and land surveying procedures, performing engineering design land surveys, and generally describes right-of-way mapping requirements. Particular emphasis is placed on accurate field surveys of existing topographic and man-made features required for mapping, and the establishment of accurate terrain models used for design and the development of earthwork quantities.

### 2.2 Regulations, Codes, Standards, and Guidelines

Refer to the *General* chapter for requirements pertaining to regulations, codes, standards, and guidelines. Surveys for the California High-Speed Train Project (CHSTP) in general will be based on the California Department of Transportation (Caltrans) manuals and guides. Where applicable, the basis for design survey and mapping standards shall be the most appropriate of the recommended practices described in the following documents:

- CHSTP Plan Preparation Manual
- California Department of Transportation (Caltrans)
  - Caltrans Surveys Manual
  - Caltrans User's Guide to Photogrammetric Products and Services
  - Caltrans Plans Preparation Manual
  - Caltrans Right of Way Manual
- Bureau of Land Management - Manual of Surveying Instructions (2009)
- U.S. Geological Survey (USGS) - National Map Accuracy Standards
- American Society of Photogrammetry and Remote Sensing - Manual of Photogrammetry

### 2.3 Quality Assurance/Quality Control

All design survey and mapping and right-of-way engineering products shall be prepared under the responsible charge and seal of a Professional Land Surveyor, or person registered as a civil engineer prior to January 1, 1982 (pursuant to the California Business and Professions Code, Section 6731 of the Professional Engineers' Act and Section 8731 of the Professional Land Surveyors' Act) licensed in the State of California. All design survey and mapping products used for construction, appraisal and property acquisition, and associated record document

1 tracking and history shall be prepared in accordance with appropriate Quality  
2 Assurance/Quality Control (QA/QC) procedures.

3 A review for the technical correctness of all design surveys and mapping shall be performed  
4 using an approved QA/QC procedure. System-wide survey control monumentation has been  
5 established by the California High-Speed Rail Authority (Authority) for the CHSTP and will be  
6 maintained during design and construction activities by the Authority.

## **2.4 Survey and Mapping**

7 Design surveys and mapping shall be based on existing system-wide primary and secondary  
8 survey control monuments. Geodetic system-wide surveys for the CHSTP have been  
9 established. Positional survey data, both horizontal and vertical, for all existing survey control  
10 monuments is on file in the office of the Authority and shall be used for all supplemental design  
11 surveys and mapping. Design surveys and mapping shall include milepost and location and  
12 stationing.

### **2.4.1 Horizontal Datum**

13 The horizontal positions of all existing CHSTP survey control monuments are based on the  
14 North American Datum of 1983, NAD83 (NSRS2007) as defined by the National Geodetic  
15 Survey. The epoch date for all established monument positions is 2007.00. All design surveys  
16 and mapping shall include a reference to the horizontal datum NAD83 (NSRS2007), and shall  
17 include a reference to the epoch date 2007.00. All supplemental design surveys and mapping  
18 shall be based on direct horizontal ties to the existing CHSTP survey control monuments.

### **2.4.2 Vertical Datum**

19 The elevations, also known as orthometric heights, of all existing CHSTP survey control  
20 monuments are based on the North American Vertical Datum of 1988 (NAVD88) as defined by  
21 the National Geodetic Survey (NGS). All design surveys and mapping shall include a reference  
22 to the vertical datum NAVD88. All supplemental design surveys and mapping shall be based  
23 on the elevations and direct vertical ties to the existing CHSTP survey control monuments. Any  
24 surveys using Global Positioning System (GPS) methods shall use the model GEOID09 for all  
25 processing and shall also be based on direct ties to existing CHSTP survey control monuments.  
26 The use of precise differential levels shall be used for all critical structures and shall be based on  
27 the elevations for the existing survey control monuments established by the Authority.

### **2.4.3 California State Plane Coordinates and Combination Factors**

28 Design surveys and mapping, right-of-way engineering, appraisal maps, design and  
29 construction for the CHSTP shall be based on the California Coordinate System of 1983 (CCS83),  
30 Zone 2 through Zone 6, as defined by the National Geodetic Survey, described in California  
31 Public Resources Code commencing at Section 8801. The required use of these State Plane Zones



for mapping, also require the use of Combination Factors. The Combination Factors allow for the conversion of horizontal zone (grid) distances to ground level distances and are especially needed for survey construction purposes. All design surveys and mapping shall show the most appropriate Combination Factor as needed. The Combination Factor for each existing survey control monument has been established by the Authority and shall be used for all design surveys and mapping. The Combination Factor for each survey control monument is shown on documents titled Survey Control Monument Data Sheet, on file in the Office of the Authority.

#### 2.4.4 Accuracy and Standards

Surveys performed shall be developed with the accuracy, standards, and field procedures as defined in the Caltrans Surveys Manual. Surveys shall conform to the specifications of accuracy per Caltrans Figure 5-1A titled "Caltrans Orders of Survey Accuracy". Applicable chapters of the Caltrans Surveys Manual include the following:

- Chapter 2 – Safety
- Chapter 5 – Classifications of Accuracy and Standards
- Chapter 6 – Global Positioning System (GPS) Survey Specifications
- Chapter 7 – Total Station Survey System (TSSS) Survey Specifications
- Chapter 8 – Differential Leveling Survey Specifications
- Chapter 9 – Control Surveys
- Chapter 10 – Right of Way Surveys
- Chapter 11 – Engineering Surveys
- Chapter 12 – Construction Surveys
- Chapter 13 – Photogrammetry

Conversion of units from U.S. Survey Foot to international meter may be necessary in order to apply standards listed above. This unit conversion factor is defined as follows:

- 1 U.S. Survey foot = 1200/3937 meters –California Public Resources Code Section 8810

#### 2.4.5 Survey Control

A geodetic control survey has been established for the CHSTP and consists of a network of existing primary survey control monuments at approximate 10-mile intervals. Additional secondary survey control monuments have been established at approximate 2-mile intervals. The existing survey control monuments established by the Authority shall be used both horizontally and vertically for all design surveys and mapping to ensure such surveys and maps are based on the same ground control and that all maps for the CHSTP, based on the California Coordinate System of 1983 (California Public Resources Code, beginning at Section

8801) are uniform and accurate. The network of survey control monuments will be maintained by the Authority.

#### **2.4.6 Photogrammetric and Topographic Mapping Surveys**

Different levels of mapping accuracy are appropriate for preliminary and advanced design. Two levels of topographic mapping may be used on the project.

- Photogrammetric mapping at 1"=100' scale controlled using Airborne GPS (AGPS) methods and generating a digital terrain model (DTM) mapping with 2-foot contour intervals is adequate for preliminary design. This mapping will be used to develop pre-final alignments and to determine earthwork quantities for estimating purposes. Field verification by ground level surveying shall be required as part of the mapping effort in order to establish that the product has met the specified accuracy standards. A technical work product report shall be prepared by the mapping team and independently checked by the design teams.
- Fully-controlled photogrammetry generating 1"=50' scale mapping with 1-foot contour intervals to National Mapping Accuracy Standards shall be used by the design teams where greater accuracy for mapping is needed to support detailed design. Field surveys shall be used to check mapping accuracy, match points, bridge and structure sites, critical alignment points, and other CHSTP facilities. The mapping shall meet Caltrans technical standards but shall not follow Caltrans review procedures.

Color digital orthorectified imagery shall be provided as a by-product to the photogrammetry deliverable. Establishment of field survey ground control is required for all photogrammetry and photogrammetric mapping services for 1"=50' design scale mapping. Field verification by ground level surveying is required as part of the mapping effort in order to establish that the product has met the specified accuracy standards. A report shall be prepared and independently checked.

Topographic mapping shall adhere to U.S. National Map Accuracy Standards. Topographic mapping requirements for preliminary and final design are summarized in Table 2-1 herein.

**Table 2-1: Topographic Mapping; Preliminary and Final Design**

	Map Scale/ Contour Interval	Max Width of Mapping Coverage	Flying Height / Photo Scale	Expected Resolution	Control	Comment
Preliminary design	1"=100' / 2'	3,780'	3600'± AMT / 1:7200, 1"=600'	0.35', pixel	AGPS may be appropriate, with field verification	Detailed planimetrics not obtainable (water valves, manholes, fences, etc.)
Final design	1"=50' / 1'	1,890'	1800'± AMT / 1:3600, 1"=300'	0.20', pixel	Fully-controlled with multiple ground targets and field verification. AGPS not appropriate	This mapping shall meet Caltrans technical standards for design scale mapping

1 Notes: AMT = Above Mean Terrain

#### 2.4.7 Digital Terrain Model

- 2 Topographic aerial mapping, supplemented by field surveys where needed, shall be used to  
3 develop a DTM that mathematically defines the existing ground surface conditions. DTM  
4 surfaces shall be used as the basis for defining vertical alignments, preparation of cross-sections,  
5 and the generation of earthwork quantities.
- 6 The DTM shall be checked for accuracy and adjusted in accordance with Caltrans  
7 photogrammetry requirements. The DTM shall be provided in a format compatible with the  
8 project design software requirements.

#### 2.4.8 Third Party Topographic Mapping Requirements

- 9 Caltrans, railroad operators, municipalities, and utilities generally have unique mapping  
10 standards. Available, existing maps as well as new maps prepared by and/or for use by third  
11 parties may need to be supplemented, converted, translated, and otherwise adjusted when  
12 intended for use on the CHSTP and enabling projects. Any such adjusted maps used for the  
13 CHSTP shall conform to the CHSTP datum as specified in Sections 2.4, 2.4.1, 2.4.2, 2.4.3, 2.4.4  
14 and 2.4.5. The quality and accuracy of all such adjusted maps shall be developed to achieve the  
15 standards and consistency of the approving agency. All maps requiring modification, shall be  
16 converted, translated and adjusted both horizontally and vertically, under the direction of a  
17 Professional Land Surveyor as specified in Section 2.3, and shall be positionally equivalent to  
18 the CHSTP datum and the existing survey control monuments established by the Authority.

## 2.5 Right-of-Way Mapping

1 The proposed right-of-way envelope shall be based on the project footprint and shall consider  
2 the site topography, drainage, ditches, retaining walls, service/access roads, utilities, the nature  
3 of the structure and side slopes selected, and other CHSTP facilities.

### 2.5.1 Property Surveys and Right-of-Way Mapping

4 During preliminary design, assessor's parcel maps and data from County Geographic  
5 Information Systems may be sufficient for preliminary design right-of-way mapping.  
6 Preliminary right-of-way base maps will be prepared using the project footprint. These maps  
7 will be used to identify parcels required for the CHSTP, including full takes and provide  
8 information on partial takes to be used for real estate appraisal purposes. Right-of-way data  
9 sheets shall be prepared showing parcel numbers, owner's names, acquisition type, areas, and  
10 other information critical for appraisal estimates. Preliminary design should identify properties  
11 that may need special attention due to ownership, impacts, or other considerations, so that  
12 design and acquisition efforts can be prioritized. Full takes and parcels with willing sellers may  
13 be acquired at this stage provided that all state and federal requirements are met.

14 Right-of-way appraisal maps shall be prepared for the alignment. These maps will be used to  
15 identify the property requirements at a scale defined in the CHSTP Plan Preparation Manual.  
16 The tasks involved in preparing these maps include review of title reports, record maps, grant  
17 deeds, property surveys, boundary analysis, and preparation of legal descriptions and exhibits  
18 for property acquisition. A Professional Land Surveyor, or person registered as a civil engineer  
19 prior to January 1, 1982 (pursuant to the California Business and Professions Code, Section 6731  
20 of the Professional Engineers' Act and Section 8731 of the Professional Land Surveyors' Act)  
21 shall be in responsible charge of these tasks. Where the project crosses or otherwise impacts the  
22 right-of-way of various agencies (i.e., Caltrans, city and county governments, railroads, utilities,  
23 etc.), right-of-way maps shall be prepared as required for the affected agency's approval. Where  
24 maps are not required, data showing recordation of parcels purchased by legal descriptions and  
25 deeds shall be shown on the Right-of-Way Record Map. Right-of-way engineering performed  
26 during final design and construction shall include monument preservation activities, final  
27 monumentation of the right-of-way post construction, and preparation of Records of Surveys as  
28 required by the California Land Surveyors' Act.

### 2.5.2 Right-of-Way Mapping Requirements

29 Requirements for right-of-way mapping shall be prepared in general conformance with  
30 Caltrans policies and procedures. Additional mapping requirements and notes are as follows:

- 31 • Right-of-way Appraisal Maps shall include limits and types of acquisition required,  
32 property information for affected and adjacent owners, encroachments, vacations,  
33 easements required for construction, excess lands and other required interests and uses.

- 1 • No spiral curves shall be used to define any lines for right-of-way purposes. Spiral curves  
2 shall only be used for track alignments. Circular curves are the only types of curves  
3 acceptable for legal descriptions and recording purposes. All curve data and tangent data  
4 shall be shown on the right-of-way maps. Tangent lines may be used to show the limits of  
5 the right-of-way when the curved track alignments are extremely flat, provided that proper  
6 clearance requirements are met. Lines and curves shall always be tangent.
- 7 • Right-of-way maps shall show the right-of-way envelope as being continuous and crossing  
8 public land as well as private land.
- 9 • The limits for the easement areas supporting all new construction (i.e., vent shafts, traction  
10 power facilities, etc.) shall be defined geometrically with a minimum of 2 ties shown  
11 wherever the location is not contiguous to the right-of-way.
- 12 • Separate drawings for vacation or transfer purposes showing the areas of public property or  
13 public right-of-way to be closed and utilized for the CHSTP shall be provided. These  
14 drawings shall be prepared in accordance with local jurisdictional requirements.
- 15 • All necessary survey data shall be shown including monuments, positions, descriptions, line  
16 and curve data, offset distances, areas, names, labels, notes, symbols, north arrows, stations,  
17 equations, centerline alignment data, milepost, etc for the purpose of acquiring and  
18 maintaining right-of-way. All data and references contained in legal descriptions shall be  
19 shown on all right-of-way maps.

## 2.6 Design Survey and Mapping Deliverables

20 All design surveys and mapping deliverables, supporting documents, and data whether  
21 completed or in process, shall be submitted to the Authority.



## Chapter 3

### Trackway Clearances

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
0.a	26 Oct 12	Revised Appendixes
1	Jun 2013	Revision 1





## Table of Contents

<b>3</b>	<b>TRACKWAY CLEARANCES .....</b>	<b>3-1</b>
3.1	Scope .....	3-1
3.2	Regulations, Codes, Standards, and Guidelines .....	3-1
3.3	Clearances to Overhead and Adjacent Structures and Facilities .....	3-2
3.3.1	Vertical Clearances .....	3-2
3.3.2	Horizontal Clearances .....	3-2
3.3.3	Clearances to Third Party Facilities .....	3-3
3.4	Vehicle Clearance Envelopes .....	3-3
3.4.1	Description of Envelopes .....	3-3
3.4.2	Structure Gauge for Non-Electrified Tracks .....	3-4
3.4.3	Effects Due to Curve Radius .....	3-4
3.4.4	Effects of Superelevation .....	3-5
3.5	Track Center Spacing .....	3-6
3.5.1	Effect of Small Radii .....	3-6
3.5.2	Effect of Superelevation on Track Centers .....	3-7
3.5.3	Clearance Point .....	3-7
3.6	Other Clearance Requirements .....	3-7
3.6.1	Passenger Station Clearances .....	3-7
3.6.2	Utility Clearances .....	3-8
3.6.3	Additional Clearance for Seismic Fault Zones .....	3-8
3.6.4	Space Around Turnouts .....	3-8
3.6.5	Space Around Derails .....	3-8
3.6.6	Core Systems Elements .....	3-8

## Tables

Table 3-1:	Minimum Vertical Clearances .....	3-2
Table 3-2:	Minimum Horizontal Clearances .....	3-3
Table 3-3:	Track Centers .....	3-6

## Figures

Figure 3-1:	Track Centers Required on Small Radius Curves .....	3-7
-------------	---	-----

## **Appendix**

- Appendix 3.A: High-Speed Equipment Only, Static Envelope and Dynamic Envelope, Tangent Track
- Appendix 3.B: High-Speed Equipment Only, Static Envelope and Dynamic Envelope Swept Path of Vehicle Rotated for Superelevation
- Appendix 3.C: High-Speed Equipment Only, Structure Gauge and Fixed Equipment Envelope, Open Section
- Appendix 3.D: High-Speed Equipment Only, Structure Gauge and Fixed Equipment Envelope, In Tunnels
- Appendix 3.E: All Passenger Equipment, Static Envelope and Dynamic Envelope, Tangent Track
- Appendix 3.F: All Passenger Equipment, Static Envelope and Dynamic Envelope Swept Path of Vehicle Rotated for Superelevation
- Appendix 3.G: All Passenger Equipment, Structure Gauge and Fixed Equipment Envelope, Open Sections
- Appendix 3.H: All Passenger Equipment, Structure Gauge and Fixed Equipment Envelope, In Tunnels

## Acronyms

GO	General Order
HST	High-Speed Train
OCS	Overhead Contact System
TCL	Track Centerline
TOR	Top of Rail



## 3 Trackway Clearances

### 3.1 Scope

This chapter provides design criteria for required clearances to the California High-Speed Train (HST) trackway and facilities. It includes an allowance for HST maintenance equipment and other equipment that may be operated within the HST tracks. Two sets of static, dynamic, fixed equipment, and structure gauge envelopes are presented in this chapter. They have been developed to accommodate the following:

- Tracks that will accommodate high-speed equipment only which includes:
  - The widest and tallest existing HST rolling stock currently under contemplation
  - International Union of Railway (UIC) GC Gauge
  - Association of American Railroads (AAR) Plate C
- Tracks that will also accommodate other passenger train equipment operating in California

### 3.2 Regulations, Codes, Standards, and Guidelines

Refer to the *General* chapter for requirements pertaining to regulations, codes, standards, and guidelines. In addition to the laws and standards relevant to any facility, there are a number of standards specific to railroad facilities that shall be applied.

- Americans with Disabilities Act (ADA)
  - California Public Utilities Commission (CPUC) General Orders (GOs) – Clearances adjacent to railroad tracks in California are governed in detail by 2 CPUC GOs:
    - CPUC GO 26 – Clearances On Railroads And Street Railroads As To Side And Overhead Structures, Parallel Tracks And Crossings
    - CPUC GO 118 – Walkways Adjacent to Railroad Trackage (provides specific minimum widths and locations for walkway areas.)
- Even where not applicable, the GOs provide useful guidance. As legal requirements, these form the Minimum Design Standards.
- Code of Federal Regulations (CFR)
    - Title 29 CFR, Occupational Safety and Health Administration (OSHA)
    - Title 49 CFR, Parts 200-299

- 1 • American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for
- 2 Railway Engineering
- 3 • Association of American Railroads (AAR) Standards
- 4 • National Fire Protection Association (NFPA) 130

### 3.3 Clearances to Overhead and Adjacent Structures and Facilities

5 Minimum clearance requirements shall be as shown on Standard and Directive drawings where  
 6 applicable. These drawings and the requirements in Tables 3-1 and 3-2 shall be used as  
 7 guidance in developing clearances for situations and conditions not clearly defined in these  
 8 drawings.

9 Where tracks carrying HSTs also carry conventional rail rolling stock or equipment, the  
 10 clearance requirements of the conventional rail owners/operations shall also be met. Where  
 11 requirements differ, the larger dimension shall govern.

#### 3.3.1 Vertical Clearances

12 Minimum vertical clearances shall be measured from the HST top of rail (TOR). Minimum  
 13 vertical clearances should be carried to a point 25 feet laterally from the centerline of the most  
 14 outside track.

**Table 3-1: Minimum Vertical Clearances**

Item	Minimum Vertical Clearance
Clearance from HST TOR to new structures	27'-0" <sup>(1)</sup>
Clearance from HST TOR to existing structures ( $V > 125$ mph)	27'-0" <sup>(1)</sup>
Clearance from HST TOR to existing structures ( $V \leq 125$ mph)	24'-0" <sup>(1)</sup>
HST tracks span over Road and/or Railroad (HST Overpass)	See Note <sup>(2)</sup>

15 Notes:

16 <sup>(1)</sup> These clearances assume that the width of the grade-separated structure above the HST track is no more than  
 17 160 feet. Structures wider than 160 feet require further Engineer approval.

18 <sup>(2)</sup> Clearances to be determined by agreements with the third party agency owning the facility.

#### 3.3.2 Horizontal Clearances

19 Minimum horizontal clearances shall be measured from the track centerline (TCL) of the closest  
 20 HST track to the feature being cleared.

**Table 3-2: Minimum Horizontal Clearances**

Item	Minimum Horizontal Clearance
TCL to railroad corridor right-of-way	See <i>Rolling Stock and Vehicle Intrusion Protection</i> chapter
TCL to highway edge of travel way	See <i>Rolling Stock and Vehicle Intrusion Protection</i> chapter
TCL to face of permanent structure	25'-0" <sup>(2)</sup> <sup>(3)</sup>
TCL to edge of platform	5'-9" <sup>(1)</sup>
TCL to centerline of OCS poles, main gantry and strain gantry	See Standard and Directive Drawings
TCL to face of fixed equipment	10'-0" <sup>(3)</sup>

**Notes:**

- <sup>(1)</sup> 5 feet-9 inches for preliminary design is based on the half-width of the widest passenger vehicle currently under consideration. Following determination of actual vehicle, the edge of platform shall be located not more than 2-3/4 inches beyond the half-width of the vehicle at the elevation of the floor and at the same height above top of rail as the elevation of the vehicle floor.
- <sup>(2)</sup> Protective structure may be required if horizontal clearance is less than 25 feet.
- <sup>(3)</sup> See Appendices 3.C, 3.D, 3.G, and 3.H for constrained areas.

### 3.3.3 Clearances to Third Party Facilities

Where facilities owned and operated by third parties are involved, the clearance requirement of this document and those of the third party shall be compared and the larger dimension used.

## 3.4 Vehicle Clearance Envelopes

The static and dynamic envelopes in Appendices 3.A and 3.E define the maximum vehicle size and limits of vehicle movement including the pantograph.

### 3.4.1 Description of Envelopes

Appendices 3.A to 3.H define the following limits:

- **Static Envelope** – The outline that limits the dimensions of the vehicle which takes into account the vehicle fabrication tolerances.
- **Dynamic Envelope** – Defines the limits of motion of a static outline size and shape vehicle. The effects of curvature and superelevation are defined by widening and rotating the dynamic outline.
- **Fixed Equipment Envelope** – Defines the closest permitted location of trackside facilities, such as utilities, continuous and intermittent elements including but not limited to the following:
  - Conduits and cables
  - Fire water lines

- Signs and markers
- Catenary system features other than the contact wire and associated wires and brackets needed for its support which may be within the fixed equipment envelope but outside the dynamic outline
- Sound/Screen walls
- Signal heads, etc.
- **Structure Gauge** – Represents the minimum distances to be provided adjacent to the tracks where other considerations do not require a larger distance. The structure gauge defines the closest permitted location of major features, including but not limited to, the following:
  - Retaining walls
  - Bridge elements
  - Tunnel sides, etc.
- **Walkway Envelope** – Indicates the allowance for walkway space and its closest allowable proximity to the track. The walkway envelope shall be defined as follows:
  - The width shall be no less than 3.00 feet.
  - The vertical clear space shall be no less than 7.50 feet above the walkway surface.
  - The top may be tapered symmetrically from full width at 6.00 feet above the walkway surface to 2.50 feet wide at the top of the envelope.
  - The trackside edge of the walkway envelope shall clear the static envelope and shall clear the dynamic envelope.
- **Width and Depth of Area below Top of Rail** – Dependent on the trackform used, refer to the *Trackwork* chapter.

### **3.4.2 Structure Gauge for Non-Electrified Tracks**

Certain tracks within maintenance facilities may not require electrification. Structure gauge for these tracks shall be as shown on Appendices 3.C, 3.D, 3.G, and 3.H except that the overhead clearance may be reduced to 19.00 feet.

### **3.4.3 Effects Due to Curve Radius**

Due to swing out of the car ends beyond the bogie positions and swing in of the middle of the car body, the “swept path” of the vehicle body will be wider on curves than on tangent track. Only the car body itself swings out. The lower parts of the Static Envelope and Dynamic Envelope that represent the position of the bogies do not swing out. Since the normal mounting position of pantographs is over the bogies, the swept path of the pantograph does not increase on curves. Those parts of the envelopes representing space for the catenary do not increase on curves, as the catenary is mounted on the structure, not on the vehicle. Refer to Appendices 3.A



and 3.E for the location of affected points. The widening of the static and dynamic envelopes shall consider lateral clearance requirements on curves for all of the candidate rolling stock. The lateral dimensions determined by the formula below provide for the maximum swing out of the mid car toward the inside of the curve and end of car toward the outside of the curve.

$$EO \text{ (feet)} = MO \text{ (feet)} = 550 / R \text{ (feet)}$$

Where:

R = radius of the curve,

EO and MO = the increased offset required to clear the vehicles on curves. EO is toward the outside of the arc and MO is toward the inside of the arc.

Appropriate application of widening is defined in the Appendices.

**Location of Placement of Transition to Increased Width in Structure Gauge** – The usual objective is to ensure sufficient space, not the precise need for additional space. Sufficiency may be achieved by beginning the transition from the unwidened section 75.00 feet into the tangent track beyond the beginning of the spiral, or of the curve if there is no spiral and achieving the full needed additional offset not less than 25.00 feet before the beginning of the full arc.

#### 3.4.4 Effects of Superelevation

**Point of Definition of Track Profile** – The track profile elevation is defined as the elevation of the top of the inside rail (low rail) of the curve. This point is commonly referred to as the Profile Grade (PG).

**Superelevation Measurement** – Superelevation is measured, whether with a level board or by survey, by determining the relative difference in elevation between the highest point on the outside rail of the curve and the highest point on the inside rail of the curve. The distance between the points is normally considered as being track gauge plus rail head width. These high points are slightly further apart than the centerline to centerline separation of the railheads due to the rail inclination toward the track center, but this small distance is normally neglected as being insignificant. Standard track gauge is 56.5 inches, measured between points 5/8 inch below the top of rail. Nominal rail head width is 3.0 inches. Thus the distance between points of measurement in the superelevation calculation is taken as being 59.5 inches.

**Point of Rotation** – The point of rotation of the track is defined as the gauge corner of the inside rail of the curve. Therefore, the point of rotation is at the profile line elevation and offset 28.25 inches from the track centerline toward the inside of the curve.

**Widening of Section** – Refer to notes on Appendices 3.C, 3.D, 3.G, and 3.H for widening requirements.

### 3.5 Track Center Spacing

Track center spacing is related to but only partially controlled by the structure gauge dimensions. The distance between HST track centers considers aerodynamics, design speed, and ease of maintenance. The distance between track centers may be increased where the increase is beneficial for maintenance, structural, or constructability considerations.

Track center distances shall be based on design speed (V) in Table 3-3. For track centers in tunnels and trench structures with a separation wall and/or walkway between the 2 tracks, refer to Standard and Directive Drawings.

**Table 3-3: Track Centers**

Condition	Recommended (feet)	Minimum (feet)
V > 160 mph	16.50	16.50
90 mph < V ≤ 160 mph	16.50	16.00
V ≤ 90 mph <sup>(1)</sup>	16.50	15.00
With OCS poles located between tracks	25.00	22.00
Between Main Track to Station Tracks or Sidings	25.00	25.00
Tracks with inter-track fences	22.00 <sup>(2)</sup>	18.00 <sup>(2)</sup>

Notes:

<sup>(1)</sup> For track centers in yards, refer to the *Track Geometry* chapter. For yard facility requirements, refer to the *Support Facilities* chapter.

<sup>(2)</sup> If the walkway is adjacent to the fence, then the distance in the condition “with OCS poles located between tracks” shall be used.

#### 3.5.1 Effect of Small Radii

For small radius curves, the track centers may need to be increased. The required increase is expressed by the following formula:

$$\text{Increase in Track Centers} = \text{Constant} / \text{Radius}$$

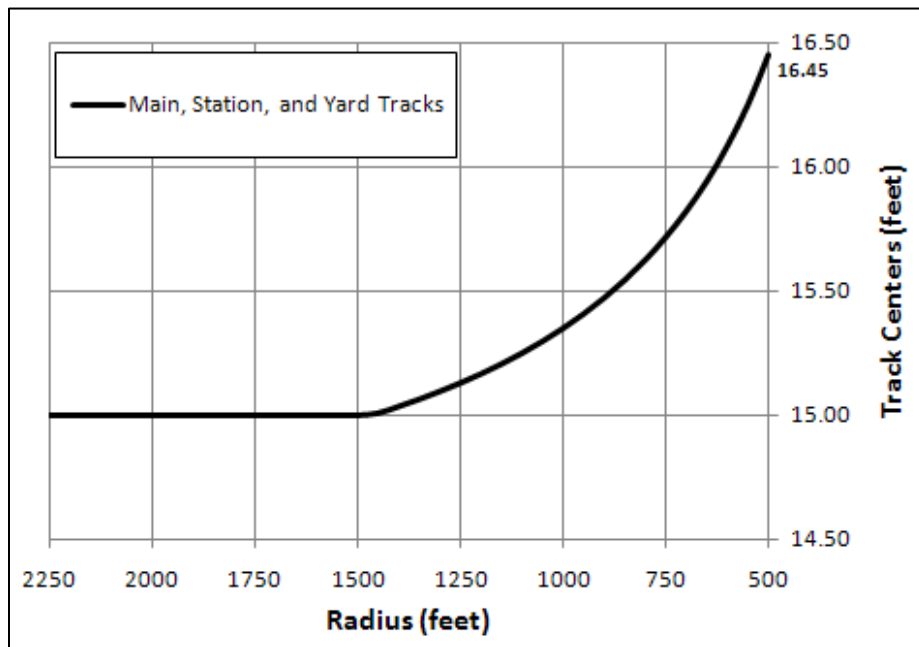
The constant is specific to the truck center and body length dimensions of the vehicle and the units of measurement. For the anticipated range of vehicle dimensions, the formula given below provides sufficient clearances between tracks. The value shall be rounded up. It shall not be rounded down.

**Track Centers Increase due to Radius** – No widening is required for tracks set at 16.50 feet track centers. For tracks spaced at less than 16.50 feet, the allowed minimum track centers shall be the greater of the defined minimum track centers for straight tracks (15.00 feet) or that determined by adding the value determined by the following formula to 14.75 feet.

Increase in Track Centers (feet) =  $1,100 / R$  (feet)

Where the use of this formula results in a required track center spacing of less than that given for the minimum spacing on straight track, it shall be taken as meaning that no increase in track centers is required. It shall not be taken to mean that track centers may be reduced.

**Figure 3-1: Track Centers Required on Small Radius Curves**



### 3.5.2 Effect of Superelevation on Track Centers

In the case of curves under 3,000 feet radius and the inside track having less superelevation than the outside track, additional space is required between tracks. This widening shall be 2 times the difference in superelevation.

### 3.5.3 Clearance Point

The clearance point (fouling point) shall be located at the point where the track centerlines are separated by not less than 16.50 feet. Where the diverging track turns parallel, the clearance point shall be located where the full track separation is achieved.

## 3.6 Other Clearance Requirements

### 3.6.1 Passenger Station Clearances

For passenger station platform offset and elevation dimensions, refer to the *Stations* chapter.

- 1 For track layout along station platforms, refer to the *Track Geometry* chapter.

### **3.6.2 Utility Clearances**

- 2 For utility clearance requirements, refer to the *Utilities* chapter.

### **3.6.3 Additional Clearance for Seismic Fault Zones**

- 3 Additional clearance for seismic fault zones shall be based on fault displacement analysis. Refer  
4 to the *Seismic* chapter. The positioning of the additional clearance shall be based on the result of  
5 the analysis of the potential magnitude of and direction of ground movement at the location of  
6 the fault zone crossing. The length and width of the additional clearance zone shall be such that  
7 subsequent to movement of the fault there will be sufficient space so that the required track  
8 realignment will permit operation without a permanent speed restriction.

### **3.6.4 Space Around Turnouts**

- 9 Space adjacent to turnouts shall be reserved for the mounting of switch machines to move the  
10 points, along with power and control cabling for those machines. This space shall be provided  
11 along 1 side of Number 9 and 11 turnouts. Refer to Standard and Directive Drawings for  
12 dimensions.

### **3.6.5 Space Around Derails**

- 13 Space adjacent to derails shall be reserved for the mounting of switch machines to move the  
14 derail. An additional width of 4 feet shall be provided on the derailing side between a point 50  
15 feet in advance of the derail to 50 feet beyond the derail for block derails or 300 feet beyond the  
16 derail for switch point derails. Space to be provided around derails shall be the same as the  
17 space around Number 9 turnouts. Refer to the *Trackwork* chapter for criteria determining derail  
18 types and locations.

### **3.6.6 Core Systems Elements**

- 19 Refer to the *Traction Power Supply System*, the *Overhead and Contact System and Traction Power*  
20 *Return System*, and the *Communications* chapters for dimensions and location of additional space  
21 requirements at wayside facilities.

## Appendix 3: High-Speed Equipment and All Passenger Equipment Static, Dynamic, Structure Gauge, and Fixed Equipment Envelopes

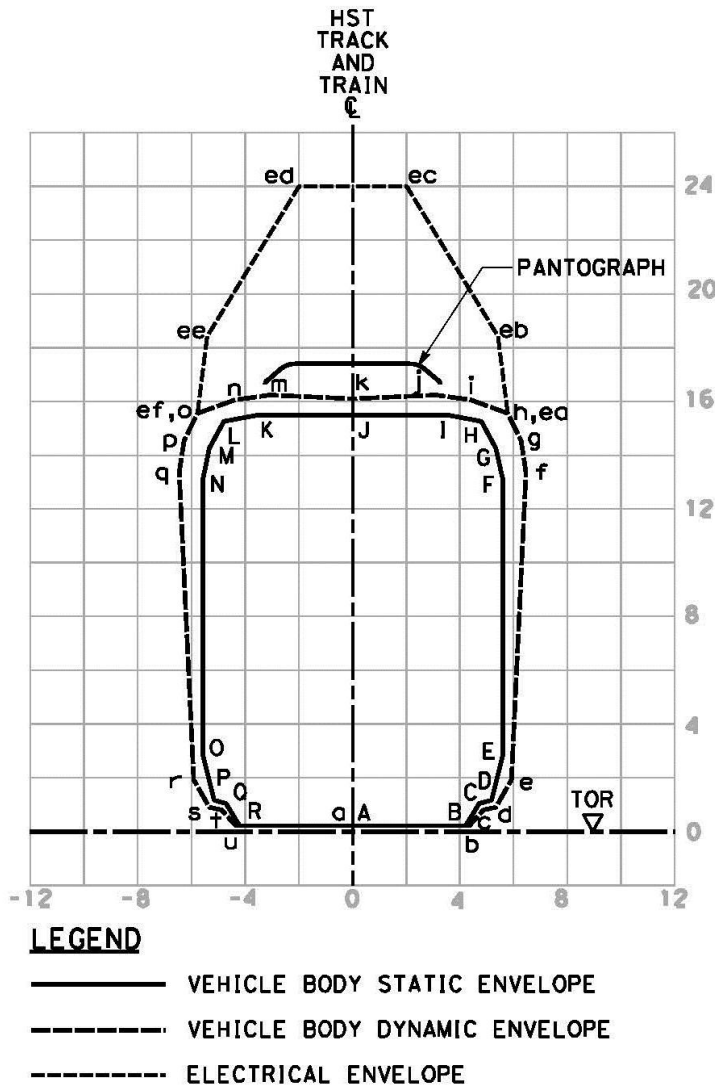
### General Notes:

1. The Vehicle Body Static Envelope defines the maximum shape and dimensions for any vehicle which may be operated on the tracks.
2. The Vehicle Body Dynamic Envelope takes the static envelope to the physical limits of motion under the maximum conceivable forces combined with the maximum allowable limits of wear and deficiencies including, for clearance purposes, motion due to track deficiencies and misalignments.
3. The Vehicle Body Static and Dynamic Envelopes, for high-speed equipment only, are based on a combination of in-service high-speed passenger equipment, Association of American Railroads (AAR) Plate C and International Union of Railways (UIC) GC Gauge.
4. The Vehicle Body Static and Dynamic Envelopes, for all passenger equipment, are based on a combination of in-service high-speed passenger equipment, Amtrak's Bi-Level Passenger Car Clearance Diagram with the addition of roof mounted equipment on cab units, and Association of American Railroads (AAR) Plate F.
5. The Pantograph, on high-speed equipment, is shown as it would be positioned with the nominal contact wire height of 17.417 feet from TOR. For all passenger equipment, the Pantograph is shown as it would be positioned with the nominal contact wire height of 18.75 feet from TOR. Refer to OCS Standard and Directive Drawings for reference.
6. The Electrical Envelope is the allowable space for overhead electrical equipment. The Electrical Envelope is lower in tunnel sections than in open air sections, however, the width is the same for both cases.
7. The total movements assumed in the development of the Vehicle Body Dynamic Envelope are:
  - a. Lateral expansion: 3.50 inches
  - b. Downward expansion: 4.50 inches
  - c. Upward expansion: 6.75 inches
  - d. Angular movement: 3.00 degrees – rotated about centerline at a point 16.5 inches above TOR
8. The width of the swept path of the vehicle body increases on curves due to mid-car and end-car overhang. Refer to each Appendix for the location of points requiring widening or not requiring widening and the example of widening cases.
9. Widening may be neglected on tracks with radii of 10,000 feet or larger.
10. Widening shall be calculated before calculating the position of the section due to the effects of superelevation.

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015



## Appendix 3.A: High-Speed Equipment Only, Static Envelope and Dynamic Envelope, Tangent Track



Tangent Track		
Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet)
<b>Vehicle Body Static Envelope</b>		
A	0.000	0.229
B, R	±4.208	0.229
C, Q	±4.708	1.042
D, P	±5.167	1.167
E, O	±5.583	2.833
F, N	±5.583	13.083
G, M	±5.333	14.250
H, L	±4.792	15.250
I, K	±3.500	15.500
J	±0.000	15.500
<b>Vehicle Body Dynamic Envelope</b>		
a	0.000	0.229
b, u	±4.354	0.229
c, t	±4.875	0.792
d, s	±5.333	0.896
e, r	±5.917	1.927
f, q	±6.458	13.354
g, p	±6.271	14.527
h, o	±5.771	15.552
i, n	±4.385	16.042
j, m	±3.083	16.229
k	0.000	16.083

Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet) (See Note 6)	
		Open Sections	In Tunnels
Electrical Envelope			
ea, ef	±5.771	15.552	15.552
eb, ee	±5.417	18.417	18.417
ec, ed	±2.000	24.000	22.333

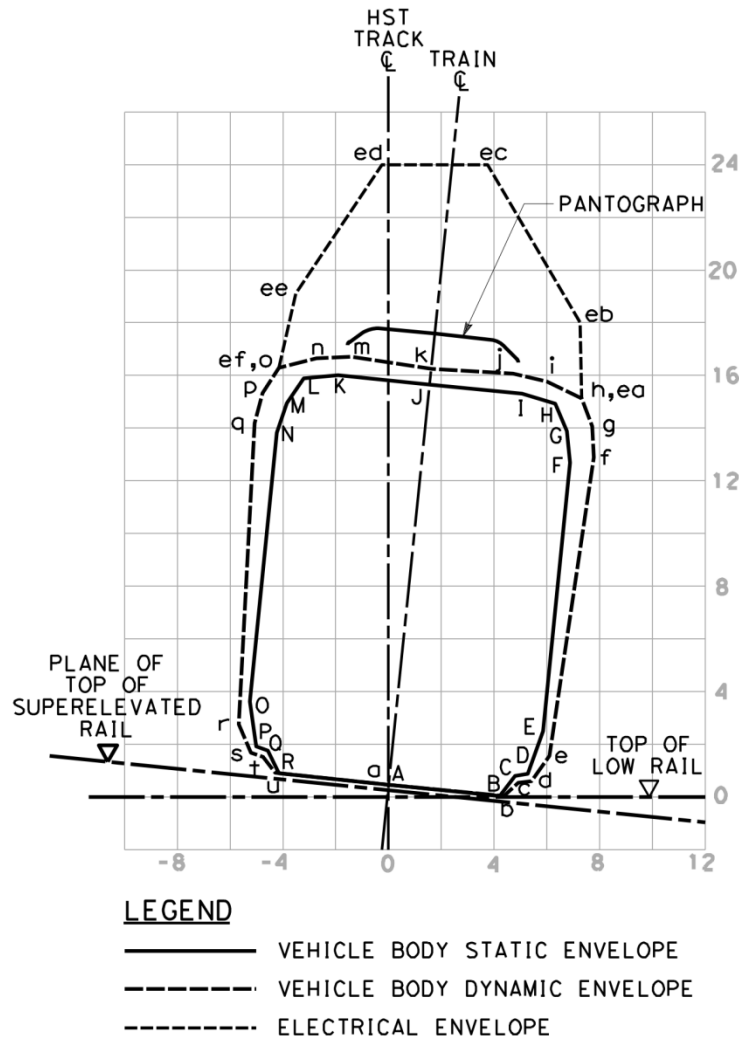
**Notes:**

1. The location of the points on the widened swept path of the vehicle body is determined by adding 550/radius (in feet) to the horizontal dimensions for points D, E, F, G, H, I, K, L, M, N, O, P, d, e, f, g, h, i, n, o, p, q, r, and s. The vertical dimensions do not change.
2. Points A, B, C, Q, R, a, b, c, t, and u need not be shifted as they relate to components that are on the vehicle at or near the bogie positions or attached to the structure.
3. As an example of the swept path dimensions for these points for a curve of 950 feet radius are given in the following table: (That radius is the internal radius of a standard No. 11 turnout.)

950 feet Radius					
Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet)	Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet)
Vehicle Body Static Envelope			Vehicle Body Dynamic Envelope		
A	0.000	0.229	a	0.000	0.229
B, R	±4.208	0.229	b, u	±4.354	0.229
C, Q	±4.708	1.042	c, t	±4.875	0.792
D, P	±5.746	1.167	d, s	±5.912	0.896
E, O	±6.162	2.833	e, r	±6.496	1.927
F, N	±6.162	13.083	f, q	±7.037	13.354
G, M	±5.912	14.250	g, p	±6.850	14.527
H, L	±5.371	15.250	h, o	±6.350	15.552
I, K	±4.079	15.500	i, n	±4.964	16.042
J	±0.000	15.500	j, m	±3.083	16.229
			k	0.000	16.083



## Appendix 3.B: High-Speed Equipment Only, Static Envelope and Dynamic Envelope Swept Path of Vehicle Rotated for Superelevation



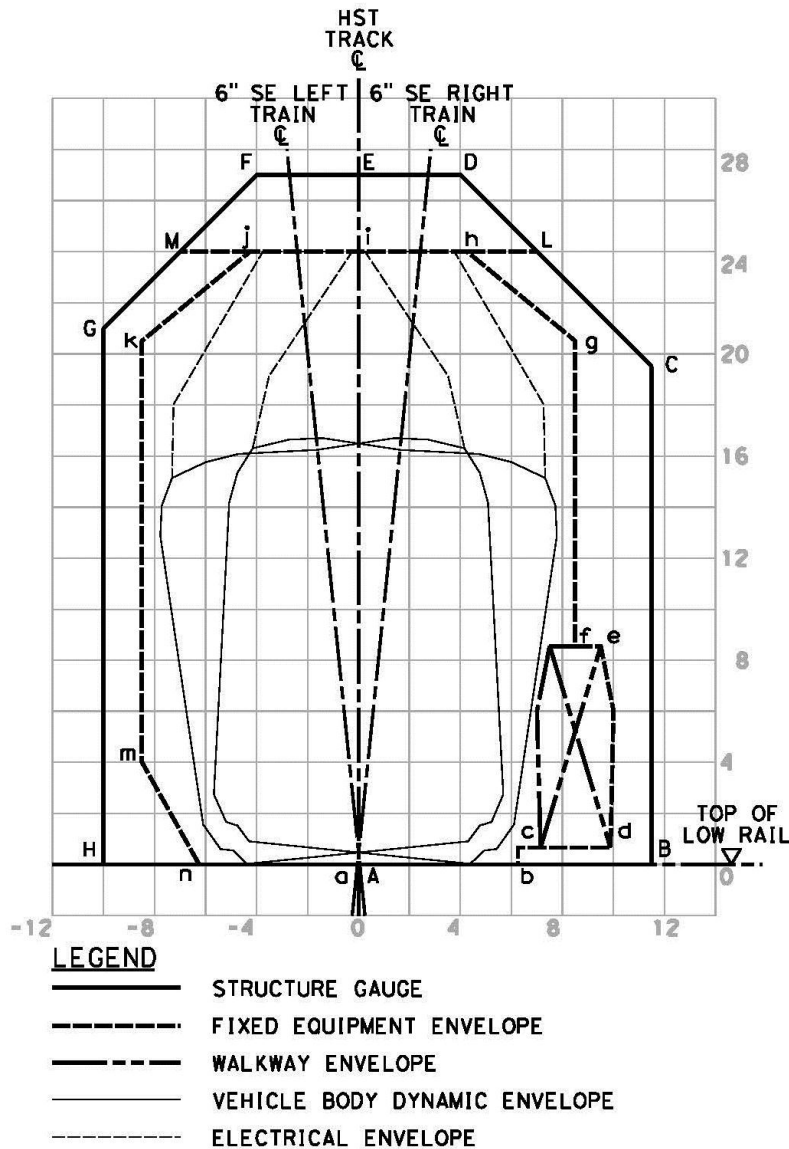
Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet) (See Note 5)	
		Open Sections	In Tunnels
Electrical Envelope			
ea	7.322	15.132	15.132
eb	7.258	18.018	18.018
ec	3.750	24.000	22.333
ed	-0.250	24.000	22.333
ee	-3.520	19.111	19.111
ef	-4.161	16.926	16.926

Superelevated, No Widening (Values for 6 inches superelevation tabulated. See notes for method of calculation for other values.)		
Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet)
<b>Vehicle Body Static Envelope (Without Increase in the Horizontal Dimension for the Effect of Small Radii)</b>		
A	0.035	0.465
B	4.222	0.041
C	4.801	0.799
D	5.270	0.877
E	5.853	2.493
F	6.886	12.691
G	6.755	13.877
H	6.317	14.926
I	5.057	15.305
J	1.575	15.658
K	-1.907	16.011
L	-3.217	15.893
M	-3.857	14.953
N	-4.224	13.817
O	-5.257	3.619
P	-5.011	1.919
Q	-4.567	1.749
R	-4.152	0.890
<b>Vehicle Body Dynamic Envelope (Without Increase in the Horizontal Dimension for the Effect of Small Radii)</b>		
a	0.035	0.465
b	4.367	0.031
c	4.924	0.538
d	5.409	0.595
e	6.093	1.562
f	7.784	12.876
g	7.713	14.063
h	7.322	15.132
i	5.993	15.759
j	4.716	16.077
k	1.634	16.243
m	-1.419	16.699
n	-2.733	16.644
o	-4.161	16.296
p	-4.762	15.327
q	-5.067	14.179
r	-5.680	2.755
s	-5.204	1.671
t	-4.758	1.521
u	-4.297	0.909

**Notes:**

1. The tables in this appendix give the position of points on the static and dynamic outlines of the vehicle rotated for 6 inches superelevation and without widening for the effects of curve radius. Refer to the following notes for the method of calculation of the position of these points.
2. Determine the widened and rotated swept path of the vehicle as follows:
  - a. Enlarge the width of the vehicle based on the radius of the curve. Refer to Appendix 3.A notes 1 and 2 for method of development of envelope due to curve radius.
  - b. Determine the angle of rotation from the applied superelevation as follows: The angle of rotation is  $\arcsin(\text{superelevation}/59.5 \text{ inches})$
  - c. Rotate the section by this angle about the point of rotation: The point of rotation for superelevation for both the static envelope and the dynamic envelope is the top inside corner of the inside rail of the curve, located at the track profile elevation and 28.25 inches (2.354 feet) offset from the track centerline.
3. The angle is 5.78756 degrees, or 5 deg 47 min 15 sec for 6 inches superelevation.
4. Points ec and ed do not rotate. They shift laterally a distance of 3.5 times the superelevation. For 6 inches superelevation, this distance is 21 inches or 1.75 feet.
5. The Electrical Envelope is lower in tunnel sections than in open air sections, however, the width is the same for both cases.

## Appendix 3.C: High-Speed Equipment Only, Structure Gauge and Fixed Equipment Envelope, Open Section

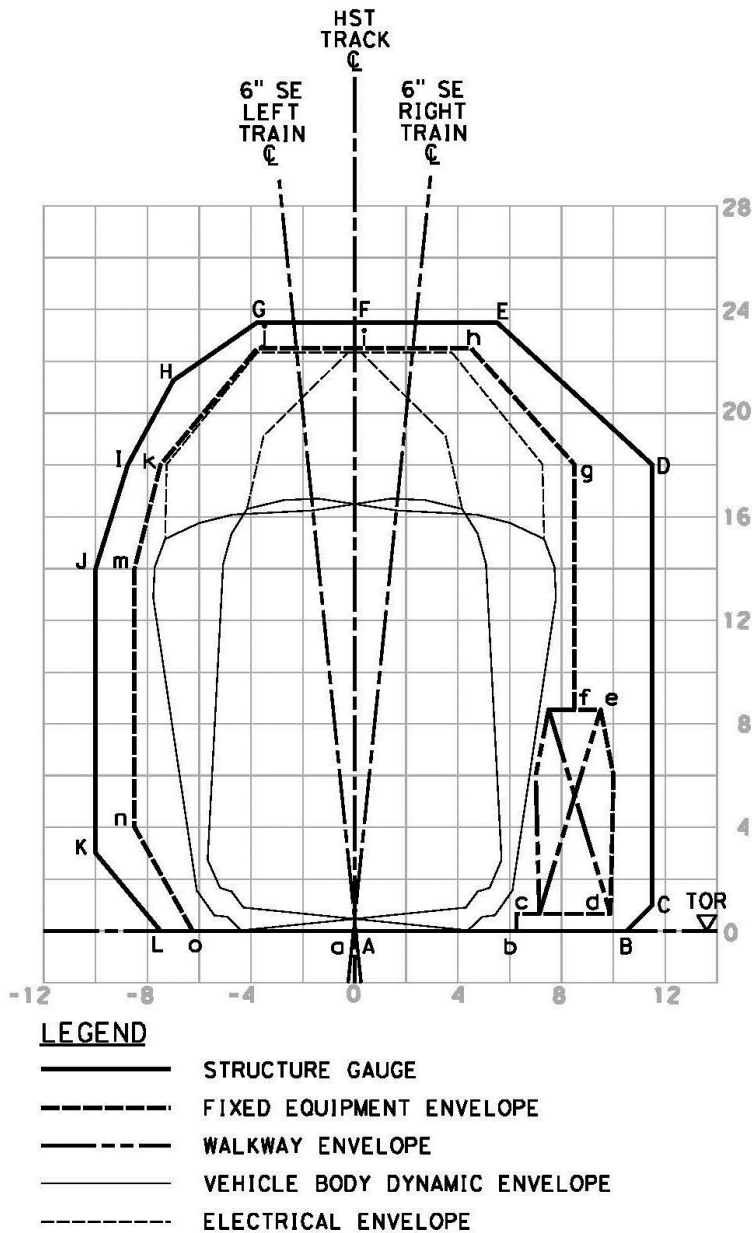


Open Section		
Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet)
<b>Structure Gauge</b>		
<b>Walkway Side (See Note 3)</b>		
A	0.00	0.00
B	11.50	0.00
C	11.50	19.50
D	4.00	27.00
E	0.00	27.00
<b>Non-Walkway Side</b>		
A	0.00	0.00
H	-10.00	0.00
G	-10.00	21.00
F	-4.00	27.00
E	0.00	27.00
<b>Under Existing Low Overhead Structures</b>		
L	7.00	24.00
M	-7.00	24.00
<b>Fixed Equipment Envelope</b>		
<b>Walkway Side (See Notes 3 &amp; 4)</b>		
a	0.00	0.00
b	6.25	0.00
c	6.25	0.67
d	10.00	0.67
e	10.00	8.17
f	8.50	8.17
g	8.50	20.50
h	4.25	24.00
i	0.00	24.00
<b>Non-Walkway Side</b>		
a	0.00	0.00
n	-6.25	0.00
m	-8.50	4.00
k	-8.50	20.50
j	-4.25	24.00
i	0.00	24.00

**Notes:**

1. Definition of the envelopes:
  - a. The Structure Gauge defines the minimum distances from track centerline to permanent features such as walls, columns and overhead structures.
  - b. The Fixed Equipment Envelope defines the allowable space for fixtures such as signs, signals, lights, piping and conduits that will be attached to the permanent features defined under Structure Gauge.
  - c. The Walkway Envelope defines a space sufficient for a person to be clear of any track mounted vehicle. The envelope illustrated is 7.50 feet high by 3.00 feet wide with the top 1.50 feet tapering to a width of 2.50 feet. Its precise location may vary depending upon local conditions. Therefore, the location of points b, c, d, e, and f will vary with the location of the walkway.
2. The Dynamic Envelopes for 6 inches superelevation each way are shown to illustrate their fit within the Structure Gauge and Fixed Equipment Envelope. Refer to Appendix 3.B for dimensions of these envelopes.
3. Right side dimensions in the figure and table are those that apply to the walkway side regardless of the actual location of the walkway. Left side dimensions in the figure and table are those that apply to the non-walkway side regardless of the actual location of the walkway.
4. Offset to structure on walkway side shall be 12.50 feet or niches shall be provided as required for placement of larger equipment.
5. Structure shall provide sufficient clear distance below TOR for the track form, under track equipment and other fixtures that must pass under the tracks.
6. Widening of sections toward the outside of the curve is not necessary. Widening of sections toward the inside of curves of under 2,400 feet radius shall be the larger of either 0.25 feet or  $550/\text{radius}$  in feet.

## Appendix 3.D: High-Speed Equipment Only, Structure Gauge and Fixed Equipment Envelope, In Tunnels



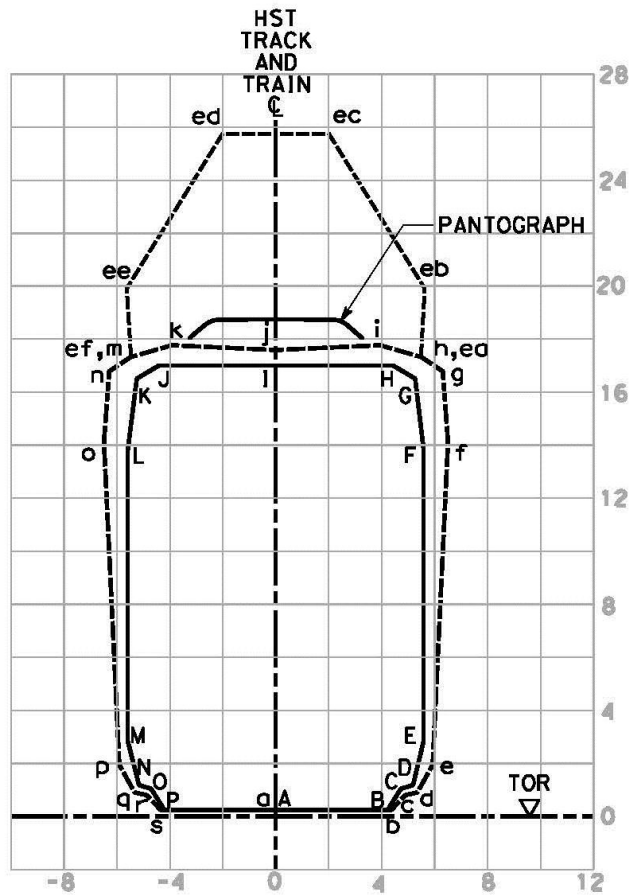
In Tunnels		
Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet)
<b>Structure Gauge</b>		
<b>Walkway Side</b> (See Appendix 3.C, Note 3)		
A	0.00	0.00
B	10.50	0.00
C	11.50	1.00
D	11.50	18.00
E	5.50	23.50
F	0.00	23.50
<b>Non-Walkway Side</b>		
A	0.00	0.00
L	-7.50	0.00
K	-10.00	3.00
J	-10.00	14.00
I	-8.75	18.00
H	-7.00	21.25
G	-3.75	23.50
F	0.00	23.50
<b>Fixed Equipment Envelope</b>		
<b>Walkway Side</b> (See Appendix 3.C, Notes 3 & 4)		
a	0.00	0.00
b	6.25	0.00
c	6.25	0.67
d	10.00	0.67
e	10.00	8.17
f	8.50	8.17
g	8.50	18.00
h	4.50	22.50
i	0.00	22.50
<b>Non-Walkway Side</b>		
a	0.00	0.00
o	-6.25	0.00
n	-8.50	4.00
m	-8.50	14.00
k	-7.50	18.00
j	-3.75	22.50
i	0.00	22.50

### Notes:

- See Appendix 3.C, Notes 1 through 6.



## Appendix 3.E: All Passenger Equipment, Static Envelope and Dynamic Envelope, Tangent Track



### LEGEND

- VEHICLE BODY STATIC ENVELOPE
- VEHICLE BODY DYNAMIC ENVELOPE
- ..... ELECTRICAL ENVELOPE

Tangent Track		
Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet)
<b>Vehicle Body Static Envelope</b>		
A	0.000	0.229
B, P	±4.208	0.229
C, O	±4.708	1.042
D, N	±5.167	1.167
E, M	±5.583	2.833
F, L	±5.583	13.833
G, K	±5.250	16.500
H, J	±4.417	17.000
I	0.000	17.000
<b>Vehicle Body Dynamic Envelope</b>		
a	0.00	0.229
b, s	±4.354	0.229
c, r	±4.875	0.792
d, q	±5.333	0.896
e, p	±5.917	1.927
f, o	±6.500	14.104
g, n	±6.292	16.792
h, m	±5.479	17.333
i, k	±3.938	17.771
j	0.000	17.583

Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet) (See Note 6)	
		Open Sections	In Tunnels
Electrical Envelope			
ea, ef	±5.479	17.333	ea, ef
eb, ee	±5.625	19.917	eb, ee
ec, ed	±2.000	25.750	ec, ed

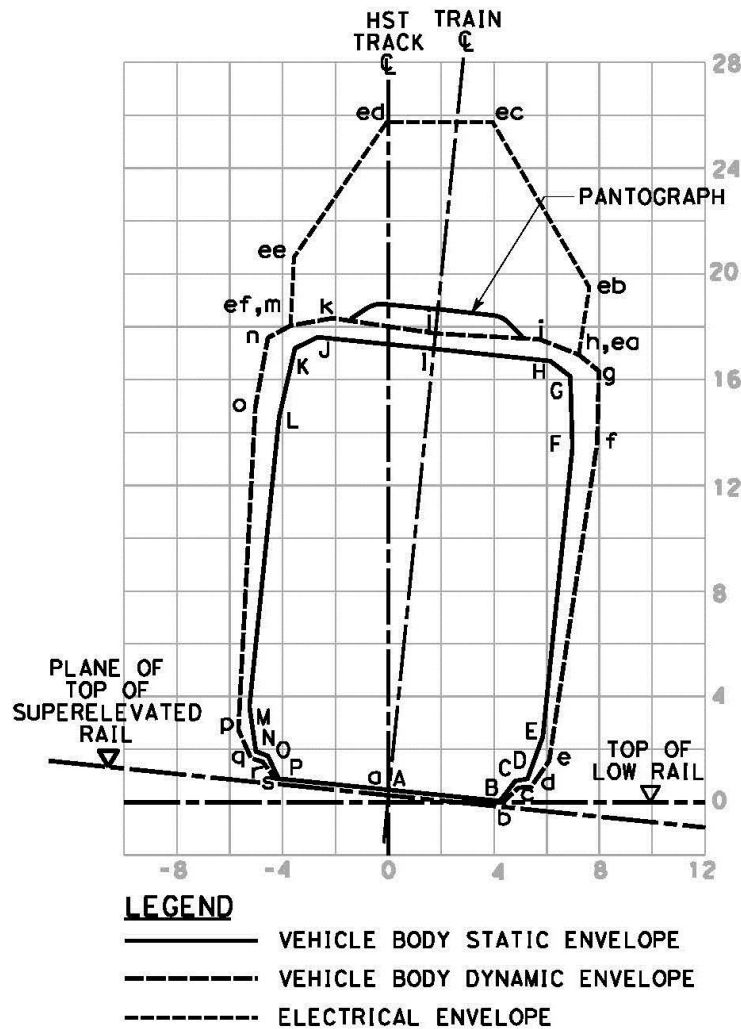
**Notes:**

1. The width of the swept path of the vehicle body increase is determined by adding 550/radius (in feet) to the horizontal dimensions for points D, E, F, G, H, J, K, L, M, N, d, e, f, g, h, m, n, o, p, and q.
2. Points A, B, C, O, P, a, b, c, r, and s need not be shifted as they relate to components that are on the vehicle at or near the bogie positions or attached to the structure.
3. As an example, the increased dimensions for these points for a curve of 950 feet radius are given in the following table: (That radius is the internal radius of a standard No. 11 turnout.)

<b>950 feet Radius</b>					
<b>Point ID</b>	<b>Horizontal Distance from TCL (feet)</b>	<b>Vertical Distance from TOR (feet)</b>	<b>Point ID</b>	<b>Horizontal Distance from TCL (feet)</b>	<b>Vertical Distance from TOR (feet)</b>
<b>Vehicle Body Static Envelope</b>			<b>Vehicle Body Dynamic Envelope</b>		
A	0.000	0.229	a	0.000	0.229
B, P	±4.208	0.229	b, s	±4.354	0.229
C, O	±4.708	1.042	c, r	±4.875	0.792
D, N	±5.746	1.167	d, q	±5.912	0.896
E, M	±6.162	2.833	e, p	±6.496	1.927
F, L	±6.162	13.833	f, o	±7.079	14.104
G, K	±5.829	16.500	g, n	±6.871	16.792
H, J	±4.996	17.000	h, m	±6.058	17.333
I	0.000	17.000	i, k	±3.938	17.771
			j	0.000	17.583



## Appendix 3.F: All Passenger Equipment, Static Envelope and Dynamic Envelope Swept Path of Vehicle Rotated for Superelevation



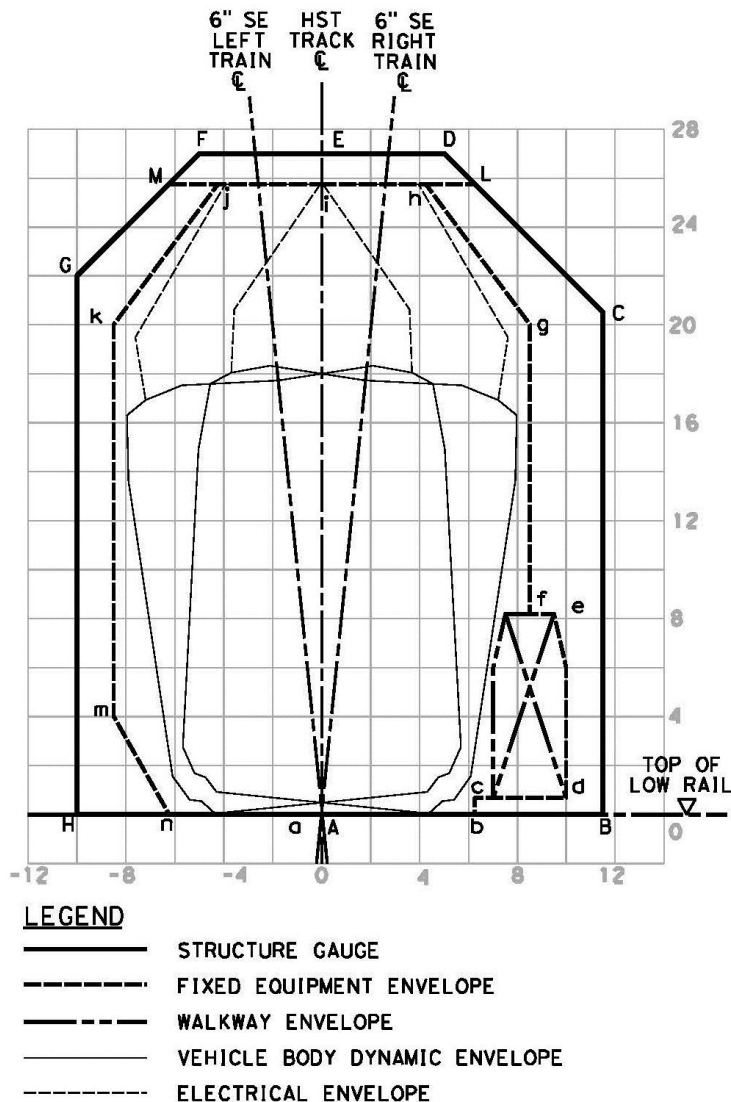
Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet) (See Note 5)	
		Open Sections	In Tunnels
Electrical Envelope			
ea	7.211	16.934	16.934
eb	7.619	19.490	19.490
ec	3.950	25.750	24.250
ed	-0.050	25.750	24.250
ee	-3.576	20.624	20.624
ef	-3.691	18.039	18.039

Superelevated, No Widening (Values for 6 inches superelevation tabulated. See notes for method of calculation for other values.)		
Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet)
<b>Vehicle Body Static Envelope</b>		
A	0.035	0.465
B	4.222	0.041
C	4.801	0.799
D	5.270	0.877
E	5.853	2.493
F	6.962	13.437
G	6.899	16.124
H	6.120	16.705
I	1.726	17.151
J	-2.668	17.596
K	-3.547	17.183
L	-4.148	14.563
M	-5.257	3.619
N	-5.011	1.919
O	-4.567	1.749
P	-4.152	0.890
<b>Vehicle Body Dynamic Envelope (Without Increase in the Horizontal Dimension for the Effect of Small Radii)</b>		
a	0.035	0.465
b	4.367	0.031
c	4.924	0.538
d	5.409	0.595
e	6.093	1.562
f	7.901	13.618
g	7.965	16.313
h	7.211	16.934
i	5.722	17.525
j	1.785	17.735
k	-2.113	18.319
m	-3.691	18.039
n	-4.554	17.582
o	-5.032	14.929
p	-5.680	2.755
q	-5.204	1.671
r	-4.758	1.521
s	-4.297	0.909

**Notes:**

1. The tables in this appendix give the position of points on the static and dynamic outlines of the vehicle rotated for 6 inches superelevation and without widening for the effects of curve radius. Refer to the following notes for the method of calculation of the position of these points.
2. Determine the widened and rotated swept path of the vehicle as follows:
  - a. Enlarge the width of the vehicle based on the radius of the curve. Refer to Appendix 3.E notes 1 and 2 for method of development of envelope due to curve radius.
  - b. Determine the angle of rotation from the applied superelevation as follows: The angle of rotation is  $\arcsin(\text{superelevation}/59.5 \text{ inches})$ .
  - c. Rotate the section by this angle about the point of rotation: The point of rotation for superelevation for both the static envelope and the dynamic envelope is the top inside corner of the inside rail of the curve, located at the track profile elevation and 28.25 inches (2.354 feet) offset from the track centerline.
3. The angle is 5.78756 degrees, or 5 deg 47 min 15 sec for 6 inches superelevation.
4. Points ec and ed do not rotate. They shift laterally a distance of 3.9 times the superelevation. For 6 inches superelevation, this distance is 23.4 inches or 1.95 feet.
5. The Electrical Envelope is lower in tunnel sections than in open air sections, however, the width is the same for both cases.

## Appendix 3.G: All Passenger Equipment, Structure Gauge and Fixed Equipment Envelope, Open Sections

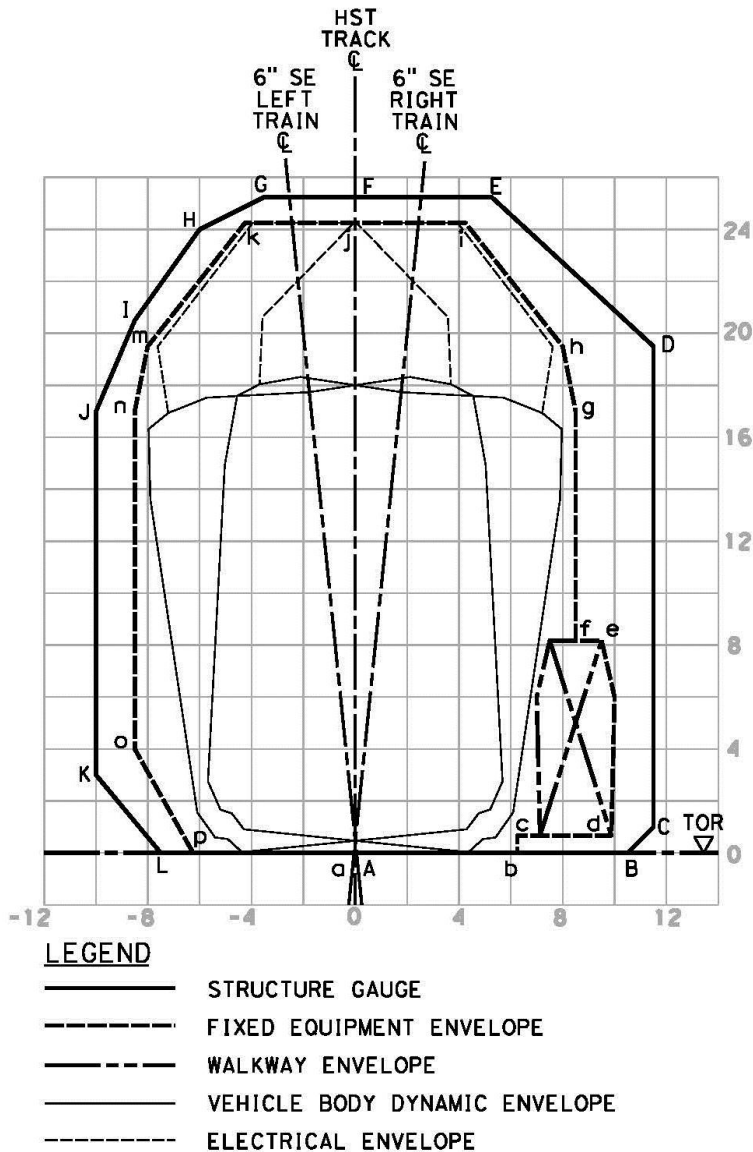


Open Sections		
Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet)
<b>Structure Gauge</b>		
<b>Walkway Side (See Note 3 &amp; 4)</b>		
A	0.00	0.00
B	11.50	0.00
C	11.50	20.50
D	5.00	27.00
E	0.00	27.00
<b>Non-Walkway Side</b>		
A	0.00	0.00
H	-10.00	0.00
G	-10.00	20.50
F	-5.00	27.00
E	0.00	27.00
<b>Under Existing Low Overhead Structures</b>		
L	6.25	25.75
M	-6.25	25.75
<b>Fixed Equipment Envelope</b>		
<b>Walkway Side (See Notes 1, 3, &amp; 4)</b>		
a	0.00	0.00
b	6.25	0.00
c	6.25	0.67
d	10.00	0.67
e	10.00	8.17
f	8.50	8.17
g	8.50	20.00
h	4.25	25.75
i	0.00	25.75
<b>Non-Walkway Side</b>		
a	0.00	0.00
n	-6.25	0.00
m	-8.50	4.00
k	-8.50	20.00
j	-4.25	25.75

**Notes:**

1. Definition of the envelopes:
  - a. Structure Gauge defines the minimum distances from track to permanent features such as walls, columns and overhead structures.
  - b. Fixed Equipment envelope defines the allowable space for fixtures such as signs, signals, lights, piping and conduits that will be attached to the permanent features defined under Structure Gauge.
  - c. The walkway envelope defines a space sufficient for a person to be clear of any track mounted vehicle. The envelope illustrated is 7.50 feet high by 3.00 feet wide with the top 1.50 feet tapering to a width of 2.50 feet. Its precise location may vary depending upon local conditions. Therefore, the location of points b, c, d, e, and f will vary with the location of the walkway.
2. The Dynamic Envelopes for 6 inches superelevation each way are shown to illustrate their fit within the Structure Gauge and Fixed Equipment Envelope. Refer to Appendix 3.F for dimensions of these envelopes.
3. Right side dimensions in the figure and table are those that apply to the walkway side regardless of the actual location of the walkway. Left side dimensions in the figure and table are those that apply to the non-walkway side regardless of the actual location of the walkway.
4. Offset to structure on walkway side shall be 12.50 feet or niches shall be provided as required for placement of larger equipment.
5. Structure shall provide sufficient clear distance below TOR for the track form, under track equipment and other fixtures that must pass under the tracks.
6. Widening of sections toward the outside of the curve is not necessary. Widening of sections toward the inside of curves of under 2,400 feet radius shall be the larger of either 0.25 feet or  $550/\text{radius}$  in feet.

## Appendix 3.H: All Passenger Equipment, Structure Gauge and Fixed Equipment Envelope, In Tunnels



In Tunnels		
Point ID	Horizontal Distance from TCL (feet)	Vertical Distance from TOR (feet)
<b>Structure Gauge</b>		
<b>Walkway Side</b> (See Appendix 3.G, Note 3)		
A	0.00	0.00
B	10.50	0.00
C	11.50	1.00
D	11.50	19.50
E	5.25	25.25
F	0.00	25.25
<b>Non-Walkway Side</b>		
A	0.00	0.00
L	-7.50	0.00
K	-10.00	3.00
J	-10.00	17.00
I	-8.50	20.50
H	-6.00	24.00
G	-3.25	25.25
F	0.00	25.25
<b>Fixed Equipment Envelope</b>		
<b>Walkway Side</b> (See Appendix 3.G, Notes 3 & 4)		
a	0.00	0.00
b	6.25	0.00
c	6.25	0.67
d	10.00	0.67
e	10.00	8.17
f	8.50	8.17
g	8.50	17.00
h	8.00	19.50
i	4.25	24.25
j	0.00	24.25
<b>Non-Walkway Side</b>		
a	0.00	0.00
p	-6.25	0.00
o	-8.50	4.00
n	-8.50	17.00
m	-8.00	19.50
k	-4.25	24.25
j	0.00	24.25

### Notes:

- See Appendix 3.G, Notes 1 through 6.



## Chapter 4

### Track Geometry

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	June 2013	Revision 1
2	Feb 2014	Revision 2
3	Feb 2016	Revision 3





## Table of Contents

<b>4</b>	<b>TRACK GEOMETRY</b>	<b>4-1</b>
4.1	Scope	4-1
4.2	Regulations, Codes, Standards, and Guidelines	4-1
4.3	Types of Rail Corridors	4-1
4.3.1	Dedicated High-Speed Rail Corridors	4-1
4.3.2	Shared Corridors	4-1
4.4	Horizontal Alignment	4-2
4.4.1	Selection of Design Speed	4-2
4.4.2	Minimum Lengths of Alignment Segments	4-2
4.4.3	Minimum Radii	4-3
4.4.4	Curves with Small Central Angles	4-4
4.4.5	Superelevation	4-4
4.4.6	Spiral Curves	4-6
4.5	Vertical Alignment	4-10
4.5.1	Grades	4-10
4.5.2	Vertical Curves	4-11
4.6	Combined Horizontal and Vertical Curves	4-11
4.7	High-Speed Turnouts (60 mph and faster)	4-12
4.8	Low and Medium Speed Turnouts (55 mph and slower)	4-13
4.9	Non-Standard Turnouts	4-15
4.10	High-Speed Crossovers	4-15
4.11	Low and Medium Speed Crossovers	4-16
4.12	Double Crossovers (Scissors Crossovers)	4-18
4.13	Track Layout along Station Platforms	4-18
4.14	Access Tracks to Yards and Maintenance Facilities	4-20
4.15	Yards Tracks	4-21
4.15.1	Connecting and Switching Tracks Inside Yards	4-21
4.15.2	Servicing and Storage Tracks	4-22
4.15.3	Simple Track Ladders	4-23
4.15.4	Double Angle Track Ladders	4-24

## Tables

Table 4-1:	Minimum Segment Lengths at Various Speeds	4-3
Table 4-2:	Recommended and Minimum Curve Radii	4-4
Table 4-3:	Recommended and Minimum Length of Spiral (Ls)	4-9
Table 4-4:	Recommended and Maximum Grades	4-11
Table 4-5:	High-Speed Turnouts	4-13
Table 4-6:	Low and Medium Speed Turnouts	4-14
Table 4-7:	High-Speed Crossovers – 16.50 feet Track Centers	4-16
Table 4-8:	Low and Medium Speed Crossovers	4-17

Table 4-9:	Geometry of Station Entry/Exit High-Speed Turnouts and Return Curves.....	4-19
------------	---	------

## Figures

Figure 4-1:	Spiral Curves Definition .....	4-7
Figure 4-2:	High-Speed Turnouts .....	4-12
Figure 4-3:	Low and Medium Speed Turnouts .....	4-14
Figure 4-4:	High-Speed Crossovers .....	4-16
Figure 4-5:	Low and Medium Speed Crossovers .....	4-17
Figure 4-6:	Detail of Station Entry/Exit High-Speed Turnout and Return Curve .....	4-19
Figure 4-7:	Simple Ladder (4 Tracks Illustrated) .....	4-23
Figure 4-8:	Double Ladder (4 Diverging Tracks Illustrated).....	4-25
Figure 4-9:	Double Ladder, Track Space Requirements (6 Diverging Tracks Illustrated) ....	4-25

## Acronyms

E / e	Equilibrium Superelevation
Ea	Actual Superelevation
Eu	Unbalanced Superelevation
HST	High-Speed Train
OCS	Overhead Contact System



## 4 Track Geometry

### 4.1 Scope

1 This chapter provides design criteria of geometric design requirements for mainline tracks,  
2 station tracks, yard tracks, turnouts, and crossovers on dedicated high-speed rail corridors of  
3 standard gauge (4'-8 1/2").

### 4.2 Regulations, Codes, Standards, and Guidelines

4 Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.  
5 Applicable codes and regulations include but are not limited to the following:

- 6 • Code of Federal Regulations (CFR) Title 49, Part 213, Track Safety Standards
- 7 • California Public Utilities Commission (CPUC) General Orders (GOs) 26D and 118
- 8 • American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for  
9 Railway Engineering

### 4.3 Types of Rail Corridors

#### 4.3.1 Dedicated High-Speed Rail Corridors

10 Dedicated High-Speed Rail Corridors are segments of right of way within the High-Speed Train  
11 (HST) System where tracks are used exclusively for HST operations, designated as such in the  
12 operating rules and where the main tracks are physically separated from other railroad tracks.  
13 There is no operation of freight trains or other passenger trains within these corridors. The  
14 operation of trains and equipment used for Maintenance of Infrastructure work is permitted in  
15 these corridors.

#### 4.3.2 Shared Corridors

16 Shared Corridors are segments along the HST System where the HST right of way is shared  
17 with other transportation system(s) including highway, freight or passenger rail.

18 Where HST tracks are shared with other passenger trains, design criteria for the maximum  
19 practicable design speed shall be used. At locations where tracks are shared with freight trains,  
20 the alignment standards for freight operations shall be checked and the more stringent criteria  
21 shall be applied.

## 4.4 Horizontal Alignment

Alignments for HST operation shall be designed to minimize the use of curves and to permit the maximum practical design speed.

When curves are used, the largest practical radii shall be used. Where the maximum design speed cannot be achieved, the highest achievable speed shall be used to define the geometry of the alignment.

The horizontal alignment shall be developed along track centerlines. It shall consist of tangents and circular curves connected by transition spirals of appropriate lengths.

When possible, double track alignment shall be designed with a constant distance in between track centerlines. Segments along straight line tracks shall be parallel and circular curves on adjacent, parallel tracks shall be concentric.

### 4.4.1 Selection of Design Speed

The speed to be used for the design of the alignment shall be the system design speed, not the operating speed, planned to be used at the time of start of operations. The purpose of determining design speed is to find the appropriate superelevation and spiral length for a particular curve in the alignment. The highest anticipated speed, superelevation, and unbalanced superelevation shall be used.

The maximum design speed for a curve shall be the same throughout the entire length of the curve from tangent points. Separate design speeds shall not be used for separate portions of a curve. If a speed limitation exists for any segment of the curve, then the design speed for the entire curve shall be the lower speed.

Refer to the *General* chapter for maximum allowed design and operating speeds.

### 4.4.2 Minimum Lengths of Alignment Segments

The minimum allowed segment length (L), in feet, shall be calculated by the following formula:

$$L = V \times 44/30 \times t$$

Where:

V = design speed (miles per hour)

t = attenuation time (seconds)

t ≥ 2.4 seconds (Recommended)

1.8 seconds (Minimum)

1.0 seconds (on diverging route of turnouts)

Minimum segment lengths shall apply to horizontal and vertical alignment segments. Where alignment segments overlap, each change in alignment shall be treated as a separate alignment element for the purpose of calculating minimum segment lengths. Where there is a vertical curve within a horizontal curve, the parts of the horizontal curve outside of the vertical curve shall be treated as separate segments when calculating segment lengths. The segment length requirement shall govern only where other design considerations for the various alignment elements do not require longer segment lengths. See Section 4.6, Combined Horizontal and Vertical Curves for further information. The segment length requirement will govern only where other design considerations for the individual alignment elements do not require longer segment lengths.

Minimum segment lengths for various design speeds are presented in Table 4-1. Additional values, for design speeds not shown, can be obtained from the formula provided in this section, rounded up to the nearest integer.

**Table 4-1: Minimum Segment Lengths at Various Speeds**

Design Speed (miles per hour)	Minimum Segment Lengths (in feet) for times of		
	2.4 seconds	1.8 seconds	1.0 seconds
250	880	660	367
220	774	581	323
200	704	528	293
175	616	462	257
150	528	396	220
125	440	330	183
110	387	290	161
90	317	238	132

#### 4.4.3 Minimum Radii

The minimum allowed curve radius shall be derived from the following formula:

$$R = 4V_{max}^2 / (Ea + Eu)$$

Where:

R = Radius (feet)

V<sub>max</sub> = Maximum design speed (miles per hour)

Ea = Actual superelevation (inches)      Ea<sub>max</sub> = 6 inches

Eu = Unbalanced superelevation (inches)      Eu<sub>max</sub> = 3 inches

Table 4-2 presents minimum values of curve radii for various design speeds. When possible, recommended values shall be used. Additional curve radii for design speeds not shown on Table 4-2 can be calculated with the formula provided above, using  $Ea = 6$  inches for Minimum values and  $Ea = 3$  inches as Recommended values. The minimum curve radius for tracks located outside the perimeter of the yards shall not be less than the value specified in Section 4.14.

**Table 4-2: Recommended and Minimum Curve Radii**

Design Speed (miles per hour)	Minimum Radius based on Superelevation Limits	
	Recommended (feet)	Minimum (feet)
250	45,000	28,000
220	35,000	22,000
200	30,000	18,000
175	22,000	14,000
150	16,000	10,000
125	10,500	7,000
110	8,100	5,400
90	5,500	3,600

#### 4.4.4 Curves with Small Central Angles

For small central angles the radius shall be sufficiently large to provide the time-based minimum arc and spiral segment lengths. There is no limitation on maximum acceptable curve radius. In general, larger radii are preferable to smaller radii as the superelevation and unbalance values become smaller as radius increases. It is desirable that the radius selected results in the length of the simple curve portion being about equal to or longer than the length of spiral. Since each portion is an alignment segment, if each segment is equal in length, the entire curve with spirals should have a minimum length not less than three times the Minimum Segment Length for the design speed of the curve. Double (back-to-back) spirals or curves with long spirals and short arc lengths shall not be used.

#### 4.4.5 Superelevation

Superelevation is the maximum difference in height between outer and inner rails on curved track, measured at the center of the rail head surface. Superelevation is used to counteract, or partially counteract, the centrifugal force acting radially outward on a train when it is traveling along the curve. A state of equilibrium is reached when the centrifugal force acting on a train is equal to the counteracting force pulling on a train by gravity along the superelevated plane of the track.



#### 4.4.5.1 Equilibrium (Balanced) Superelevation

Equilibrium superelevation (E) may be derived by the simplified formula:

$$E = 4.0 V^2 / R$$

Where:

E = Equilibrium superelevation (inches)

V = Design speed (miles per hour)

R = Radius of curve (feet)

E is also expressed as:

$$E = E_a + E_u$$

Where:

E<sub>a</sub> = actual superelevation (inches)

E<sub>u</sub> = unbalanced superelevation (inches)

Thus:  $E = (E_a + E_u) = 4.0 V^2 / R$

#### 4.4.5.2 Actual Superelevation

Actual superelevation (E<sub>a</sub>) shall be accomplished by maintaining the top of the inside (or low) rail at the “top of rail profile” while raising the outside (or high) rail by the amount of the E<sub>a</sub>. The inside rail is designated as the “grade rail” and the outside rail is designated as the “line rail”.

The E<sub>a</sub> shall be determined to the nearest 1/4 inch by the formulas above. For any curve calculation on the main track which yields less than 1/4 inch of required superelevation, 1/4 inch shall be specified.

Curves within special trackwork shall not be superelevated. Yard tracks and other low speed tracks on which trains or equipment will normally be stationary for long periods shall not be superelevated. Yard lead tracks and other running tracks shall be superelevated as described in the discussion of those type tracks.

It is recommended that the E<sub>a</sub> be limited to 6 inches.

#### 4.4.5.3 Unbalanced Superelevation

Unbalanced superelevation (E<sub>u</sub>), also referred to as cant deficiency, is the amount of superelevation not applied to the curve. E<sub>u</sub> can also be defined as the difference between the equilibrium superelevation (E) and the E<sub>a</sub>.

$$E_u = E - E_a$$

Where:

Ea = actual superelevation that is applied to the curve

Eu = unbalanced superelevation

The maximum Eu shall be limited to 3 inches.

#### **4.4.5.4 Ride Quality and Superelevation**

Ride quality on curves is determined by the amount of lateral acceleration which in curve design is expressed as Eu. Curves shall not be superelevated to balance the design speed, the calculated average speed, or the maximum operating speed. Eu values shall be kept between 1 and 3 inches for ride comfort and smooth running of the vehicles through curves.

- Minimum Eu shall be 1.0 inch, except where Ea+Eu is less than 2.0 inches, in which case Ea and Eu shall be set to be approximately equal.
- Maximum Eu shall be 3.0 inches, based on a lateral acceleration limit of 0.05g.

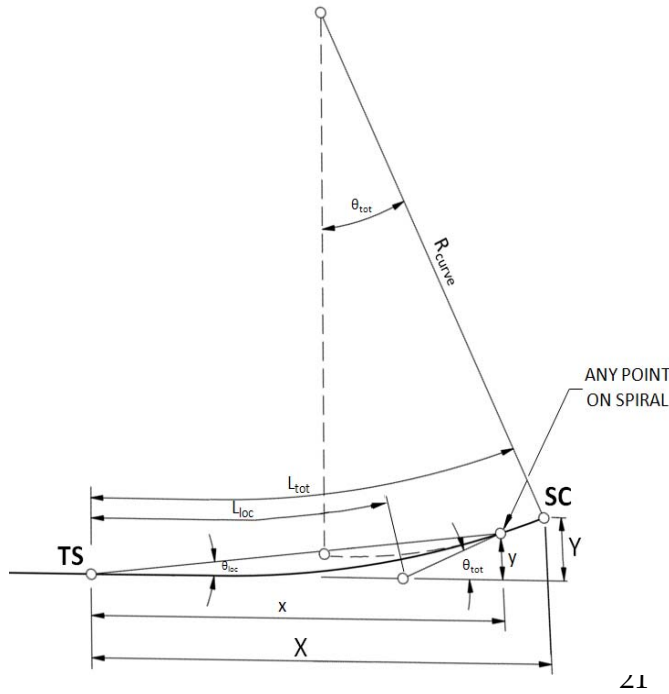
#### **4.4.6 Spiral Curves**

Spiral curves shall be used to transition from tangent tracks to tracks on circular curves and to gradually develop full track superelevation. Figure 4-1 illustrates the geometry of spiral transition curves.

Spiral transition curves used in high-speed track alignment shall be of the following types:

- Half-Sine spiral curves (variable rate transitions)
- Clothoid spiral curves (constant rate transitions)

1 **Figure 4-1: Spiral Curves Definition**



Where:

- TS Tangent Spiral point: the point of change from tangent to spiral
- SC Spiral Curve point: the point of change from spiral to circular curve
- R<sub>curve</sub> Radius of circular curve, in feet
- L<sub>loc</sub> Spiral length from TS to a specific location.
- L<sub>tot</sub> Total length of spiral from TS to SC (or SCS) in feet
- x = Distance from TS point to any point on the curve, measured along the extended initial tangent
- X= Total x at the end of the transition curve from TS to SC
- y= Tangent offset distance to any point along the spiral, measured perpendicular to the extended initial tangent.
- Y = Tangent offset of the SC point.
- θ<sub>loc</sub> = Spiral angle at any point along the spiral
- θ<sub>tot</sub> = Total spiral angle

#### 4.4.6.1 Half-Sine Spirals

Half-Sine spirals (also known as Sine Half-Wavelength Diminishing Tangent Curves and Cosine Spirals) provide a variable rate of change in curvature between the tangent and circular curved track. Half-Sine spirals shall be used on:

- All curves along HST mainline tracks
- Curves having design speeds of 110 mph or more
- Curves associated with turnouts having design maximum speed of 110 mph or more

Half-Sine spirals are defined by the following formulas (angles in these formulae are in radians):

$$y = \frac{x^2}{R_{\text{curve}}} \left[ \frac{\alpha^2}{4} - \frac{1}{2\pi^2} \{1 - \cos \alpha\pi\} \right] \quad \alpha = \frac{x}{X}$$

$$R_{\text{loc}} = \frac{2R_{\text{curve}}}{\left[ 1 - \cos \left( \pi \frac{L_{\text{loc}}}{L_{\text{tot}}} \right) \right]}$$

$$Ea_{\text{loc}} = 0.5Ea_{\text{curve}} \left[ 1 - \cos \left( \pi \frac{L_{\text{loc}}}{L_{\text{tot}}} \right) \right]$$

Where:

$Ea_{loc}$  = Variable actual superelevation at a specific location along the spiral, in inches ( $Ea_{loc} = Ea_{curve}$  at the SC location)

$Ea_{curve}$  = Actual superelevation at the SC and throughout the circular curve, in inches

#### 4.4.6.2 Clothoid Spirals

Clothoid spirals provide a constant rate of change in curvature between the tangent and the connecting circular curve. Clothoid spirals shall be used on tracks having design speed lower than 110 mph. Clothoid spirals may be used on large radius curves that require small amounts or no superelevation and small unbalanced superelevation.

Clothoid spiral are defined by the following formulas:

$$\theta_{loc} = \frac{L_{loc}^2}{2R_{curve}L_{tot}}$$

$$R_{loc} = \frac{R_{curve}}{\left(\frac{L_{loc}}{L_{tot}}\right)}$$

$$Ea_{loc} = Ea_{curve} \left(\frac{L_{loc}}{L_{tot}}\right)$$

#### 4.4.6.3 Spiral Lengths

The length of the spirals shall be the longest length determined by calculating the length requirements per Table 4-3. These lengths are the following:

- Length determined by allowed rate of change in superelevation, which controls the speed of car rotation around the track centerline (roll).
- Length determined by allowed rate of change in  $E_u$ , which controls the acceleration caused by centrifugal force not balanced by the  $E_a$  (lateral jerk).
- Length determined by limitation on twisting over the vehicle body.
- Length needed to achieve Attenuation Time

**Table 4-3: Recommended and Minimum Length of Spiral (Ls)**

Half-Sine (Variable Change) Spirals <sup>(1)</sup>		
Spiral Design Factor	Recommended	Minimum
Superelevation	1.63 Ea V	1.30 Ea V
Unbalance	2.10 Eu V	1.57 Eu V
Twist <sup>(2)</sup>	140 Ea	118 Ea
Minimum Segment	2.64 V	2.20 V
Clothoid (Linear Change) Spirals		
Superelevation	1.47 Ea V	1.17 Ea V
Unbalance	1.63 Eu V	1.22 Eu V
Twist	90 Ea	75 Ea
Minimum Segment	2.64 V	2.20 V

**Notes:**

- <sup>(1)</sup> Longer lengths of half-sine spirals are due to the variability in the ramp rate.  
<sup>(2)</sup> Provides maximum twist rates identical to the twist rate of the clothoids.

**Where:**

- Ls = Spiral length (feet)  
Ea = Actual elevation (inches)  
Eu = Unbalanced elevation (inches)  
V = maximum speed of the train (mph)

After calculation and selection of length, based on the governing requirement, the spiral length should then be rounded up to a convenient value for further calculation and use in the alignment.

**4.4.6.4 Special Situations**

**Spirals on Large Radius Curves** – Clothoid spirals may be used instead of half-sine spirals regardless of track type or design speed if the following conditions are met: The required superelevation and unbalanced superelevation are both under 1.0 inch at the maximum design speed; and the “Minimum Segment” length for the spiral is more than twice the length required by any other factor.

Spirals may be omitted if the following conditions are met:

- The required superelevation is zero (balancing superelevation for the maximum speed less than 0.5 inches); and
- The calculated offset of the curve due to application of the spiral is less than 0.05 feet in ballasted track or less than 0.02 feet in non-ballasted track.

**Reverse Curves** – Reverse curves shall only be allowed when there is insufficient distance between spiral curves to provide the minimum required length of tangent segment. In these cases, the spirals shall be extended to provide a reversing curve.

- 1    **Compound Curves** – Compound curves shall not be used on mainline tracks.

## 4.5      **Vertical Alignment**

- 2    The vertical alignment is defined as the top of rail profile grade. In curves with superelevation,  
3    the vertical alignment is the top of the low rail.

4    Vertical alignment shall be designed to have the smoothest practical profile while optimizing  
5    earthwork, structures, tunnels, and drainage. Use of multiple short grades and multiple changes  
6    in grade within any particular change of elevation (“sawtooth profiles”) shall be avoided to the  
7    extent practical. In addition to increasing operational costs and difficulty by requiring frequent  
8    changes in power, a line with multiple changes in grade is aesthetically unappealing. As a check  
9    on the reasonableness of the profile developed, it shall be drawn up at a highly condensed  
10   horizontal scale so that the vertical changes are exaggerated, otherwise, the alignment can  
11   appear deceptively smooth. Changes in top of rail profile gradients shall be connected by  
12   vertical curves.

### 4.5.1      **Grades**

13   Grades are expressed in absolute values. Grades shall be as low as practical. In areas of  
14   relatively flat terrain, the grades should not exceed the recommended values per Table 4-4. In  
15   mountainous terrain, grades should be minimized in order to maximize operating efficiency  
16   which most often means lower gradients than the surrounding terrain.

17   The average grade over any 6.0 miles of line should not exceed 2.5 percent.

18   Maximum gradient shall not exceed 2.5 percent on ballasted track and 3.5 percent on non-  
19   ballasted track. When these limit values are used, the low end of the grade shall not be less than  
20   2.0 miles beyond the end of a passenger station platform.

21   Maximum gradient through passenger station platform shall be 0.25 percent.

22   Minimum gradient through cuts, tunnels, and trenches shall be 0.25 percent.

23   Maximum segment length of continuous 3.5% grade shall not exceed 20,000 feet.

24   In areas occupied by turnouts and other special trackwork, grades up to 1.75 percent in  
25   ballasted track and 3.50 percent in non-ballasted track may be used where the use of lower  
26   grades would result in the requirement for lower speed turnouts.

27   For grade limitation at phase breaks, refer to the *Traction Power Supply System* chapter.

**Table 4-4: Recommended and Maximum Grades**

Track type and conditions	Recommended	Maximum
Ballasted	1.25%	2.50%
Non-ballasted	1.25%	3.50%
Ballasted track through turnouts and other special trackwork	0.50%	1.75%
Non-ballasted track through turnouts and other special trackwork	1.25%	3.50%
Mainline tracks through Station Platforms	0%	0.25%

## 4.5.2 Vertical Curves

Vertical Curves shall be Parabolic. The length of vertical curves shall be rounded up to nearest 100-foot increment where practical.

### 4.5.2.1 Vertical Curve Acceleration Rates

The acceleration value to be used for vertical curves shall not exceed 0.90 ft/sec<sup>2</sup>.

### 4.5.2.2 Minimum Vertical Curve Lengths ( $L_{VC}$ )

The minimum vertical curve lengths ( $L_{VC}$ ), in feet, on lines carrying HSTs only shall be the longer of the following:

$$L_{VC} = 3.5 V \text{ or } L_{VC} = 2.15 V^2 (\Delta\%/100) / 0.90 \text{ ft/sec}^2, \text{ but not less than } 200 \Delta \%$$

Where:

$V$  = Design speed (miles per hour)

$\Delta\%$  = algebraic difference of the gradients (in %)

### 4.5.2.3 Vertical Curves in Shared Corridors

Where HST tracks closely parallel lines for other passenger or freight trains such that a common profile is desirable, the longest vertical curve length determined by separate calculation for each type of traffic shall determine the vertical curve length to be used for all tracks. The length of vertical curve for the other systems shall be based on the standards of the systems involved.

## 4.6 Combined Horizontal and Vertical Curves

Horizontal and vertical curves may overlap. It is preferred to avoid overlap of vertical curves and spiral curves. Overlaps may be used if this consideration causes an increase in cost, increases the height of fill or aerial structures, or results in other aspects of the alignment being reduced below recommended values. For example, when there is a vertical curve within the body of a horizontal curve, the parts of the horizontal curve outside of the vertical curve will be treated as separate segments when calculating segment lengths. Horizontal and vertical

1 segment ends may coincide if it is not practical to separate them by the minimum segment  
2 length distance.

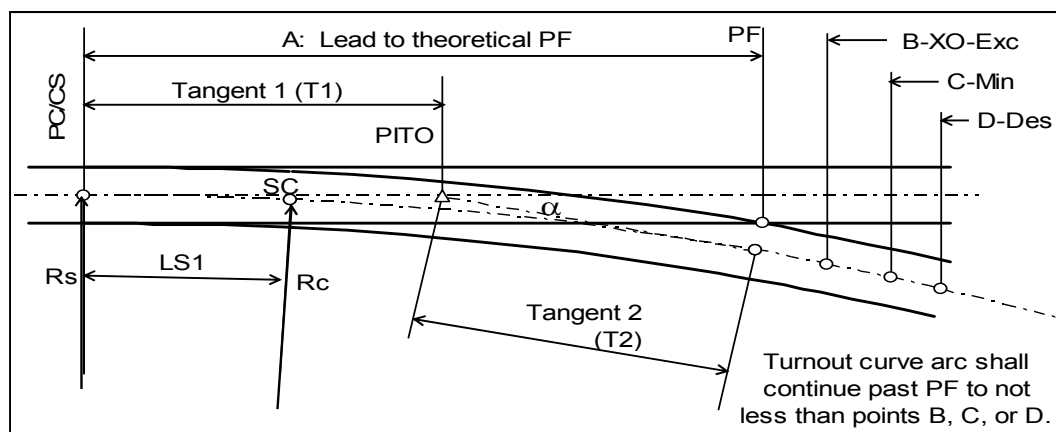
## 4.7 High-Speed Turnouts (60 mph and faster)

3 Turnout geometries are presented for the following speeds: 60 mph, 80, mph, 110 mph, and 150  
4 mph. The requirements of this section are limited to geometric considerations only. Track  
5 components for turnouts and other special trackwork can be found in the *Trackwork* chapter.  
6 Other spatial considerations, including distance between track centers and space beside and  
7 around turnouts can be found in the *Trackway Clearances* chapter.

8 High-speed turnout and crossover designs are based on the following criteria:

- 9 • Eu not to exceed 3 inches
- 10 • Minimum time over any turnout segment or curve connected to a turnout, including spirals  
11 on the frog end of turnouts and spirals into a curve on the diverging track that is adjacent to  
12 the turnout, about 1.0 second
- 13 • Maximum Virtual Transition Rate at switch point: 4.5 inches/second
- 14 • Ratio of entry radius to turnout body radius: Not less than 2:1.
- 15 • Curved frogs
- 16 • Spirals shall be kept out of frogs

17 **Figure 4-2: High-Speed Turnouts**





**Table 4-5: High-Speed Turnouts**

Geometry of Turnout and its Segments, in feet unless stated otherwise				
Design Speed	60 mph	80 mph	110 mph	150 mph
Turnout Entry Radius	10,000.00	18,000.00	34,000.00	80,000.00
Turnout Body Radius	5,000.00	9,000.00	17,000.00	32,000.00
Switch Spiral Length and Desirable Frog End Spiral Length	90.00	120.00	160.00	220.00
A. Distance to Theoretical Point of Frog (Zero Point, also called Fine Point)	237.53	318.53	436.76	610.07
Angle at Theoretical Point of Frog	2d27m49s	1d 50m12s	1d20m14s	0d58m27s
Derived Frog Number (AREMA method)	23.25	31.2	42.8	58.8
Tangent 1 (T1)	128.06	171.67	333.14	461.99
Tangent 2 (T2)	109.48	146.87	276.93	363.30
Turnout Body Curve Arc Length, SC to PF	147.50	198.51	276.74	375.18
B. Distance to point of 5.85 ft. separation	262.62	352.18	482.98	673.52
C. Distance to point of 7.00 ft. separation	285.48	382.85	525.11	731.34
D. Distance to point of 8.00 ft. separation	303.85	407.49	558.97	777.81

**Notes:**

Values in table are for illustration purposes, and so are generally given to 2 decimal places. This is not to be construed as the necessary limit for the alignment calculations.

To provide for future OCS design and construction, sufficient distance is required between:

1. two adjacent points of switches of adjacent universal crossovers
2. point of switch of turnout and adjacent point of switch of crossover

The preferred distance between adjacent switch points along the main tracks is 1,400 feet. The minimum distance between adjacent switch points along the main track is 1,000 feet. Placement of high-speed turnouts in relation to alignment features shall be based on 1.0 seconds of run time of the slower alignment element, whether another turnout or the end of a spiral or vertical curve.

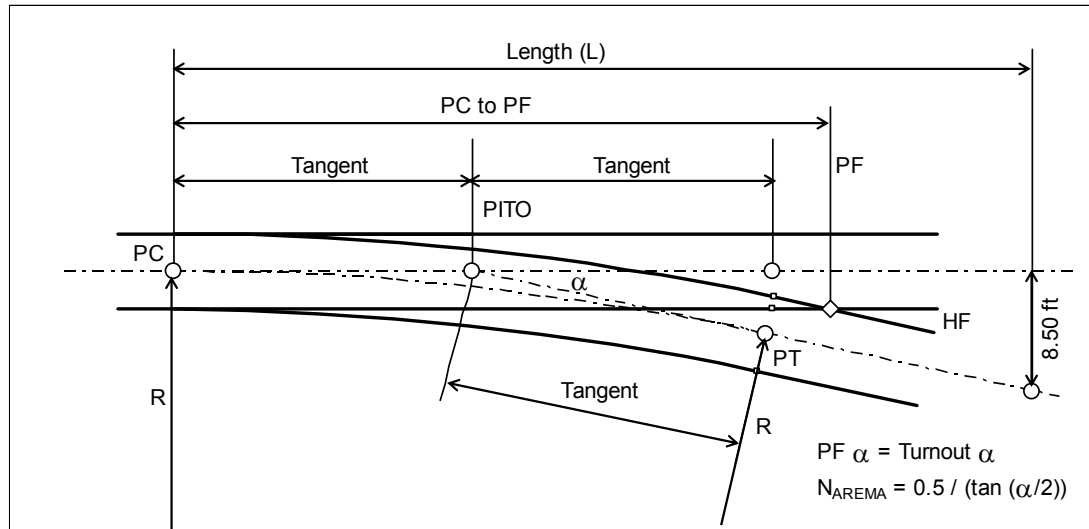
#### 4.8 Low and Medium Speed Turnouts (55 mph and slower)

Turnouts to storage and refuge tracks, yard connection tracks, and within yards and any other low and medium speed locations shall use AREMA standard frogs. The standard turnout sizes to be used shall be Numbers: 9, 11, 15, and 20.

Number 11 turnouts shall be used as the standard yard turnout, and as the minimum size turnout to be installed in main tracks with speeds of 125 mph or less and in station tracks. Main line turnouts to yard Leads or other tracks shall be no less than Number 20 turnouts if the conditions allow it. Turnouts from station tracks to stub end storage tracks shall be Number 11.

- 1 Number 9 turnouts may be used in yard tracks where geometric constraints make the use of
- 2 Number 11 turnout impractical.

3 **Figure 4-3: Low and Medium Speed Turnouts**



**Table 4-6: Low and Medium Speed Turnouts**

Number	9	11	15	20	24
Defined Angle (degrees/minutes/seconds)	6d21m35s	5d12m18s	3d49m06s	2d51m51s	2d23m13s
Radius	620 feet	950 feet	1750 feet	3250 feet	4650 feet
Tangent	34.44 feet	43.18 feet	58.33 feet	81.25 feet	96.87 feet
Lead, PC to ½ inch PF	77.19 feet	95.43 feet	129.58 feet	176.25 feet	210.87 feet
PC to 8.5 feet separation	110.71 feet	136.49 feet	185.69 feet	251.14 feet	300.79 feet
Tangent Rail, ½ inch PF to Curve PT	8.31 feet	9.07 feet	12.92 feet	13.75 feet	17.13 feet
Maximum Diverging Speed	20 mph	25 mph	35 mph	45 mph	55 mph
Unbalance at Max. Diverging Speed	2.58 inches	2.63 inches	2.80 inches	2.49 inches	2.61 inches

**Notes:**

Values in table are for illustration purposes and so are generally given to 2 decimal places. This is not to be construed as the necessary limit for the alignment calculations.

Spatial considerations, including space beside and around turnouts can be found in the *Trackway Clearances* chapter. For track components, see the *Trackwork* chapter.

To provide for future OCS design and construction, sufficient distance is required between:

1. two adjacent points of switches of adjacent universal crossovers
2. point of switch of turnout and adjacent point of switch of crossover

The preferred distance between adjacent switch points along the main tracks is 600 feet. The minimum distance between adjacent switch points along the main track is 400 feet. This requirement does not apply for the yard turnouts. Run time considerations are not relevant to the location and spacing of low and medium speed turnouts. Vehicle twist and relative end offsets are the controlling factors. It is recommended to provide at least 75.00 feet of straight track in advance of a switch. Where practical, these turnouts shall be spaced so that the length between turnouts is at least equal to the sum of vehicle truck centers plus one end overhang. Where the usage of switches that are point to point is such that trains are unlikely to use both turnouts, the switch points may be placed closer, down to 30 feet apart. It is desirable that the track off the frog end of the turnout be straight to at least the end of the switch tie set, which may be taken as the point at which the tracks are 8.50 feet apart. In the development of crossovers, track ladders, and track fans, it will be seen that these values are not always achievable.

#### 4.9 Non-Standard Turnouts

Turnouts on curves or in locations where standard turnouts cannot be used shall be designed as special cases. These turnouts shall be designed such that the lateral forces and rates of change in these forces are similar to those in standard design turnouts.

For all turnouts, the Eu shall not exceed 3.0 inches on either side of the turnout.

For high-speed turnouts, the following governs:

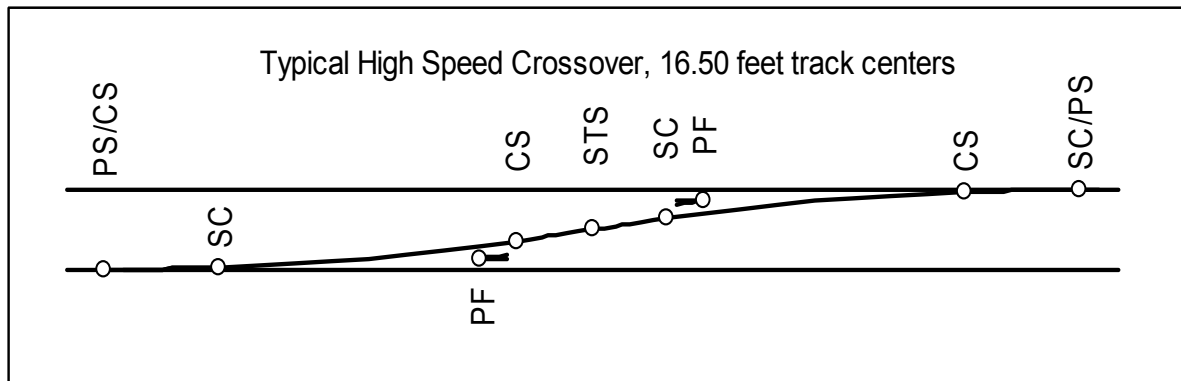
- Switch end spiral having a transition rate not more than 4.5 inches per second
- Eu at the point of switch: 1.5 inches
- If the curve does not continue beyond the turnout on the frog end, a frog end spiral having a transition rate of not more than 4.5 inches per second shall be applied.
- Minimum time over any turnout segment or curve connected to a turnout shall be approximately 1.0 second, and not less than 0.9 seconds.

For low and medium speed turnouts, compound internal curves shall not be used. If a curved frog is used, the end of the curve shall be outside the casting portion of the frog.

#### 4.10 High-Speed Crossovers

Crossovers in high-speed turnouts are more complex, as the curve continues through the frog. In order to place crossovers for 60 mph or faster between tracks at the standard track center spacing of 16.50 feet, the frog end spiral must be shortened to keep the spiral out of the frog. The length of the 2 spirals combined achieves the minimum 1.0 second run time when they are considered as 1 design element. Figure 4-4 shows the normal relationship between crossover components in a crossover between 16.50 feet track centers.

1 **Figure 4-4: High-Speed Crossovers**



2  
3  
**Table 4-7: High-Speed Crossovers – 16.50 feet Track Centers**

Geometry of Turnout and its Segments, in feet unless stated otherwise				
Design Speed	60 mph	80 mph	110 mph	150 mph
Track Centers	16.50	16.50	16.50	16.50
Total Length along main track	618.74	829.97	1,138.63	1,583.92
Total Length along Crossover Track	619.05	830.20	1,138.80	1,584.04
Turnout Entry Radius	10,000.00	18,000.00	34,000.00	80,000.00
Turnout Body Radius	5,000.00	9,000.00	17,000.00	32,000.00
Switch Spiral Length	90.00	120.00	160.00	220.00
Frog Spiral Length	45.00	62.00	85.00	115.00
Angle at STS	3d01m31s	2d 15m15s	1d38m28s	1d11m49s
Length of Entry Curve	0.00	0.00	0.00	0.00
Length of Turnout Body Curve	173.52	233.10	324.40	457.02

4 **Notes:**

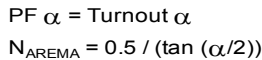
5 Values in table are for illustration purposes and so are generally given to 2 decimal places. This is not to be  
6 construed as the necessary limit for the alignment calculations.

7  
8 For high-speed crossovers between track centers of between 16.50 feet and 21.50 feet, longer  
9 spirals between turnouts may be used, but with the limitation that they be kept out of the frog.  
10 Where the track centers are 21.50 feet or greater, a full length spiral shall be used. High-speed  
11 crossovers shall not be used between tracks having track centers of under 16.50. For other  
12 spatial requirements see the *Trackway Clearances* chapter.

## 4.11 Low and Medium Speed Crossovers

13 The essence of a crossover is 2 turnouts connected at their frog ends. This occurrence is  
14 common. The distance of concern in crossovers is the central tangent, shown as “Tangent” in

### Figure 4-5: Low and Medium Speed Crossovers



Number	9	11	15	20	24
Defined Angle	6d21m35s	5d12m18s	3d49m06s	2d51m51s	2d23m13s
Radius	620 feet	950 feet	1750 feet	3250 feet	4650 feet
Allowed Speed	20 mph	25 mph	35 mph	45 mph	55 mph
Length (L) end to end of crossover, 15.00 feet track centers	203.47 feet	251.02 feet	341.42 feet	462.31 feet	553.60 feet
PITO to PITO distance on tangent, 15.00 feet track centers	134.58 feet	164.66 feet	224.75 feet	299.82 feet	359.85 feet
Change in length per 1.00 foot change in track centers, either of the above	8.972 feet	10.978 feet	14.983 feet	19.988 feet	23.990 feet
Tangent length on diagonal, 15.00 feet track centers	66.53 feet	78.98 feet	108.58 feet	137.69 feet	166.41 feet
Change in length per 1.00 foot change in track centers	9.03 feet	11.02 feet	15.02 feet	20.01 feet	24.01 feet

Page 4-17  
February 2016, Rev. 3

1  
2 Since small radii curves in turnouts result in short component life and working the equipment  
3 to near its limits of movement is undesirable, it is recommended that the turnouts in crossover  
4 be Number 11 or larger. It is also recommended to keep the track centers at 15.00 feet or larger  
5 for this and other reasons.

## **4.12 Double Crossovers (Scissors Crossovers)**

6 Where space is constrained and crossovers allowing universal moves are desired, crossovers  
7 may be overlapped to form a double crossover. This form of crossover is sometimes called a  
8 scissors crossover, as on some systems the term “double crossover” means two single  
9 crossovers of opposite hand placed in succession.

10 Double (scissors) crossovers shall be used only where their use keeps other aspects of the  
11 alignment from being reduced to less than minimum values due to their high cost and  
12 maintenance requirements. Double crossovers using high-speed turnouts shall not be used  
13 unless the track centers are wide enough that the crossing diamond may be straight, and  
14 preferably where the crossing angle is equal to or less than that in a Number 15 double  
15 crossover.

16 The following double crossovers may be used:

- 17 • Number 9 (Shall not be used in main tracks)
- 18 • Number 11 at 15.00 feet or larger track centers (Shall not be used in main tracks)
- 19 • Number 15 at 15.00 feet or larger track centers

20 Double (scissors) crossovers with frog angles larger than that of a Number 15 turnout require  
21 movable center frogs, and therefore should be used only where use of smaller crossovers affects  
22 run time.

## **4.13 Track Layout along Station Platforms**

23 Turnouts connecting the platform track with the main track shall permit an operating speed of  
24 60 mph or faster. Other than the main line turnouts, the normal train operation into or out of  
25 the platform shall not pass through the curved side of turnouts.

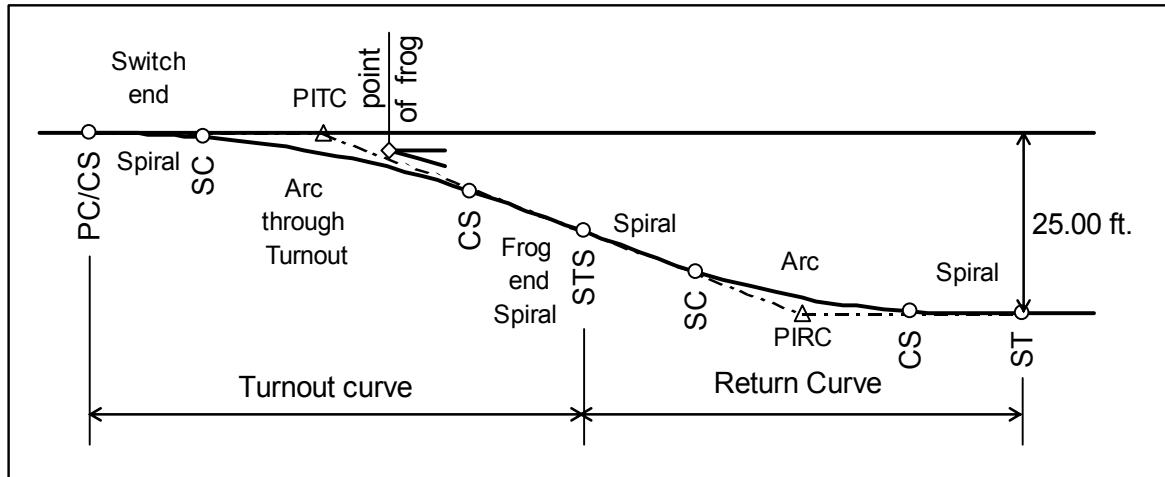
26 The station platform track between the entry turnout and the exit turnout along the main track  
27 shall have a 3,350 foot minimum length centered symmetrically on the midpoint of the station  
28 platform.

29 Platform tracks shall be tangent through the platform length and to a distance of not less than  
30 75 feet beyond the ends of the platform. If the platform track must be curved, the largest

practical radius of curve shall be used, and other means used to provide for accessibility in accordance with Americans with Disabilities Act (ADA) requirements.

Other tracks connected to platform tracks shall turn out of the tangent portion of the platform tracks. Turnouts shall be placed not less than 75 feet beyond the ends of the platform.

**Figure 4-6: Detail of Station Entry/Exit High-Speed Turnout and Return Curve**



**Table 4-9: Geometry of Station Entry/Exit High-Speed Turnouts and Return Curves**

Geometry of Connection and its Segments, in feet unless stated otherwise			
Design Speed	60 mph	80 mph	110 mph
Platform Track Offset	25.00	25.00	25.00
Turnout Entry Radius	10,000.00	18,000.00	34,000.00
Turnout Body Radius	5,000.00	9,000.00	17,000.00
Switch Spiral Length	90.00	120.00	160.00
Frog Spiral Length	90.00	120.00	160.00
Return Curve Radius	4,000.00	7,000.00	13,500.00
Curve Spiral Length	90.00	120.00	160.00
Total Length along main track	743.65	991.80	1,364.60
Total Length along Platform Track	744.25	992.25	1,364.92
Angle at STS	3d44m07s	2d 48m04s	2d02m17s
Length of Entry Curve	0.000	0.000	0.000
Length of Turnout Body Curve	213.47	290.02	404.71
Length of Return Curve	170.78	222.24	320.21

**Notes:**

Values in table are for illustration purposes and so are generally given to 2 decimal places. This is not to be construed as the necessary limit for the alignment calculations.

The turnouts for storage and refuge tracks at passenger stations will depend upon the operational requirements. Turnouts smaller than Number 11 shall not be used. Spirals need not be applied to the return curve for a stub end track. If the track is for yard access instead of to storage, a spiral appropriate to the design speed of the access track shall be applied.

#### 4.14 Access Tracks to Yards and Maintenance Facilities

The criteria contained in this section are intended for the geometric design of tracks connecting the mainline to yards, Maintenance of Infrastructure (MOI), terminal layup and storage facilities. See section 4.13 for the geometric design of tracks connecting the mainline to station platforms.

Site constraints may lead to large distances between mainline access points and these facilities. For the purpose of minimizing time required to clear revenue tracks, these tracks shall be designed much like a secondary mainline railroad. The design speeds of the turnouts that are used between the mainline and these tracks shall be 60 mph. The design speed of crossovers between main tracks associated with these turnouts shall be 60 mph unless they also serve another purpose that requires a higher speed.

The minimum length between mainline turnout fouling point and the first yard or MOI turnout shall be not less than 1600 feet. The following are the minimum/maximum design parameters for these tracks

- Design speed: 60 mph, site conditions permitting. Where conditions do not permit 60 mph, a lower design speed may be used. This lower design speed shall be as high as site conditions permit.
- Minimum Curve Radii: 900 feet
- Maximum Actual Superelevation ( $E_a$ ): 3 inches
- Maximum Unbalanced Superelevation ( $E_u$ ): 3 inches
- Spiral Lengths (Clothoid): 62 feet per inch of superelevation or unbalanced superelevation, whichever gives the greatest length
- Minimum Length of Tangent between curves in the same direction: 0 feet. Compound curves must be joined by spirals of length equal to 62 feet per inch of change in superelevation or unbalance, whichever gives the greater length
- Minimum Length of Tangent between reversing curves. The length may be reduced by one-half the combined lengths of the adjacent spirals.  $L_{min} = 9,400,000 / (R_1)^2 + 9,400,000 / (R_2)^2$ , but not less than 40 feet
- Recommended Turnouts: not less than Number 15
- Minimum Turnouts: Number 11



- Minimum Track Centers, not including allowance for Overhead Contact System (OCS) poles, drainage, walkways, roadways, or other facilities that will be placed between tracks in some areas: 15.00 feet
- Minimum Track Centers on small radius curves may need to be larger than the values given above. If the following calculation results in a larger value, this value shall be used:  $14.75 + 1,100 / \text{Radius (in feet)}$ , but not less than 15.00 feet
- Maximum Grade: 2.50 percent
- Vertical Curves: 100 feet minimum length with a rate of change of not more than 1.00 percent per 100 feet

## 4.15 Yards Tracks

The specific track arrangement for each yard will depend upon the purpose of the yard and tracks in the yard. Therefore the basic layout will be determined by operational requirements. The requirements developed in this chapter are therefore limited to those of a general nature except for those relating to geometric constraints due to:

- Curvature related constraints due to vehicle characteristics
- Track length constraints due to train and individual vehicle length
- Profile and grade related issues

Other than the tracks connecting the yards to the revenue tracks, the design parameters for these tracks are speed-independent.

### 4.15.1 Connecting and Switching Tracks Inside Yards

The following standards apply to tracks on which trains will not be stored or left standing but are installed for the purpose of connections between yard tracks and yard access tracks within the area designated as yards, all types, and other low speed tracks.

- Minimum Curve Radii: 620 feet
- Minimum Length of Tangent between curves in the same direction: 0 feet (compound curve)
- Minimum Length of Tangent between reversing curves:  $L_{\min} = 9,400,000 / (R_1)^2 + 9,400,000 / (R_2)^2$ , but not less than 40 feet.
- Minimum Turnout Number: 9 (internal radius 620 feet). If in a track with high volume traffic, the minimum shall be a Number 11.
- Minimum Track Centers: 15.00 feet
  - On small radius curves, minimum track centers shall be increased if the following calculation results in a larger value:  $14.75 + 1,100 / \text{Radius (in feet)}$ , but not less than

15.00 feet. This value does not include allowance for OCS poles, drainage, walkways, roadways, or other facilities that will be located between tracks.

- Track centers shall be increased for OCS poles, light poles, drainage, signal masts, equipment cases, walkways, service aisles or other facilities placed between tracks. Maximum Grade: 2.50 percent
- Minimum Length of Vertical Curve: 50 feet with a rate of change of not more than 2.00 percent per 100 feet.

For additional criteria on walkways and service aisles see *Civil* chapter.

#### 4.15.2 Servicing and Storage Tracks

The following standards apply to those portions of tracks on which trains or equipment will be left standing, serviced, or stored and do not apply on the approach portions of those tracks. These standards apply only to the usable length of track and any overrun distances or, in the case of stub end tracks, the portion between usable length and the bumping post or other end of track device.

- **Usable Length of Track** – The usable length of track is defined as the length of track which is usable for its defined purpose. Usable length does not include space for bumping posts or other end of track devices, defined set back from the end of track device, defined set back from signals, space occupied by road crossings, turnouts to other tracks, and any other feature that render the equipment on the track inaccessible to service, if the purpose of the track is to hold equipment while being serviced, or unusable for storage if the purpose of the track is to store passenger trains or other equipment.

Usable length of track for train servicing and storage tracks is defined based on the maximum potential train length. Sufficient length beyond train length to hold a switch engine shall also be provided. Minimum length shall be 1400 feet.

Usable length of track for other purposes: For tracks not intended to hold full length trains, the usable length shall be defined by the length of equipment that it is intended to hold plus some allowance for placement of equipment, and desirably additional length sufficient to hold a switch engine. Minimum length shall be 75 feet plus the length to be occupied by equipment.

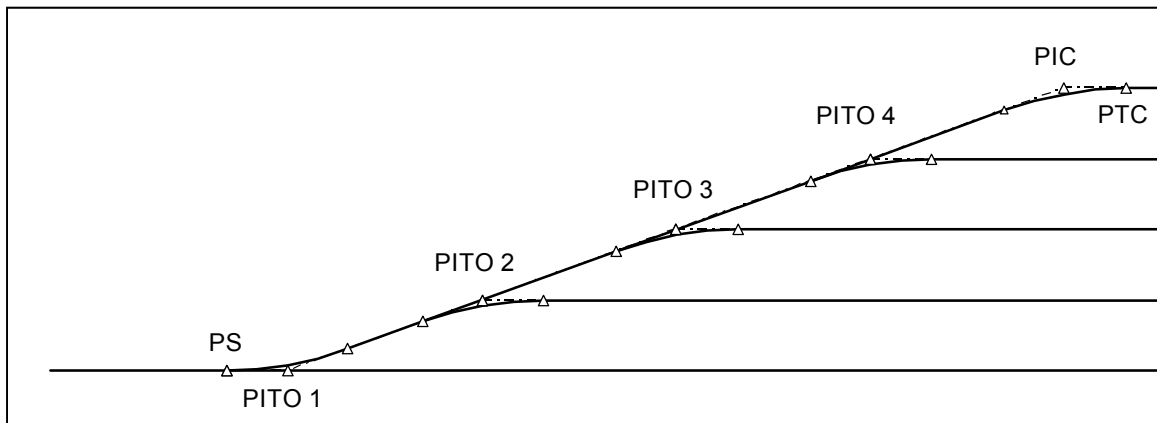
- **Minimum Curve Radii for curves within the usable length** – 10,000 feet
- **Minimum Grade within the usable length** – between 0.00 percent and 0.20 percent or between 0.30 percent and 0.50 percent down from the access point. In the case of double ended tracks, the low end access track shall not be lower than the highest point within the portion designated as usable length.
- **Minimum Length of Vertical Curves** – 50 feet minimum length with a rate of change of not more than 1.00 percent per 100 feet.

- **Minimum Track Centers, between tracks on which servicing of equipment will be performed** – alternating spacing of 28.00 feet and 20.00 feet. These track centers provide space between tracks for roadways on the wider centers and cart paths or walkways on the narrower centers. However, these do not include allowances for OCS poles, light poles, drainage, signal masts, electrical cases, inspection platforms and pits, or other facilities that may interfere with the use of the aisles as traveled ways. Wider track centers shall be provided where these facilities are needed.
- **Minimum Track Centers, between tracks on which no servicing of equipment will be performed** – 15.00 feet. Wider track centers shall be provided if OCS poles, light poles, drainage, signal masts, electrical cases, major walkways or other facilities must be placed between tracks.

#### 4.15.3 Simple Track Ladders

A track ladder is a series of turnouts used to connect a group of parallel tracks to each other in conjunction with either an approach track or a stub end track to permit equipment to be accessed or shuttled between tracks. The most common form of connection of multiple parallel tracks is a straight ladder, also called a simple ladder. A simple ladder is a series of turnout connected end to end so as to access all the parallel tracks. Its primary advantage is its simplicity in design, construction and maintenance. Its disadvantage is its length when more than a few tracks are involved.

**Figure 4-7: Simple Ladder (4 Tracks Illustrated)**



Calculation of the points on these ladders is straightforward. The Point of Switch (PS) to Point of Intersection of the Turnout (PITO) 1 dimension is a property of the turnout used. The PITO 1 to PITO 2 and PITO 2 to PITO 3 and so forth lengths parallel to the tracks are simply track spacing divided by the tangent of the frog angle of the turnout. PI to PI lengths along the ladder track are track spacing divided by the sine of the frog angle of the turnout. When summed and the length of the final curve tangent added, the length of the entire ladder is determined.

1 Dimensions for the basic ladder connecting tracks at 15.00 centers using Number 11 turnouts  
2 are as follows:

- 3 • Between PITO's parallel to the lead track: 164.66 feet
- 4 • Between PITO's on the ladder track: 165.34 feet
- 5 • Total distance, PS entry turnout to curve PT for the case illustrated: 745.01 feet
- 6 • Total PS to PS distance for double ended tracks with 1,500 feet clear length: 2,990 feet
- 7 • Length utilized by ladder for each additional track: 329.33 feet (double ended)

8 When more than a few tracks are involved, the total length of this arrangement quickly becomes  
9 impractical, particularly where track centers are large. Thus, the need for compound ladders to  
10 shorten the overall yard length.

#### **4.15.4 Double Angle Track Ladders**

11 Considerable space can be saved by use of double angle ladder tracks, as the larger angle  
12 considerably reduces the length required to achieve the required offsets. The following is  
13 provided for assistance in understanding the design of multi-track ladder tracks.

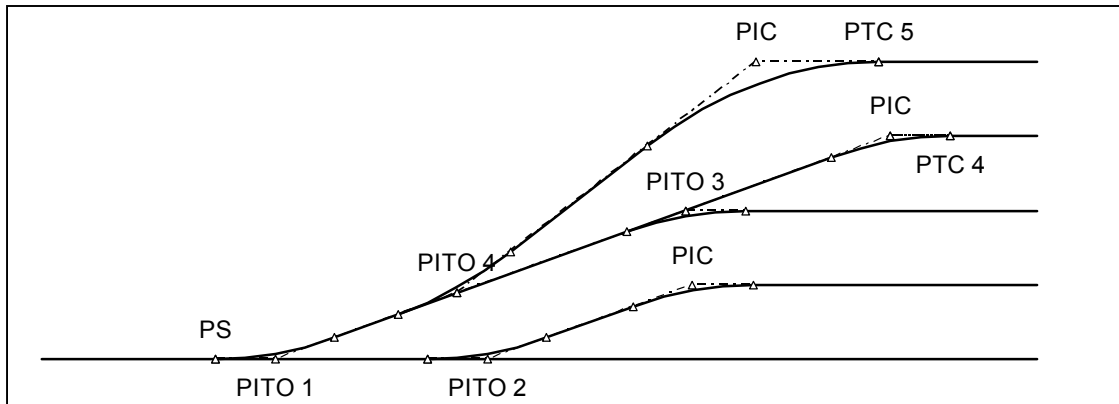
14 First, look at the situation with the same number and spacing of tracks as used in the simple  
15 ladder illustration. The single frog angle ladder using Number 11 turnouts and 15.00 feet track  
16 centers was 745 feet long from first point of switch to the point of development of the full width.  
17 By taking only one track off the outside, the length is reduced to approximately 580 feet, a  
18 saving in length of over 320 feet if the yard is double ended.

19 This method can be carried forward with additional tracks to whatever extent is necessary. The  
20 greater the number of tracks, the greater is the saving in length. For the illustrated six diverging  
21 track arrangement, the length from beginning point to end of last curve is about 734 feet, using  
22 Number 11 turnouts. The same number of tracks using a simple ladder would utilize  
23 approximately 1074 feet. Thus, for a double ended arrangement, the length saving is 680 feet.

24 The greater the number of tracks, the greater is the savings in overall yard length. For large  
25 numbers of tracks, the arrangement can be carried at least one step further to go to a triple  
26 ladder. Figure 4-8 and Figure 4-9 illustrate the nature of these savings.

27 When developing this form of track arrangement, the need to provide space for switch  
28 machines must not be overlooked. In addition, with these more complex track ladder  
29 arrangements, consideration must be given to the location of OCS poles since complex track  
30 layouts equate to complex overhead wiring layouts, including the need for wire termination  
31 poles and downguys.

**Figure 4-8: Double Ladder (4 Diverging Tracks Illustrated)**

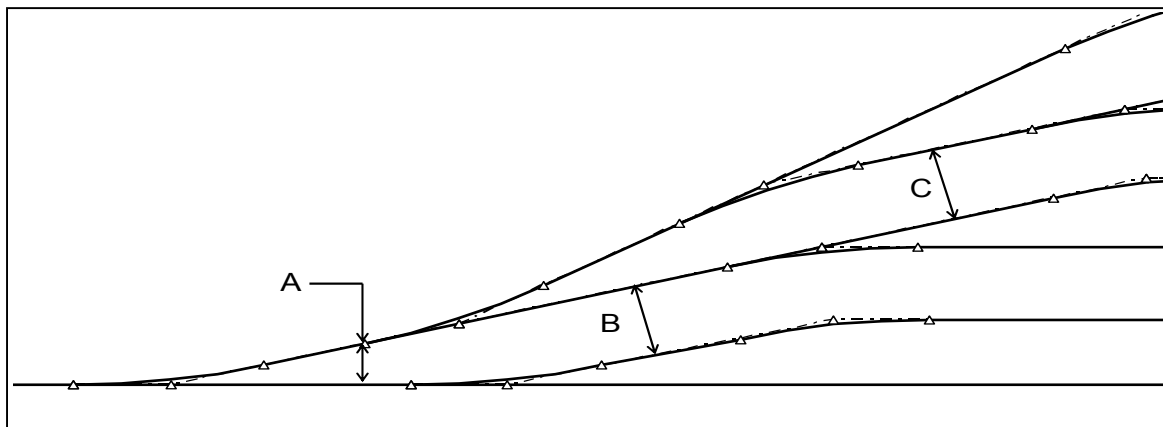


The following considerations shall be used in the development of these designs:

- **Separation at switch point** – Recommended: 9.00 feet, Minimum: 9.0 feet.
- **Space between track centers** – Recommended: 20.0 feet, Minimum 18.0 feet.
- **Space between track centers with switch points approximately opposite** – Recommended: 25.0 feet; Minimum 20.0 feet; if at least one switch machine can be turned away.

The above considerations are required to provide space for the switch tie sets of adjoining turnouts to fit together without overlapping. While overlapping tie sets are constructible, these are undesirable because they create the need for non-standard, site-specific ties and fixtures that add to yard cost and complexity. These space requirements generally will provide adequate clearance for switch machines to be located clear of adjacent tracks. However, the specifics of each yard layout may create localized conditions of interference. Ultimately the yard ladders must be laid out with dimensionally accurate switch machines and tie layouts, and adjacent roads and facilities must be overlaid to verify fit.

**Figure 4-9: Double Ladder, Track Space Requirements (6 Diverging Tracks Illustrated)**



## 4.16 Operations and Maintenance Considerations

Universal crossovers between main tracks shall be located no farther than one mile from the station platform track entry and exit turnouts at each end of the station. Additionally, universal crossovers between main tracks shall be located nominally 20 miles apart, no greater than 25 miles apart, and spaced out equidistantly between the universal crossovers at consecutive stations. Universal crossovers shall be capable of handling diverging moves at speeds of 80 mph.

Crossovers shall be located on the main tracks no farther than 0.5 mile from the mainline track turnouts at each end of the MOI facility or siding.

Turnouts shall be located so that the point of switch on one end of the turnout and/or the point of frog on the other end is/are a minimum of 150 feet from any grade separated structure passing over the HST tracks.

## Chapter 5

### Trackwork

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	13 Mar 12	Initial Release, R0
1	May 2013	Revision 1





## Table of Contents

<b>5</b>	<b>TRACKWORK</b>	<b>5-1</b>
5.1	Scope	5-1
5.2	Regulations, Codes, Standards, and Guidelines	5-1
5.3	Interfaces and General Requirements	5-2
5.3.1	In-Service Conditions	5-2
5.3.2	Interface between Track and Track Support Systems	5-2
5.3.3	Clearances around Tracks	5-2
5.3.4	Maintenance Considerations	5-3
5.3.5	Track Drainage	5-3
5.3.6	Seismic and Ground Movement Considerations	5-3
5.3.7	Electric Properties	5-3
5.3.8	High-Speed Tracks	5-3
5.3.9	Lower Speed Main Tracks and Station Tracks	5-3
5.3.10	Yard, Storage, and Low Speed Tracks	5-4
5.4	Rail	5-4
5.4.1	Selected Section	5-4
5.4.2	Use of Standard and High Strength Rails	5-6
5.4.3	Rolling Tolerances and Straightness of Rails	5-6
5.5	Rail Welding and Joints	5-8
5.5.1	Rail Welding Standards	5-8
5.5.2	Zero Stress Temperature	5-8
5.5.3	Bolted Joints – Insulated and Non-insulated	5-8
5.6	Rail Fastenings	5-10
5.7	Other Track Material	5-10
5.7.1	Emergency Guard Rails	5-10
5.7.2	Bumping Posts – Sliding and Fixed	5-10
5.7.3	Derails	5-10
5.8	Track Support Systems	5-11
5.8.1	Track At-Grade	5-11
5.8.2	Track on Concrete Base	5-12
5.8.3	Transitions between Trackforms	5-12
5.9	Non-Ballasted Track	5-12
5.9.1	Precast Segmental Slab Non-Ballasted Track	5-13
5.9.2	Transit System Type Plinth and Fasteners	5-14
5.9.3	Embedded Tie Blocks in Elastic Boots	5-14
5.9.4	Special Considerations in Seismically Active and other Unstable Areas	5-15
5.10	Ballasted Track	5-15
5.10.1	Concrete Ties	5-15
5.10.2	Wood Ties and Other Types of Ties	5-17
5.10.3	Ballast	5-17
5.11	Turnouts and Other Special Trackwork Components	5-17
5.11.1	Rails in Special Trackwork	5-18
5.11.2	Switch Rails	5-18
5.11.3	Turnout Frogs	5-20
5.11.4	Turnout Guard Rails (also called Check Rails)	5-24

5.11.5	Turnout and Special Trackwork Support Systems .....	5-25
5.12	Grade Crossings .....	5-26
5.12.1	Yard Traffic Grade Crossings .....	5-26
5.12.2	Main Line Pedestrian Crossings .....	5-26

## Tables

Table 5-1:	Rail Straightness Requirements .....	5-7
Table 5-2:	Continuous Welded Rail (CWR) Zero Stress Temperature (ZST) .....	5-8

## Figures

Figure 5-1:	AREMA 141 RE Rail .....	5-5
Figure 5-2:	Precast Segmental Non-Ballasted SlabTrack .....	5-13
Figure 5-3:	Low Vibration Track Embedded Tie Block .....	5-14
Figure 5-4:	Example of Low Vibration Track, Level .....	5-15
Figure 5-5:	Example of Low Vibration Track with Superelevation .....	5-15
Figure 5-6:	EN 60E1A3 Asymmetric Switch Rail (EN 13674-2 Figure A.7) .....	5-19
Figure 5-7:	AREMA Switch Tip Rounding – Side View of End of Switch Point .....	5-20
Figure 5-8:	Welded Boltless Manganese Frog .....	5-22
Figure 5-9:	Movable Point Frog (1,200 meter radius turnout in JIS 60 kg/m rail) .....	5-23
Figure 5-10:	Movable Point Frog: Rail Housing (Left) and Manganese Steel Cast Housing (Right) .....	5-23
Figure 5-11:	EN 33C1 Check Rail (EN 13674-3 Figure A.1) .....	5-24

## Acronyms

AAR	Association of American Railroads
AREMA	American Railway Engineering and Maintenance-of-Way Association
CFR	Code of Federal Regulations
CWR	Continuous Welded Rail
EFB	Electric Flash Butt
EN	European Standard (EuroNorm)
FRA	Federal Railroad Administration
LVT	Low Vibration Track
RBM	Rail Bound Manganese
WBM	Welded Boltless Manganese
ZST	Zero Stress Temperature

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015



## 5 Trackwork

### 5.1 Scope

1 This chapter provides the basis of design for track materials and components for both ballasted  
2 and non-ballasted trackforms. This chapter is not intended to be a specification for track  
3 materials or track construction.

4 The objective of this chapter is to define the standards and major components of track for design  
5 speeds of 250 mph. The track shall also be compatible with equipment having Association of  
6 American Railroads' running gear. Track gauge is standard—that is 56-1/2 inches measured  
7 between points 5/8 inch below the top of the head of the rail. The general basis for materials  
8 standards shall be the standards of the American Railway Engineering and Maintenance-of-  
9 Way Association (AREMA). Some reference to and use will be made of other standards,  
10 European and Asian, as appropriate. Exotic or complex components shall not be used any more  
11 than necessary for the track to perform its function.

12 Rails shall be continuously welded throughout, including through turnouts. Expansion joints  
13 shall not be used.

### 5.2 Regulations, Codes, Standards, and Guidelines

14 Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.  
15 Certain requirements of the Federal Railroad Administration (FRA), among them:

- 16 • Code of Federal Regulations (CFR)
  - 17 – Title 49, Part 213, Track Safety Standards, generally and also in particular Subpart G –
  - 18 Train Operations at Track Classes 6 and Higher.

19 Various codes and standards referenced herein include but are not necessarily limited to the  
20 following:

- 21 • American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for  
22 Railway Engineering, in particular the following chapters:
  - 23 – Chapter 1: Roadway and Ballast
  - 24 – Chapter 4: Rail
  - 25 – Chapter 5: Track
  - 26 – Chapter 17: High-Speed Rail Systems
  - 27 – Chapter 30: Ties

- American Railway Engineering and Maintenance-of-Way Association (AREMA) Portfolio of Trackwork Plans, commonly referenced as the AREMA Portfolio.
- European Standards (EN for European Norms)
  - EN 13674-2, Railway Applications – Track-Rail-Part 2: Switch and Crossing rails used in conjunction with Vignole railway rails 46 kg/m and above
  - EN 13674-3, Railway Applications – Track-Rail-Part 3: Check rails

## **5.3 Interfaces and General Requirements**

### **5.3.1 In-Service Conditions**

Track and track components shall be designed and fabricated to be durable and perform under the prevailing and extreme climatic and environmental conditions occurring within the geographic extent of the system. For climatic conditions, see the *General* chapter.

### **5.3.2 Interface between Track and Track Support Systems**

Prior to placement of tracks, structures and embankments shall be surveyed to ensure compliance with the contract documents.

For track-structure interaction, refer to the *Structures* chapter.

For grounding and bonding requirements of the track support structure, refer to the *Grounding and Bonding Requirements* chapter.

### **5.3.3 Clearances around Tracks**

**Between the Rails** – There shall be nothing between the rails higher than 4 inches below the plane of the top of rails with the exception of required power and signal devices and connections, joint bars and bolts, and turnout rails which include frog guard rails.

**Outside the Rails** – There shall be nothing outside the rails higher than 4 inches below the plane of the top of rails to a minimum of 6.50 feet offset from track centerline with the exception of 6.25 feet offset for long aerial structures and with the exception of required power and signal devices and connections, rail braces, joint bars and bolts, and turnout rails.

Clearances related to passage of vehicles and offsets to fixed elements above the plane of the rails are described in the *Trackway Clearances* chapter.

These requirements do not apply to station platforms or end steps and ramps down from station platforms that have offsets not less than 6 inches greater than the platform edge offset. These requirements do not apply to non-revenue tracks that have speed limits of 25 mph or less.

#### 5.3.4 Maintenance Considerations

Durability, low maintenance, and ease of maintenance shall be a primary consideration in selection of track types and track components. Maintainability shall be second only to performance requirements in selection of appropriate components. Components that require adjustments shall be avoided. Components that require full or partial disassembly to be inspected shall be avoided.

Under normal circumstances, maintenance work will be performed outside of normal service hours.

#### 5.3.5 Track Drainage

Positive drainage shall be provided for spaces between the tracks. Refer to the *Drainage* chapter for trackway drainage requirements.

#### 5.3.6 Seismic and Ground Movement Considerations

The alignment crosses several faults, both active and inactive. The potential frequency and magnitude of movement for each of these shall require analysis and specific appropriate mitigations developed on a case-by-case basis. Use of non-ballasted track across these zones shall be the base case for the analysis. Refer to the *Seismic* chapter.

#### 5.3.7 Electric Properties

The minimum electrical resistance from rail to rail shall not be less than 10 ohms per thousand feet.

The minimum electrical resistance from rail to rail shall not be less than 20,000 ohms for each set of rail supports.

#### 5.3.8 High-Speed Tracks

Homogeneity in track type is recommended. Frequent changes in trackform shall be avoided. The continuous minimum length of any trackform shall not be less than the minimum length of alignment segment for a run time of 1.8 seconds at the design speed of the segment, as specified in the *Track Geometry* chapter.

#### 5.3.9 Lower Speed Main Tracks and Station Tracks

Non-ballasted track shall be used for all tracks adjacent to and to 75 feet beyond the end of station platforms. Use of non-ballasted track is recommended for all tracks carrying revenue trains, lead tracks into yards, and stub end storage and refuge tracks associated with stations.

### **5.3.10 Yard, Storage, and Low Speed Tracks**

- 1 Yard, storage, and low speed tracks may be either ballasted or non-ballasted tracks. Ballasted
- 2 track is the recommended trackform in yards. There will also be special track types in certain
- 3 areas of the yards and shops, such as embedded rails in shop floors, pedestal supported tracks
- 4 over pits, and other non-standard types.

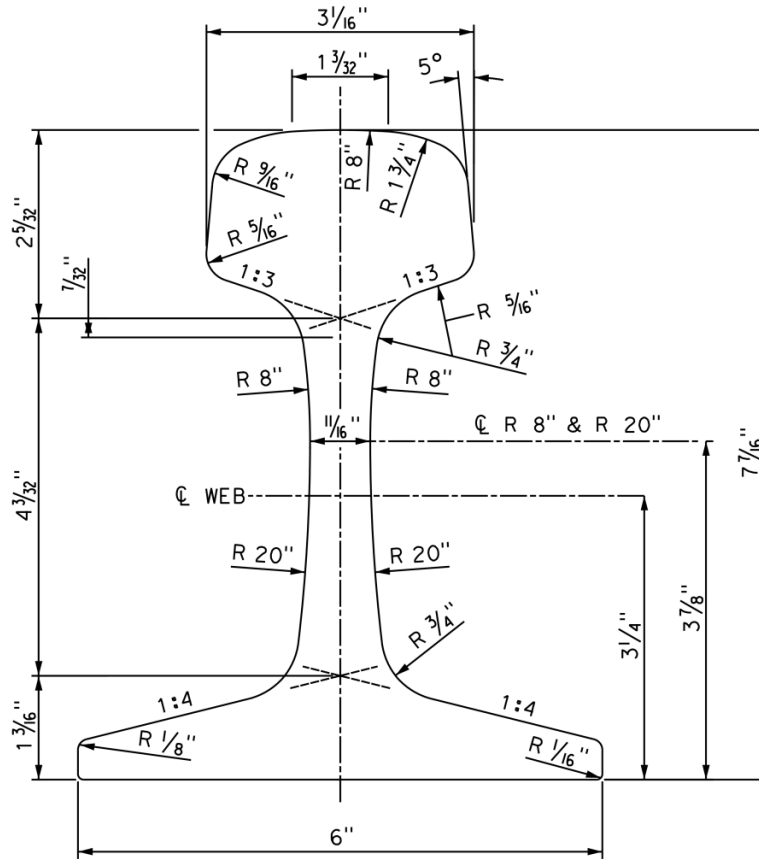
## **5.4 Rail**

### **5.4.1 Selected Section**

- 5 The 141RE rail section shall be used for high-speed tracks on the California High-Speed Train
- 6 Project (CHSTP).
- 7 The outline and properties of the 141RE section as given in the AREMA Manual, Chapter 4,
- 8 Part 1 are presented in Figure 5-1:



1 **Figure 5-1: AREMA 141 RE Rail**



## 141 RE RAIL SECTION

1. Rail Area, head	5.3724 inch <sup>2</sup>
Rail Area, web	3.5547 inch <sup>2</sup>
Rail Area, base	4.8701 inch <sup>2</sup>
Rail Area, whole rail	13.7972 inch <sup>2</sup>
2. Rail Weight (based on specific gravity of rail steel = 7.84)	140.7002 lb/yd
3. Moment of Inertia about the neutral axis	100.44 inch <sup>4</sup>
4. Section modulus, head	25.24 inch <sup>3</sup>
Section modulus, base	28.97 inch <sup>3</sup>
6. Lateral moment of inertia	14.91 inch <sup>4</sup>
7. Lateral section modulus, head	9.74 inch <sup>3</sup>
Lateral section modulus, base	4.97 inch <sup>3</sup>
8. Height of shear center above base	1.88 inches
9. Torsional rigidity is 'KG' Where G is the modulus of rigidity and: K = (error for K >10%)	5.97 inch <sup>4</sup>

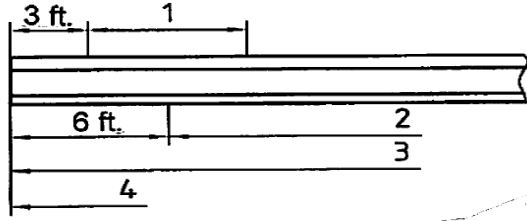
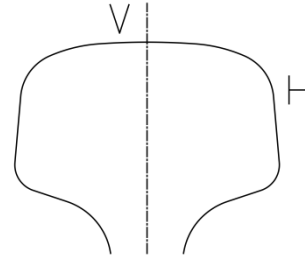
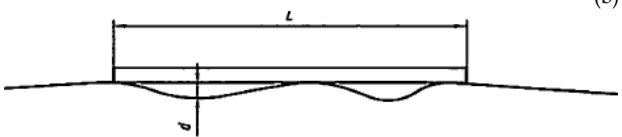
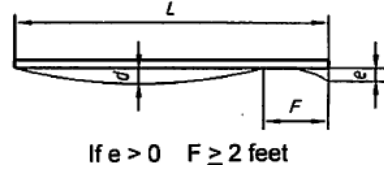
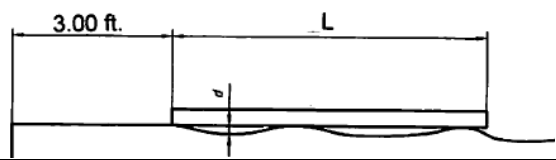
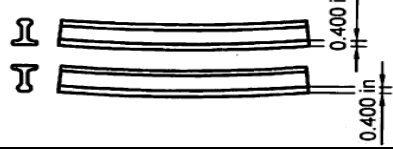
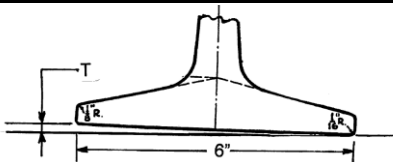
#### **5.4.2 Use of Standard and High Strength Rails**

- 1 Running rail, rails in turnouts including switch point rails, frog rails, and guard rails shall
- 2 comply with the metallurgy, hardness, and strength requirements of the AREMA Manual,
- 3 Chapter 4, Part 2, Standard Strength or High Strength as appropriate.
- 4 Either Standard Chemistry or Low Alloy Chemistry may be used for standard strength rail.
- 5 Low Alloy Chemistry shall be used for high strength rail.
- 6 Standard Strength Rail shall be used throughout except in the following areas where High
- 7 Strength Rail shall be used:
- 8
  - Revenue service tracks having a radius of 2,000 feet or less
- 9
  - Yard lead tracks having a radius of 1,500 feet or less

#### **5.4.3 Rolling Tolerances and Straightness of Rails**

- 10 Straightness surface flatness and twist tolerances given in Table 5-1 shall be substituted for
- 11 those given in the AREMA Manual, Chapter 4.

Table 5-1: Rail Straightness Requirements

					
1. Overlap 2. Body 3. Whole Rail 4. End "E"				1. V and H Location of flatness measurements. 2. The position of H is 5/8" below top of rail.	
		Class A			
		d	L		
BODY FLATNESS <sup>(a)</sup>	Vertical V	$\leq 0.012$ in.	10 ft. <sup>(c)</sup>		
	Horizontal H	$\leq 0.018$ in.	5 ft. <sup>(c)</sup>		
ENDS <sup>(a)</sup>	End "E"	6 ft.		 If $e > 0$ $F \geq 2$ feet	
	Vertical straightness	$d \leq 0.016$ in.	6 ft.		
	Horizontal straightness	$d \leq 0.024$ in.	6 ft.		
OVERLAP <sup>(a)</sup>	Length of overlap	6 ft.			
	Vertical flatness V	$d \leq 0.012$ in.	6 ft. <sup>(c)</sup>		
	Horizontal flatness H	$d \leq 0.024$ in.	6 ft. <sup>(c)</sup>		
WHOLE RAIL	Upsweep and downsweep	0.40 in.			
	Side sweep	Curve radius $R > 5000$ ft.			
TWIST	Whole Rail	$T \leq 0.100$ in.			
	Within any 3 ft.	$T \leq 0.020$ in.			

Notes:

- (a) Automatic measurement equipment shall measure as much of the rail as possible but, at least the body. If the whole rail satisfies the body specification, then measurement of end and overlap is not mandatory.
- (b) Automatic measurement techniques are complex and are therefore difficult to define but finished rail flatness shall be capable of being verified by straight edge as shown in the above figure.
- (c) 95% of delivered rails shall be within limits specified with 5% of rails allowed outside tolerances by 0.004 inch.
- (d) Reference L sliding over end E.

## 5.5 Rail Welding and Joints

Rails in all tracks shall be welded and welded by the Electric Flash Butt (EFB) welding method to the greatest extent practical. The lengths of strings of welded rail shall be as long as practical. Use of Thermit welds shall be limited to locations and conditions where use of EFB welding is impractical. Use of bolted joint bars shall be limited to insulated joints and other locations as described in Section 5.5.3.

### 5.5.1 Rail Welding Standards

**AREMA Manual Requirements** – Welding of rail shall be in accordance with the standards of the AREMA Manual Chapter 4, Rail; Part 3, Joining of Rail and the following sections.

- **Straightness** – The straightness of welds shall be to standards not less than that for the rail between welds.
- **Staggering of Welds** – Welds in opposite rails of the same track shall not be set square with each other. They shall be staggered. The amount of stagger shall be in the range of 15 to 33 feet. In constrained locations such as turnouts, the stagger shall be between two and three spacings of the rail support.

### 5.5.2 Zero Stress Temperature

Rails shall be installed so as to be free of longitudinal stress, that is in neither compression or tension at the temperatures given in Table 5-2.

**Table 5-2: Continuous Welded Rail (CWR) Zero Stress Temperature (ZST)**

Section Limits	ZST, Open Air (°F)	ZST, Tunnel (°F)
San Francisco to 22 miles south of San Francisco	110	90
22 miles south of San Francisco to east side of Pacheco Pass	115	95
East side of Pacheco Pass to 30 miles south of Bakersfield	120	100
Sacramento to Junction with SF – LA line	125	105
30 miles south of Bakersfield to 25 miles north of Los Angeles	115	95
25 miles north of Los Angeles to Los Angeles to Anaheim	120	100
Los Angeles to 35 miles north of San Diego	120	100
35 miles north of San Diego to San Diego	115	95

### 5.5.3 Bolted Joints – Insulated and Non-insulated

Rail joints not welded, both insulated and non-insulated, shall be bonded. Bonded joints shall have full contact D-bars of a cross section appropriate for the 141RE rail section. Joints shall be in accordance with the requirements of AREMA Manual Chapter 4, Part 3, Joining of Rail except

as modified herein. Joints shall not to be installed either square or at the same spacing as axle spacings in the trucks. In certain shop tracks in or adjacent to buildings, the use of non-bonded joints may be necessary.

Joints in tracks carry in-service passenger trains or other tracks have a speed limit of 30 mph or more shall be eight-hole bars not less than 50 inches long. The fourth hole each way from center of bar and end of rail shall be spaced 6 inches from the third hole. The end 9 inches of the bar shall be tapered from full width of bar to not extend beyond the rail head at the end of bar. Joints in tracks outside yards and other facilities that do not carry in-service passenger trains and have a speed limit of less than 20 mph or less may have 6-hole bars.

Insulated joints shall be located to meet the requirements of the signal and train control system, refer to the *Automatic Train Control* and *Yard Signaling* chapters. Non-insulated joints may be used in locations where use of electric flash butt welds is impractical and use of thermit welds is inadvisable.

#### 5.5.3.1 Bonded Joints

When installed in revenue track having a design speed of 60 mph or higher, straightness shall be as follows:

- **Vertical misalignment** – Crown: +0.015 inch, Dip: zero, measured with a 36-inch straightedge
- **Horizontal misalignment** – 0.020 inch, measured with a 36-inch straightedge
- **Vertical offset** – 0.015 inch
- **Horizontal offset** – 0.010 inch

For tracks having design speeds lower than 60 mph, the straightness limits given in the AREMA Manual may be used.

#### 5.5.3.2 Joint Stagger

Joints in opposite rails of the same track shall not be set square with each other.

Insulated joints shall be staggered between 4 feet and 6 feet.

Non-insulated joint stagger shall be to the same limits as that given for welds.

#### 5.5.3.3 Non-Bonded Temporary Joints

Temporary joints shall have 36-inch-long, 6-hole joint bars in accordance with the requirements of the AREMA Manual, Chapter 4, Part 3. Temporary joints shall be staggered to the limits given in Section 5.5.3.2.

## **5.6 Rail Fastenings**

The spring clip shall be the Pandrol Fastclip or a clip of similar characteristics. Any alternative clip proposed shall be tested and all characteristics proven to be equal to or better than the following:

- The rail fastening system shall consist of the smallest practical number of pieces.
- Fastening systems shall be boltless to the greatest extent practical. Spring clips shall not be held in position by bolts or any other method that uses a separate piece beyond the spring clip insulator.
- The spring clip load deflection curve shall be within 15 percent of a straight line for deflections of between 0 and 3/8-inch. The zero position is defined as the spring clip position when fully applied to the rail base and with the rail firmly seated.
- Strength requirements per rail seat:
  - Longitudinal – not less than 2,400 pounds without slippage of the rail
  - Lateral movement of the rail base not more than 1/4-inch under a load of 8,000 pounds

## **5.7 Other Track Material**

Other track material consists of other track features that are common to both ballasted and non-ballasted tracks (e.g., guard rails, bumping posts, etc.).

### **5.7.1 Emergency Guard Rails**

Emergency guard rails and similar devices shall not be used.

### **5.7.2 Bumping Posts – Sliding and Fixed**

Fixed bumping post similar to the Hayes type shall be installed at the end of track in yards and shops, and other tracks where the design speed for trains or equipment is 20 mph or less.

Bumping post having either hydraulic or friction devices allowing a movement of not less than 10 feet shall be used at the ends of all tracks that are designed to carry revenue trains. The bumping post shall be capable of stopping a 16-car train at a speed of 5 mph.

Wheel stops may be used on tracks which are designed to hold only maintenance equipment and also have a design speed of 20 mph or less.

### **5.7.3 Derails**

The FRA requires that all tracks connecting to tracks carrying passenger trains operating at speeds above 110 mph shall be equipped with a derail that is interconnected with the signal system (49 CFR 213, paragraph 213.357). The type of derail is not specified in the CFR.

Derails shall be located at or beyond the point where track centers are 16.50 feet. For track centers less than 16.50 feet, the derail shall be located at or beyond the point where the tracks achieve their full defined track centers.

Each derail shall be clearly visible. When in a locked position, a derail shall be free of any lost motion which would prevent it from performing its intended function.

#### 5.7.3.1 Switch Point Derails

Each track, other than another track carrying revenue passenger trains, that connects with a track carrying revenue passenger trains shall be equipped with a switch point derail that is interconnected with the signal and train control system. The switch point derail shall be the same as the switch end of a No. 11 turnout up to the point of 18 inches separation between gauge lines.

The derail interconnection shall be such that the derail operation coincides with the turnout operation.

#### 5.7.3.2 Block Derails

Block derails shall be similar to the Hayes type. Block derails shall be applied on tracks used to store maintenance equipment and other locations where the speed limit will be 20 mph or less. They may be either powered or non-powered as the situation requires.

### 5.8 Track Support Systems

#### 5.8.1 Track At-Grade

The top of the trackbed shall be graded so that there is a 24:1 (H:V) minimum cross-slope towards the adjacent ditch, subdrain system, or embankment slope.

##### 5.8.1.1 Subgrade

Prepared subgrade is the trackbed that supports the railroad loads transmitted through the rails, ties, ballast, and subballast. Prepared subgrade shall be sloped and side ditches or subdrain system shall be provided to keep the subgrade free of standing water.

The designer shall analyze the existing subgrade and determine whether the material is considered to be compressible, soft, loose, or otherwise unsuitable to support the design dead and live loads. If the existing subgrade is compressible, soft, loose, or otherwise unsuitable as described in the Standard Specifications, then it shall be either (1) removed and replaced with approved compacted backfill or (2) stabilized with ground improvement techniques to ensure settlement criteria in the *Geotechnical* chapter.

##### 5.8.1.2 Subballast or Asphalt Underlayment

Where the ground is stable and soils are not subject to pumping through the subballast, subballast may be used under the track ballast. Where the ground is subject to pumping or the

1 subgrade material is subject to volume changes with changes in moisture content, an asphalt  
2 base course underlayment shall be used.

3 **Subballast** – The subballast material shall be based on the standard aggregate base course used  
4 by Caltrans in the geographic area of application. The minimum thickness of subballast shall be  
5 9 inches. A thicker layer may be required. The thickness shall be determined by analysis of the  
6 support required.

7 **Asphalt Underlayment** – The asphalt underlayment shall be based on the standard asphalt base  
8 course used by Caltrans in the geographic area of application. The minimum thickness shall be  
9 6 inches. The width of the asphalt section shall be beyond the influence line brought down to  
10 the bottom of the asphalt underlayment at a slope of 1:1 (H:V) but shall not extend beyond the  
11 toe of the ballast. The area between the edge of asphalt underlayment and top of slope or ditch  
12 line shall have a layer of subballast of the same or greater thickness as the asphalt  
13 underlayment.

## **5.8.2 Track on Concrete Base**

### **5.8.2.1 Track on Aerial Structures and Bridges**

### **5.8.2.2 Track in Tunnels**

### **5.8.2.3 Track in Trenches or Cut-and-Cover Structures**

## **5.8.3 Transitions between Trackforms**

14 For transition of embankments to structures, refer to the *Geotechnical* chapter. For transition of  
15 trackforms, refer to Standard and Directive Drawings.

## **5.9 Non-Ballasted Track**

16 The design depth from top of rail to the top of structure, subballast, or equivalent at the track  
17 centerline shall not be less than 2.50 feet, refer to Standard and Directive Drawings.

18 The average fastener spacing in this type of trackform shall be 27 inches. Rail supports need not  
19 be directly opposite each other. Spacing may be varied between 24 inches and 30 inches, with  
20 the average spacing being within  $\pm 1.0$  inches of the 27 inches over any 10 fasteners.

21 The vertical spring rate of the non-ballasted track system shall be designed to be 3000 lb/in/in.  
22 This requirement does not apply to tracks in yards.

23 It may be assumed that each fastener carries about 25 percent of the wheel load for purposes of  
24 deflection calculation. Deformation of the elastomer shall not exceed 10 percent of the thickness  
25 of the elastomer layer.



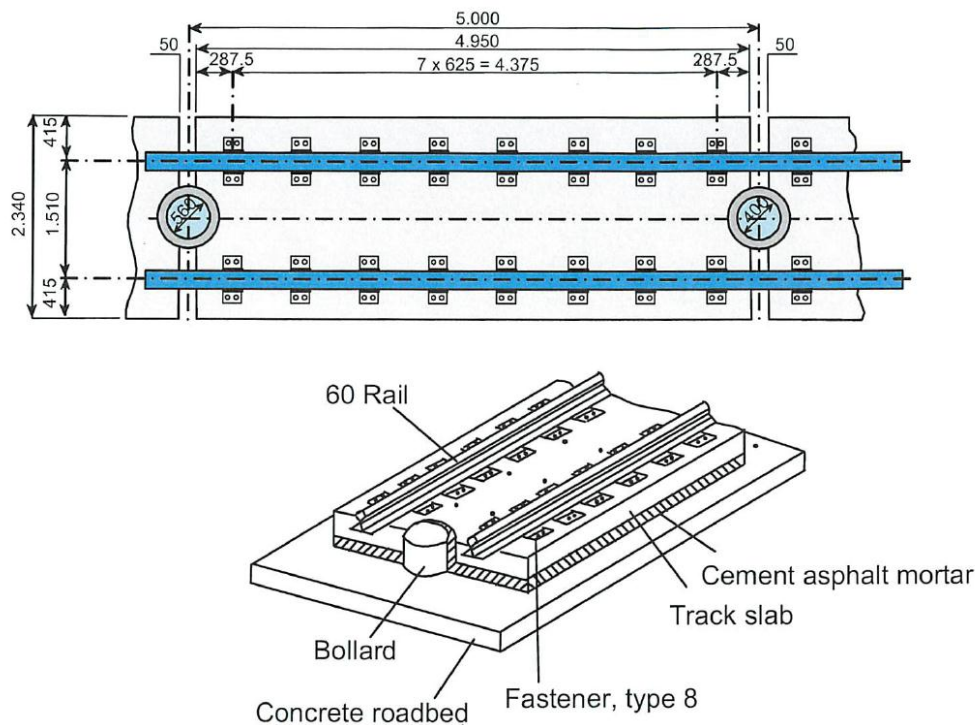
Provision for passage of signal and train control cables across the trackform and from outside to between the rails shall be provided. The location, spacing, and nature of these provisions will be shown on Standard and Directive Drawings.

For additional guidance on vertical alignment and grade limitation, refer to the *Track Geometry* chapter.

### 5.9.1 Precast Segmental Slab Non-Ballasted Track

The recommended trackform shall be the non-ballasted precast segmented slab type trackform. The precast slab system will be similar to the standard non-ballasted trackform used in the Shinkansen system. Figure 5-2 provides a typical layout of this trackform. A standard design for the CHSTP will be developed.

**Figure 5-2: Precast Segmental Non-Ballasted Slab Track**



Modifications and modernizations made to this system for application to the CHSTP should include the following:

- Use single piece transit system type rail fasteners instead of the multi-piece rail fasteners. See Section 5.9.2.
- Rationalization of dimensions
- Modification of bollard size and shape

- Development of a precast segmented slab track design specific to the California conditions and including “lessons learned” since the original design will follow at a later date.

## 5.9.2 Transit System Type Plinth and Fasteners

The transit system type plinth and fasteners may be used. This type is very familiar to contractors that have built tracks for transit systems in the U.S. It can be designed to be applicable for high-speed tracks.

The ratio between the quasistatic deflections and the dynamic deflections under forces applied and released at a rate approximating that of a train passing at full speed shall not be greater than 2:1 (H:V).

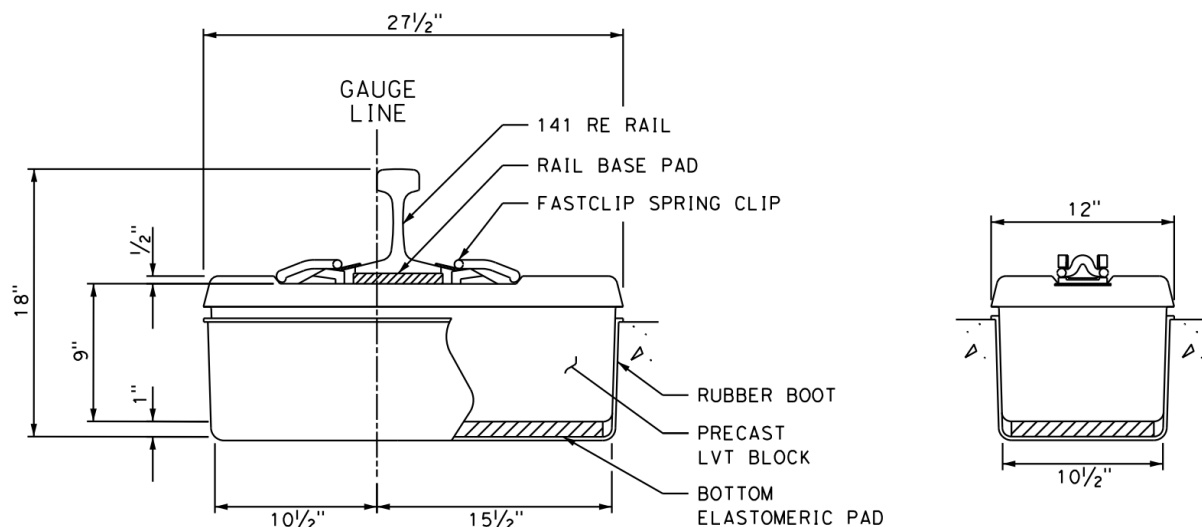
The fastener shall not be dependent upon the elastomer in shear to prevent excessive lateral or longitudinal movement of the rail in relation to the track slab.

## 5.9.3 Embedded Tie Blocks in Elastic Boots

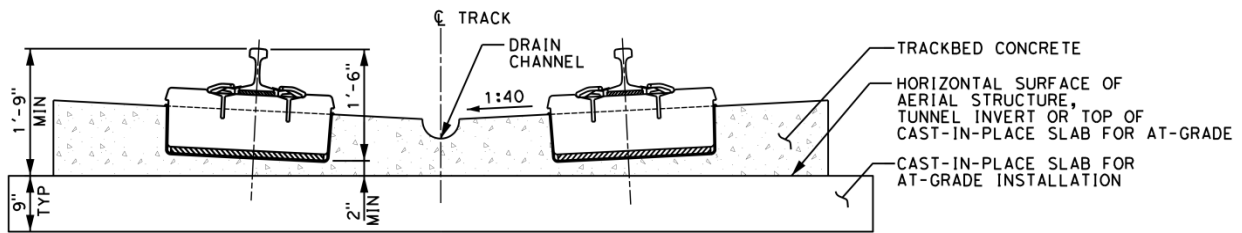
In areas where there is little or no potential for seismic activity, ground movement or structure movement, and maximum speed is 125 mph or less, a system consists of embedded tie blocks in elastic boots, commonly called Low Vibration Track (LVT) may be used.

For booted block systems, all the requirements for concrete tie fasteners shall apply, except the pullout force may be reduced to 6,000 pounds so long as all other requirements are met.

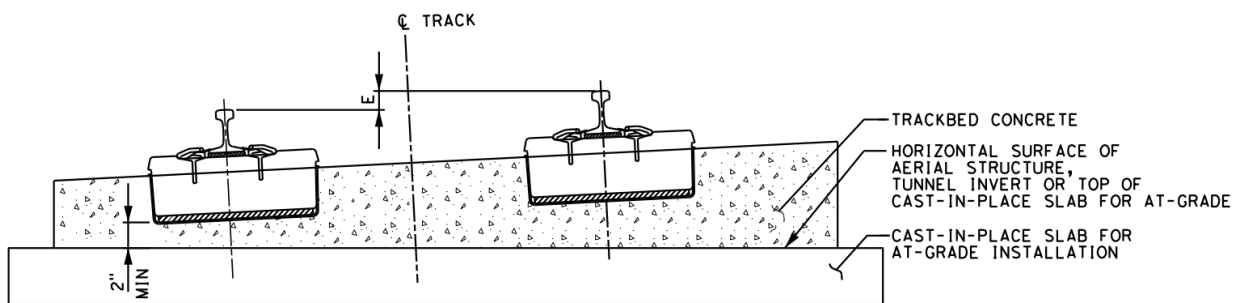
**Figure 5-3: Low Vibration Track Embedded Tie Block**



**Figure 5-4: Example of Low Vibration Track, Level**



**Figure 5-5: Example of Low Vibration Track with Superelevation**



#### 5.9.4 Special Considerations in Seismically Active and other Unstable Areas

In areas subject to ground movement from seismic activity, the Precast Segmental Non-Ballasted Slab track shall be the basis for the non-ballasted trackform. For these areas, the bollards shall be taller than normally required and the slab supporting the slabs shall be wider. The minimum length of this type of trackform and the exact amounts of the additional widening and heights shall be based on the potential amount of movement.

### 5.10 Ballasted Track

The design depth from top of rail to the top of structure, subballast, or equivalent at the track centerline shall not be less than 2.50 feet, refer to Standard and Directive Drawings. For ballasted structures, the design depth from top of rail to the structure deck or invert shall not be less than 2.75 feet, plus allowance for waterproofing layer where it is used.

For additional guidance on vertical alignment and grade limitation, refer to the *Track Geometry* chapter.

#### 5.10.1 Concrete Ties

Ballasted tracks shall be constructed with concrete crossties to the greatest extent practical.

### 5.10.1.1 Dimension and Properties

Concrete crossties shall be designed and fabricated in accordance with the requirements of the AREMA Manual Chapter 30, Ties, Part 4, Concrete Ties, including strength and other requirements with the following modifications:

- Ties – shall be scallop sided and equipped for the Pandrol Fastclip fastening system.
- Rail seats – shall provide a 1:40 (H:V) slope rail inclination.
- Weight – shall not be less than 600 pounds and not more than 750 pounds each.
- The minimum tie dimensions shall be the following:
  - Length – Crossties – Not less than 8.25 feet long and not more than 8.5 feet.
  - Width at bottom – Not less than 10.25 inches and not more than 11 inches
  - Width at top – Not less 7.25 inches and not more than 9 inches, but no greater than the bottom width minus 2.5 inches.
  - Thickness of crossties at ends – Not less than 8 inches and not more than 9 inches.
  - Thickness of crossties at center – Not less than 6.25 inches and no thicker than 0.50 inch less than the thickness at the ends.

### 5.10.1.2 Rail Fastening System

In addition to the requirements listed in Section 5.6, the following requirements shall apply:

- Bolted baseplates shall not be used on concrete ties in plain line.
- Where bolted baseplates are required, for example in turnouts, female inserts in the ties shall be used. There shall be no exposed threads or threads positioned so that a separate cap is required to cover the threads.
- Inserts in concrete ties shall be able to withstand a pull out force of 12,000 pounds. This applies to both spring clip inserts and insert to hold baseplates that support the rails.
- Inserts in concrete ties shall be able to withstand a torque of 250 foot-pounds. This applies to both spring clip inserts and insert to hold baseplates that support the rails.
- Rail seat pads shall be used between the rail base and the concrete tie.
  - Rail seat pad stiffness shall be between 1,500,000 pounds per inch and 3,000,000 pounds per inch for a pad that is 6 inches by 8 inches in area.
  - Where bolted baseplates are used, a pad of the same properties and material shall be used, but it may be either between the baseplate and the tie or the baseplate and the rail.

### 5.10.1.3 Placement and Spacing

Concrete ties shall be spaced at an average of 27 inches in main tracks, station tracks, and other high volume tracks. The spacing shall be 30 inches in yard tracks having radii of 1,100 feet or

greater. For lesser radii, the spacing shall be 24 inches within the curve and associated spirals or to 50 feet beyond the ends of the curve on curves without spirals.

### 5.10.2 Wood Ties and Other Types of Ties

Wood ties are acceptable in yard tracks and other tracks that have a speed limit of 40 mph or less. Alternate types of ties, such as plastic, reconstituted wood, etc. shall not be used. No special fastening devices are required.

**Dimensions** – The standard wood tie shall be manufactured and treated in accordance with the requirements of the AREMA Manual, Chapter 30, Part 3, Solid Sawn Timber Ties. The nominal dimensions of the ties shall be 7 inches by 9 inches by 8 feet 6 inches.

**Placement and Spacing** – Wood ties shall be spaced at an average of 21 inches in main tracks, station tracks, and other high volume tracks. The spacing shall be 24 inches in yard tracks having radii of 1,100 feet or greater. For lesser radii, the spacing shall be 21 inches within the curve and to 50 feet beyond the ends of the curve.

### 5.10.3 Ballast

- **Materials**

- AREMA Manual Chapter 1, Part 2, Ballast.

- **Gradation**

- Revenue tracks, yard leads, and other tracks where employees are unlikely to be walking: Size No. 24 as given in the AREMA Manual Chapter 1, Part 2 Table 1-2-2. Recommended Ballast Gradations
- Yard tracks and other tracks where employees are likely to be walking: Size No. 4 as given in the AREMA Manual Chapter 1, Part 2 Table 1-2-2. Recommended Ballast Gradations

- **Minimum Depth of Ballast under Rail Seats**

- On earthworks – 12 inches
- On short length structures – 15 inches, not including thickness of waterproofing, etc.

## 5.11 Turnouts and Other Special Trackwork Components

The requirements of this section relate to materials and component types to be used in turnouts and other special trackwork. Only major components are discussed. Details of location and application of these components and details of minor components are shown on the Standard and Directive Drawings. Placement of and internal geometry of turnouts and associated connecting tracks is described with the alignment related requirements.

### **5.11.1 Rails in Special Trackwork**

1 The shape and properties of the rail used in special trackwork shall be the same as that used in  
2 open track. Rails through turnouts in revenue tracks shall be set at an inclination of 1:40.  
3 Standard Strength Rail shall be used except High Strength Rail shall be used where the  
4 following conditions apply:

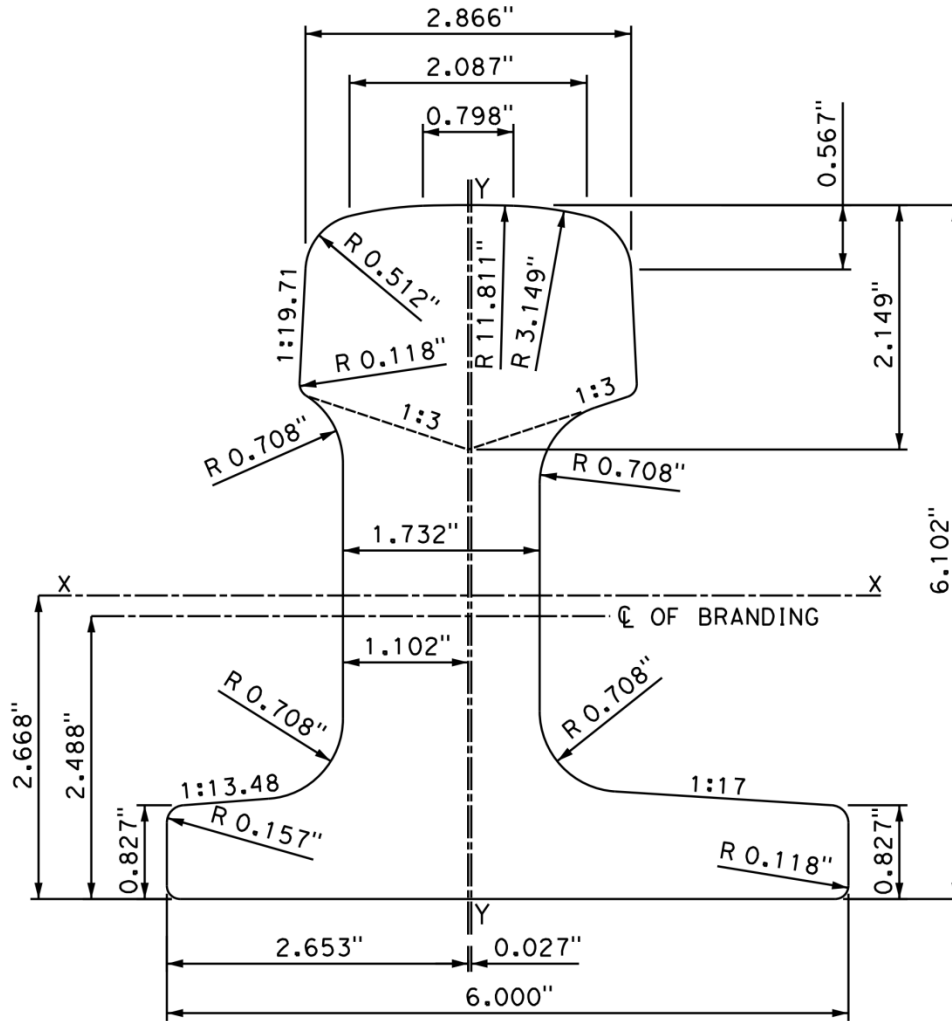
- 5 • Turnouts in revenue service tracks having an internal radius of 3,000 feet or less and where  
6 not less than 30 percent of the traffic operates on the curved side of the turnout.
- 7 • Turnouts in yard lead tracks having a radius of 2,000 feet or less and where not less than 30  
8 percent of the traffic operates on the curved side of the turnout.

### **5.11.2 Switch Rails**

9 Switch rails shall be of the EN 60E1A3 section as shown in Figure 5-6 (EN 13674-2 Figure A.7).

10 Switch point rails shall comply with the metallurgy, hardness, and strength requirements of the  
11 AREMA Manual, Chapter 4, Part 2, for High Strength Rails.

1 **Figure 5-6: EN 60E1A3 Asymmetric Switch Rail (EN 13674-2 Figure A.7)**



## 60 E1A3 RAIL SECTION

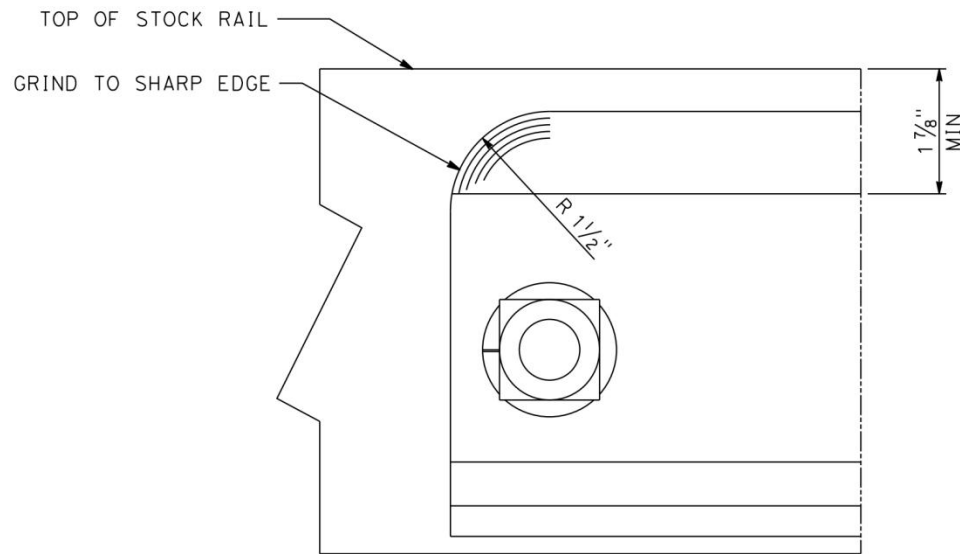
(NOTE: UNITS CONVERTED FROM METRIC UNITS)

2

Property	U.S. units
Centerline of branding	3.002 inches ATR
Mass per meter	168.6 lb/yd
Cross-sectional area	16.51 in <sup>2</sup>
Moment of inertia X-X axis	65.42 in <sup>4</sup>
Section modulus – Head	19.02 in <sup>3</sup>
Section modulus – Base	24.56 in <sup>3</sup>
Moment of inertia Y-Y axis	21.56 in <sup>4</sup>
Section modulus Y-Y axis, left	8.04 in <sup>3</sup>
Section modulus Y-Y axis, right	6.49 in <sup>3</sup>

3

1 **Figure 5-7: AREMA Switch Tip Rounding – Side View of End of Switch Point**



2  
3 Note: from AREMA Portfolio Plan 221

### 5.11.3 Turnout Frogs

4 Three types of frogs shall be used, with the type determined by the application.

5 Wheel contact areas on frogs of all types shall be machined to match the head shape of the  
6 141RE rail section. Rails used in frogs shall comply with the metallurgy, hardness, and strength  
7 requirements of the AREMA Manual, Chapter 4, Part 2, for High Strength rails. Manganese steel  
8 castings used in frogs shall be explosion hardened. References herein to AREMA Portfolio Plans  
9 and other plans are general in nature.

10 Frogs in high-speed turnouts shall be Movable Point Frogs.

11 Frogs in low and medium speed turnouts shall be either Rail Bound Manganese (RBM) Frogs,  
12 Welded Boltless Manganese (WBM) Frogs, or Spring Frogs.

13 • **RBM or WBM Frogs** shall be used for yard turnouts. This type shall be used in revenue  
14 tracks where all of the following conditions apply:

- 15 – The design speed for trains on the straight (through) side of the turnout is 90 mph or  
16 less.
- 17 – Anticipated train volume on the diverging side is 30 percent or more of the train volume  
18 on the through route.
- 19 – Terminal station throat turnouts
- 20 – Turnouts in yard and secondary tracks

21 • **Spring Frogs** shall be used where all the following conditions apply:



- The design speed for through trains on the straight side of the turnout is 110 mph or less.
- Anticipate train volume on the diverging route is less than 30 percent of the volume on the through route.
- The turnout is number 20 or less
- **Movable Point Frogs**, also called Swing Nose Frogs or Crossings, shall be used in turnouts where the design speed of either track is more than 110 mph. This type of frog shall be used for turnouts in lower speed tracks where the conditions permitting and the other types of turnouts do not exist.

#### **5.11.3.1 AREMA Rail Bound Manganese (RBM) Fixed Frogs**

Materials and fabrication of RBM Frogs shall be as shown in the AREMA Portfolio of Trackwork Plans, Plans 623 through 625, except that tapering and planing of the wing rails shall be a two stage planning similar to that shown on AREMA Portfolio Plan No. 504-71 (Note: This is not the current version of this plan). Details of end planing will be shown on Standard and Directive Drawings.

#### **5.11.3.2 Welded Boltless Manganese (WBM) Frogs**

A recent development is a one-piece cast manganese steel frog that can be welded into the track. It is an “Americanized” version of the European cast manganese steel frog with welded on rail steel legs which enables it to be welded. This frog design is patented, U.S. patent number 5,170,932. The patent is held by Voest-Alpine Eisenbahnsysteme. The design is also patented in Austria and Germany. Figure 5.8 is provided for illustration purposes. This particular frog is in a freight service main track.

1 **Figure 5-8: Welded Boltless Manganese Frog**



2  
3  
4 Tapering of the wing rails shall be a two stage taper similar to that shown on AREMA Portfolio  
5 Plan No. 504-71 (Note: This is not the current version of this plan). Details of end planing will be  
6 shown on Standard and Directive Drawings.

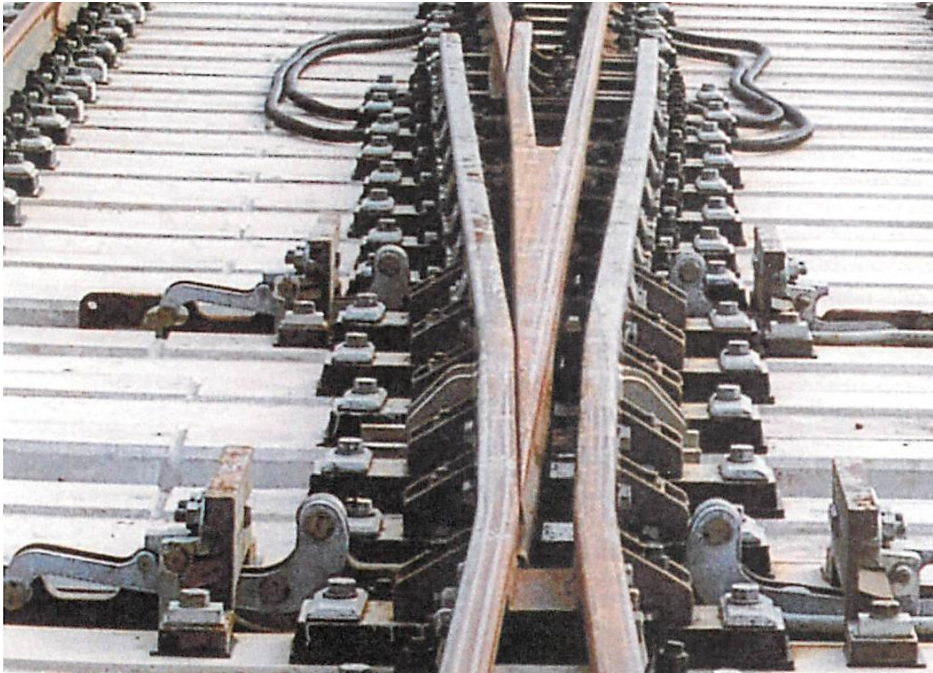
**5.11.3.3 AREMA Spring Frogs**

7 Materials and fabrication shall be as shown in AREMA Portfolio Plans, 401, 407 and 490, except  
8 that tapering and planing of the wing rails shall be a two stage planning similar to that shown  
9 on AREMA Portfolio Plan No. 504-71. Note: This is not the current version of this plan. Details  
10 of end planing will be shown on Standard and Directive Drawings.

**5.11.3.4 Swing Nose (Movable Point) Frogs**

11 There are several variations in design in these frogs. Three examples are shown in the following  
12 figures. There are other design variations.

1 **Figure 5-9: Movable Point Frog (1,200 meter radius turnout in JIS 60 kg/m rail)**



2

3

4 **Figure 5-10: Movable Point Frog: Rail Housing (Left) and Manganese Steel Cast Housing (Right)**

5

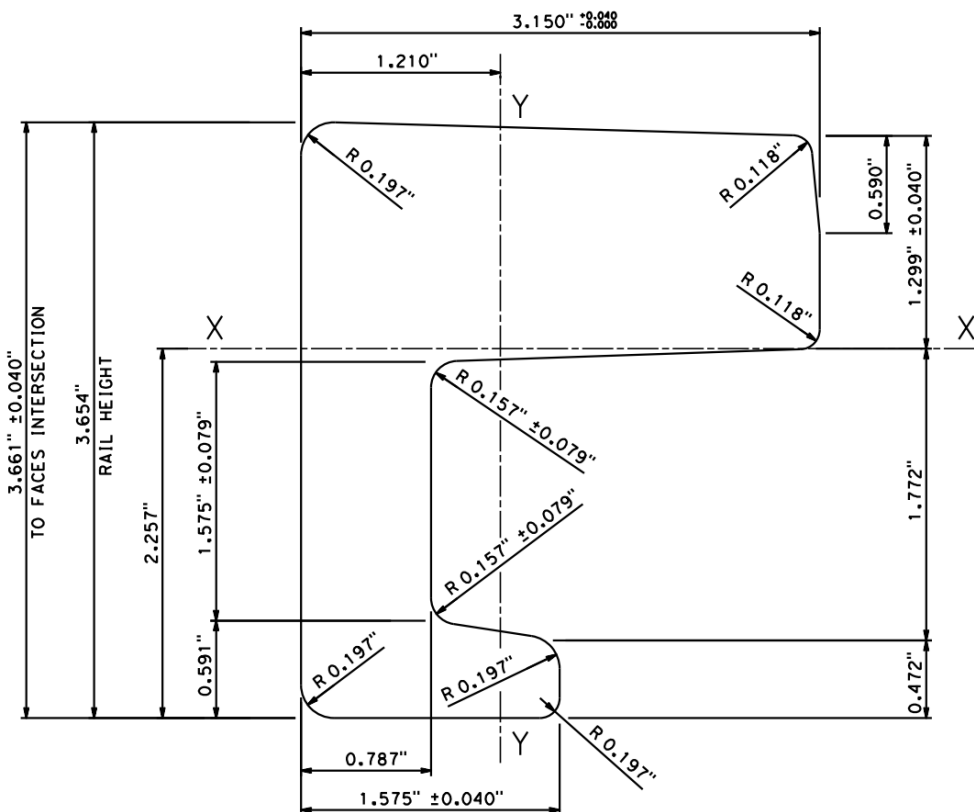


6

#### 5.11.4 Turnout Guard Rails (also called Check Rails)

Turnout guard rails shall be of the 33C1 Section as shown in Figure A.1 in EN 13674-3. This section has also been referred to as the U69, UIC33, and RL 1-60. Metallurgy, hardness and strength of the section shall comply with that for High Strength Rail in the AREMA Manual, Chapter 4, Part 2.

**Figure 5-11: EN 33C1 Check Rail (EN 13674-3 Figure A.1)**



#### EN 33C1 CHECK RAIL SECTION

(NOTE: UNITS CONVERTED FROM METRIC UNITS)

Property	U.S. units
Mass per meter	66.50 lb/yd
Cross-sectional area	6.51 in <sup>2</sup>
Moment of inertia X-X axis	7.14 in <sup>4</sup>
Section modulus – Head	5.11 in <sup>3</sup>
Section modulus – Base	3.16 in <sup>3</sup>
Moment of inertia Y-Y axis	5.26 in <sup>4</sup>
Section modulus Y-Y axis, left	4.34 in <sup>3</sup>
Section modulus Y-Y axis, right	2.71 in <sup>3</sup>

End tapering and planing shall be a two stage planning similar to that shown on AREMA Plan No. 504-71. Note: This is not the current version of this plan. Details of end planing will be shown on Standard and Directive Drawings.

#### 5.11.5 Turnout and Special Trackwork Support Systems

The turnout support system shall be identical with or similar to the support system of the adjacent track. The components used in the support system shall be identical with or similar in nature to those used in the same trackform in track outside the limits of turnouts and special trackwork. That is, where the trackform is ballasted track, the turnouts shall be installed on ballast and ties. Where the trackform is non-ballasted, the turnouts shall be placed on a modification of the same non-ballasted trackform.

##### 5.11.5.1 Non-Ballasted Track

The trackform through turnouts and special trackwork shall be the same as the trackform in the adjacent track. Fastening and support layouts will be shown on Standard and Directive Drawings. For non-standard units, these standard layouts shall be used as guidance along with the following:

- Support spacing through the turnout shall average no more than 24 inches under the switch and 27 inches through the rest of the turnout
- The use of special supports shall be limited to locations where standard supports cannot be made to fit. Rail supports need not be directly opposite each other. Fastener spacing may be adjusted at certain locations if needed to reduce the number of fasteners supporting two rails.

##### 5.11.5.2 Ballasted Track

Turnouts in revenue tracks shall be placed on concrete switch ties. Turnouts within yards shall be placed on wood switch ties. Number 15 and smaller turnouts located in other non-revenue tracks outside yards may be placed on either wood or concrete ties. Number 20 turnouts and larger shall be placed on concrete ties.

- **Concrete switch ties** shall be designed and fabricated in accordance with the requirements of the AREMA Manual Chapter 30, Ties, Part 4, Concrete Ties, including strength and other requirements, the general requirements for concrete ties in this document and the following:
  - **Length** – Not less than the distance between gauge lines of the outside rails plus 3.50 feet and not more than the distance between gauge lines of the outside rails plus 4.00 feet. Switch ties supporting switch machines, helper rods, or associated operating hardware may be longer as required.
  - **Thickness** – Portions supporting rails and within 2.50 feet of the gauge line of the rail shall be no less than 8 inches thick and no more than 9 inches thick. Switch ties may be of a constant thickness throughout their length. Extended portions supporting switch

1 machines, helper rods, or associated operating hardware may be thinner, but no less  
2 than 6.50 inches thick.

- 3 – **Spacing** – Average spacing shall not exceed 27 inches. Range of spacing may be between  
4 24 inches and 30 inches. Spacing with the areas of drive rods and other rodding and  
5 signal equipment shall be spaced as needed for these facilities.

- 6 • **Wood switch ties** shall be manufactured and treated in accordance with the requirements of  
7 the AREMA Manual, Chapter 30, Part 3, Solid Sawn Timber Ties. The nominal cross  
8 sectional dimensions of switch ties shall be 7 inches by 9 inches.

- 9 – **Length** – Increments in length shall be 1 foot located and positioned so that the  
10 difference between switch tie length and distance between gauge lines of the outside  
11 rails is between 3.50 feet and 4.50 feet. Switch ties supporting switch machines, helper  
12 rods, or associated operating hardware may be longer as required.

- 13 – **Spacing** – Average spacing shall not exceed 21 inches under the switch and frog areas  
14 and shall not exceed 24 inches under the remainder of the turnout. Range of spacing to  
15 be between 19 inches and 22 inches under the switch and frog and 22 to 26 inches under  
16 the remainder of the turnout.

## 5.12 Grade Crossings

### 5.12.1 Yard Traffic Grade Crossings

### 5.12.2 Main Line Pedestrian Crossings

## Chapter 6

### Rolling Stock and Vehicle Intrusion Protection

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	Jun 2013	Revision 1
2	Feb 2014	Revision 2
3	Feb 2016	Revision 3





# Table of Contents

**6 ROLLING STOCK AND VEHICLE INTRUSION PROTECTION .....6-1**

6.1 Scope .....6-1

6.2 Regulations, Codes, Standards, and Guidelines.....6-1

6.3 Protection of HST Operating Infrastructure from Vehicle Intrusion.....6-1

6.3.1 Protection Against Intrusion of Conventional Trains.....6-2

6.3.2 Protection Against Intrusion of Roadway Vehicles .....6-3

6.4 Containment of HST Rolling Stock .....6-5

RFP No.: HSR 14-32 – EXECUTION VERSION

## Acronyms

Authority	California High-Speed Rail Authority
Caltrans	California Department of Transportation
CHSTP	California High-Speed Train Project
HST	High-Speed Train
OCS	Overhead Contact System
TCL	Track Centerline

## 6 Rolling Stock and Vehicle Intrusion Protection

### 6.1 Scope

This chapter provides the separation requirements for the California High-Speed Train (HST) alignment adjacent to, crossing over, and crossing under other transportation systems. The requirements are established to protect the HST operating infrastructure from intrusion by rolling stock and vehicles from adjacent transportation systems (i.e., passenger and freight rail tracks, and state and local highways/roadways). Application of these requirements will be determined by a site-specific hazard analysis, in conformance with the hazard management process in the CHSTP Safety and Security Management Plan. This chapter is intended to serve primarily as the basis of the track, earthwork, and structural design and mitigation measures where minimum clearances cannot be met. This chapter does not define requirements for access control devices. Refer to the *Civil* chapter for protection of the Authority's right-of-way and facilities against trespass by unauthorized persons and animal intrusion.

### 6.2 Regulations, Codes, Standards, and Guidelines

Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards. Applicable codes and regulations include but are not limited to the following list:

Code of Federal Regulations (CFR)

- Title 49, Part 213, Section 361 for protection of the right-of-way for Class 8 and 9 tracks

- Title 49, Part 214, Railroad Workplace Safety

California Public Utilities Commission (CPUC) General Order (GO) No. 26-D

Federal Railroad Administration (FRA) guidelines regarding the separation and protection of adjacent transportation systems and conventional railroads

- High-Speed Passenger Rail Safety Strategy published by FRA (November 2009)

American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for Railway Engineering

California Department of Transportation (Caltrans), Highway Design Manual, Traffic Manual and Standard Plans

### 6.3 Protection of HST Operating Infrastructure from Vehicle Intrusion

The HST operating infrastructure, which includes the operating envelope, traction power facilities, wayside power cubicles, communication cabinets, cable troughs, piers and walls supporting HST structures, shall be protected from intrusion in order to preserve safe and

reliable operations. The limits of the HST operating envelope is defined as the area from the outer face of the Overhead Contact System (OCS) pole foundations in width and from top of the OCS poles to the trackbed supporting the HST tracks in height. In locations where the HST operating envelope is located within an open trench, on retained fill, or on an aerial structure, the limit of operating envelope shall be extended to the outer face of retaining walls, trench walls, abutments and piers of aerial structures.

### 6.3.1 Protection Against Intrusion of Conventional Trains

Passenger and freight trains that operate in shared corridors or adjacent to the HST system shall be prevented from entering into the HST operating infrastructure by lateral separation or by a physical barrier (e.g., earth berms, ditches, or reinforced concrete walls) when lateral separation between railway systems is insufficient.

#### 6.3.1.1 Protection Measures without Physical Barriers

The preferred protection is to locate HST operating infrastructure at a sufficient distance from passenger and freight (conventional) railroad systems to avoid intrusion. A lateral distance of 102 feet or greater measured between the closest HST track centerline (TCL) to conventional railroad right-of-way does not require a physical barrier for intrusion protection. Alternatively, when the HST alignment is on embankment and its trackbed is 10 feet or higher than the freight/conventional railroad top of rail, use of a physical barrier for intrusion protection of HST operating infrastructure is not required. Additionally, separation requirements of conventional railroad owners and operators shall be considered in establishing required separation.

#### 6.3.1.2 Protection Measures with Physical Barriers

When lateral separation between the closest HST TCL to conventional railroad right-of-way is less than 102 feet, or separation requirements of conventional railroad owners and operators, physical barriers shall be installed. The intrusion protection shall be designed to mitigate the risk of a train derailment from adjacent conventional railroad intruding into the HST operating envelope. For train collision loads, refer to the *Structures* chapter. For grounding and bonding of reinforced concrete barrier refer to the *Overhead Contact System and Traction Power Return System*, and *Grounding and Bonding Requirements* chapters. The intrusion protection is achieved by the following measures:

- HST Guideway At or Below Grade
  - Use of a minimum 10-foot-high berm, or 10-foot-deep ditch, or a 5-foot-deep ditch and a 5-foot-high berm combination, as an intrusion protection measure when HST guideway is at-grade. Refer to Standard and Directive Drawings for typical sections of various intrusion protection measures.
  - Use of a minimum 10-foot-high reinforced concrete barrier as an intrusion protection measure when HST guideway is at or below grade, or if required by conventional railroad owners and operators. Refer to Standard and Directive Drawings for placement of the wall within HST right-of-way.

- When there is a concrete barrier as an intrusion protection between a conventional railroad and the HST and there is an aerial structure pier between HST and the railroad, the concrete barrier shall transition to protect the pier. Refer to AREMA Pier Protection requirements. The minimum height of the barrier protecting the pier shall be 10 feet. The transition of the concrete barrier from inside HST right-of-way to the pier shall be at 3:1 slope or flatter.
  - Sufficient separation between the physical intrusion protection and the HST right-of-way shall be provided to allow maintenance of the intrusion protection barrier. This separation shall not be less than 5 feet.
  - Refer to Standard and Directive Drawings for typical sections of various intrusion protection measures.
- HST Guideway supported on retaining walls
    - When HST guideway is supported on MSE retaining walls, intrusion protection measures shall be identical to intrusion protection for HST guideway at-grade.
    - When HST guideway is supported on cast-in-place retaining walls, the wall shall be designed for train collision loads, refer to the *Structures* chapter.
  - HST Elevated Guideway
    - Where the side clearance from the closest conventional rail TCL is less than 25 feet to the face of a HST structure, such as a pier or a retaining wall (with the exception of a trench wall), a 6-foot high reinforced concrete barrier shall be constructed at a minimum distance of 1 foot from the face of the HST supporting structure. Where the side clearance is 12 feet or less, the height of the reinforced concrete barrier shall be 12 feet. The reinforced concrete barriers shall be designed to protect HST supporting structures from a direct impact by a derailed conventional railroad locomotive.
    - .
- These guidelines are for physical separation and do not include right-of-way considerations that may require additional separation. Additionally, separation requirements of freight railroad owners and operators shall be considered in establishing required separation.

### 6.3.2 Protection Against Intrusion of Roadway Vehicles

Protection against highway/roadway vehicles from intruding into the HST operating infrastructure shall be provided through sufficient lateral separation between state highway systems or local roadways and the HST system, or the installation of barriers. For highway vehicle collision loads, refer to the *Structures* chapter.

#### 6.3.2.1 Protection Against Intrusion of Roadway Vehicles into the HST Operating Infrastructure

For state highway systems, protection against errant roadway vehicles from intruding into HST operating infrastructure shall be provided. Caltrans requires protection for errant roadway vehicles when HST fixed objects are located within the highway Clear Recovery Zone (CRZ).

Caltrans Highway Design Manual establishes 52 feet as the CRZ for the high-speed rail project. Therefore, when a high-speed rail corridor is constructed longitudinal to a freeway, expressway, or a conventional highway with posted speeds over 40 mph, the nearest fixed object or feature associated with the operation of the rail facility shall be located at a minimum of 52 feet horizontally from the planned ultimate edge of the traveled way. When the HST alignment is not longitudinal to a Caltrans freeway, expressway, or highway, the standard Caltrans 30 feet requirement for CRZ shall apply.

If these clearances cannot be provided, a design exception shall be obtained from Caltrans and the Authority along with appropriate roadside protection mitigation measures, such as installation of a metal beam guard rail or concrete barrier.

For protection of HST operating infrastructure, appropriate required type of roadside protection shall be site specific, based on site specific hazard analysis, and shall consider factors such as traffic volumes, speed, highway geometry, side slopes, accident history, and others. For instance, in locations where high volumes of cargo and tanker trucks are present with high probability of intrusion into HST operating infrastructure, a more stringent intrusion protection is required and shall be provided, such as a concrete wall up to 7.5-foot high meeting design force requirements specified for AASHTO TL-6 with a Caltrans type 60D barrier or metal beam guard rail installed for protection. However in most cases, a 56-inch high concrete barrier meeting design force requirements specified for AASHTO TL-5 is required to protect HST operating infrastructure from intrusion by errant vehicles.

For local roadways, protection against adjacent roadway vehicles from intruding into HST operating infrastructure shall be provided based on site specific hazard analysis and per the requirements of the local jurisdictions.

For both the state highway system and local roadway systems, the intrusion protection shall be designed to mitigate the risk of errant vehicles from an adjacent roadway intruding into the HST operating infrastructure. Refer to Standard and Directive Drawings for various conditions where intrusion protection measures are required along the HST alignment.

### **6.3.2.2 Protection Against Intrusion of Roadway Vehicles over the HST Operating Infrastructure**

Protection against intrusion of roadway vehicles on grade separated structures onto the HST operating infrastructure below the structure shall be provided. The overhead structure shall be designed to include vehicular railing with sufficient strength to withstand collision loads defined in the *Structures* chapter. The vehicular railing shall extend to the nearest intersection or 100 feet beyond the end of the overhead structure with appropriate taper to redirect vehicles that may travel down the roadway embankment and into the Authority's right-of-way. In conjunction with keeping the roadway vehicle from intruding into the HST operating infrastructure, a protective screening and barrier shall be provided to prevent contact with the OCS, to prevent pedestrians from falling onto, and to reduce the risk of objects being dropped onto the HST operating infrastructure. Refer to the *Overhead Contact System and Traction Power*

1 *Return System* chapter for minimum requirements of this protective screening and barrier. Refer  
2 to Standard and Directive Drawings for typical section of intrusion protection measures on and  
3 along a roadway structure over the HST.

4 Supplemental protection shall be achieved through the use of intrusion detection technology in  
5 the fencing around HST operations. When the intrusion detection system is activated, HST  
6 operation is stopped, speed is reduced, or other appropriate actions will be taken by the  
7 signaling system and/or operations. Intrusion protection, if required, shall be designed in  
8 conjunction with the hazard analysis to determine the need for a physical barrier.

### 6.3.2.3 HST Pier and Wall Protection

9 Where HST piers, trench walls, and other structures are located within the Caltrans Clear  
10 Recovery Zone, install Caltrans-type concrete barriers to redirect errant vehicles from intrusion  
11 into the HST operating envelope.

## 6.4 Containment of HST Rolling Stock

12 HST rolling stock shall be contained within the operational envelope in order to reduce the  
13 potential for intrusion into an adjacent transportation system. Strategies to ensure containment  
14 include but are not limited to the following:

15 Use modern HST rolling stock, which have documented performance likely to minimize the risk  
16 of the train derailment and extending beyond its operating envelope. These protections may  
17 include use of bogie-mounted components and use of plate springs to prevent rails from large  
18 lateral movement.

19 Use of articulated trainsets to keep the HST vehicles upright and in line in the event of a  
20 derailment.

21 The raised track side cable trough walls in tunnels, trenches, and aerial structures provide  
22 derailment containment to protect cable troughs and OCS poles due to their close proximity to  
23 the track as compared with OCS poles and cable troughs located in the at-grade section. Refer to  
24 Standard and Directive Drawings for minimum side clearance requirements for grade separated  
25 structures.

26 Ensure the appropriate level of maintenance of infrastructure (per FRA standards) and rolling  
27 stock (per manufacturer requirements), to mitigate the risk of derailment.

28 On aerial structures, protection shall be provided by a derailment protection wall designed so  
29 that the HST remains within its operating infrastructure. Refer to the *Structures* chapter.

## Chapter 7

### Civil

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	07 Mar 12	Initial Release, R0
1	May 2013	Revision 1
2	Sept 2014	Revision 2
3	Feb 2016	Revision 3





## Table of Contents

1	<b>7 CIVIL</b>	<b>7-1</b>
2	7.1 Scope	7-1
3	7.2 Regulations, Codes, Standards, and Guidelines	7-1
4	7.3 Grading, Side Slopes, and Retaining Walls for Roadways and Sites	7-2
5	7.3.1 Side Slopes	7-2
6	7.4 Construction Site Best Management Practices	7-3
7	7.5 Slope/Surface Protection Systems Best Management Practices	7-3
8	7.6 Site Work – Non-Authority Facilities	7-4
9	7.6.1 Roadways	7-4
10	7.6.2 Traffic Control Devices	7-5
11	7.6.3 Sidewalk, Driveways, Curb Cuts	7-5
12	7.6.4 Miscellaneous Facilities	7-6
13	7.7 Site Work – Authority Facilities	7-6
14	7.7.1 Authority Roadways	7-6
15	7.7.2 Site and Roadway Geometrics	7-9
16	7.7.3 Traffic Control Devices	7-13
17	7.7.4 Signage	7-13
18	7.7.5 Parking	7-14
19	7.7.6 Sidewalks and Driveways	7-17
20	7.7.7 Walkways and Cable Trough	7-17
21	7.7.8 Service Aisles	7-19
22	7.7.9 End-of-Line Walkway	7-19
23	7.7.10 Crosswalks	7-20
24	7.7.11 Emergency Stairs	7-20
25	7.7.12 Maintenance Access	7-20
26	7.8 Access Control Devices	7-21
27	7.8.1 Access Control	7-21
28	7.8.2 Access Control by Type of HST Trackway	7-26
29	7.8.3 Access Control by Type of Facility	7-30
30	7.8.4 Access Control at Other Locations	7-31
31	7.9 Wildlife Crossing	7-34
32	7.10 Maintenance and Protection of Traffic during Construction	7-34
33	7.10.1 Railroad Operations	7-35

## Tables

Table 7-1:	Stall Dimensions (in feet)	7-15
Table 7-2:	Disabled Parking Spaces	7-16
Table 7-3:	Access Control - Gates	7-26
Table 7-4:	Access Control Fencing – Trackway	7-30
Table 7-5:	Access Control Fencing – Authority Facilities	7-31
Table 7-6:	Access Control Fencing – Other Locations	7-34

## Figures

Figure 7-1: Slope Rounding, Stepping, Terracing, and Contouring .....	7-4
Figure 7-2: Access Road Dead End Turn-Around (Emergency Vehicle Staging).....	7-8

## Acronyms

AASHTO	American Association for State Highway and Transportation Officials
AD	Access Deterring
ADA	Americans with Disabilities Act
AR	Access Restricting
BMP	Best Management Practice
Caltrans	California Department of Transportation
CCR	California Code of Regulations
CHSTP	California High-Speed Train Project
HST	High-Speed Train
MOI	Maintenance of Infrastructure
MUTCD	Manual on Uniform Traffic Control Devices
OCS	Overhead Contact System
RSM	Rolling Stock Maintenance
SA	Station Area
SWPPP	Storm Water Pollution Prevention Plan



## 7 Civil

### 7.1 Scope

This chapter provides design criteria to be used in the civil/site design and related work including grading, slope protection, erosion control, site work for Authority's and non-Authority's facilities, access control, and maintenance and protection of traffic during construction.

### 7.2 Regulations, Codes, Standards, and Guidelines

Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards. The following codes, guidelines, and references were used in the development of this chapter.

- Americans with Disability Act (ADA) Standards for Transportation Facilities adopted by 49 CFR part 37
- California Code of Regulations (CCR)
  - Title 24, Part 2 – California Building Code
  - Title 24, Part 9 – California Fire Code
- California Public Utilities Commission (CPUC), Applicable General Orders
- American Association for State Highway and Transportation Officials (AASHTO)
  - A Policy on Geometric Design of Highways and Streets Roadside Design Guide
- American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for Railway Engineering
- California Department of Transportation (Caltrans)
  - Standard Specifications
  - Standard Plans
  - Highway Design Manual
  - California Manual on Uniform Traffic Control Devices (MUTCD)
  - Stormwater Quality Handbooks

### 7.3 Grading, Side Slopes, and Retaining Walls for Roadways and Sites

For embankments, cuts, retaining walls, etc. required to support the California High-Speed Train (HST) trackway, refer to the *Geotechnical* chapter and the *Structures* chapter for design requirements.

Areas of construction shall be cleared, grubbed, and stripped. Areas disturbed by construction shall be protected by an erosion control system. Refer to Section 7.4 Construction Site Best Management Practices (BMPs) for criteria and standards.

Construction of fills shall be a controlled fill. Subgrade preparation and compaction shall be as recommended by Geotechnical Engineer.

Storm runoff (and ground water, if applicable) flow shall be prevented from infiltrating the trackbed. Cut and fill side slopes shall include earth or concrete intercept ditches on top of a cut, on a face of a slope and between the trackbed and a slope, and benches with maintenance access. For additional drainage requirements, refer to the *Drainage* chapter.

Due to limited space, a portion of or the entire slope may be replaced by a retaining wall. The same requirements for preventing the storm runoff/groundwater from entering the trackbed shall apply; therefore retained trackway shall include intercepting ditches. Tiered retaining wall trackway shall include benches with construction and maintenance access. In retained trackway, storm runoff/groundwater shall be collected by an underdrain system. For additional drainage requirements, refer to the *Drainage* chapter.

Permanent erosion control for cut and fill slopes shall be provided after completion of earthwork.

Landscaping design shall comply with requirements of the *Stations* chapter.

Retaining walls that do not support the HST trackway shall be designed per *Caltrans Standards* or the requirements of the authority having jurisdiction, including requirements for weep holes.

#### 7.3.1 Side Slopes

Design of cut and fill slopes shall be based on geotechnical evaluation of soil and materials for construction of embankment or excavation and geotechnical recommendations for slope stability, including steepness and depth/height of cut and fill slopes, height between tiers, use of filter fabric, required material compaction and other elements of cut or fill for trackway.

Side slopes shall be as flat as available right-of-way permits, except that slopes flatter than 10 horizontal to 1 vertical shall not be used.

Where heights are less than 4 feet, slopes shall be 4 horizontal to 1 vertical or flatter. When the right-of-way is restricted, the following applies:

- Cut slopes shall be 2 horizontal to 1 vertical, unless otherwise recommended by the Geotechnical Engineer. The top of cut slopes, other than those in rock, shall be rounded, recommended 10 feet and minimum 2 feet, on both sides of the intersection of the slope planes.
- Fill slopes for embankments shall be 2 horizontal to 1 vertical, unless otherwise recommended by the soils report or the Geotechnical Engineer. Slope rounding shall be used at the top of fill slopes in the same manner as prescribed for cut slopes.

#### 7.4 Construction Site Best Management Practices

Areas of proposed construction shall be cleared, grubbed, and stripped. Construction Site Best Management Practices (BMPs) shall be applied during construction activities to reduce the pollutants in stormwater discharges throughout construction. SWPPP shall conform to the rules, regulations, and practices of the authority having jurisdiction. Designer shall refer to State Water Resources Control Board (SWRCB) for general permitting requirements.

Refer to the *Drainage* chapter for BMPs.

Refer to *Caltrans Standard Plans* for erosion and sediment control standards.

#### 7.5 Slope/Surface Protection Systems Best Management Practices

Surface protection includes permanent design measures that are used alone or in combination to minimize erosion from completed, but not-vegetated (bare) surfaces. Vegetated surfaces may offer advantages to paved surfaces, including lower runoff volumes and slower runoff velocities, increased times of concentration, and lower cost.

Where site or slope-specific conditions would prevent adequate establishment and maintenance of a vegetative cover, hard surfacing shall be considered.

Vegetated surfaces shall consist predominantly of established native grasses and mixed shrubs.

Vegetated surfaces shall be established on disturbed soil areas after construction activities in that area are completed, and after the slope has been prepared. Vegetated surfaces shall only be considered for areas that will support the selected vegetation long-term.

A rough surface may be added to a slope by various methods all of which run parallel to the slope contour over the entire face of the slope. The purpose of slope roughening is to prevent surface erosion that can cause downstream pollution by reducing the velocity of surface runoff. This BMP may also be considered a Low Impact Development (LID) technique.

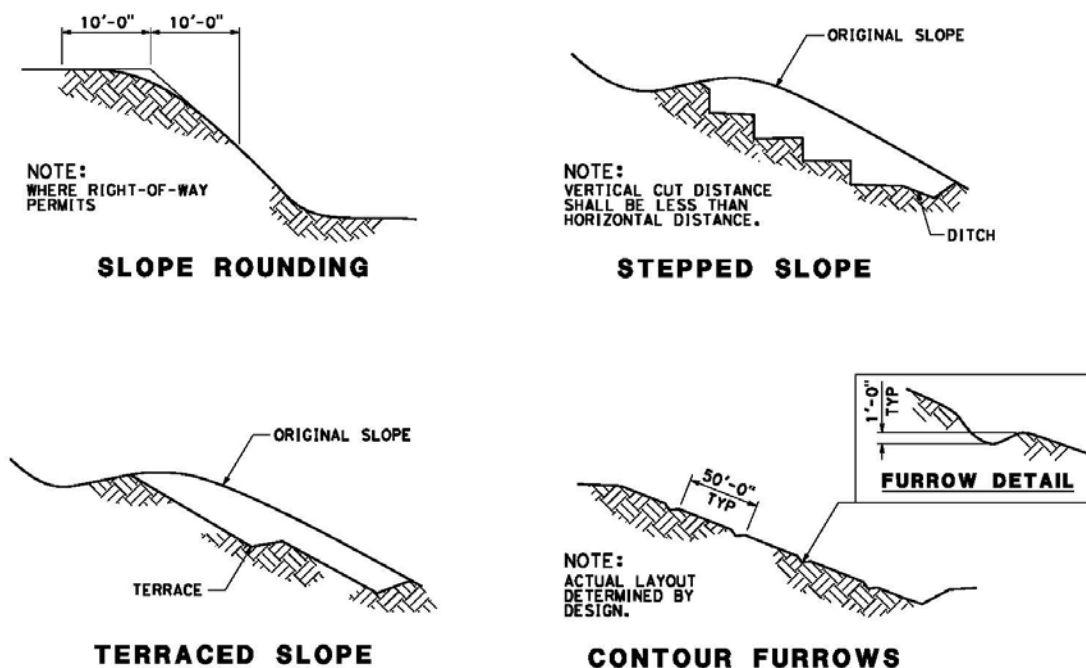


Roughening and terracing are techniques for creating furrows, terraces, serrations, stair-steps, or track-marks on the soil surface. These treatments increase adhesion of erosion control materials and improve vegetation establishment.

Slope rounding is used to minimize the formation of concentrated flows; and used on embankment or cut slopes, prior to the application of temporary or permanent erosion control.

Slope roughening, terracing, rounding, and stepping, shall be implemented as depicted on Figure 7-1.

**Figure 7-1: Slope Rounding, Stepping, Terracing, and Contouring**



## 7.6 Site Work – Non-Authority Facilities

Design of publicly and privately maintained facilities including roadways, sidewalks, driveways, traffic devices and signs not under the jurisdiction of the Authority, shall be in accordance with the applicable requirements of the authority having jurisdiction.

### 7.6.1 Roadways

**At-grade Crossings** – At-grade (level) roadway crossings of the HST trackway are prohibited and require the grade-separation of existing crossings within the Authority's right-of-way.

**Paving** – Roadway pavement structural sections shall be designed in accordance with standards of the authority having jurisdiction. If no standards are available, restored pavements shall be of materials in-kind to that originally employed.

### 7.6.2 Traffic Control Devices

Design of traffic control devices, where roadway intersections are affected by the Authority's project, shall conform to the following:

- Maintenance, relocations of traffic control devices, temporary or permanent, and restoration of these facilities shall be in accordance with the practices and requirements of the authority having jurisdiction and the Authority. Where the authority having jurisdiction has no standards, the *California MUTCD* shall be followed.
- Traffic signal design and details shall conform to the requirements of the authority having jurisdiction.

Roadway signing and striping shall consider the following:

- Roadway signage and striping plans shall be subject to the approval of the authority having jurisdiction.
- Traffic striping, raised pavement markers, delineators, pavement legends, arrows and other markings installed as a part of the project shall be in accordance with *California MUTCD* or the standards of the authority having jurisdiction.
- Work involving relocation, restoration, and temporary installation of street signing shall conform to *California MUTCD* or the standards of the authority having jurisdiction.

### 7.6.3 Sidewalk, Driveways, Curb Cuts

**Sidewalks** – Sidewalks shall be in accordance with the standards of the authority having jurisdiction.

**Driveways** – Driveway minimum and maximum widths and numbers shall be in accordance with the standards of the authority having jurisdiction.

**Curb Cuts** – Ramps and Curb Cuts shall consider the following:

- Design of curb cuts and ramps shall be in accordance with the applicable provisions of the ADA and Title 24, CCR Part 2, "Regulations for the Accommodation of the Disabled in Public Accommodations."
- Ramps and curb cuts located in third party areas shall follow the standards of the authority having jurisdiction and shall be in accordance with the ADA and Title 24.

#### 7.6.4 Miscellaneous Facilities

- 1 **Parking Meters** – Removal, storage, restoration, and reinstallation of parking meters shall be in  
2 accordance with the standards of the authority having jurisdiction.

### 7.7 Site Work – Authority Facilities

3 This section establishes criteria and standards for the design of roadways, parking facilities,  
4 pedestrian facilities, and driveways, including signage, marking and striping, which are to be  
5 maintained by the Authority. Refer to the *Structures* chapter for vehicular load requirements on  
6 roadways.

7 Roadway design in public right-of-way shall be in conformance with the requirements of the  
8 authority having jurisdiction.

9 Additional criteria relevant to this section may be found in the following chapters:

- 10 • Trackwork
- 11 • Stations
- 12 • Drainage
- 13 • Geotechnical
- 14 • Tunnels
- 15 • Facility Power and Lighting Systems
- 16 • Overhead Contact System and Traction Power Return System
- 17 • Traction Power Supply System
- 18 • Communications

#### 7.7.1 Authority Roadways

19 Authority Roadways shall meet the requirement of AASHTO “A Policy on Geometric Design  
20 Highway and Street”, for Local Rural Road using vehicle type WB 40 or equivalent, with a  
21 minimum 45 foot turning radius, and design speed of 30 mph in the level terrain and 20 mph in  
22 rolling and mountainous terrain.

##### 7.7.1.1 Access Roads

23 Access roads are roads connecting public streets to access gates located along the perimeter  
24 fencing of HST trackway and Authority facilities.

25 Access roads shall be constructed at-grade on exclusive right-of-way where practicable.

26 Access roads shall be sized to permit access by emergency responders and allow turning and  
27 staging of emergency vehicles.

Access roads shall be provided to within 100 feet of tunnel portals and emergency access/egress points. Refer to the *Tunnels* chapter for tunnel portal site criteria.

Traction Power Facilities shall be accessed by access roads. The road shall be designed to accommodate low-load type vehicles. Refer to the *Traction Power Supply System* chapter for additional site and access requirements.

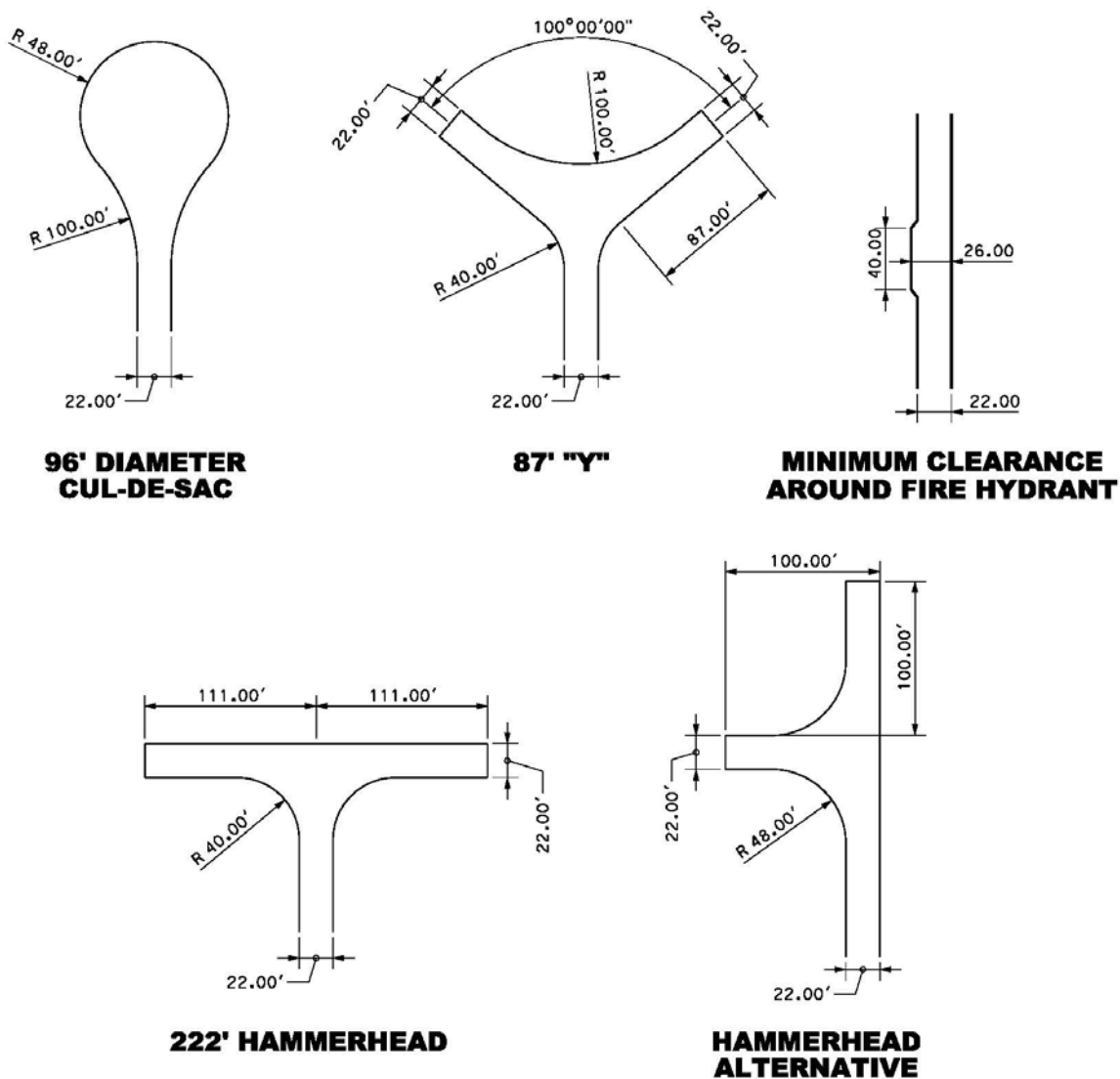
Standalone Radio Sites shall be accessed by access roads. Refer to the *Communications* chapter for additional site and access requirements.

Interlockings shall be accessed by access roads.

Minimum requirements for an access roadway shall consist of the following:

- Roadway Width – Access roads shall have an unobstructed width of not less than 22 feet inclusive of shoulders. If a fire hydrant is located along the road, the minimum roadway width shall be 26 feet per California Code and Regulation, Fire Code.
- Vertical Clearance – Unobstructed vertical clearance of not less than 14.5 feet.
- Pavement – Shall be all-weather use and designed to withstand a 75,000 pound (37.5 ton) load.
- Turning radius – Shall be 45 feet minimum.
- Dead ends – Dead end access roads in excess of 150 feet in length shall be provided with space appropriate for the turn-around of fire apparatus as identified in Figure 7-2. This space can also provide room for emergency vehicle staging.
- Bridges – Shall be designed to withstand 75,000 (37.5 ton) load. Vehicle load limits shall be posted at both entrances to bridges.
- Grade – The gradient for an access road shall not exceed 10 percent. Grades steeper than 10 percent will require approval by the Authority.
- Access Gates – Where gates restrict public access to the access road, such gates shall have a minimum opening of 20 feet, constructed of the swinging or sliding type, the ability to be opened by 1 person.
- There shall be a vehicle turnaround area at the roadway terminus.
- Refer to *Standard and Directive Drawings* for additional details of the access road requirements.

1 **Figure 7-2: Access Road Dead End Turn-Around (Emergency Vehicle Staging)**



### 7.7.1.2 Service Roads

- 2 Service roads are roads located within the Authority right-of-way for the purpose of inspecting,
- 3 servicing, and maintaining Authority property.
- 4 Service roads allow vehicle access to the various service areas to service trains and onsite
- 5 equipment. Such roadways shall also serve as fire access roads. A service road or service aisle
- 6 shall be provided between alternate tracks in yards. Refer to Section 7.7.8 for criteria on Service
- 7 Aisles.
- 8 Minimum roadway width requirements for a service road should match requirements for an
- 9 access road.

A. Roadways for Rolling Stock Maintenance (RSM) Facilities and Maintenance of Infrastructure (MOI) Facilities

- 1 Buildings within a facility shall be accessed by a service road.
- 2 A 2-way service road, 24 feet wide with 2 foot un-paved shoulders on each side, follows the
- 3 interior perimeter of each facility.
- 4 For the RSM facilities, a 50-foot wide asphalt “apron” would surround the main shop building
- 5 to provide access for emergency vehicles to any point around the structure.
- 6 Yard traffic at-grade crossings shall be concrete. Refer to the *Trackwork* chapter for criteria.

**7.7.2 Site and Roadway Geometrics**

**7.7.2.1 Grades**

A. Parking Facilities

- 7 • General
- 8 – 5 percent maximum slope in a parking stall
- 9 – 6 percent maximum slope in a traveled area
- 10 – 2 percent recommended slope in any direction
- 11 – 0.3 percent minimum slope in any direction
- 12 • Comply with ADA Standards for Transportation Facilities regarding slopes at parking
- 13 spaces and access aisles designated for accessible parking. Accessible parking spaces shall
- 14 be located at an optimum location within the parking lot to provide easy access to the
- 15 station.
- 16 • Driveway Slopes and Ramps
- 17 – 20 percent maximum slope on driveway or ramp
- 18 – 10 percent maximum cross slope of a driveway or ramp
- 19 • Transition slopes between the ramp and driveway are required when the slope of the
- 20 driveway or ramps exceeds 12 percent.

B. Roadways within Authority and Parking Facilities

- 21 • Maximum: 6 percent Recommended: 5 percent
- 22 • Minimum: 0.5 percent Recommended: 1 percent
- 23 • Cross-slope: 2 percent
- 24 • Crown cross-section, except on curves

C. Maximum Grade Differential for Roadways within Authority and Parking Facilities

- Crest Vertical Curve: 9 percent
- Sag Vertical Curve: 6.5 percent

Note: Crest and Sag Curves at top and bottom of ramps without parking may exceed these differentials, but shall use a vertical curve as per Section 7.7.2.4.

**7.7.2.2 Design Speeds**

A. Access Roadways

- Thirty mph in the level terrain and 20 mph in rolling and mountainous terrain

B. Roadways within Authority Facilities and Parking Facilities

- Ten mph

**7.7.2.3 Clearances**

Refer to the *Trackway Clearances* and *Utilities* chapters for additional criteria on clearances.

A. All Authority Roadways

- Vertical - 14 feet–6 inches minimum
- Horizontal - 2 feet–6 inches minimum from edge of shoulder or face of curb to fences, light standards and pedestrian barriers

B. Parking Facilities

- Vertical - 7 feet–0 inches minimum (with height restrictions noted)
- Minimum 9 feet–6 inches vertical clearance provided for van accessible parking spaces

**7.7.2.4 Vertical Curves on Authority Roadways**

Crest Curves -  $L_{min} = 28 A$

Sag Curves -  $L_{min} = 35 A$

Where:

$L_{min}$  = minimum vertical curve length

A = Algebraic difference in grades

No vertical curves shall be less than 20 feet.

**7.7.2.5 Sight Distance at Intersections**

Site distance at intersections shall comply with "A Policy on Geometric Design of Highway and Street," current edition, published by the American Association of State Highway and Transportation Officials (AASHTO).

#### 7.7.2.6 Curbs and Gutters

- 1 Curbs shall be Caltrans Curb Type A1-6.
- 2 Gutters should be integral with curbs, Caltrans Type A2-6.
- 3 Curbs and gutters shall be cast-in-place concrete, and shall be in compliance with *Caltrans*
- 4 *Standard Plans and Specifications*.

#### 7.7.2.7 Pavement

##### A. Roadbed Design

- 5 The roadbed cross-section (from top to bottom) shall consist of pavement, aggregate base,
  - 6 aggregate subbase, filter course, and subgrade material. The aggregate subbase course may be
  - 7 eliminated where the subgrade material has a Sand Equivalent of 20 minimum and a Resistance
  - 8 (R) Value in excess of 40 when tested in accordance with California Test Methods CTM 217 and
  - 9 CTM 301, respectively. The filter course shall not be eliminated.
- 10 • Pavement shall be dense graded asphalt concrete consisting of asphalt binder and close
  - 11 graded mineral aggregates, Caltrans HMA Type A, unless otherwise required due to special
  - 12 conditions or use.
  - 13 • Aggregate base shall be used under all pavements.
  - 14 • Aggregate subbase shall be of a granular material conforming to the class designated.
  - 15 • Filter course shall consist of 1 layer of filter fabric placed on the subgrade below the
  - 16 aggregate subbase course. The subgrade upon which the filter fabric is placed shall have
  - 17 been compacted to a relative compaction of 95 percent as determined by ASTM D1557.

##### B. Structural Design of the Roadbed

- 18 The required thickness of the individual layers making up the roadbed shall be determined
  - 19 from the following considerations and procedures:
- 20 • For purposes of structural design of the roadbed only, the Authority roadways shall be
  - 21 classified according to the character of traffic thereon as follows:
    - 22 – **Type A Roadways** – Roadways which will carry truck traffic, except those classified as
    - 23 Type C. Type A roadways may carry passenger car traffic.
    - 24 – **Type B Roadways** – Roadways that will carry only passenger cars.
    - 25 – **Type C Roadways** – Roadways having a low volume of traffic, on the order of not more
    - 26 than 25 cars or 5 trucks daily, that will be used for service or maintenance purposes, and
    - 27 are not intended for use by the general public. Bicycle paths shall also be classified as
    - 28 Type C.
  - 29 • Structural design of the roadbed shall be in accordance with the procedure for design of
  - 30 flexible pavements in the *Caltrans Highway Design Manual, Chapter 630, Flexible Pavement*.



- Traffic Indexes to be used shall be as follows:
  - 9.0 for Type A Roadways
  - 7.0 for Type B Roadways
  - 6.0 for Type C Roadways
- Additional gravel equivalent thickness for "Factor of Safety" called for in the *Caltrans Highway Design Manual* Empirical Method shall not be provided for the Authority roadways.
- Structural design of the roadbed shall be reviewed and approved by the responsible Geotechnical Engineer.
- Structural design of the roadbed shall be based on the use of Resistance (R) values, representing the structural quality of the subgrade material. The determination of the (R) value of a given material requires a soil test which shall conform to California Test Method (CTM) 301.
- The thickness of the individual layers of the roadbed shall be determined using data shown above.

#### C. Other Paved Areas - Roadbed Design

Parking lot roadbed, which includes parking areas as well as aisles and circulation roads, shall consist of the pavement, aggregate base, aggregate subbase, filter course, and subgrade.

Permeable pavements in parking lots shall be used when local conditions permit. Permeable pavement types may include porous asphalt, rubberized asphalt, and pervious concrete.

Paving materials used at parking lot facilities shall have a Solar Reflectance Index (SRI) 29 or higher.

Structural design of parking lots and bicycle paths shall be as follows:

- Parking lots which will contain only passenger cars shall be designed as Type B roadways. Parking lots for vehicles other than passenger cars shall be designed as Type A roadways.
- Bicycle paths immediately adjacent to the station shall be designed as Portland cement concrete sidewalks. Paths away from the station site area shall be asphalt concrete. Most paths will be used occasionally by maintenance vehicles, and therefore will have a pavement structure equivalent to a Type C roadway.

Bus pads shall be as follows:

- Concrete bus pads shall be provided at bus stops which are constructed or reconstructed in conformance with the standards and specifications of the authority having jurisdiction.

- Continuous concrete pads may be required in the vicinity of the station entrance when subject to heavy bus traffic or required by the authority having jurisdiction.
  - For bus pad detail, refer to Standard and Directive Drawings.
- For bus and taxi loading zone criteria refer to the *Stations* chapter.

### 7.7.3 Traffic Control Devices

- Curb markings, signs and striping used on Authority roadways and parking facilities shall be standard facilities as required by the latest edition of *California MUTCD*.
- Traffic signing equipment, materials, and installation processes shall be as specified in the *Caltrans Standard Plans and Specifications* and CPUC GO 75-D.
- Placement of signs shall be coordinated with actual site conditions to prevent masking or blocking of safety-sensitive signs.
- Authority signage shall be provided at the entrance to the commuter parking area, and pathfinder signs shall be placed to provide directions from state highway routes.

### 7.7.4 Signage

- This section refers to all signage, inclusive of traffic signs noted in Section 7.7.3 on Traffic Control Devices.
- Regulatory and safety, transportation, and maintenance of way signs and graphics shall meet the requirements of the applicable regulatory agency.
- The following types of signage shall be provided for HST trackway and facilities:
- Location
    - Mile Post
    - Political Subdivision Signs
    - Standard Right-of-Way Sign and Monument Marker
    - No Trespassing Signs
  - Maintenance-of-Infrastructure
    - Maintenance Limit Signs
    - Roadway Structures Signs
    - Snowplow Signs
    - Alignment Signs or Markers
    - Elevation Markers

- 1   •   Transportation
- 2       –   Speed Control Signs – Temporary and Permanent
- 3       –   Location Signs
- 4   •   Safety
- 5       –   Restricted Clearance Signs
- 6       –   Fire Hazard Signs
- 7       –   Electrical Hazard Signs
- 8       –   Highway Grade Crossing Signs (To be used inside Maintenance Facilities)
- 9       –   Barricade Signs (To be used inside Maintenance Facilities)
- 10      –   Highway and Barricade Signs
- 11      –   Power Operated Switch Signs
- 12      –   Warning Signs
- 13      –   Directional Signs
- 14      –   Identification
- 15   For signage requirements at electrical facilities refer to the *Overhead Contact System and Traction*
- 16   *Power Return System* chapter.

#### **7.7.5      Parking**

- 17   For parking lot design within the station area, the Designer shall consider providing adequate
- 18   designated parking stalls for car share, vans, low-emission vehicles, and security personnel.
- 19   Security parking shall be located close to the station entrance.

20

**Table 7-1: Stall Dimensions (in feet)**

Stall Angle	(Parallel)	45°	60°	90°
<b>Stall Dimension</b>				
Normal Car				
width	9.0	9.0	9.0	9.0
length	22.0	18.0	18.0	18.0
Disabled spaces				
width	—	14.0	14.0	14.0
length	—	18.0	18.0	18.0
Disabled Van Space				
width	—	17.0	17.0	17.0
length	—	18.0	18.0	18.0
Small Car				
width	—	8.0	8.0	8.0
length	—	15.0	15.0	15.0
Clear Aisle	width (Roadway)	13.0 (one way)	19.0 (one way)	27.0 (two way)

Note: Parking stalls at wayside facilities shall be angled at 90 degrees with 12 feet width by 18 feet length.

Disabled parking spaces shall be provided in accordance with Title 24, Part 2, Section 1129B, CCR (refer to *Caltrans Standard Plan Accessible Parking Off-Street*). Disabled parking spaces shall be located as near as practical to a primary entrance to a facility (building, station entrance). The space shall be located so that a disabled person does not have to wheel or walk behind parked cars other than his/her own. Pedestrian ways shall be provided so as to ensure an accessible pathway from each such parking space to the facility; walks and sidewalks shall conform to Title 24, Part 2, Section 1133B.7, CCR.

When parking is provided for patrons, employees, or visitors, the minimum number of disabled spaces required is as follows:

**Table 7-2: Disabled Parking Spaces**

Total Number of Parking Spaces	Number of Disabled Parking Spaces Required
1 – to 25	1
26 – 50	2
51 – 75	3
76 – 100	4
101 – 150	5
151 – 200	6
201 – 300	7
301 – 400	8
401 – 500	9
501 – 1,000	2 percent of total
1,001 – Over	20 plus 1 for each 100 over 1000

Note: One (1) Van Parking Space for every 8 disabled spaces

The requirements in Title 24, CCR, shall be met for other factors such as signage, minimum dimensions of accessible spaces, slope of parking surfaces, and entrances/vertical clearances for parking structures.

For motorcycle parking layout details, refer to *Standard and Directive Drawings*.

Parking structures shall be designed with “Crime Prevention through Environmental Design” principles, including clear lines of sight, well lit, and no areas where persons could secrete themselves or weapons.

For parking lot lighting and electrical charging station requirements refer to the *Facility Power and Lighting Systems* chapter.

#### **7.7.5.1 Parking Facilities for Wayside Facilities**

A minimum of 4 parking stalls shall be provided within the fenced area for maintenance personnel at ventilation structures and traction power facilities, unless otherwise indicated on the preliminary engineering documents.

A minimum of 2 parking stalls shall be provided at standalone radio sites.

Parking shall be provided, although not necessarily within fenced areas, for other wayside facilities.

#### **7.7.5.2 Parking Facilities for RSM Facilities and MOI Facilities**

A minimum of 150 parking spaces shall be provided in proximity to the facility for management and administrative personnel, visitors, deliveries, and priority parking.

1 Parking spaces shall be provided for 50 crew, 50 rolling stock preparations, and 150 yard  
2 support employees.

### 7.7.5.3 Multilevel Parking Structures

3 For Multilevel Parking Structures criteria refer to the *Stations* chapter.

### 7.7.5.4 Kiss-and-Ride Facilities

4 For Kiss and Ride Facilities criteria refer to the *Stations* chapter.

## 7.7.6 Sidewalks and Driveways

### 7.7.6.1 Sidewalks

5 Maximum cross slope shall be 2 percent, and match the elevation of existing building(s)  
6 finished floor elevation(s), flow away from the building(s).

7 Minimum slope shall be 0.5 percent.

8 Federal and State accessibility requirements shall be met for sidewalk areas behind and  
9 adjoining driveways, alley openings, and pedestrian ramps.

10 Minimum sidewalk width shall be 5 feet.

11 For design details, refer to *Standard and Directive Drawings*.

### 7.7.6.2 Driveways

12 Where the Authority's driveways cross sidewalks which are under another authority having  
13 jurisdiction, the driveway/sidewalk details shall be governed by the more stringent  
14 requirements of either the authority having jurisdiction or the Authority.

15 Driveway entrances that provide access to RSM Facilities and MOI Facilities shall normally be  
16 paved per *Caltrans Standard Plans* details. This pavement shall be considered adequate for  
17 normal truck use and for infrequent overloads. Greater pavement thickness shall be provided  
18 where warranted by the volumes or type of traffic using the driveway.

## 7.7.7 Walkways and Cable Trough

19 This section addresses criteria for retained fill and at-grade walkways. For trench, aerial, and  
20 underground walkway criteria refer to the *Structures* and *Tunnels* chapters.

21 For walkway structural loads refer to the *Structures* chapter.

22 Continuous 3-foot minimum width walkway(s) shall be provided along the following:

- 23 • **Two-track locations** – There shall be 1 outside walkway on each side.

1 • **Four-track locations (at stations)** – With 2 outer station tracks, there shall be a walkway  
2 between the main tracks and the station tracks, along the inter-track fence next to the  
3 through tracks.

4 • **Four-track locations** – There shall be a walkway outside the outermost tracks only.

5 Walkways shall have a longitudinal slope not greater than 1 inch vertical to 8 inches horizontal.

6 Walkways along tracks shall be the top of the cable trough where practicable. The covers of the  
7 cable trough will form part of the walkway surface.

8 As the cable trough will also act as part of the emergency walkway, it shall provide an even,  
9 stable walking surface without creating a tripping hazard and have a non-slip surface.

10 Cable trough shall be in line with the top surface of the sub-ballast or bearing base layer.

11 Cable trough will carry the train control, power, and communications cables required to operate  
12 the HST. For dimensions and cable trough layout transition layouts, refer to *Standard and*  
13 *Directive Drawings*.

14 Cable troughs and connection boxes shall be drained. The drainage within the cable trough  
15 shall be provided to drain any percolated surface runoff. The water drained from cable troughs  
16 shall be properly diverted into the side drainage system. If drainage material is used for this  
17 purpose, the material shall meet the Terzaghi's filter criteria.

18 Walkway design layouts shall avoid "dead ends".

19 For walkway envelope, refer to the *Trackway Clearances* chapter.

20 Where walkways are adjacent to a wall exceeding 5 feet in height above the top of adjacent rail  
21 and more than 100 feet in length, the walkway shall be in conformance with top of the adjacent  
22 rail. Handrails shall be installed 3 feet – 6 inches above walkway surface providing a route to a  
23 safe area. Handrails shall not interfere with the stated clearance envelopes, including the  
24 walkway clearance envelope.

25 Yard walkways shall be paved, 5-foot wide, and located between yard tracks. Refer to *Standard*  
26 *and Directive Drawings* for additional guidance.

#### 7.7.7.1 Stations

##### A. Walkways at End of Station Platform

27 Walkways shall provide access to the underside of vehicles and the refuge space below the  
28 platform.

29 The length of the walkway beyond the platform, not including ramps or stairs, shall be 70 feet.

30 For further guidance and standards refer to the *Stations* chapter.

### 7.7.8 Service Aisles

Each storage track in yards shall have service aisles for its entire length, from fouling point to fouling point or from end of track to fouling point.

Service aisles shall be continuous, connecting to service roads.

Service aisles shall be paved and a minimum of 10 feet wide without obstructions.

Whenever possible, service aisles shall be located between tracks and serve both adjacent tracks.

Service aisles shall be designed to meet all of the requirements of walkways, except as modified in this section. Refer to *Standard and Directive Drawings*.

No stairs shall be used on service aisles.

Service aisles shall not be located within public areas.

At-grade crossings shall be as follows:

- Service aisles at at-grade crossings with yard tracks shall be avoided whenever possible.
- If an at-grade crossing is required for a service aisle, it shall meet all of the requirements for crosswalks.
- Service aisles crossings shall not be located on main track, nor on any track where the trains speed is in excess of 10 mph.

In areas where service aisles are used and a pedestrian crossing or a hi-rail vehicle access point exists on any track subject to automatic train operations, or train speed is in excess of 10 mph, the pedestrian crossing or hi-rail access point shall be protected to prevent inadvertent use by carts. Protection shall be provided using bollards, stairs, fence, gate or other appropriate measure that will not interfere with its intended use.

Service aisles and inspection walkways shall not be located closer to the track than the safe distance from the track for workers. Nominal distance from center line of track to edge of service aisle or walkway shall be 12 feet minimum in open air. The specific distance shall be determined following selection of rolling stock and in conjunction with the operating plan.

### 7.7.9 End-of-Line Walkway

Wherever it is determined by the Authority that service aisles are not required along end-of-line storage tracks, a 3-foot wide lighted and paved walkway, level with the top of tie, shall be provided for accessibility to each end-of-line storage track. A single walkway between tracks is preferred.



### 7.7.10 Crosswalks

Crosswalks shall be provided along HST trackway, connecting the 2 outside walkways, at access and egress points.

Crosswalks shall be provided at track level to assure walkway continuity where safety walkways are discontinued on 1 side of the trackway and continued on the opposite side and where access is required from safety walkways and track walkways to egress points, i.e., exit stairs, cross-passages, or maintenance stairs.

Crosswalks shall be concrete paved and have a minimum width of 8 feet and a walking surface at top of rail elevation.

Elevated safety walkway shall be brought down to the top of rail level by using ramps or stairs at crosswalks. The length of the lower walkway shall be the width of the crosswalk.

Crosswalks shall not be located where the track is superelevated in excess of 5 ½ inches to ensure the maximum slope does not exceed 8.3 percent.

Crosswalks shall not be located between, on, or within special trackwork.

Handrails shall not be used at crosswalks.

Where a parked train can block a crosswalk, an alternative exit path shall be provided. There shall not be “dead-end” walkways.

### 7.7.11 Emergency Stairs

For trackway emergency stairs criteria refer to the *Structures* chapter, and for location refer to the *System Safety and Security* chapter.

### 7.7.12 Maintenance Access

Maintenance access shall be provided along CHSTS infrastructure to allow inspection and maintenance of CHSTS infrastructure and facilities. The following maintenance access requirements shall be provided:

#### A. Aerial Structures

- Fifteen-foot wide maintenance access along the entire length of aerial structure measured from both outside edges of structure.
- Fifteen-foot wide clearance around the columns and foundations.

Where continuous maintenance access cannot be provided on both sides of the structure due to existing obstacles, continuous access shall be provided at least on 1 side. Where existing obstacles do not allow providing continuous access on either side, such as river or cross roads, maintenance access shall be established beyond the obstacle as soon as practical.

B. Underground Structures

- Fifteen-foot (10-foot [minimum] when right-of-way is constrained) wide surface and subsurface maintenance easement, measured from the outer edges of a bored tunnel wall for ground improvements installed during construction.
- Fifteen-foot (5-foot [minimum] when right-of-way is constrained) wide surface and subsurface maintenance easement, measured from the outer edges of cut-and-cover tunnels.
- Fifteen-foot (10-foot [minimum] when right-of-way is constrained) wide maintenance access from the outside edge of the trench wall and evacuation structure of the trench section.

C. Retained Fill or Cut Structures

- Fifteen-foot (10-foot [minimum] when right-of-way is constrained) wide maintenance access, measured from the Access Restriction (AR) fence to the face of the wall (for retained fill) or to back of the wall (for retained cut), along the entire retaining walls. If retaining wall is at the right-of-way and AR fence is on top of the wall, a 10-foot wide maintenance access easement measured from the outside face of the wall shall be provided.

D. At-Grade

- Fifteen-foot wide maintenance access measured from the catch point of fill or cut slope to the right-of-way fence for at-grade sections. For cut slopes higher than 30 feet, a 20-foot maintenance access would be required.

E. Stations

- Access requirements around the CHST stations will be developed at a later time.
- All maintenance access shall be drivable. Refer to *Standard and Directive Drawings* for typical sections of maintenance access along HST alignment.

## 7.8 Access Control Devices

For fence and gate location and additional requirements refer to the *System Safety and Security* chapter.

Access control devices are considered as ancillary structures and shall be designed to accommodate loads and load combinations defined in the *Structures* chapter.

### 7.8.1 Access Control

The entire Authority's right-of-way shall be protected at the property line with an anti-climb fence, wall or fence-wall, fence-traffic barrier or wall-traffic barrier combination to prevent unauthorized access by people, vehicles or animals. Refer to the *Overhead Contact System and Traction Power Return System* chapter for clearances and protection against electric shock.

### 7.8.1.1 Fences

Fencing shall be installed during construction as a means of protecting Authority properties. If temporary fencing is installed, it shall be replaced by permanent fencing prior to completion of construction.

Permanent fencing to be used is as follows:

- Access Restriction (AR) Fencing
- Access Deterring (AD) Fencing
- Station Area (SA) Fencing

Refer to the *Stations* chapter for Architectural and Station Area fencing within the station site areas, including transitions to AR and AD fencing, and pedestrian and vehicle gates on station site.

#### A. Access Restriction Fencing

AR fencing is permanent fencing used to deny access to the HST trackway and to protect the Authority's property.

AR fences shall meet the following minimum material and height requirements.

Unless otherwise specified, fencing shall extend from ground level to a minimum height of 8 feet. AR fence shall be welded wire mesh as indicated in *Standard and Directive Drawings*. In high security areas, the AR fence fabric shall be expanded metal mesh as indicated in *Standard and Directive Drawings*. The locations of high security shall be determined through a Site-Specific Hazard Analysis.

.

Fence posts shall be cast into concrete footings, set into concrete retaining walls or set in rigid traffic barriers.

AR fences shall be located inside the Authority's right-of-way within a distance of 1 foot from the right-of-way line.

Combinations of walls or barriers with welded wire fabric or expanded metal mesh with a total height of 8 feet measured from the highest ground surface adjacent to the wall or barrier may be used as AR fencing.

#### B. Access Deterring Fencing

AD fencing is permanent fencing used to deter access and/or prevent from passing through to areas that do not require a high degree of security. AD Fencing is primarily within Authority's right-of-way. AD fences may also be used in areas where the risk of trespassing is low, such as storage area inside a wayside facility. AD fencing shall be 6 feet high, consisting of 6 feet of

welded wire mesh. Fence posts shall be cast into concrete footings, set into concrete retaining walls or set in rigid traffic barriers.

#### C. Grounding of Fencing

Permanent fencing shall be bonded and grounded to prevent electric shock from induced voltage. Refer to the *Grounding and Bonding Requirements* chapter for additional details on the requirements for grounding of fences.

#### D. Vegetation Control along Fences

Vegetation along fenced areas of Authority property shall be controlled to assure that no large trees or shrubs provide access over the fence by people or animals. Fencing and trees, including branches, shall be kept apart a minimum of 5 feet. Refer to the *Overhead Contact System and Traction Power Return System* chapter for vegetation clearance to electrical lines within the Authority right-of-way. Future growth of vegetation shall be considered when planning new landscaped areas.

#### 7.8.1.2 Walls

Walls may be used to prevent intrusion by vehicles into Authority property. When appropriate, walls may be used in combination with fences at the following locations:

- Where there is vertical separation from trackway
- Where there is close proximity between the HST trackway and an adjacent transportation facility

Refer to the *Rolling Stock and Vehicle Intrusion Protection* chapter for vehicular intrusion protection.

#### 7.8.1.3 Traffic Barriers

Traffic barriers may be required where the Authority right-of-way abuts public and private roads and highways and at highway overpasses where there is a potential of vehicles accidentally entering the Authority right-of-way. Traffic barriers used to protect HST trackway shall comply with the requirements of the *Rolling Stock and Vehicle Intrusion Protection* chapter and Caltrans Standards:

##### A. Traffic Barriers Types

Traffic Barriers shall be either rigid or semi-flexible depending on the location as indicated herein. Refer to *Caltrans Standard Plans* for installation and construction details. Refer to the *Rolling Stock and Vehicle Intrusion Protection* chapter and *Standard and Directive Drawings* for barrier types and clearance requirements between Authority and Caltrans facilities.

- **Rigid Traffic Barrier** –Refer to the *Rolling Stock and Vehicle Intrusion Protection* chapter for types of rigid traffic barriers. When rigid traffic barriers are placed at Authority right-of-way, the barrier shall be used in conjunction with AR fences to prevent intrusion into HST trackway from adjacent roadways.

- **Semi-flexible Traffic Barrier** – Shall be used along service roads located within Authority right-of-way except when such roads are located in close proximity to any track in which the placement of barriers may compromise the trackway clearance envelope. Semi-flexible barriers shall be placed at the outside of curve as required by safety considerations, to delineate the roadway and maintain vehicles within the trackway. Semi-flexible traffic barriers shall be Caltrans Metal Beam Guard Railing of the appropriate type for the local condition. Design and installation details for Metal Beam Guard Railing are found in *Caltrans Standard Plans*.

#### **7.8.1.4 Fencing and Traffic Barriers in combination**

AR fencing shall be located a minimum of 3 feet from the back side of a semi-flexible traffic barrier.

If 3 feet separation cannot be achieved, a rigid traffic barrier shall be used and fence height shall be increased by a height equal to the height of the barrier.

If AR or AD fencing is installed on top of a rigid barrier the combined height of fence and barrier shall not be less than 8 feet.

#### **7.8.1.5 Gates**

Gates with locking devices shall be provided along fenced areas to allow access to authorized personnel, emergency vehicles, and maintenance equipment.

Gates shall be constructed of the same material and height as adjacent fence and shall not decrease the level of security provided by the fences.

Gates shall be either swinging or sliding type. Sliding gates shall be utilized where swinging gates foul the walkway or vehicle envelope when opened. For gate details, refer to *Standard and Directive Drawings*.

Gate locations along fencing within freeway right-of-way shall require Caltrans approval.

Gate locations shall be coordinated with (i.e., placed adjacent to or near) the location of Authority wayside facilities requiring access from outside Authority right-of-way.

At aerial sections, access to the trackway shall be made from stations or emergency stairs or by mobile ladder equipment from roadways adjacent to the trackway. If no adjacent or crossing roadways exist, construction of access roads is required. Gates shall be provided at emergency stairs landing along aerial structures at intervals of 2.5 miles nominal on either side of the trackway (not on both).

For gate placement at communications, train control, and traction power facilities refer to *Standard and Directive Drawings*.

Gates shall be bonded and grounded to prevent electric shock from induced voltage. Refer to the *Grounding and Bonding Requirements* chapter for additional details on the requirements for grounding of gates.

A. TYPES OF GATES

• Walking Gates

Gates for personnel and equipment access (walking gates) shall have a minimum width of 4 feet.

For Traction Power Facilities, walking gates shall be 6 feet wide.

• Driving Gates

Gates for vehicular access (driving gates) shall have a minimum width of 12 feet.

Gates along right-of-way fencing may require approval by the local fire protection authority having jurisdiction. For emergency responders vehicular access minimum gate width shall be 20 feet.

Driving swinging gates shall be a pair and shall be hinged from the inside. Provision shall be made for swinging gates to swing not less than 90 degrees away from Authority facilities.

Driving gates shall be provided in conjunction with either access roads or at locations where existing roads make it practicable for emergency vehicles to get to the trackway.

Along at-grade trackway, driving gates shall be located at 2.5 mile nominal intervals on either right-of-way side. When possible, access gates shall be staggered.

Driving gates shall be provided at traction power facilities. Minimum gate width shall be 20 feet.

Trackside access driving gates shall be provided at Authority facility locations. If this cannot be provided due to site constraints, an alternative method of providing vehicular access to the trackside from the Authority facility shall be submitted to the Authority for review and concurrence.

**Table 7-3: Access Control - Gates**

Type of HST Trackway	Nominal Gate Spacing Interval
At-Grade, Unretained Fill, and Unretained Cut	2.5 miles
Aerial	2.5 miles
Retained Fill (Embankment) or Retained Cut	2.5 miles

#### 7.8.1.6 Fence Signage

Fencing shall be provided with signs warning of hazards from operations, high voltage electrical installations, and any other relevant hazards, at any location where the public may reasonably be expected to approach the right-of-way.

Signs bearing the words “Danger”, “High Voltage Lines” and “Keep Away” in letters at least 3 inches in height, shall be installed at intervals of not more than 500 feet along each fence enclosing the rights-of-way, at every gate and at each station or passenger loading platform, at a height between 5 and 6 feet from the finished ground outside Authority property. The signs may carry other information relative to the hazard present, but the three required phrases “Danger”, “High Voltage Lines”, and “Keep Away” shall be larger size type than the type used for additional items.

Signs bearing the message “No Trespassing” in letters of at least 3 inches in height, and including the California Penal Code section number for trespassing in smaller letter size, shall be installed along the fence at a minimum, every 500 feet, at a height between 5 and 6 feet from the finished ground outside Authority property.

Signage shall be consistent throughout the Authority system.

#### 7.8.2 Access Control by Type of HST Trackway

Access to non-public Authority property shall be controlled by installing perimeter fences (AR) along the right-of-way with locked gates to allow access and egress of maintenance and emergency personnel.

Within the vicinity of a passenger station the right-of-way fencing shall be installed to guide the passengers to the designated platform entrances and to prevent unsafe shortcut to the platform.

##### 7.8.2.1 At-Grade Trackway

AR fencing shall be provided continuously along each side of at-grade trackway sections, including transitions to underground or aerial sections. Fence construction shall be designed, installed and maintained in such manner as to deny access over, under or through the fencing

to animals and unauthorized persons. The HST system shall have no at-grade public road crossings or at-grade crossings of other rail systems.

#### 7.8.2.2 At-Grade Trackway within Roadway Corridor

A combination of AR fence and an appropriate rigid traffic barrier shall be constructed along the Authority right-of-way when it is at-grade, runs parallel and within Caltrans Clear Recovery Zone to a roadway, including locations where the trackway shares a common corridor in a roadway median.

The rigid traffic barrier shall be located within the Authority right-of-way at a minimum distance of 1 foot inside the Authority right-of-way.

Refer to the *Rolling Stock and Vehicle Intrusion Protection* chapter for traffic barrier requirements.

#### 7.8.2.3 At-Grade Trackway Adjacent to Conventional Railroad

Protection against accidental intrusion shall be provided where a HST trackway is in close proximity to a conventional railroad corridor. Refer to the *Rolling Stock and Vehicle Intrusion Protection* chapter for additional requirements.

#### 7.8.2.4 At-Grade Trackway through High-Risk Trespassing Areas

Special consideration shall be given to areas determined to have a high-risk of trespass such as, but not limited to, HST trackway adjacent to parks, playgrounds, schoolyards, highly populated urban areas or areas within the pathway to and from any of these places, which require a higher degree of security. The AR fence at high-risk trespassing areas shall be expanded metal mesh. The surface under the fence shall be paved to prevent undermining or fence fabric shall be embedded a minimum of 1 foot into the ground.

#### 7.8.2.5 Trackway in Cut or Fill (Embankment) Section

AR fencing shall be used for HST trackway along embankment and cut sections. AR fences shall not be placed on the slope surfaces of the cut or embankment sections.

When the HST trackway section is in cut, AR fences shall be located at a distance from the top of slope, after slope rounding:

- Recommended 15 feet
- Minimum 10 feet

When the HST trackway section is on embankment, AR fences shall be located at a distance from the toe of slope:

- Recommended 15 feet
- Minimum 10 feet



### 7.8.2.6 Trackway on Aerial Structure

At abutments of aerial structures, AR fencing from adjoining sections shall be continued beyond the abutment to a point where the soffit of the structure is 10 feet or more above the finished grade. At that point, the right-of-way fences on each side of the aerial structure shall be joined under the aerial structure.

Where the ground level is less than 10 feet below the soffit of the structure, aerial structure sections shall be protected with AR fencing located 15 feet from the drip-line of the structure. Fencing is not required under an aerial structure where the right-of-way adjoins property which is already fenced in a manner consistent with these criteria.

Fencing is not required on aerial structure except at abutments. Area around the columns and foundations shall remain accessible for 15 feet outside of the foundation limits.

### 7.8.2.7 Trackway on Retained Fill

Retained fill trackways shall have at least 1 vertical wall exceeding 5 feet in height measured from finished surface along the non-trackway side of the wall.

AR fencing shall be installed on top of retaining walls along retained fill sections on the following conditions:

- When retaining walls are located at the right-of-way line along HST trackway
- The adjacent land outside Authority property is less than 10 feet below the top of the wall. Refer to *Standard and Directive Drawings* for fence details.

The combined height of the wall with traffic barrier and fence above the adjacent ground outside the Authority right-of-way shall not be less than 8 feet.

Where a retaining wall is within the right-of-way line, security fencing shall not be placed on top of the wall. However, railing shall be required along the top of wall for fall protection, refer to the *Structures* chapter.

Fencing between at-grade and retained fill sections shall be continuous.

Where a retaining wall is used as support for both fencing and poles supporting lights, overhead contact system (OCS), or signs, the open space between the fence and the pole shall be less than 4 inches. Refer to the *Overhead Contact System and Traction Power Return System* chapter for OCS safety barrier requirements.

### 7.8.2.8 Trackway on Retained Cut

Retained cut trackways are defined as trackways having at least 1 of its vertical walls exceeding 5 feet in height measured from the top of rail.

AR fencing shall be installed on top of retaining walls or rigid traffic barriers along retained cut sections on the following conditions:

• When retaining walls are located at the right-of-way line along HST trackway and the adjacent land outside Authority property is less than 10 feet above the top of the wall. Refer to *Standard and Directive Drawings* for fence details.

• Concrete barriers located adjacent to a HST trackway on the right-of-way line.

The combined height of the wall or traffic barrier and the fence above the adjacent ground outside the Authority right-of-way shall be not less than 8 feet..

Where a retaining wall is well within the right-of-way line, security fencing shall be located in accordance with other requirements and shall not be placed on top of the wall. However, railing shall be required along the top of wall for fall protection, refer to the *Structures* chapter.

Fencing between at-grade and retained sections shall be continuous.

Where a retaining wall is used as support for both, fencing and poles supporting lights, OCS or signs, the open space between the fence and the pole shall be less than 4 inches.

#### **7.8.2.9 Trackway Underground (Bored, Mined, and Cut-and-Cover Tunnels)**

Where at-grade sections adjoin underground sections, the right-of-way AR fencing shall extend beyond the portal by a minimum of 30 feet where possible and be continuous across the right-of-way at that point.

Where the minimum of 30 feet cannot be achieved because of physical constraints or an adjacent public way, the fencing shall extend across the right-of-way at the point of constraint.

Authority property above underground sections of HST trackway shall be protected in accordance with these criteria and the facility and/or use of the land above ground. If the use of the land above ground has not been defined and no HST facility has been constructed, perimeter AD fencing with gates shall be installed to prevent unauthorized access to the property.

**Table 7-4: Access Control Fencing – Trackway**

High-Speed Train Infrastructure	Fence Type AR	Walls / Barrier, Other
At-grade	✓	
At-grade adjacent to Roadways <sup>(1)</sup>	✓	✓
At-grade within Highway Corridor	✓	✓
At-grade adjacent to Conventional Railroad <sup>(1)</sup>	✓	✓
At-grade through High Risk Trespassing Areas	✓	
Sloped Cut or Fill (Embankment) Section	✓	
Aerial structure	✓	
Retained Fill	✓	✓
Retained Cut	✓	✓
Underground (Bored, Mined and Cut-and-Covered Tunnels)	✓	

<sup>(1)</sup> Use of walls and barriers for intrusion protection shall be made in conjunction with site-specific risk assessment. Refer to the *Rolling Stock and Vehicle Intrusion Protection* chapter for additional requirements.

### 7.8.3 Access Control by Type of Facility

This section prescribes right-of-way fencing for wayside facilities such as yards, maintenance facilities, train control, communications, and traction power facilities.

#### 7.8.3.1 Yards and Maintenance Facilities

AR fencing with vehicular and pedestrian access gates equipped with locking devices shall be installed along the perimeter of Authority facility.

#### 7.8.3.2 Train Control, Communications and Traction Power Facilities

Train control, traction power facilities, and communications equipment locations shall be either fenced with AR fencing or an 8 foot minimum high wall enclosure with secured gates.

Where there is public access or trespass is likely, anti-climbing protection shall be provided at buildings and other structures supporting energized parts of the OCS. In addition to anti climbing protection measures, warning signs shall be installed on the fence or the wall indicating “DANGER HIGH VOLTAGE”. Access to fixed ladders, particularly at signal poles and signal gantries, and the means of access to any roof or other place which could allow non-authorized persons to approach energized parts, shall be secured or otherwise protected.

1 For fencing placement and further requirements at OCS, communications, train control and  
2 traction power facilities refer to the *Traction Power Supply System, Overhead Contact System and*  
3 *Traction Power Return System, Communications, and Automatic Train Control* chapters.

### 7.8.3.3 Passenger Station

4 Station area fence (SA), shall be installed along the right-of-way within the vicinity of a station  
5 platform to prevent unsafe shortcuts to the platform and to guide passengers to the designated  
6 station entrances. Refer to the *Stations* chapter for station area fence standards.

#### A. Limits of Platforms

7 Station area fences with locked gates shall be installed at the ends of station platforms, along the  
8 sides of platforms perpendicular to the tracks, to prevent unauthorized access to the trackway.

#### B. Inter-Track Fencing or Protection Screens (Walls)

9 Inter-track fences or protection screens (walls) shall be provided between through track and  
10 station track or between adjacent tracks at station platforms for the full length of the platforms  
11 and at least 150 feet beyond each platform end. For track spacing refer to the *Trackway*  
12 *Clearances* chapter and for additional criteria on inter-track fencing/protection screens refer to  
13 the *Stations* chapter.

**Table 7-5: Access Control Fencing – Authority Facilities**

High-Speed Train Infrastructure	Fence Type		Walls / Barrier, Other
	AR	AD	
Yard and Maintenance Facilities	✓		
Train Control and Traction Power Facilities	✓		
Passenger Stations	✓		
• Limits of Platforms			✓ <sup>(1)</sup>
• Inter-Track Fencing / Protection Screens			✓ <sup>(1)</sup>
Parking Facilities		✓	

14 <sup>(1)</sup> Station Area Fence

### 7.8.4 Access Control at Other Locations

#### 7.8.4.1 End of Line and Storage Tracks

15 End of line tracks and end of storage tracks that extend from the station tracks, shall be  
16 protected with AR fencing located at the following:

- 17 • One (1) foot minimum distance from the Authority right-of-way line in the direction parallel  
18 to the tracks, and
- 19 • Two (2) feet minimum distance from the right-of-way side of the bumping structure in the  
20 direction perpendicular to the tracks.

#### 7.8.4.2 Roadway Overhead Crossing HST Trackway

Where HST trackway is traversed by a roadway overhead crossing, the overhead crossing shall be provided with a modified AD combination of rigid traffic barrier and a protective solid barrier. The protective solid barrier shall be installed on top of traffic barriers. The limits of the solid barrier shall be over the entire HST right-of-way. For protective solid barrier requirements refer to the *Overhead Contact System and Traction Power Return System, Grounding and Bonding Requirements* chapters and *Standard and Directive Drawings*. For further roadway vehicle containment requirements, refer to the *Rolling Stock and Vehicle Intrusion Protection* chapter.

The modified AD with protective solid barrier at roadway overhead crossings with sidewalk shall be constructed with a curved top to prevent the throwing of objects onto HST trackway. The minimum height of the modified AD with protective solid barrier with the traffic barrier shall be 8 feet. Refer to *Standard and Directive Drawings*.

#### 7.8.4.3 Streets Ending at HST Trackway

A rigid traffic barrier shall be installed at the end of dead-end streets, cul-de-sacs, or "T" intersections adjacent to at-grade segments of HST trackway. The barrier length and height shall be sufficient to intercept all possible vehicular paths from within the traveled way of the approaching street.

**Case 1** – Where the longitudinal grade of the streets dead-ending at HST trackway is 2 percent maximum going down towards Authority property, the barrier shall be a minimum of 4 feet–8 inches above the street surface at the barrier.

**Case 2** – Where steep grades and close proximity of the track require a substantial physical barrier against runaway vehicles, the barrier shall consist of an 18-inch thick reinforced concrete wall. The top of the barrier shall be from 4 feet–8 inches to 6 feet above the street surface at the barrier. Design drawing for these concrete walls shall be submitted to Authority for review and concurrence.

#### 7.8.4.4 Authority Roadways

If conditions along areas of the Authority roadways require installation of traffic barriers, semi-flexible traffic barriers shall be installed, provided that the location of such barriers do not obstruct the clear pathway from walkways to emergency exits or encroach into the clearance envelope of any Authority facility.

Access roads and service roads are not required to be protected with fence.

#### 7.8.4.5 Emergency Exits and Equipment Rooms in Tunnels

Emergency exits, rooms containing fixed equipments, corridors, stairwells, and other controlled areas in tunnels shall have doors and/or gates with a lock system capable of preventing unauthorized access from outside and a release mechanism that makes it possible to open them from the inside for evacuation purposes.

For additional system-wide criteria on securing exits refer to the *System Safety and Security* chapter.

#### 7.8.4.6 Drainage Structures

Where drainage requires passage under the trackway, pipes or box culvert structures shall be used.

HST trackway over box culvert structures shall be protected with AR fencing around headwall as shown in *Standard and Directive Drawings*.

HST trackway over box culvert structures shall be evaluated for their security risk potential by preparation of a threat and vulnerability analysis. If action is required to enhance the structure to address security concerns, options considered shall not conflict with structures that have been designed as designated wildlife crossings. If culvert crossings are designated as wildlife crossing, no grates shall be installed at the entrances of the culverts, subject to approval by appropriate regulatory agency.

For culvert structure sizing refer to the *Drainage* chapter.

Open ditches alongside the trackway may require to be connected to a drainage system outside the Authority access controlled right-of-way. In such cases, open ditches crossing under AR fence shall not be allowed unless the ditch is concrete lined and the open section of the ditch is protected by steel welded grid as shown on *Standard and Directive Drawings*.

**Table 7-6: Access Control Fencing – Other Locations**

High-Speed Train Infrastructure	Fence Type		Walls / Barrier, Other
	AR	AD	
End of Line and Storage Tracks	✓		
On Roadway Overhead Crossings HST		✓ <sup>(1)</sup>	✓
Streets Dead-Ending at HST Trackway			
• Roadway is 2% maximum going down towards Authority property	✓		✓
• Steep grades and close proximity of the track	✓		✓
Access Roads			✓
Service Roads			
Emergency Exits and Equipment Rooms			✓
Drainage Structures			✓

<sup>(1)</sup> Modified AD with protective solid barrier, refer to Section 7.8.4.2.

## 7.9 Wildlife Crossing

Authority facilities shall be designed and constructed such that wildlife movement corridors are preserved and do not create a barrier to wildlife movement in the area.

Refer to Final Environmental Impact Statement, Biological Resources and Wetlands Section for identified wildlife movement corridors that need to be preserved and mitigations measures to follow.

The mitigations measures to be incorporated as a project design feature include wildlife corridor undercrossing, wildlife fencing, and wildlife artificial dens.

## 7.10 Maintenance and Protection of Traffic during Construction

For the duration of the CHSTP construction, every effort shall be made to minimize the interruption of surface traffic, be that pedestrian and vehicular, adjacent to, under or over the construction site.

Access provisions for the following shall be maintained within construction zones:

- Emergency services and emergency vehicles
- Local access to businesses and residences

Temporary interruptions to business and residence access shall be coordinated and approved by the local authority having jurisdiction.

### 7.10.1 Railroad Operations

- 1 Construction activities affecting surface transportation, including freight or other rail operator,
- 2 shall be planned and scheduled in cooperation with the relevant authorities/agencies.
- 3 Temporary structures for the support and maintenance of surface traffic adjacent to under or
- 4 over the construction site shall be designed and constructed in accordance with prevailing
- 5 codes, standards and regulations and are subject to review and approval by the local authority
- 6 having jurisdiction and the Authority.
  
- 7 During construction, interruption of, and interference with passenger and freight rail operations
- 8 shall be avoided unless otherwise approved by the authority having jurisdiction. Wherever
- 9 possible, the design and the sequencing of the construction activities shall allow for such
- 10 uninterrupted railway operations.



## Chapter 8

### Drainage

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	Jun 2013	Revision 1
2	Feb 2014	Revision 2
3	Feb 2016	Revision 3



## Table of Contents

1	<b>8 DRAINAGE.....</b>	<b>8-1</b>
2	8.1 Scope .....	8-1
3	8.2 Regulations, Codes, Standards, and Guidelines .....	8-1
4	8.3 Goals and Objectives .....	8-2
5	8.4 Hydrological Analysis .....	8-3
6	8.4.1 Time of Concentration .....	8-3
7	8.4.2 Intensity .....	8-3
8	8.4.3 Design Storm Frequency/Recurrence Interval .....	8-3
9	8.4.4 Snowmelt .....	8-4
10	8.4.5 Storm Runoff .....	8-4
11	8.4.6 Floodplain Information .....	8-4
12	8.5 Hydraulic Design .....	8-5
13	8.5.1 Basic Parameters .....	8-5
14	8.5.2 Channel Hydraulics .....	8-6
15	8.5.3 Culvert Hydraulics .....	8-20
16	8.6 Trackway and Facility Drainage Systems .....	8-23
17	8.6.1 Track Drainage Systems .....	8-23
18	8.6.2 Embankments and Cut Slopes .....	8-23
19	8.6.3 Bridges/Aerial Structures .....	8-23
20	8.6.4 Tunnels .....	8-26
21	8.6.5 Retaining Walls .....	8-26
22	8.6.6 Trenches .....	8-27
23	8.6.7 Critical HST Structures/Facilities .....	8-27
24	8.6.8 Stations and Platforms .....	8-27
25	8.6.9 Roadways .....	8-27
26	8.7 Detention/Retention of Surface Water Runoff .....	8-29
27	8.7.1 Peak Flow Reduction and Release Rate .....	8-29
28	8.7.2 Required Storage Estimate .....	8-29
29	8.7.3 Basin Sizing .....	8-30
30	8.7.4 Outlet Structures and Emergency Spillways .....	8-31
31	8.8 Stormwater Quality Management .....	8-31
32	8.9 Application of Approved Software .....	8-32

## Tables

Table 8-1:	Design Storm Frequencies .....	8-4
Table 8-2:	Maximum Permissible Velocities for Lined/Non-Erosive Channels .....	8-7
Table 8-3:	Hydraulic Parameters for Channel Sections .....	8-8
Table 8-4:	Suggested Depth and Spacing of Pipe Underdrains for Various Soil Types .....	8-12
Table 8-5:	Maximum Manhole Spacing Requirements .....	8-28

Figures

Figure 8-1: Trapezoidal Hydrograph.....8-30

## Acronyms

AREMA	American Railway Engineering and Maintenance-of-Way Association
Authority	California High-Speed Rail Authority
BMP	Best Management Practice
Caltrans	California Department of Transportation
FHWA	Federal Highway Administration
HDM	Highway Design Manual
HDS	Hydraulic Design Series
HDPE	High-Density Polyethylene
HEC	Hydraulic Engineering Center
HST	High-Speed Train
IDF	Intensity-Duration-Frequency
PVC	Polyvinyl Chloride
RCP	Reinforced Concrete Pipe
RWQCB	Regional Water Quality Control Board
SWDR	Storm Water Data Report
SWPPP	Storm Water Pollution Prevention Plan
USBR	United States Bureau of Reclamation



## 8 Drainage

### 8.1 Scope

This chapter provides design criteria for the hydrologic analysis and design of hydraulic facilities and provides guidelines for hydraulic facility implementation and for Best Management Practices (BMPs) for surface water quality treatment.

Unless otherwise noted, design guidance shall follow California Department of Transportation (Caltrans) Highway Design Manual (HDM) requirements for hydrologic analysis and hydraulics design. Regional criteria shall be used to determine surface water runoff data. Refer to the *General* chapter of this design manual for design life requirements for storm drain structures.

Locations where the California High-Speed Train (HST) alignment crosses existing drainage channels, drainage requirements for roadways and other structures located in or adjacent to the California High-Speed Rail Authority's (Authority's) right-of-way may be subject to regulations and additional requirements by other jurisdictions. Supplemental hydrologic and hydraulic requirements shall be considered for drainage facilities owned or operated by third-party rail operators/agencies, and property owners impacted by these improvements. Design requirements of local municipalities shall be considered for discharge within those jurisdictions. Where a drainage facility is required to connect to a third-party drainage facility, the Designer shall coordinate with the utility owner to determine if an upgrade to the existing facilities may be required.

### 8.2 Regulations, Codes, Standards, and Guidelines

Refer to the *General* chapter for requirements pertaining to regulations, codes, standards, and guidelines. Regulations, codes, and standards, such as but not limited to the following, shall be the one applicable.

- California Public Utility Commission (CPUC) General Orders (GOs)
  - CPUC GO 95 - Rules Governing Overhead Electric Line Construction
  - CPUC GO 128 - Rules for Underground Electric Construction
- California Department of Transportation (Caltrans)
  - Caltrans Highway Design Manual (HDM), English Version
  - Caltrans Standard Plans and Standard Specifications
  - Caltrans Bridge Design Specifications (CBDS)
  - Caltrans Storm Water Quality Handbook: Project Planning and Design Guide

- 1 • Federal Highway Administration (FHWA), Hydraulic Design Series (HDS)
- 2 • U.S. Army Corps of Engineers
- 3 • U.S. Bureau of Reclamation (USBR)
- 4 • U.S. Bureau of Land Management
- 5 • Federal Emergency Management Agency (FEMA)
- 6 • American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for
- 7 Railway Engineering
- 8 • American Association of State Highway and Transportation Officials (AASHTO)
- 9 – Highway Drainage Guidelines
- 10 – Model Drainage Manual
- 11 • Applicable Local Ordinances
- 12 Applicable local building, planning, and zoning codes and laws shall be reviewed for facilities,
- 13 particularly those located within multiple municipal jurisdictions, state rights-of-way, and/or
- 14 unincorporated jurisdictions.

### **8.3 Goals and Objectives**

- 15 The following goals and objectives shall be considered in development of drainage design:
- 16 • The CHSTP shall not adversely impact the existing floodplain of the area adjacent to the
  - 17 HST corridor.
  - 18 • Ensure critical HST structures/facilities are protected against 100- and 500-year flood events.
  - 19 Critical HST structures/facilities, in this chapter, refers to HST structures/facilities that are
  - 20 critical to safe operation of HST system, refer to Section 8.6.7.
  - 21 • Comply with regulatory requirements.
  - 22 • Contain drainage within the Authority's right-of-way.
  - 23 • Keep runoff from outside the Authority's right-of-way from entering into the Authority's
  - 24 right-of-way.
  - 25 • To the extent that is reasonable and practical, avoid placement of third-party drainage
  - 26 access points from within the Authority's access controlled right-of-way.



## 8.4 Hydrological Analysis

Hydrologic design and analysis shall conform to industry standards, codes, guidelines, and utilize applicable software. The criterion for each factor involved in hydrologic analysis to obtain optimum runoff calculations are outlined in this section. For criteria not included in this section, references shall be used as follows:

- Caltrans HDM for rainfall hydrological analyses
- FHWA HDS-02 for criteria not found in Caltrans and for snowmelt analyses

### 8.4.1 Time of Concentration

The time of concentration ( $T_c$ ) shall be used to determine approximate rainfall intensity. The time of concentration is the sum of 2 travel times, including sheet flow/overland flow and shallow concentrated flow, usually in a gutter, swale or channel.

The minimum time of concentration recommended in urban areas is 5 minutes and in rural areas is 10 minutes. For gutter, pipe and channel flow, Manning's equation shall be used, per Section 8.5.2.

The Caltrans HDM Hydrology chapter shall be referenced for detailed methodologies on  $T_c$  calculation.

### 8.4.2 Intensity

Intensity is defined as the time rate of rainfall depth and is commonly given in inches per hour. The time of concentration depends on an initial estimate of an intensity value found from Intensity-Duration-Frequency (IDF) curves or the National Oceanic and Atmospheric Administration (NOAA) Precipitation Frequency Data Server (PFDS) using the following link: [http://hdsc.nws.noaa.gov/hdsc/pfds/pfds\\_map\\_cont.html?bkmrk=ca](http://hdsc.nws.noaa.gov/hdsc/pfds/pfds_map_cont.html?bkmrk=ca)

IDF curves shall be obtained from local agencies or Caltrans for the most current and accurate information.

### 8.4.3 Design Storm Frequency/Recurrence Interval

Frequency establishes the frame of reference for how often precipitation with given characteristics is likely to occur. The design storm frequencies for the design of various storm facilities shall be as shown in Table 8-1.

**Table 8-1: Design Storm Frequencies**

Storm Facility	Rural	Urban
Drainage facilities crossing the track (e.g., culverts)	2% (50-yr) <sup>(1)</sup>	1% (100-yr) <sup>(1)</sup>
Drainage facilities not crossing the track (e.g., parking lots, access roads, station drainage facilities)	10% (10-yr) <sup>(1)</sup>	2% (50-yr) <sup>(1)</sup>
Ditches/storm drainage systems adjacent to the track	4% (25-yr) <sup>(1)</sup>	2% (50-yr) <sup>(1)</sup>
Freeways – Minor Ramps and Frontage Roads	10% (10-yr) <sup>(2)</sup>	
Conventional Highways – High volume, multilane or urban with speeds 45 mph and under		
Freeways – Through traffic lanes, branch connections, and other major ramp connections	4% (25-yr) <sup>(2)</sup>	
Conventional Highways – High volume, multilane or low volume, rural with speeds over 45 mph		
All State Highways	2% (50-yr) <sup>(2)</sup>	
Drainage systems crossing under bridge structure and on the right-of-way	2% (50-yr) <sup>(1)</sup>	1% (100-yr) <sup>(1)</sup>
Critical HST Structures/Facilities <sup>(3)</sup>	Min 0.2% (500-yr) <sup>(3)</sup>	Min 0.2% (500-yr) <sup>(3)</sup>

**Notes:**

- (1) Based on Standard Engineering practices employed by other railroad operators within California.  
(2) Caltrans HDM, Table 831.3 shall be referred to for Roadway Drainage Guidelines.  
(3) For critical HST Facilities, refer to Section 8.6.7.

#### 8.4.4 Snowmelt

For runoff calculations in areas where snowmelt may occur, refer to the FHWA HDS-02 Hydrology report.

#### 8.4.5 Storm Runoff

Storm runoff shall be calculated in accordance with criteria and methodologies specified in the Caltrans HDM's Hydrology chapter and the applicable local procedures.

#### 8.4.6 Floodplain Information

The proposed elevation of the track subballast (bottom) shall be a minimum of 2 feet higher than the 100-year Base Flood Elevation. Drainage facilities located within a floodplain shall be designed so that the proposed improvements will not result in the following:

- 1 • Increase the flood flow rate or inundation hazard to adjacent upstream or downstream
- 2 property
- 3 • Raise the flood level of drainage way
- 4 • Reduce the flood storage capacity or obstruct the movement of floodwater within a drainage
- 5 way
- 6 Refer to the Caltrans HDM General Aspects chapter for FEMA guidelines where encroachment
- 7 on floodplains is anticipated.

## 8.5 Hydraulic Design

### 8.5.1 Basic Parameters

#### 8.5.1.1 Discharge of Storm Drains into Local Drainage System

8 Discharge of stormwater from trackway sections, station sites, parking lots, and wayside  
9 facilities into local drainage system shall comply with environmental and regulatory permit  
10 requirements. Appropriate mitigation measures to prevent pollutants shall be taken as required  
11 before site drainage is discharged into local drainage system.

#### 8.5.1.2 Debris Control

12 Debris may consist of trash, natural streambed material such as boulders, silts, sands, clays,  
13 sticks, tree limbs and other vegetation. Buoyant material will float during a storm event and  
14 other materials will roll or skip along channel bed. The frequency of the storm event will affect  
15 the quantity of debris that is carried along the channel; the more discharge in the channel will  
16 result in more debris carried.

17 Debris control shall be a significant consideration during the design of hydraulic structures such  
18 as catch basins, culverts, storm drain systems, and outlet structure from detention basins.  
19 Depending on the type of debris and location where the debris is controlled, there are several  
20 options for debris control structures such as debris racks, debris risers, debris cribs, and debris  
21 fans. The type and quantity of debris shall be determined and an appropriate debris control  
22 measure shall be implemented.

#### 8.5.1.3 Access Control

23 For drainage structure access control requirements, refer to the *Civil* chapter.

#### 8.5.1.4 Grounding and Corrosion Control

24 All metallic pipes and appurtenances shall be grounded and protected against corrosion. Refer  
25 to the *Grounding and Bonding Requirements* chapter and the *Corrosion Control* chapter for  
26 requirements.

## 8.5.2 Channel Hydraulics

### 8.5.2.1 Open Channel Hydraulics

This section presents the minimal criteria and design standards for the hydraulic evaluation and design of open channels. For criteria not included in this section, references shall be used as follows:

- AREMA's design criteria shall be followed for design of new open channels.
- Refer to Caltrans HDM for design criteria not available in AREMA, and for existing open channels.
- Local criteria shall be followed as required by the governing agency.

Transverse channels that pass through culverts shall join parallel ditches at an angle of approximately 30 degrees to minimize aggradations and deposition.

#### A. Open Channel Flow

The computation of uniform flow and normal depth are based on Manning's formula:

$$Q = \frac{1.49}{n} A R^{2/3} S^{1/2}$$

Where:

Q = Flow Rate (cfs)

n = Roughness Coefficient<sup>1</sup>

A = Flow Area (feet<sup>2</sup>)

R = Hydraulic Radius, A/P, (feet)

P = Wetted Perimeter, (feet)

S = Slope of the Energy Grade Line, (feet/feet)

#### B. Maximum Permissible Velocity

Open channel flow velocities shall not erode nor cause deposition in the channel. For maximum permissible velocities of unlined channels refer to the Caltrans HDM. The maximum permissible velocities for lined/non-erosive channels are presented in Table 8-2

---

<sup>1</sup> The Caltrans HDM Physical Standards chapter shall be referred to for Manning's roughness coefficients.

**Table 8-2: Maximum Permissible Velocities for Lined/Non-Erosive Channels**

Type of Lining	Maximum Permissible Velocity (feet per second)
Unreinforced vegetation	5.0
Loose riprap	10.0
Grouted riprap	15.0
Gabions	15.0
Soil-Cement	15.0
Concrete	35.0

Source: Clark County Regional Flood Control District, Hydrological Criteria and Drainage Design Manual, August 1999.

**C. Freeboard**

Freeboard is the vertical distance between the design water surface elevation and the bottom of subballast or bottom of a bridge girder/soffit. The minimum freeboard required for track side ditches shall prevent saturation and infiltration of stormwater into the subballast and ballast sections of the track. The minimum recommended water depth in any channel section shall be 2 feet below the bottom of subballast or bottom of a bridge girder/soffit.

For superelevation requirement on curved open channel alignment for water surface elevation, refer to the Caltrans HDM.

**D. Grade Control**

If the ditch grade is steeper than the grade that results in maximum permissible velocities, drop structures shall be considered to maintain design velocities, and prevent erosion and scour to the channel bed and embankments. To mitigate sediment aggradation and degradation, a drop structure may be installed across a channel to create a vertical drop or a short sloping drop. Drop structures may be built with gabions, sheet piling, riprap, or concrete walls with footings.

**E. Channel Section**

Selection of a channel cross section may involve changing/improving the existing natural waterway (channel) or designing a new channel section. Where feasible, the channel section is preferred to have a flat bottom.

**Changing a natural channel** – Existing hydraulic conditions of a natural channel shall be assessed to confirm channel stability and evaluate the impact of proposed improvements. Natural waterways shall have adequate capacity to pass the flows from a design storm event. The stream mechanics shall be studied to analyze the need for erosion control structures.

**Design of new channel section** – The first estimation of a channel cross section is based on normal depth plus freeboard. The shape of the channel shall consider terrain, flow velocity, available right-of-way for the corridor, and quantity of flow to be conveyed. Channels shall be sized for the anticipated design runoff and to allow the subballast to drain. Open channels

- 1 along the toes of trackway embankments shall be designed with sufficient depth to carry the
- 2 peak flow with design water surface elevation below the bottom of subballast.
- 3 Hydraulic parameters of basic channel sections are provided in Table 8-3. For trackside ditch
- 4 details, refer to Standard and Directive Drawings.

**Table 8-3: Hydraulic Parameters for Channel Sections**

Channel Section	Area (A)	Wetted Perimeter (P)	Hydraulic Radius (R)
Triangular V-ditch	$zy^2$	$2y\sqrt{1+z^2}$	$\frac{zy}{2\sqrt{1+z^2}}$
Trapezoidal	$(b + zy)y$	$b + 2y\sqrt{1+z^2}$	$\frac{(b + zy)y}{b + 2y\sqrt{1+z^2}}$
Rectangular	$by$	$b + 2y$	$\frac{(by)}{(b + 2y)}$

Notes:

- 6 b = base of rectangle or base of trapezoidal
- 7 z = side slope of trapezoid or V-ditch
- 8 y = depth of flow in channel

**F. Channel Lining**

Channel lining is the key factor that determines the roughness coefficient of a channel. The most commonly used channel materials are the following:

**Grass lined channel (vegetative lined)** – Grass linings provide protection to the channel from erosion. Due to the frequent maintenance involved, grass lined channels are not recommended within the Authority's right-of-way. They may be used as a best management practice for stormwater quality control where pollution prevention devices are required. Grass lined channels shall not be designed with side slopes steeper than 3:1 (H:V).

**Riprap lined channel** – Riprap lining is suitable for a short but steep channel reach. The nature of high friction of rocks contributes to the effectiveness of energy dissipation. Riprap channels may be considered where right-of-way is constrained and/or erosion occurs.

The roughness coefficient,  $n$ , of a riprap channel is correlated to the intermediate riprap rock size,  $D_{50}$ , in feet.

$$n = 0.0393 D_{50}^{1/6}$$

Where:

$n$  = Roughness, (non-dimensional)

The criteria for sizing of  $D_{50}$  and thickness of riprap lining shall be as follows:

- $D_{50}$  (maximum) = 24 inches
  - Thickness of riprap lining =  $2 \times D_{50}$
  - Thickness of riprap lining shall be 50 percent more for water deeper than 3 feet.
- Concrete lined channel** – Concrete lining shall be designed to withstand various forces due to high gradient. Criteria for design of concrete lined channels include the following:
- Thickness of concrete lining = 6 inches for  $V < 30$  feet per second
  - Thickness of concrete lining = 7 inches for  $V > 30$  feet per second
  - Channel section shall be adjusted for superelevation changes in water surface.
  - Side slopes shall be a maximum of 2:1 (H:V), or a structurally reinforced wall if steeper.
  - A concrete cutoff wall shall be provided at both the upstream and downstream termini.
- Composite channel** – Composite channels shall be considered where the open channel shall have the hydraulic capacity to handle a wide spectrum of storm events. Due to the maintenance level involved, composite channels are not permitted within the Authority's right-of-way but may be designed outside the Authority's right-of-way where floodplain mitigation and/or pollution prevention/mitigation measures are required. The channel cross section shall be designed to have 2 sections: the lower section or the main channel, usually shallow, narrow and has a hard bottom, with a side slope ranging from 0.5:1 (H:V), to 2:1 (H:V) and an overbank section usually wide, flat and grass lined, with mild slopes of 10:1 (H:V).
- Composite channels may be designed for more specific purposes such as low flow channels, trickle channels and wetland channels.

#### 8.5.2.2 Overside Drains

Overside drains are used to prevent erosion of embankments and other steep-sloped surfaces by collecting surface runoff and conveying it to a stable or less erosive drainage facility. Water conveyance at cut slopes or fill slopes are generally erosive and are more likely to need an overside drain. The spacing of overside drains shall consider the quantity of flow and capacity limitation of the gutter or ditch and the ground configuration.

For cut slopes, the angle of the slope shall be considered for overside drain slopes. The overside drain shall be sized to convey a larger storm, so that the drain is not washed out following a major storm event. Rock riprap shall be considered to minimize the velocity, but may need a wire mesh to prevent rock slippage.

Fill slopes may require the use of overside drains. For roadways, curb openings will generally allow flow to discharge down an embankment into an open channel. Ditch openings or small spillways may be used to alleviate water from trackside drainage ditches. Riprap material and an aesthetic design shall be considered.

1 Water from overside drains shall not be diverted to watersheds that originally did not contain  
2 the water, or negatively affect downstream properties.

3 Oversedrain drains may be designed as pipe downdrains, flume downdrains and spillways as  
4 follows:

5 • **Pipe downdrains** – Usually made of plastic or metal material, pipe downdrains are  
6 recommended for slopes of 4:1 (H:V) or steeper and a minimum diameter of 8 inches. When  
7 overside drains are designed in areas where sediment debris is likely and the drain is  
8 expected to be longer than 50 feet, a larger pipe size shall be considered to minimize  
9 clogging in the pipe. Oversedrain pipes shall be buried along the corridor, or designed to  
10 blend with the existing natural landscape.

11 • **Flume downdrains** – Generally rectangular in shape, flume downdrains are open channel  
12 chutes that can discharge water at steeper grades, 2:1 (H:V) or steeper. The flume invert  
13 shall be below surface grade so that the flume is even with the surface slope. Sharp bends in  
14 the flume are not permitted.

15 • **Spillways** – Asphalt concrete is typically used to create a spillway on slopes flatter than 4:1  
16 (H:V). Spillways are most commonly V-shaped and shall be placed on compacted soil to  
17 prevent further erosion. Sharp turns are not permitted to prevent splash over of discharge.

18 Grate inlets may be used in cases where a depression is not feasible. Outlet velocities shall be  
19 mitigated by use of energy dissipators.

### 8.5.2.3 Underdrain System

20 Underdrains shall be located in areas where it is anticipated that groundwater may interfere  
21 with the stability of tracks, roadbeds and side slopes. The underdrain system helps to draw the  
22 water table down, preventing softening of sub grade soils, sloughing, or instability of slopes.  
23 The utilization of underdrain pipes shall consider subsurface conditions and geotechnical  
24 studies focusing on infiltration and percolation recommendation. Underdrain pipes shall be  
25 bedded in clean, granular or crushed aggregate material enclosed in an envelope of non-woven  
26 geotextile fabric. For criteria not included in this section, reference shall be made to Caltrans  
27 HDM design criteria for underdrain systems within roadways or highways.

#### A. Pipe Size

28 Underdrain pipe shall be a minimum of 6 inches in diameter for segment length less than 500  
29 feet and a minimum of 8 inches for segment lengths over 500 feet. A minimum 6-inch PVC pipe  
30 shall be used to carry the water from underdrain system to an onsite drainage system or to the  
31 municipal stormwater system.

#### B. Location

32 Underdrains shall be used in the following locations:



- Under ballast in ballasted trackway which does not naturally drain towards the outside of the trackway, to intercept ground water and trackbed surface drainage infiltration through the ballast
- Along the toe of a cut slope to intercept seepage
- Along the toe of a fill on the side from which groundwater emanates
- Across the track or roadway at the downhill end of a cut
- Along the periphery of any paved area under which groundwater is likely to collect
- In retained cuts and on retained embankments
- Under the track slab at station platforms
- Between tracks at locations of outside station platforms and or where several sets of tracks are adjacent
- At low points in the profile, and 100 feet each side of a low point

C. Pipe Material

Underdrain pipes shall be perforated type and installed with perforations pointing down towards the bottom of the trench. Underdrain pipe shall be made of porous concrete, steel, aluminum, corrugated metal, rigid plastic, or polyethylene. For track drainage at stations and at-grade sections, perforated PVC or high-density polyethylene of schedule 80 shall be used.

D. Access Holes/Cleanouts and Risers

Access holes/cleanouts for underdrains shall have convenient access for maintenance crews and equipment. Cleanouts for underdrain systems shall be spaced at a maximum of 300 feet. Pipe materials for cleanouts to be used within trackway shall be plastic with metal risers. Risers shall be provided at beginning of underdrain runs and at a maximum of 300-foot intervals. For track drain/underdrain cleanout and riser details, refer to Standard and Directive Drawings.

E. Cover

Underdrain pipes for track drainage system shall have a minimum cover of 36 inches from the top of finished grade.

F. Depth and Spacing

Underdrain depth and spacing shall consider the permeability of the soil, the elevation of the water table and the amount of drawdown and time needed to ensure stability. Depth and spacing of underdrains shall be as follows:

**Table 8-4: Suggested Depth and Spacing of Pipe Underdrains for Various Soil Types**

Soil Class	Soil Composition			Drain Spacing (feet)			
	Percentage Sand	Percentage Silt	Percentage Clay	3 feet Deep	4 feet Deep	5 feet Deep	6 feet Deep
Clean Sand	80-100	0-20	0-20	110-150	150-200	-	-
Sandy Loam	50-80	0-50	0-20	50-100	100-150	-	-
Loam	30-50	30-50	0-20	30-60	40-80	50-100	60-120
Clay Loam	20-50	20-50	20-30	20-40	25-50	30-60	40-80
Sandy Clay	50-70	0-20	30-50	15-30	20-40	25-50	30-60
Silty Clay <sup>(1)</sup>	0-20	50-70	30-50	10-25	15-30	20-40	25-50
Clay <sup>(1)</sup>	0-50	0-50	30-100	15 (max)	20 (max)	25 (max)	40 (max)

Source: Caltrans HDM, Table 842.4.

**Notes:**

<sup>(1)</sup> Drainage blankets or stabilization trenches shall be considered.

**G. Slope**

Underdrain grades shall be not less than 0.5 percent. If 0.5 percent is not feasible, a slope that would provide a minimum velocity of 2 feet per second shall be provided in the full pipe condition.

**H. Separation of Underdrain Systems**

Design of underdrain systems shall consider separation of drainage flows:

- Pipes carrying surface water shall not discharge into underdrains. Surface water shall also be prevented from seeping into underdrains other than those provided to collect trackbed surface drainage.
- Where underdrains are located under trackbeds or paved areas, other than sidewalks, the filter material and filter fabric shall extend up to the top of prepared subgrade.
- Where underdrains other than those provided to collect trackbed surface drainage are located under unpaved areas or sidewalks, the filter material shall extend up to 6 inches below finished grade and the filter material shall be covered with impervious backfill material.

**8.5.2.4 Energy Dissipators**

Where the anticipated outlet velocity for a waterway exceeds the maximum permissible velocity for the bed material of the receiving channel, an acceptable means of energy dissipation shall be used to reduce the velocity to safe limits. Commonly used energy dissipators include natural scour holes, drop structures, internal dissipators, external dissipators, and stilling basins. These facilities decrease the chance of a hydraulic jump as well as erosion/scour.

This section presents the minimal criteria and design standards for the hydraulic evaluation and design of energy dissipators. For criteria not included in this section, the following references shall be used:

- Refer to Caltrans HDM for design criteria of energy dissipators on highways and freeways.
- Local criteria shall be followed as required by the governing agency.

To permit debris to be carried with the flow, dissipators employing obstructions shall be avoided unless it can be demonstrated that such obstructions will not collect debris.

#### A. Natural Scour Holes

This option consists of providing an area in which flows through the culvert will be allowed to form a natural scour hole. Scour holes shall be lined over an area sufficient to cover the potential scour hole with the class of riprap appropriate for the culvert exit velocity.

The following shall be considered in the design of natural scour holes:

- Undermining of the culvert outlet will not occur or it is impracticable to be checked by a cutoff wall.
- The expected scour hole will not cause costly property damage.
- Right-of-way or drainage easements at the site are sufficient to encompass the entire scour hole which may often be quite large.
- Environmental concerns due to sedimentation will not be a factor.
- There are no aesthetic concerns or other nuisance effects, such as insect breeding.

#### B. Drop Structures

**Inclined or sloping drop structures** – Where the difference in elevation from the upper channel bottom to the lower channel bottom is 10 feet or lower, these drop structures shall be used. The top of the crest wall shall be placed at a height above the upstream channel bottom. A downstream apron shall be provided to transition from the drop structure to the downstream channel.

**Vertical drop structures** – A vertical drop structure is designed to force the hydraulic jump to occur within a stilling basin next to a rectangular weir. When the flow line of the channel is too steep for the design condition, erosion and scour may occur to the channel bottom or the toe of the embankment. To mitigate sediment aggradation and degradation, a drop structure shall be considered. The drop structure shall have sufficient length and water cushions to prevent scouring of the downstream channel bed due to a nappe or hydraulic jump.

**Material** – The material used to construct the drop structure depends on the availability of materials, the height of drop required, and the width of the channel. Rock riprap and timber pile construction have been successful on channels having small drops and widths less than

100 feet. Sheet piles, gabions, and concrete structures are generally used for larger drops on channels with widths ranging up to 300 feet.

**C. Internal Dissipators**

Containing the hydraulic jump within the culvert is a form of internal energy dissipation. Internal dissipators shall be used where the scour hole at the culvert outlet is unacceptable, the right-of way is limited, debris is not a problem and moderate velocity reduction is needed. The 3 types of internal dissipators are tumbling flow, increased resistance, and broken back culverts.

**Tumbling flow (roughness elements)** – Tumbling flow may be applicable where culvert slopes are between 10 percent and 15 percent. Tumbling flow in culverts shall be the following:

- Use 5 rows of uniformly sized roughness elements in box culverts or open chutes
- Spacing (L) between the roughness element rows is set by choosing a ratio of L/h to be between 8.5 and 10, where h is the height of the element.

**Increased resistance** – Increasing resistance may cause a culvert to change from partial flow to full flow in the roughened zone. Velocity reduction is accomplished by increasing the wetted surfaces as well as by increasing drag and turbulence by the use of roughness elements. Increased resistance shall be used where a culvert is flowing partially full with inlet control and the requirement for outlet velocities is between critical and normal. The following criteria shall be considered for increased resistance design on culverts:

- Where slopes of culverts are less than 4 percent
- Five rows of roughness elements may be used.
- Where the height of element is 5 percent to 10 percent of the diameter

**Broken-back culverts** – Substituting a "broken-slope" flow line for a steep, continuous slope in a culvert may be used for controlling outlet velocity. The steep slope of the culvert is replaced by breaking the slope into a steeper portion near the inlet followed by a horizontal runout section. Broken-back culverts, at minimum, shall have the following:

- There shall be sufficient tailwater and sufficient friction and length in the runout section of the culvert
- Steep sections for which slope shall be less than or equal to 1:1.4 (H:V)
- Hydraulic jump may be completed within the culvert barrel

In situations where the runout section is too short and/or there is insufficient tailwater for a jump to be completed (or initiated) within the barrel, an outlet weir or a drop of the outlet followed by an outlet weir shall be designed. Sills are effective in forcing the hydraulic jump in broken-back culverts and in spreading the water back to the natural stream width.

#### D. External Dissipators

External dissipators shall be designed where the outlet scour hole is not acceptable, a moderate amount of debris is present, and the culvert outlet velocity is moderate,  $Fr < 3$ . Various types of external dissipators are discussed in the following sections.

**Impact basin U.S. Bureau of Reclamation Type VI** – The USBR Type VI basin was developed by the USBR and is contained in a relatively small box-like structure, with a vertical baffle. An opening is provided between the bottom of the baffle and the floor of the box.

The use of impact basin USBR Type VI is not recommended where debris or ice buildup may cause substantial clogging. The design of impact basin USBR Type VI shall achieve the guidelines presented below:

- Valid for discharges up to 400 cubic feet per second and velocities as high as 50 feet per second.
- In situations where the culvert entering the basin has a slope greater than 27 percent, the basin shall be constructed on a horizontal grade.
- The culvert shall provide a horizontal section at least 4 culvert widths in length immediately upstream of the dissipator.
- The end of the basin shall be provided with a low sill that, where feasible, shall be set at the same elevation as the downstream channel.
- Where needed to retain the roadway embankment, the end of the basin may be provided with an alternate end sill and 45 degree wingwalls.
- Where the velocities of flows exiting the basin exceed 5 feet per second, the channel downstream of the basin shall be provided with a riprap apron, per the guidelines presented in the section on 'Riprap Aprons'.
- A moderate depth of tailwater will improve its performance. However, the tailwater depth shall not be above half of the height of the baffle.

**Hook type impact basin energy dissipator** – The hook energy dissipator is a type of impact basin that abates culvert outflow velocities by means of 3 hook-shaped blocks and an end sill in a uniform trapezoidal channel or a warped wingwall basin. The minimum criteria for hook type basins are presented below:

- Hook type Basin with Uniform Trapezoidal Channel
  - The side slopes of the basin shall be between 1.5:1 (H:V) and 2:1 (H:V), and the bottom width of the basin shall be 1 to 2 times the effective opening width of the culvert.
  - Where scour may occur, a riprap apron shall be provided downstream of the basin, per the guidelines presented in the section on 'Riprap Aprons'.
  - A cutoff wall shall be provided at the end of the basin.

- These basins may be used where the Froude number of the culvert outflow is between 1.8 and 3.0.
- Hook type basin with warped wingwalls
  - Wingwalls warped from vertical at the culvert outlet to side slopes of 1.5:1 (H:V) at the end sill are recommended.
  - The recommended ratio of hook width/culvert width is 0.16.
  - The spacing between hooks shall be within the range of 1.5 to 2.5 times the hook width.
  - The height of wingwalls shall be at least twice the flow depth at the culvert exit.
  - A flare angle of 5.7 degrees per side is the optimum value for  $Fr > 2.45$ .
  - Where scour may occur, a riprap apron shall be provided downstream of the basin, per the guidelines presented in the section on 'Riprap Aprons'.

**Riprap aprons** – Riprap aprons for culverts shall be designed in accordance with outlet protection criteria mentioned in Section 8.5.3.4 of this chapter.

Riprap aprons for energy dissipators shall be designed per the equation:

$$D_{50} = \frac{0.692}{(S-1)} \left( \frac{V^2}{2g} \right)$$

Where:

$D_{50}$  = median rock size (feet)

$S$  = rock specific gravity (pound per cubic foot)

$V$  = velocity at the end of energy dissipator (feet per second)

#### E. Stilling Basins

Stilling basins shall be used where the outlet scour hole is not acceptable, debris is present, and the culvert outlet velocity ( $V_o$ ) is high,  $Fr > 3$ .

**Riprap basin** – The riprap basin shall be considered where the standard riprap apron or other energy dissipators are inadequate. A riprap basin is a depressed area of riprap placed at the outlet of a high velocity culvert, storm drain outlet, or open channel. Recommended minimum criteria for riprap basin are as follows:

- The basin shall be pre-shaped and lined with riprap that is at least  $2 \times D_{50}$  thick.
- The ratio of depth of scour ( $ds$ ) to rock size ( $D_{50}$ ) shall be greater than 2.
- The length of the energy dissipating pool shall be  $10 \times (ds)$ , and the length of the apron  $5 \times (ds)$ .

- A riprap cutoff wall or sloping apron shall be constructed if downstream channel degradation is anticipated.

**Saint Anthony Falls stilling basin** – The Saint Anthony Falls stilling basin uses a forced hydraulic jump to dissipate energy. The design consists of a sloping chute with chute blocks at its base, followed by blocks on the floor of the basin. The basin floor also has a sill located at the downstream end. The basin sidewalls may be parallel for a rectangular stilling basin or may diverge, beginning at the downstream toe of the chute to create a flared stilling basin. A cut-off wall and wingwalls shall be provided at the end of the stilling basin. Minimum design criteria for a Saint Anthony Falls stilling basin is provided below:

- Recommended where  $Fr = 1.7$  to 17
- Requires a sufficient tailwater for efficient operation
- Sidewall flare shall not be greater than 0.5:1, 0.33:1, or flatter
- Height of the baffles is set equal to the entering flow depth
- Wingwalls shall be equal in height and length to the stilling basin sidewalls. Top of the wingwall shall have a 1:1 (H:V) slope.

#### 8.5.2.5 Siphons

Inverted siphons (sometimes called sag culverts or sag lines), although not desirable, may be used to convey water by gravity under roads, railroads, other structures, various types of drainage channels and depressions. An inverted siphon structure shall operate without excess head when flowing at design capacity and shall not be used for drainage or irrigation where freezing may block the siphon's waterway.

Inverted siphons shall be used as follows:

- To carry flow under obstructions such as sanitary sewers, water mains, or any other structure or utility that may be in the path of the storm drain line
- Where avoidance or adjustment of the utility is not practical

This section presents the minimal criteria and design standards for the hydraulic evaluation and design of inverted siphons. For criteria not included in this section, the following reference(s) shall be used:

- Caltrans HDM design criteria shall be followed for design of siphons or sag culverts within roadways or highways.

##### A. Pipe Material and Size

Several pipe materials may be used for siphon construction:

- Welded smooth steel pipe with internal ceramic coating

- Precast reinforced concrete pressure pipe
- Reinforced plastic mortar pressure pipe

The conduit size through the inverted siphon used as a storm drain system shall be the same size as either the approaching or exiting conduit. In no case shall the conduit size be smaller than the smallest of the approaching or exiting conduit.

**B. Transitions, Head Losses, Cover and Slope**

Transitions are defined as the inlet and outlet portion of an inverted siphon and shall be used to reduce head losses and prevent channel erosion in unlined channels. Siphon transitions shall be located outside the Authority's right-of-way. Concrete inlet and outlet transitions shall be used for the following:

- Siphons crossing tracks and paved roadways
- 36-inch diameter and larger siphons crossing narrow (< 30 feet), unpaved, off-system roads
- Siphons in unlined channels with water velocities in excess of 3.5 feet per second in the siphon

Because an inverted siphon includes slopes of zero and adverse values, head losses through the structure due to friction, bends, junctions, and transitions shall be accounted. Sound engineering judgment shall be used to determine the maximum limits of head losses and if determined unacceptable, an alternative for the siphon design shall be considered. The total computed head loss shall be increased by 10 percent as a safety factor to ensure the siphon is not causing unexpected backwater.

The siphon profiles shall satisfy requirements of cover, siphon slopes, bend angles, and submergence of inlet and outlet.

- At siphons crossing tracks and roadways or highways, the minimum cover shall be based on the structural requirements of the siphon material.
- At siphons crossing under natural drainage channels, a minimum of 3 feet of compacted earth cover shall be provided.
- At siphons crossing under an earth channel, a minimum of 3 feet of compacted earth cover shall be provided.
- At siphons crossing under a lined channel, a minimum of 3 feet of compacted earth cover shall be provided between the bottom of the channel lining and the top of the siphon.

Siphon slopes shall not be steeper than 2:1 (H:V) and shall not be flatter than a slope of 0.005 feet/feet.



C. Velocity

The following velocity criteria are to be used in determining the length of the siphon:

- 3.5 feet per second or less for a short siphon not located under a trackway or roadway with only earth transitions provided at entrance and exit
- 2.5 feet per second or less for a short siphon located under tracks or roadways with either a concrete transition or control structure provided at both inlet and outlet
- 10 feet per second or less for a long (> 200-foot) siphon with either a concrete transition or control structure provided at the inlet and a concrete transition provided at the outlet

D. Collars and Blowoff Structures

Collars are placed at intervals along the siphon to reduce the velocity of any water moving along the outside of the siphon or through the surrounding earth, thereby preventing removal of soil particles (piping) at the point of emergence. Siphon collars shall not be used unless piping computations or observations of burrowing animals indicate they are needed.

Blowoff structures are provided at or near the low point of inverted siphons to permit draining of the siphon for inspection and maintenance or shutdown. Siphons greater than 18 inches in diameter shall be equipped with a blowoff structure. An access hole or similar access shall be included with a blowoff on long siphons 36 inches and larger in diameter to provide an access point for inspection and maintenance. To facilitate removal of any accumulated sediments and to expedite the draining process, an 8-inch minimum gate valve shall be used. The drain pipe shall outfall where drain water will not cause any damage.

E. Freeboard

Upstream channel freeboard is commonly provided to accommodate intercepted storm runoff, improper operation or drift blockage. Freeboard criteria for siphons are as follows:

- The channel bank freeboard upstream from siphons shall be increased 50 percent or 1.0 foot maximum to prevent washouts at these locations.
- The increased freeboard shall extend upstream a distance from the structure such that damage caused by overtopping the channel banks would be minimal.
- If the freeboard extends upstream from the transition inlet, a minimum distance as determined by dividing the freeboard height by the channel slope shall be used. And, for freeboard downstream from the outlet transition, a minimum distance of 50 feet or to the Authority's right-of-way, whichever is less, shall be used.

F. Wasteways

Wasteways are often placed upstream from a siphon transition to divert the channel flow in case of an emergency. A wasteway, either separate or integral with the inlet transition, shall be provided where significant damage would occur due to escaping channel waters. Escaping

1 waters shall be conveyed to a point and released in a manner to avoid trackway, roadway or  
2 property damage. Wasteways are not permitted within the Authority's right-of-way.

### G. Safety Devices

3 Safety measures (e.g., fences, grates) shall be provided near siphons to protect persons and  
4 animals from injury and loss of life. A hazard can occur both when the siphon is operational or  
5 dry. Inlet and outlet transitions shall be hydraulically efficient and have removable grates to  
6 minimize the hazard associated with human or animal ingress or debris blockage.

#### **8.5.2.6 Pump Stations**

7 The use of pump stations at sag or sump points shall be avoided, when practical. Long-term  
8 operation and maintenance costs shall be identified and considered prior to implementation of  
9 pumps. The use of a gravity system shall be fully evaluated through the use of long pipelines or  
10 adjustments to the grade or track profiles, before pumps can be considered for the project.  
11 Where a gravity system cannot be provided and pump stations are unavoidable, pump station  
12 design within the corridor shall conform to the following:

- 13 • FHWA, Hydraulic Engineering Center (HEC)-24 on Highway Stormwater Pump Station  
14 Design
- 15 • *Mechanical, Supervisory Control and Data Acquisition Subsystems*, and *Facility Power and*  
16 *Lighting Systems* chapters of this Design Criteria

17 For pump stations installed for the project, but owned, operated, and maintained by local  
18 agencies or a third party, the criteria to use shall be in accordance with the local governing  
19 agency.

20 Pump stations shall be designed to accommodate the inflow from a storm event according to  
21 Section 8.4.3. All possible flow shall bypass or pass-through downstream of the pump station, to  
22 reduce pumping requirements. Hydrological analyses of the watersheds that may discharge to  
23 the pump stations shall be carefully evaluated to avoid potential off-site drainage diversions  
24 from adjacent watersheds. The pump station design shall also address future build-out of the  
25 tributary watershed to verify that the pump station can handle increases in flow.

26 Pump stations shall be designed to accommodate space for equipment cabinets and conduit and  
27 cabling to provide for SCADA control.

#### **8.5.3 Culvert Hydraulics**

28 Existing drainage facilities within the corridor shall not be negatively impacted due to the  
29 proposed design. Where a transverse undercrossing is required to convey surface runoff, flood  
30 waters, and/or existing streams across the Authority's right-of-way, the crossing shall be  
31 provided within a culvert. When runoff is increased, existing culverts shall be upsized to allow  
32 for increase in flow.

The following sections outline minimal culvert criteria. For criteria not addressed in this section, the following references shall be used:

- AREMA shall be followed for design of culverts along the corridor.
- Caltrans HDM shall be followed for design of culverts along roadways and highways impacted by the improvements.
- Local criteria shall be followed if AREMA and Caltrans criteria do not specify.

#### 8.5.3.1 Design Elements

Numerous cross sectional shapes are available to be used as culverts. The shape selection shall be based on the cost of construction, the limitation on the upstream water surface elevation, track and roadway embankment, available cover, and hydraulic performance.

The following culvert criteria shall be applied:

- Minimum culvert diameter/rise for trunk drains and culverts crossing under the track shall be 36 inches.
- Minimum culvert diameter/rise for lateral drains shall be 18 inches.
- Culverts and drains under platforms and station areas shall be a minimum of 18 inches.
- Culverts crossing under trackway shall not be placed within the limits of prepared subgrade unless the backfill material for the culvert and its compaction meet the requirements of prepared subgrade, refer to *Geotechnical* chapter for limits of prepared subgrade, material and compaction requirements. Culverts crossing under trackway shall be a minimum of 6 feet below top of rail and 3 feet below the flow line of ditch along the trackway.
- Culverts, within 45 feet of track centerline, shall have a minimum cover of 4 feet; culverts elsewhere shall have a minimum cover of 3 feet.
- In locations where the above criteria are not practical, reduced clearance may be provided with approval of the governing agency.

The selection of a culvert material shall consider structural strength, hydraulic performance and roughness, durability, corrosion and abrasion resistance. Common culvert materials are concrete, corrugated aluminum and corrugated steel. Culverts and storm drains passing beneath tracks or maintenance roadways shall be reinforced concrete pipe (RCP). Culverts and drains under platforms or in station areas that are not under tracks shall be RCP, polyvinyl chloride (PVC), high-density polyethylene (HDPE), or corrugated steel.

#### 8.5.3.2 Location, Skew, and Slope

Culverts shall be placed to allow for cross-passage of surface runoff, flood waters, and where existing streams may exist. This may reduce embankment erosion, minimize debris buildup and, if placed often enough, limit carryover of drainage from one watershed to another.

1 Culverts shall be in alignment and on the same gradient with existing streambeds. Curvatures  
2 in the alignment of the culvert and angle points shall be avoided.

3 The slope of the culvert shall be the same gradient of the existing streambed unless the  
4 topography is generally flat, in which case the invert of the inlet and outlet structures shall be  
5 designed to avoid sedimentation in the culvert. A minimum self cleaning velocity of 2.5 feet per  
6 second shall be used for culvert design.

### **8.5.3.3 Inlet Structure**

7 For inlet control, a maximum allowable headwater of 1.5 times the culvert diameter/rise shall be  
8 used. For the 100-year storm event, a minimum freeboard between the water surface elevation  
9 and the subballast shall be 2 feet.

10 Upstream properties shall be protected from ponding and backwater effects from an undersized  
11 culvert. Ponding at the inlet structure shall be prevented. A larger culvert size shall be  
12 implemented if ponding and discharge backup are anticipated at the culvert entrance.  
13 Overtopping of the tracks is not permitted.

14 Sound engineering judgment shall be applied to inlet structure design.

#### **A. Headwalls**

15 Headwalls shall be used in locations where right-of-way is constrained, in areas where  
16 vegetation growth on slopes is limited, to improve the culverts appearance and increase the  
17 hydraulic efficiency. The headwall shall be designed to have adequate strength and proportion  
18 to prevent sliding or overturning from excessive soil pressures and to prevent settlement.

#### **B. Wingwalls**

19 Wingwalls shall be considered to prevent erosion and scour and provide soil stability around  
20 the proposed culvert. Perpendicular, oblique, or parallel wingwalls, or a combination thereof,  
21 shall be used, depending on the physical and hydraulic conditions involved. Wingwalls shall be  
22 carefully designed not to alter the historical flow patterns of the existing stream and prevent  
23 turbulence during peak storm events. An entrance apron and cutoff wall shall also be  
24 considered to prevent erosion and scour and increase hydraulic efficiency at the inlet.

#### **C. Flared End Sections**

25 Flared end sections improve hydraulic performance of the culvert by allowing a smooth  
26 transition between the natural channel and culvert and are aesthetically appealing. Due to these  
27 reasons, flared end sections are preferred to headwalls and wingwalls within the corridor.

### **8.5.3.4 Outlet Structure**

28 Due to the unnatural constriction and material of culverts, the outlet velocity is generally higher  
29 than natural stream velocities. Energy dissipators or outlet embankment protection, such as  
30 slope paving, riprap, headwalls and wingwalls, end sections, cutoff walls and toe walls, shall be  
31 provided at culverts along the corridor to minimize downstream erosion and reduce drainage

- 1 velocities. Natural flow patterns of the existing stream shall be restored downstream of the
- 2 culvert, with special consideration of the channel transition for the 100-year flood.
- 3 Refer to Section 8.5.2.4 for design of energy dissipators.

## 8.6 Trackway and Facility Drainage Systems

### 8.6.1 Track Drainage Systems

- 4 Standing water along rail tracks may shunt the signal circuits causing signal failures. Hence,
- 5 standing water along rail tracks is not permitted and shall drain away from the tracks. Track
- 6 drainage shall be provided to drain stormwater from tracks including cable troughs and right-
- 7 of-way. Drainage criteria for cable troughs in at-grade sections are provided in the *Civil* chapter.
- 8 A minimum 4-foot flat bottom ditch shall be used for drainage along the trackway toe of
- 9 embankments or bottom of cut sections. Refer to Standard and Directive Drawings. Where a
- 10 minimum 4-foot flat bottom ditch cannot be accommodated due to constraints, a V-ditch may
- 11 be used. Underdrain track drainage pipe systems shall be used where right-of-way constraints
- 12 make the standard flat bottom ditch or V-ditch unfeasible. Track drainage systems shall be used
- 13 as a longitudinal drainage system to capture the onsite runoff generated from the trackway. The
- 14 collected runoff may be conveyed to a local storm drain system, based on the hydraulic capacity
- 15 or, conveyed to the nearest natural water body, after applying appropriate treatment measures
- 16 as defined in the BMP section of this chapter. Refer to Standard and Directive Drawings for
- 17 detailed track drainage configuration. Pipe size, material, slope, and access hole requirements
- 18 for a closed track drainage system shall be consistent with the criteria in Section 8.5.2.3.
- 19 In a shared corridor, drainage shall be kept separate from the drainage system of the third party
- 20 operator. If any upgrades to the third party's drainage system are anticipated, they shall be
- 21 performed based on the third party's design requirements.
- 22 For criteria not included in this section, references shall be used as follows:
- 23
  - AREMA shall be followed for design of new track drainage facilities.

### 8.6.2 Embankments and Cut Slopes

- 24 For embankments that support the HST trackway and for cut slopes, refer to the *Geotechnical*
- 25 chapter for drainage and slope protection requirements.

### 8.6.3 Bridges/Aerial Structures

- 26 Design of HST bridges and aerial structures over waterways and associated drainage facilities
- 27 shall be coordinated with local agencies or the U.S. Army Corps of Engineers. The 2 basic
- 28 designs involved in this section are the HST aerial structures and HST structures over
- 29 waterways (HST bridges).

Single track HST bridge and aerial structure decks shall have a 2 percent cross slope towards the cable trough. Double track HST bridge and aerial structure decks shall have an inverted crown with 2 percent cross slope. Sufficient number of drain inlets shall be provided on the structure deck to drain the entire deck section without passage of runoff over structure expansion joints. Design of track structure shall allow passage of water towards the deck drain inlet is located. Drain inlets shall be 8 inches in diameter. In non-ballasted structures, drain inlets shall have a slotted brass cover securely attached to the inlet with screws. In ballasted structures, an 8-inch diameter half-dome perforated corrugated galvanized pipe shall be placed on the flow line leading to the drain inlet.

Drain inlets shall be connected to 6-inch diameter minimum drain pipe which run inside the box girder of the structure. The drain pipes shall have a minimum 1 percent slope. Drain pipes crossing the expansion joint shall be designed to allow multi directional movement of the pipe during a seismic event. The range of movement shall accommodate the maximum movement of the expansion joint.

Drainage of track sections on embankment or retained fill leading to a bridge or aerial structure shall be captured outside of the structure abutment.

Discharge of deck drainage through columns is permissible provided it meets the receiving body's regulatory requirements. If discharge pipes are located within columns, cleanouts shall be provided outside the column plastic hinge limit as specified in the *Structure Chapter*. Drain pipes crossing the gap between the box girder and the column shall have flexible connections to allow multi directional movement of the pipe during a seismic event.

This section presents the minimal criteria and design standards for the hydraulic evaluation and design associated with bridge drainage within the corridor. For criteria not included in this section, the following references shall be used:

- AREMA's "Roadway and Ballast" chapter for detailed information on the magnitude and level of scour created at piers and abutments and countermeasures
- HDS-01, Hydraulics of Bridge Waterways, FHWA for bridge hydraulic analyses
- HEC-21, Design of Bridge Deck Drainage, FHWA for bridge deck drainage design
- HEC-09, Debris Control Structures Evaluations and Countermeasures, FHWA for mitigating debris impacts to bridge structures
- Refer to local agency manuals for local criteria within each jurisdiction.
- The *Structures* chapter shall be referenced to ensure coordination with structural design of bridges and aerial structures.

#### 8.6.3.1 Freeboard Protection

For the hydraulic design of bridges and aerial structures, a minimum of 2 feet of freeboard above the design frequency water surface elevation shall be provided.

### 8.6.3.2 Erosion Control and Scour Protection

Protection measures shall be taken to protect bridges or aerial structures, piers, and embankments from erosion and scour. Measures include providing riprap and/or vegetation on the slopes and streambed and increasing the distance between embankments. Where applicable, energy dissipators may be provided in accordance with criteria provided in Section 8.5.2.4.

### 8.6.3.3 Pier Design and Location

For structures over waterways, the spacing and location of the structural piers can significantly affect the hydraulic characteristics of the existing waterways. In locations where pier columns and protection walls interfere with drainage, an alternative drainage facility shall be provided to collect and carry water to a drainage system.

Piers shall be located outside of drainage channels and natural washes, where possible, to minimize negative impacts associated with scour and erosion at the pier. Where piers are located within channels, a streamlined design at the pier nose shall be considered. This shall be obtained by providing circular or rounded shapes at the upstream and downstream faces of piers in order to reduce flow separation, aligning bents with the direction of flow and increasing the length of the bridge to decrease velocities.

Debris buildup may occur at piers which can reduce the hydraulic capacity of the channel, increase the local scour, and potentially cause the pier to fail. The design shall consider the type of debris that could impact the pier. Depending on the debris type, protective devices such as steel plates, debris deflectors, wingwalls, and upstream debris catchment structures shall be used.

### 8.6.3.4 Deck Drainage System

Stormwater on a bridge or aerial structure surface can affect the water spread on the structure into the trackway, cause complications with maintenance, and negatively impact the aesthetics of the structure, by corrosion or debris. The deck drainage system includes the bridge or aerial structure deck, gutters, inlets, pipes, downspouts and end collectors, which are discussed in the following subsections.

#### A. Bridge/Aerial Structure Deck

A longitudinal drainage system shall be provided along the deck to minimize standing water on the bridge or aerial structure. Criteria for bridge or aerial structure deck design are as follows:

- For bridges and aerial structures with ballasted track, the minimum size of a half circle or U-shaped perforated corrugated galvanized drain channel shall be 8 inches. The drain channel shall be installed on top of the bridge deck waterproofing membrane over the invert of the deck, refer to Standard and Directive Drawings.
- For bridges and aerial structures with non-ballasted track, a drainage trough shall be designed to convey the deck drainage.
- The cross slope of the bridge/aerial structure deck shall be 2 percent.

- Standing water on the bridge or aerial structure shall not be permitted.

#### **B. Inlets and End Collectors**

The bridge and aerial structure end collectors are drainage inlets that collect flow before it reaches the structure and prevent flow from leaving the bridge or aerial structure. End collectors are typically drop inlets which convey a higher capacity; slotted drains may be used. Stormwater upstream of a bridge or aerial structure shall be fully collected prior to reaching the structure. To avoid flooding on the bridge or aerial structure and backup in the pipes, inlets and drains on the bridge or aerial structure shall account for a 50 percent clogging factor. For ballasted bridge/aerial structure decks, the half dome drain channel shall be terminated at inlets before the bridge deck expansion joint, refer to Standard and Directive Drawings. Inlets shall be provided at intervals to collect the flow into the storm drainage system.

#### **C. Pipes and Downspouts**

The minimum longitudinal slope of drain pipes inside the box girder shall be 1 percent or generate a minimum velocity of 2 feet per second. Downspouts shall be considered in the aesthetics of the bridge or aerial structure. Pipes and downspouts located within the concrete of the structure provide more challenges for access and maintenance. Cleanouts shall be provided at convenient and accessible locations along the pipe. Cleanouts shall be located such that these can be reached from the ground for easy access for personnel, and at places where the pipes bend and debris build-up may occur. Cleanout locations shall be identified.

Outfalls from downspouts may discharge directly into storm drains, or nearby receiving water, considering the water is treated before discharging offsite. If the downspout discharge is directly to surface drainage, the free-falling water shall not come into contact with the structure members to avoid corrosion and deterioration. Stormwater from the bridge or aerial structure shall also not negatively impact the surface below; erosion control devices may be necessary at the outfall location and the surface channel shall be designed to carry and transfer the increase in flow. Downspouts that discharge directly to a storm drain shall connect to a manhole for easy access. The outfall invert shall be a minimum of 0.25 feet higher than the manhole invert to avoid debris clogging the storm drain.

Refer to Standard and Directive Drawings for drainage of aerial structures.

### **8.6.4 Tunnels**

For drainage requirements in tunnels, refer to Standard and Directive Drawings. Drainage from tunnel and cut-and-cover structures shall discharge to portals or to a low-point sump in pump station.

### **8.6.5 Retaining Walls**

Provide a concrete lined gutter behind retaining walls to redirect storm runoff away from the walls, refer to Standard and Directive Drawings. For other drainage requirements at retaining walls, refer to the *Geotechnical* chapter.



### 8.6.6 Trenches

Where the ground water table is high and interferes with the stability of track bed and side slopes, the trackway shall be in a trench section. Trench drainage discharges to a low-point sump in pump station and is discharged to local drainage system. Refer to the *Structures* chapter for trench wall heights for flood protection. For drainage concept in trench sections, refer to Standard and Directive Drawings.

### 8.6.7 Critical HST Structures/Facilities

HST critical facility sites, such as traction electrification system, automatic train control, communications, vent structures, traction power supply sites, operation control centers, yards, etc., shall be designed to drain so that the finish floor elevation or top of slab foundation of the facility sites remain 6 inches above a 500-year flood elevation or 2 feet above 100-year flood elevation, whichever is greater.

### 8.6.8 Stations and Platforms

Refer to the *Stations* chapter for station area and station platform drainage requirements. Station area drainage should discharge to the nearest municipal drainage collection system after proper treatment as necessary. Refer to Standard and Directive Drawings for station trackway drainage system within the station area.

Station drainage shall accommodate deluge system for depressed and underground stations.

### 8.6.9 Roadways

The following sections outline minimal drainage criteria for roadways, street improvements, parking lots and storm drains. For criteria not addressed in this section, the following references shall be used:

- Caltrans HDM criteria shall be followed for design of roadway drainage, including subsurface drainage facilities within the corridor.
- For third party roadway drainage requirements, refer to third-party's drainage criteria.

#### 8.6.9.1 Surface Drainage

Physical characteristics of surface drainage for roadways leading to stations, yards, and other wayside facilities shall accommodate the following:

- Cross flow shall not be more than 0.1 cubic feet per second.
- Ponding shall be prevented in parking lots where vehicle stalls are located.
- Sound engineering judgment shall be used with careful consideration to train, vehicular and pedestrian traffic.

### 8.6.9.2 Storm Drain Design

Storm drains shall be designed in coordination with roadway and trackway surface flow. Inlets, subsurface piping, and maintenance access shall be considered when designing storm drains. Refer to the *Utilities* chapter for clearance requirements of storm drain facilities with adjacent infrastructure facilities.

### 8.6.9.3 Inlets and Maintenance Access

Inlet types may include curb-opening inlets, grate inlets, slotted drain inlets and a combination of all these types. Inlet type shall consider the proposed location of the inlet and the required capture capacity the inlet must convey. The location and spacing of inlets shall be designed to prevent ponding and flooding in traveled lanes. Inlets are generally placed upstream of pedestrian ramps and street intersections. Grate inlets are not recommended for pedestrian pathways. Inlets shall be placed prior to a superelevation reversal to prevent cross flow on the roadway.

Inlet capacity depends on the size and shape of the opening, grate type and the roadway geometry upstream of the inlet. To mitigate back-up and ponding of water, a minimum clogging factor of 50 percent shall be used for all inlet designs.

Design of manholes shall conform to the following maintenance hole spacing requirements:

**Table 8-5: Maximum Manhole Spacing Requirements**

Pipe Diameter	Maximum Spacing Requirements
Any pipe that cannot reach a self-cleaning velocity of 2.5 feet/sec.	300 feet
Less than 24-inches	400 feet
24 inches to 48 inches	400 feet to 800 feet
Greater than 48 inches	1,200 feet

Engineering judgment shall be used in determining spacing requirements. Access logistics for pipe maintenance shall be considered. Manholes and associated storm drains shall be located out of the roadway traveled way and intersections to avoid disruptions to traffic. If the subsurface drain is proposed to cross the tracks, manholes shall be considered for use at the Authority's right-of-way limits.

### 8.6.9.4 Pipe Characteristics

Subsurface pipes shall be designed at full flow capacity. To determine the appropriate pipe size, shape and material, Manning's equation shall be applied as described in Section 8.5.2. Storm drains may operate under pressure, provided the pipe material will not be jeopardized due to pressurization; however, the hydraulic grade line shall not rise above the manhole or inlet structure, and shall be a minimum of 1 foot below the surface finished grade.

Generally in urban areas, storm drain pipes are RCP material to provide a longer life of the system and minimize maintenance over the life of the pipe. RCP pipes shall be designed for facilities crossing under the tracks. Other pipe materials, including corrugated steel, HDPE and PVC may be considered along the corridor and at train stations, based on a life cycle cost analysis to justify the use of pipe type. For corrosion protection of metallic and RCP pipes refer to the *Corrosion Control* chapter. Criteria for pipe size and cover are as follows:

- Minimum pipe size within roadways shall be 18 inches
- A 3-foot minimum cover shall be provided for pipes

## 8.7 Detention/Retention of Surface Water Runoff

The main purpose of a detention basin is to temporarily store runoff volume to reduce peak discharge by allowing flow to be discharged at a controlled rate. This section provides minimal design criteria for hydraulic evaluation and design of detention basins, for the purpose of flood control and stormwater management.

Caltrans identifies detention basins as a design pollution prevention BMP, which temporarily detains runoff to allow sediment and pollutant to settle. If the detention basins are designed to include the purpose of a BMP, reference shall be made to Section 8.8. Proper treatment of polluted runoff shall be included in the design of the detention/retention basins. Retention storage can be defined as a depression or low point where water accumulates with no possibility for escape as runoff. These facilities allow for infiltration into underlying soils, and are considered as “Infiltration Devices.”

Several factors such as the peak outflows, spillway sizing, and sedimentation govern the design of detention facilities. For design criteria and detailed design methodologies of detention basins not included in this section, refer to HEC-22, Urban Drainage Design Manual, FHWA.

### 8.7.1 Peak Flow Reduction and Release Rate

The facility's outlet structure shall limit the maximum outflow to allowable release rates. The maximum release rate may be a function of existing or developed runoff rates, downstream channel capacity, potential flooding conditions, and/or local agency regulations.

The system shall be designed to release excess stormwater expeditiously to ensure that the entire storage volume is available for subsequent storms. The facilities may need a paved low flow channel to ensure complete removal of water and to aid in nuisance control.

### 8.7.2 Required Storage Estimate

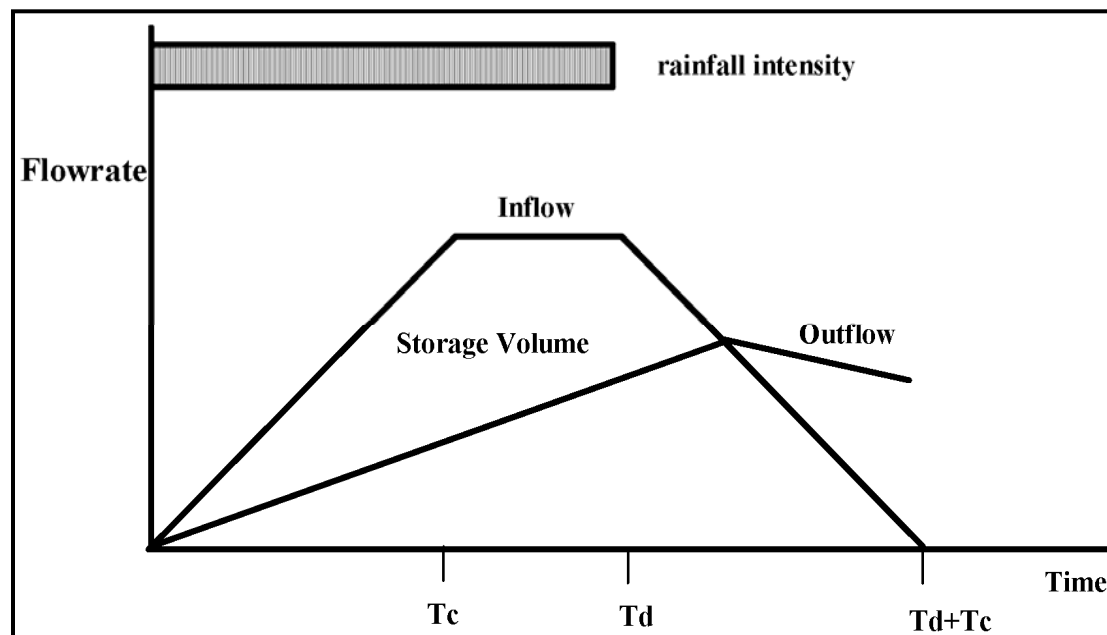
For watersheds greater than 150 acres, the required volume of storage for a detention basin that is necessary for peak flow reduction can be estimated as the difference between the inflow and the outflow hydrographs, using the Hydrograph Method.

The Volumetric Method shall be used to estimate the runoff detention/retention volume for watersheds less than 150 acres for the urban catchment. In this case, uniform rainfall is assumed and the required storage volume can be estimated by the volume difference between the rainfall volume coming to the basin and the runoff volume released from the basin. For simplicity, trapezoidal hydrograph depicted on Figure 8-1 can be considered for volume calculation. The inflow hydrograph has a linear rising limb over the time of concentration of the tributary watershed and the peaking portion of the inflow hydrograph is a plateau from the time of concentration, to the end of rainfall event. The effective rainfall volume is represented by the storage volume shown in the graph, where:

$T_c$  = Time of Concentration (minutes)

$T_d$  = Rainfall Duration (minutes)

**Figure 8-1: Trapezoidal Hydrograph**



Source: "Detention Basin Sizing for Small Urban Catchments", ASCE J. of Water Resources Planning and Management, Vol. 125, No. 6, November.

### 8.7.3 Basin Sizing

A stage-storage relationship defines the relationship between the depth of water and storage volume in a detention basin facility. The basin width to length ratio shall be greater than 2 so that the flood flows can sufficiently expand and diffuse into the water body to enhance the sedimentation process. Slopes on embankments shall maintain the bank slope stability. Slopes on earthen embankments shall not be steeper than 4:1 (H:V) and on riprap embankments shall not be steeper than 3:1 (H:V).

Detention basins may be designed in rectangular, triangular, trapezoidal or elliptical shapes, depending on the available right-of-way and required volume of storage. Once the storage volume is determined, the storage basin configuration is determined by multiple layers to accommodate the 10-, 25-, and 100-year storm event storage volumes.

#### 8.7.4 Outlet Structures and Emergency Spillways

Outlet structures of a detention basin consist of low flow outlets and emergency spillway. The outlet structure is formed by risers, perforated plates, orifices, weirs, and culverts. The minimum size of a low flow outlet shall be 18 inches. The low flow outlets are designed based on the orifice principle, while the emergency spillway is designed based on weir equations. The invert of the emergency spillway shall be 2 feet above the major design stormwater surface elevation. It is recommended to have a minimum of 1 foot freeboard.

The obstruction to low flow conduits by debris can reduce outlet design release rate and cause the premature filling of the detention basin with stormwater, reducing the flood protection provided by the structure. All outlet works and low flow conduits shall be provided with a trash rack for debris control. The trash rack shall provide a maximum bar spacing not to exceed two-thirds of the outlet opening or diameter. The total area of the trash rack shall allow for passage of the design flow with 50 percent of the trash rack blocked.

### 8.8 Stormwater Quality Management

The Stormwater Quality Management program shall comply with requirements of the stormwater discharges from facilities regulated by National Pollutant Discharge Elimination System permit issued by the State Water Resources Control Board, Regional Water Quality Control Boards (RWQCBs) permit and various environmental permits issued by the appropriate local regulatory agencies.

Where a local storm drainage system is not available, such as in rural farm land areas, water treatment requirements shall follow the environmental permits issued by the appropriate regulatory agencies.

An effective stormwater management program involves incorporating stormwater BMPs during the planning and design phases (permanent BMPs), as well as during the construction phases (temporary BMPs) of a project. The Designer shall identify the pollutants of concern in the stormwater discharge because they can have numerous negative impacts such as the following:

- Reducing the storage capacity of hydraulic facilities due to the deposition of sediment and silt
- Increasing toxic release to aquatic life due to the metal dust and toxic fluids from train/vehicle leaks

- 1 • Contributing to non biodegradable pollutants such as street litter
- 2 Two major types of permanent BMPs are design pollution prevention BMPs and treatment
- 3 BMPs. The Storm Water Data Report (SWDR) is a project specific document that documents the
- 4 permanent BMPs, and shall be prepared for each phase of the project. The SWDR shall be
- 5 prepared based on Caltrans procedures. The most common temporary BMPs are the
- 6 construction site BMPs, usually documented using a Storm Water Pollution Prevention Plan
- 7 (SWPPP). For guidance on evaluation, selection, and design criteria on permanent BMPs,
- 8 SWDR, temporary BMPs, and SWPPP, the following references shall be used:
- 9 • Caltrans Storm Water Quality Handbooks, Project Planning and Design Guide, for design of
- 10 BMPs
- 11 • Caltrans HDM design criteria
- 12 • Caltrans Storm Water Quality Handbooks, Construction Site Best Management Practices
- 13 manual for implementation of stormwater BMPs during construction
- 14 • RWQCB shall be contacted and/or referenced for guidance, where necessary.

## **8.9 Application of Approved Software**

15 The use of industry accepted hydrologic/hydraulic design programs is recommended. Where  
16 the drainage facilities impact or connect to facilities owned by others, local agency criteria shall  
17 be applied. The Caltrans HDM General Aspects chapter may be referred for approved software  
18 in use.

## Chapter 9

### Utilities

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	07 Mar 12	Initial Release, R0
1	Jun 2013	Revision 1
2	Sep 2015	Revision 2
3	Feb 2016	Revision 3





## Table of Contents

<b>9</b>	<b>UTILITIES.....</b>	<b>9-1</b>
9.1	Scope .....	9-1
9.2	Regulations, Codes, Standards, and Guidelines .....	9-1
9.3	Goals, Objectives, and Agreements .....	9-2
9.3.1	Goals and Objectives .....	9-2
9.3.2	Agreements .....	9-2
9.4	HST Utilities .....	9-3
9.4.1	Electrical .....	9-3
9.4.2	Fuel Lines .....	9-4
9.4.3	Water .....	9-4
9.4.4	Sewer .....	9-4
9.4.5	Public Telephones .....	9-5
9.4.6	Service Connections to HST Utilities .....	9-5
9.5	Third Party Utilities .....	9-5
9.5.1	High Risk and Low Risk Utilities .....	9-6
9.5.2	Right-of-Way Encroachment .....	9-6
9.5.3	Encroachment Justifications .....	9-6
9.5.4	Rearrangement .....	9-7
9.5.5	Utility Clearances .....	9-13
9.5.6	Safety and Protection Measures .....	9-16
9.5.7	Miscellaneous Utilities and Supporting Facilities .....	9-17

## Acronyms

Authority	California High-Speed Rail Authority
AREMA	American Railway Engineering and Maintenance-of-Way Association
Caltrans	California Department of Transportation
CFR	Code of Federal Regulations
CHST	<i>Not Used; use HST for California High-Speed Train</i>
CHSTP	California High-Speed Train Project
CPUC	California Public Utilities Commission
GO	General Order
HST	High-Speed Train
HV	High Voltage
NFPA	National Fire Protection Association
OCS	Overhead Contact System

## 9 Utilities

### 9.1 Scope

This chapter provides guidance on standards and procedures for the location, assessment, protection, relocation and placement of underground and overhead utilities located within and in proximity to the California High-Speed Rail Authority's (Authority's) right-of-way (shared or exclusive).

The approach for assessing and addressing third party utility impacts is broadly based on the desire to limit the number of transverse encroachments, eliminate longitudinal encroachments, and eliminate the need for maintenance of third party utilities within the Authority's right-of-way. Utilities specified herein shall include: all physical services, such as electrical power, gas, water, telephone, cable TV, fiber optics, sanitary sewers, other public and private utilities and supporting facilities required to be placed or relocated in the Authority's right-of-way. This chapter does not address California High-Speed Train (HST) storm drains and irrigation systems. Guidance for design and identification of these utilities and supporting facilities will be included in separate chapters of this Design Criteria.

### 9.2 Regulations, Codes, Standards, and Guidelines

Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards. In addition to the laws and standards relevant to any facility, standards specific to railroad facilities shall be applied.

- Code of Federal Regulations (CFR), Title 49
- California Government Codes
- California Public Utilities Commission (CPUC) General Orders (GOs), Public Utility Codes, Rules of Practice and Procedure, and the Policies and Guidelines
- National Fire Protection Association (NFPA) Standards
- California Department of Transportation (Caltrans)
  - Caltrans Highway Design Manual (HDM)
  - Caltrans Project Development Procedures Manual (PDPM)
  - Caltrans Right-of-Way Manual, Chapter 13
- American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for Railway Engineering
- American Society of Mechanical Engineers (ASME) Standards

- 1 • National Electric Safety Code (ANSI C2/NESC) Standards
- 2 • American Water Works Association (AWWA) Standards
- 3 • American Petroleum Institute (API) Standards
- 4 For installation of utilities, applicable local building, planning and zoning codes and laws shall
- 5 be reviewed for the passenger stations, maintenance or service facilities as they may be located
- 6 within multiple municipal jurisdictions or state right-of-way.

## **9.3 Goals, Objectives, and Agreements**

### **9.3.1 Goals and Objectives**

7 The following goals and objectives shall be considered in development of utility design:

- 8 • Protect the Authority's facilities and right-of-way against damage caused by presence of a
- 9 utility.
- 10 • To the extent that is reasonable and feasible, exclude access points for third party utilities
- 11 from within access controlled right-of-way. This objective is intended to provide a safe
- 12 environment for operations, minimize inconvenience to the traveling public, and eliminate
- 13 conditions detrimental to the safety of utility employees during maintenance of its facilities.
- 14 • Provide a means of inspection, maintenance, and repair of utilities, as may be required,
- 15 without disruption of train service.
- 16 • Satisfy requirements and criteria of the affected utility owners, however, no betterment shall
- 17 be included unless specifically agreed between the utility owner and the Authority.

### **9.3.2 Agreements**

- 18 • Coordinate with the Authority and utility owner as needed for the rearrangement of the
- 19 utilities within and adjacent to the Authority's right-of-way.
- 20 • Technical requirements of pertinent utility agreement shall be incorporated into the design.
- 21 • Coordinate plans developed or being developed by others in adjoining areas to ensure that
- 22 the overall utility system is comparable to those existing before start of construction and will
- 23 be compatible with project operations.
- 24 • New construction and the protection, support, restoration, and rearrangement of utilities
- 25 shall be in conformance with the latest technical specifications and practices of the
- 26 respective utility owner.
- 27 • Third party utility design performed by the Designer requires approval by utility owner in
- 28 coordination with the Authority.

- In the event that there are no standards, design shall be in accordance with the requirements of this chapter.

## 9.4 HST Utilities

HST utilities are defined as utilities and supporting facilities that serve HST facilities.

### 9.4.1 Electrical

Electrical service utilities and supporting facilities transmit electrical power from its traction power or facility power substations to facilities such as Overhead Contact System (OCS), ventilation structures, train control houses, passenger stations, and other appurtenant wayside facilities.

Various local or regional power companies provide power to traction power and facility power substations.

If a power company's substation is located within the Authority's right-of-way, the design of the substation site shall follow the requirements of that power company, Section 9.5.5.8 of this chapter on Aboveground Utility Facilities, the *Facility Power and Lighting Systems*, and the *Traction Power Supply System* chapters.

Electrical distribution facilities shall be surface mounted on foundations, supported on structures, or installed underground. Refer to the *Facility Power and Lighting Systems* chapter for design requirements.

Unless otherwise specified, each utility power company designs, furnishes, and installs all of its cables and/or overhead conductors, including all high voltage (HV) utilities and supporting facilities and connection, from the utility's network up to the point of common coupling (interface connection point).

HST Core Systems (Traction Electrification, Overhead Contact System, Automatic Train Control, and Communications) ductbanks, manholes, conduits and cables located within the Authority's right-of-way or from interface connection point shall be designed by the Designer.

#### 9.4.1.1 Ductbanks

For design of underground ductbanks, refer to the *Facility Power and Lighting Systems* chapter, *Communications* chapter, and Standard Specifications.

#### 9.4.1.2 Manholes and Handholes

Standard manhole and handhole sizes shall be used whenever possible. Refer to the *Facility Power and Lighting Systems* chapter, *Communications* chapter, and Standard Specifications for more information.

### **9.4.2 Fuel Lines**

The design and installation of fuel lines (gas and petroleum) to facilities from service main up to the meter is performed by the individual utility companies providing the fuel, unless otherwise indicated in the agreement between the Authority and the utility company.

The size of fuel supply line and the meter shall be based on peak-demand calculations provided by the Designer.

The clearance requirements for the fuel pipe shall be per the requirements of the utility owner (up to the meter) and of this chapter (from the meter to the facilities).

The design and installation fuel lines within Authority's right-of-way shall comply with the requirements of NFPA 30 and local jurisdictional requirements.

The meter for fuel lines shall be in public right-of-way or in a location accessible to the utility company.

Corrosion control measures for fuel pipelines shall be applied in accordance with the following:

- Federal Safety Standards for Gas Lines, Title 49 CFR, Part 192
- Section 9.5.6.4 of this chapter on Cathodic Protection and Corrosion Control
- The utility company standards and practice

### **9.4.3 Water**

Fire and domestic (potable and non-potable) water service connections shall be designed by the Designer and installed by the Contractor. The size of the domestic connection and meter shall be based upon a peak-demand pressure-loss calculation. The domestic service meter shall be located in public right-of-way or in a location accessible to the utility company and shall meet the requirements of the water company providing the water meter.

Fire flow and pressure data in the existing water main will be furnished by the municipal agency in consultation with the Fire Marshal having jurisdiction. Unless otherwise specified, the fire service in public right-of-way and domestic services up to the Authority's right-of-way and including the meter is installed by the utility. Location of fire hydrants within the Authority's right-of-way shall be coordinated with and approved by the Fire Marshal having jurisdiction of the site. For standpipe design and installation within passenger station areas and HST tunnels, refer to the *Fire Protection* chapter.

### **9.4.4 Sewer**

Sewer services shall meet the requirements of the Sanitary Sewer District and the appropriate municipal building code. The Designer shall be responsible for sizing and establishing the slope of the sewer services, however, the minimum allowable diameter for a sanitary sewer shall be 8

1 inches with a minimum slope of 0.7 percent. Location of sanitary sewer manholes shall be  
2 accessible to maintenance staff.

#### 9.4.5 Public Telephones

3 Conduits and wiring for public telephones in passenger stations and parking lots, from the  
4 telephone service points to each public telephone location, shall be designed by the Designer  
5 and installed by the Contractor. The location and elevation of the conduit stub at the property  
6 line shall be indicated on the plans. Unless otherwise specified in the agreement between the  
7 Authority and the public telephone service provider, the design and installation of conduits and  
8 wirings from telephone distribution network to the Authority's right-of-way will be performed  
9 by the public telephone service provider and the portion within the Authority's right-of-way  
10 shall be performed by the Contractor. Public telephones are installed by the telephone company  
11 or the service provider. All public telephone service provider conduits and cables shall be  
12 terminated at the Entrance Facility Room at each passenger station, building, or facility as  
13 described in the *Communications* chapter. Also, refer to the *Stations* chapter for general location,  
14 quantity, and description of public phones in passenger station areas.

#### 9.4.6 Service Connections to HST Utilities

15 Utility service connections to utilities and supporting facilities that lie outside of structures,  
16 such as traction power substations, train control houses, etc., shall be shown on the utilities  
17 plans. The Designer shall coordinate the service connections with other items of work. The  
18 Designer shall ensure that service connections are indicated on its plans and plans for the  
19 adjacent construction contracts.

20 For utility service connections, the Designer is responsible for design of utility service lines from  
21 the interface point (i.e., a manhole or a meter) to HST facilities. The utility owner is responsible  
22 for design and construction of the utility service through the interface point unless otherwise  
23 stated in a utility agreement.

### 9.5 Third Party Utilities

24 Third party utilities are defined as utilities and supporting facilities belonging to governmental  
25 agencies, public utility corporations, railroads, privately owned companies, and private parties  
26 for the provision of sewer, water, drainage, gas, electrical, steam, telephone, cable TV,  
27 petroleum product pipelines, fiber optics, and other communications systems. Third party  
28 utilities are mainly transmission lines, distribution lines, and the service connections to adjacent  
29 properties that are not related to HST operations.

30 Third party utilities to be relocated, abandoned, or protected in place during construction,  
31 whether done by the Contractor or the utility owner, shall be addressed in the construction  
32 documents.

### 9.5.1 High Risk and Low Risk Utilities

Utilities conducting toxic or flammable gases or liquids, pressurized waterlines greater than 80 psi normal operating pressure or in pipelines greater than 8 inches in diameter, and underground electrical supply lines that have potential to ground more than 300 volts, which do not have concentric grounded or effectively grounded metal shields or sheaths, are considered High Risk utilities.

All other utilities are considered Low Risk utilities.

### 9.5.2 Right-of-Way Encroachment

An encroachment is defined as any structure or object of any kind that is within the Authority's right-of-way but not part of the HST facility. Encroachments allow temporary or permanent use of Authority's right-of-way by a utility, a public entity, or a private party. Encroachments include all public and private utilities within the Authority's right-of-way.

To meet the Authority's goal and objective, to the extent that is reasonable and feasible, exclude access points for utilities from within access controlled right-of-way. This objective is intended to provide a safe environment for operations, minimize inconvenience to the traveling public, and eliminate conditions detrimental to the safety of utility employees during maintenance of its utilities and supporting facilities.

High Risk utilities and supporting facilities shall not be allowed on HST aerial structures, tunnels or through any underground/subgrade trackway under any circumstances.

### 9.5.3 Encroachment Justifications

Access to third party utilities from the Authority's right-of-way is an exception to criteria and requires Authority's approval.

Where longitudinal or transverse encroachments and installations are required, the Designer shall ensure the following:

- An alternate location is not feasible, from the standpoint of providing efficient utility services in a manner conducive to safety, durability, and economy of maintenance and operations.
- The accommodation will not adversely affect the design, construction, operation, maintenance, safety, or stability of HST system and supporting facilities.
- The accommodation will not interfere with or impair the proposed use or future expansion of HST system and supporting facilities.
- The disapproval of the use of the Authority's right-of-way would result in an immitigable impact to the owner, the environment, or the public.



- The utility will be located in such a manner that it can be serviced, maintained, and operated without being accessed from within the Authority's right-of-way and will not adversely affect safety or cause damage to HST system and supporting facilities.

#### 9.5.3.1 Longitudinal Encroachments

**Existing Utility Longitudinal Encroachments** – Longitudinal utility encroachments within the Authority's right-of-way shall be considered on a case-by-case basis.

Existing longitudinal utilities located within the existing or proposed Authority's right-of-way shall be relocated to the outside of the Authority's right-of-way unless it can be shown to meet the encroachment justification requirements noted above.

**New Utility Longitudinal Encroachments** – New third party utilities are not be permitted to be installed longitudinally within access controlled areas. Deviation from this requirement requires Authority's approval.

#### 9.5.3.2 Transverse Encroachments

New utility installations and adjustments or relocation of existing utilities may be permitted to cross the Authority's right-of-way.

To the extent feasible and practicable, they should cross on a line generally normal to, but not less than 60 degrees from the railroad longitudinal alignment.

Transverse crossings that are at less than 60 degrees from the railroad longitudinal alignment shall be classified as a longitudinal encroachment.

Transverse utility encroachments shall comply with the encroachment justification requirements noted above.

With the exception of HV transmission lines and the utilities that can be placed in roadway structures or independent utility bridge structure with a solid surface going over Authority's right-of-way, all utility transverse crossings shall be undergrounded. Refer to the *Overhead Contact System and Traction Power Return System* chapter for HV clearance requirements over HST OCS facilities.

Air space leases for wireless communications facilities fall under the general guideline for transverse encroachments and are to be reviewed by and concurred with the Authority which may develop special guidelines for wireless communication facilities.

#### 9.5.4 Rearrangement

To the extent that is reasonable and feasible, proposed third party utilities shall be located outside the Authority's right-of-way. The location, design, and construction of relocated or proposed utilities shall meet the requirements of CPUC GOs and the provisions of this chapter.

In performing utilities rearrangements, due consideration shall be given to the needs of the HST Core Systems design, the requirements and obligations of the utility owners, and goals and objectives established by the Authority. If temporarily relocated, existing utilities shall be restored upon completion of work. If permanently relocated, the new utility shall be operational before or coincident with the termination of the existing service.

#### **9.5.4.1 Identification of Existing Utilities**

The Designer is responsible for determining the identification and location of all utility facilities that lie within the planned construction right-of-way boundaries. This is accomplished by the following:

- Review of procurement documents
- Review of railroads, State, County, or other local jurisdiction's as-builts, permit records, and geographic information system
- A joint field review of the project area with the Authority or its designee (Utility Coordinator) and utility owner's representatives

The need for utility identification and verification is twofold:

- To identify all potential/project conflicts so they may be cleared before project construction commences, either through avoidance or relocation
- To meet the requirements of California Government Code Section 4215

#### **9.5.4.2 Utility Verification Request to Owner**

Prepare and furnish verification maps to each utility owner detailing existing or potentially existing utilities and supporting facilities within the project area. The request letter shall include the elements shown in Caltrans Right-of-Way Manual Chapter 13 Exhibit 13-EX-10. The Designer shall request the utility owner to update the maps for all utilities and supporting facilities including any abandoned facility. Normally the utility owner is allowed 30 days to respond. The Designer shall be responsible for follow-up to ensure timely completion of verification. (Refer also to CPUC GO No. 128, Rule 17.7 for legal requirements for regulated utility owner to provide facility location information.)

#### **9.5.4.3 Positive Location of Underground Utilities**

Positive location of all High Risk or Low Risk utilities and supporting facilities shall be accomplished by potholing; methods of potholing shall be determined by the utility owner to determine horizontal and vertical position of the facility. Combination of methods may be more effective than a single method. The Designer, through the Authority, shall make a written request to the utility owner for the positive location of their utility facility. With the concurrence of the utility owner, potholing and surveying of the utility shall be performed and shown on the contract documents.

All utilities and supporting facilities within the construction area shall be positively located within 0.5 feet for both horizontal and vertical location. For transverse utilities and supporting facilities, the positive location should be made on either side of the trackway and other critical locations. For longitudinal utilities and supporting facilities, the locations shall be done at intervals sufficient to establish the location of the line but in no event at greater than 100-foot intervals. Machine excavation to expose the High Risk or Low Risk utilities and supporting facilities in order to physically locate them shall be done by, or at the authorization of, the utility/facility owner.

#### 9.5.4.4 Level of Service and Service Interruptions

A level of service equivalent to the existing service shall be maintained for adjacent properties, residents, and businesses throughout construction by supporting utilities in place, diverting utilities, or providing alternative temporary utilities and supporting facilities.

Interruption of existing utilities service shall be minimized. Service shall not be interrupted without the prior written consent of utility owners.

#### 9.5.4.5 Placement

Pipes shall be designed to support dead loads imposed by earth, subbase, pavement, ballast, structures, track, and dynamic forces exerted by anticipated train loads when the pipe is operated with the design range of internal pressure from maximum to zero.

Utility lines crossing beneath at-grade trackway shall conform to the Pipelines section of the AREMA Manual for Railway Engineering, except as modified in these criteria, and the following:

- Casing pipes shall be provided for all utility carrier pipes crossing trackway and structure foundations and buildings.
- Electrical ducts, telephone, and fiber optic conduits in ducts may not require casing pipe where the ducts are encased in concrete and the strength of the utility facility is capable of withstanding rail system loading. Refer to the *Structures* chapter for rail system loading requirements.
- Where new trackways are constructed over existing utilities that are not in conflict with the construction and operations of the HST Core Systems conduits and existing utilities to be continued in service, the utility facilities shall be uncovered and encased in accordance with this Design Criteria.
- Utilities shall not be placed in any manner or position which may cause damage to or impair the safety of the HST Core Systems conduits.
- Utilities shall not be placed within culverts and/or under waterway crossings.
- Where a utility crosses a HST utility within the Authority's right-of-way, the HST utility shall be maintained above the third party utility.

1 When utility and supporting facility rearrangement are designed by the utility or facility owner,  
2 the Designer shall coordinate and furnish data so that the owner may complete its design in a  
3 timely fashion. The Designer shall review the facility owner's proposal to ensure that it is  
4 compatible with the HST Core Systems conduit design and that of other affected owners. The  
5 Designer shall include such arrangement as facility alignments on its plans and shall provide  
6 appropriate copies for distribution to affected facility owners.

#### 9.5.4.6 Carrier Pipes

7 For non-flammable substances, the carrier pipe and joints within the Authority's right-of-way  
8 shall be of acceptable material and construction per AREMA requirements. Joints for carrier  
9 pipe operating under pressure shall be mechanical or welded per AREMA and ASTM  
10 requirements. Resilient type joints shall be used where the combination of fill height and  
11 foundation soil compressibility could adversely affect the permanence and watertightness of  
12 rigid type joints.

13 For flammable substances, the carrier pipe that has to be relocated or replaced within the  
14 Authority's right-of-way shall be designed, installed, and tested in accordance with the current  
15 standards of the utility owner and the following:

- 16 • Title 49 CFR Part 192 "Transportation of Natural and Other Gas by Pipeline: Minimum  
17 Federal Safety Standards", Part 193 "Liquefied Natural Gas Facilities: Federal Safety  
18 Standards", and Part 195 "Transportation of Hazardous Liquids by Pipeline"
- 19 • California Government Codes, Section 51010–51019.1
- 20 • ASME B31.8 "Gas Transmission and Distribution Piping Systems" and ASME B31.4 "Liquid  
21 Transportation System for Hydrocarbons, Liquid Petroleum Gas, Anhydrous Ammonia and  
22 Alcohols"
- 23 • "API Recommended Practice (RP 1102) Steel Pipeline Crossing Railroads and Highways"
- 24 • CPUC GO No. 112-E "Rules Governing Design, Construction, Testing, Operation, and  
25 Maintenance of Gas Gathering, Transmission, and Distribution Piping Systems" except that  
26 allowable stresses for the design of steel pipe shall conform to AREMA requirements
- 27 • Steel carrier pipes shall be protectively coated and provided with a cathodic protection  
28 system and test monitoring facilities in conformance with the requirements of this chapter  
29 and the *Corrosion Control* chapter.
- 30 • Pipelines carrying flammable gas products shall, where practicable, cross any portion of the  
31 HST Core Systems conduits where tracks are carried on an embankment.
- 32 • Liquid-petroleum lines in proximity or crossing underground structures shall also conform  
33 to the requirements of NFPA 30.
- 34 • Steel natural gas pipeline facilities crossing under Authority's right-of-way may be uncased  
35 provided that they meet the following requirements:

- Design and installation of the pipeline shall be in accordance with the AREMA requirements for uncased natural gas pipeline crossing
- Inspection and maintenance of natural gas pipeline shall be performed by the gas company without entering the Authority's right-of-way
- Sectionalizing valves for natural gas pipeline shall be installed and maintained in accordance with 49 CFR Part 192 and shall be activated to shut off the flow of gas across the Authority's right-of-way in case of emergency
- In case of a gas leak, the pipeline under Authority's right-of-way shall be abandoned in place with the requirements of the Authority and a new pipeline be installed

#### 9.5.4.7 Casings

Underground utilities and supporting facilities located within the Authority's access controlled right-of-way shall be encased. Casings are required to protect HST safe operation and facilitate maintenance of utilities crossing under Authority's access restricted right-of-way without the need to enter Authority's right-of-way.

- Casings shall consist of steel, corrugated steel, ductile iron, or reinforced concrete pipe in accordance with the latest industry codes and standards, AREMA requirements, and the requirements specified below.
- Steel casings shall be 3/8 inch minimum thickness with welded joints.
- Electrical and communication lines can be installed in non-metallic casing.
- Casing pipes and joints shall be watertight and shall be capable of withstanding train loading as specified in the *Structures* chapter.
- Casings shall be sloped to drain.
- Casings shall be protected against corrosion by using corrosion resistant casing material and interior and exterior protective coatings, or other approved methods in conformance with the requirements of this chapter and the *Corrosion Control* chapter. Care shall be taken to select materials which will not be damaged through contact with dissimilar metal.
- Casings shall be cathodically protected per the requirements of this chapter and the requirements of the utility owners.
- Metallic casings shall be grounded and bonded in conformance with the requirements of the *Grounding and Bonding Requirements* chapter.
- Steel casing pipe shall have a minimum yield strength of 35,000 psi. Wall thickness for steel casing pipe shall conform to AREMA requirements for steel casing pipe for E80 loading.

- Ductile iron pipe may be used for a casing provided the method of installation is by open trench. Ductile iron pipe shall conform to ANSI/AWWA C151/A21.51 “Ductile-Iron Pipe, Centrifugally Cast”. The pipe shall be of the mechanical-joint type or plain-end pipe with compression-type couplings.
  - For non-flammable substances the strength of ductile iron pipe shall be computed in accordance with ANSI/AWWA C150/A21.50 “Thickness Design of Ductile-Iron Pipe”, to sustain external loads.
  - For flammable substances ductile iron pipe shall conform to ANSI/AWWA C151/A21.51 “Ductile-Iron Pipe, Centrifugally Cast”.
- Reinforced concrete pipe, with gasketed watertight joints, can be used for a casing pipe for non-pressurized utilities.
- The inside diameter of the casing pipe shall be sized to allow the carrier pipe to be removed subsequently without disturbing the casing pipe or trackway. All joints or couplings, supports, insulators, and centering devices for the carrier pipe within a casing shall be taken into account. In no case shall the casing pipe be sized smaller than the following:
  - The inside diameter of the casing pipe shall be at least 4 inches greater than the largest outside diameter of the carrier pipe, joints and couplings.
  - The minimum size of a casing pipe shall not be less than 24 inches.
- Casing pipe shall extend beyond the Authority’s right-of-way to a sufficient distance to allow for access to the casing by open excavation. The minimum distance outside the right-of-way shall be 3 feet plus the distance between the bottom of the casing and top of finished grade at the right-of-way line, measured at the right angle to the right-of-way. For flammable utilities, the extent of casing shall not be less than 45 feet from the track centerline, measured at the right angle to the track centerline.
- Where the ends of the casing are below ground, they shall be properly protected against the entrance of foreign material, but shall not be tightly sealed.
- Where the ends of the casing are at or above ground surface and above high-water level, they may be left open provided drainage is afforded in such a manner that leakage will be conducted away from the tracks or structures.
- CPUC GO No. 128, “Rules for Construction of Underground Electrical Supply and Communication Systems” shall govern for all pertinent applications.
- Casings carrying fluids and gases shall be properly vented. Vent pipes shall be of sufficient diameter, but in no case less than 2 inches in diameter, projecting through the ground surface beyond the Authority’s right-of-way line. Vent pipes shall extend no less than 4 feet above the ground surface. Top of vent pipes shall be fitted with a down-turned elbow, properly screened.

### 9.5.5 Utility Clearances

The minimum requirements for utility clearances shall be as defined by the CPUC GOs as applicable, Caltrans HDM and PDPM, AREMA, utility owner's requirements, and these Design Criteria. These requirements apply to HST Core Systems related facilities as well as those owned by others.

In addition, High Risk and Low Risk utilities shall comply with the following requirements:

- High Risk Utilities

- Maintain 500 feet minimum horizontal separation from flammable gas or hazardous liquid utility to other High Risk utilities

- Maintain 50 feet minimum horizontal separation from non-flammable gas or hazardous liquid utility to other High Risk utilities

- Maintain 5 feet minimum horizontal separation from Low Risk utilities

- Maintain 20 feet minimum horizontal separation from non-load carrying and load carrying structural elements, including OCS pole foundations and downguys

- Maintain 3 feet minimum vertical separation from drainage pipes/structures

- Low Risk Utilities

- Maintain 3 feet minimum horizontal separation from other Low Risk utilities

- Maintain 5 feet minimum horizontal separation from load carrying structural elements, including OCS pole foundations and downguys, and 3-foot minimum horizontal separation from other structures

- Maintain 3 feet minimum vertical separation from drainage pipes

The above clearances are minimum requirements. The Designer shall verify the adequacy of these standards.

Electrical and communication lines within the Authority's right-of-way shall comply with the above requirements except that a concrete encased ductbank can be used in lieu of steel casing pipe. All underground electrical utilities and supporting facilities within the planned construction area shall meet the minimum clearance requirements as defined in CPUC GOs.

#### 9.5.5.1 Underground Utilities within At-Grade Section

Where a portion of the utility line crosses under HST right-of-way, the utility shall be encased. The casing under the track shall be a minimum of 6 feet below top of rail and a minimum of 3 feet below the flow line of the ditch or drainage pipe next to the track. Refer to Standard and Directive Drawings for utility clearance requirements. Casing shall be provided for utilities

located outside HST right-of-way but within the zone of train loading influence or if the future utility maintenance requires excavation that can impact the integrity of HST infrastructure.

Casing pipes shall not be placed within the limits of prepared subgrade unless the backfill material for the culvert and its compaction meet the requirements of prepared subgrade, refer to the *Geotechnical* chapter for limits of prepared subgrade, material, and compaction requirements. Casing pipes shall not be less than 6 feet below top of rail and at the closest point. Deeper installations may be required to avoid conflicts with the HST Core Systems under track conduits and buried facilities. Casing for flammable gas or hazardous liquid utility pipe shall be a minimum of 10 feet below top of rail.

Where the casing is not directly beneath the trackbed, the depth of ground cover shall not be less than 4 feet. A 6-inch-thick layer of reinforced red concrete shall be placed over the casing pipe if 3 feet of ground cover cannot be provided between top of casing pipe and bottom of ditch.

Utilities that cannot be relocated underground shall be located on an independent utility bridge structure, encased, and appropriately protected from 25 kV Electrification System. The independent utility bridge structure shall meet the following requirements:

- Have a solid surface to prevent casings from falling off the bridge and provide walking surface for inspection
- Constructed independent of the HST infrastructure for its intended use and maintained in accordance with recognized industry standards
- Be part of utility owner's system and hence included in owner's inspection and maintenance plan. However, the inspection and maintenance of the utility bridge and the utility crossing Authority's right-of-way shall be limited to non-operating hours of HST
- Have warning signs be installed per the regulatory requirements
- Be grounded and bonded in accordance with the *Grounding and Bonding Requirements* chapter
- Meet the clearances requirements of the *Trackway Clearances* chapter

#### **9.5.5.2 Underground Utilities within HST Core Systems Facility Sites**

Utilities located within the HST Core Systems facility sites, such as traction power facility and Stand Alone Radio Sites, shall be relocated outside of the Authority's right-of-way.



### 9.5.5.3 Underground Utilities within Authority's Roadways and Parking Lots

Underground utilities within roadways and parking areas shall have a minimum of 3 feet cover over casings. Additional cover shall be provided where necessary to comply with the utility owner's standards.

### 9.5.5.4 Underground Utilities within HST Aerial Structure Section

Underground utilities within 5 feet of a HST pier or abutment foundation shall be relocated in accordance with the requirements of this chapter and the utility owner's clearance requirements. Existing utilities that do not need to be relocated shall be encased in accordance with the requirements of this chapter with the following exceptions:

- Where utilities are within a jurisdictional authority's roadway or railroad's right-of-way
- Casings do not need to be designed per Cooper E-80 loading requirements but are subject to site specific loading requirements.

Access manholes to utilities shall be relocated outside Authority's right-of-way unless such manholes are located within roadways or access roads of other jurisdictional authorities.

Designer shall submit a utility protection and monitoring plan for utilities within the zone of influence of excavation limits to the utility owner for review and approval.

### 9.5.5.5 Underground Utilities within HST Trench Section

Where a trench section is 8 feet or less from the original ground, the utilities shall cross under trench sections in casing and top of casing shall be a minimum of 8 feet below top of rail.

Where a trench section is deeper than 8 feet, utilities shall cross over the trench section in an independent utility bridge structure that spans the entire width of trench section. The minimum clearance from the bottom of utility bridge structure shall be per requirements of the *Trackway Clearances* chapter and the Standard and Directive Drawings for trench sections.

In all other cases, the utility shall be relocated so that the utility crossing is outside the trench limits.

### 9.5.5.6 Underground Utilities within HST Tunnel Section

For utility clearance requirements within a tunnel section, refer to Standard and Directive Drawings for tunnel sections.

### 9.5.5.7 Overhead Utilities

Overhead utilities shall cross the tracks at public roadway, or highway overpasses. Such utilities shall be contained within the overpass structure such that in case of failure the overhead lines fall on the roadway structure or inside a casing embedded in the overpass structure. The casing shall be per the requirements of this chapter and shall be grounded and bonded in accordance with the *Grounding and Bonding Requirements* chapter. Where electrical lines with voltage less

than 25 kV and communication lines cannot be accommodated in an overpass structure, they shall be relocated underground per clearance requirements established in this chapter.

Clearances for overhead electrical lines with voltage higher than 25 kV shall be governed by CPUC GO No. 95 “Overhead Electric Line Construction” or National Electrical Safety Code (ANSI C2/NESC) wire to wire clearance requirements, whichever is more stringent. Design and construction of overhead electrical lines shall also comply with the requirements of CPUC GO No. 176.

#### **9.5.5.8 Aboveground Utility Facilities**

All above ground utilities shall be moved outside the Authority’s right-of-way or conform to the requirements of Sections 9.5.5.1 to 9.5.5.5.

#### **9.5.6 Safety and Protection Measures**

The Designer shall support the Authority in preparation of emergency response access and procedures to handle a situation in which a pipeline leak or railroad derailment or incident may jeopardize the integrity of the pipeline. Local conditions shall be considered when developing these procedures. Refer to the *System Safety and Security* chapter for guidelines.

##### **9.5.6.1 Call Before You Dig**

The Designer shall include in its plans the instruction to contact Underground Service Alert and other railroad owners, such as Union Pacific Railroads’ “Call Before You Dig” requirements, within the railroad’s right-of-way prior to any excavation.

##### **9.5.6.2 Shut-off Valves**

Accessible emergency shut-off valve(s) shall be located as close to the Authority’s right-of-way as practicable and as mutually agreed to by the Designer, the Authority, and the utility owner. These valves shall be marked with signs for identification. Where there are existing automatic control shut-off valve stations at locations and within distances acceptable to the Authority, additional valves may not be required. Valves shall not be located within the Authority’s right-of-way.

##### **9.5.6.3 Utility Markers**

Utility markers shall be placed at points where the centerline of the utilities intersects the boundaries of the right-of-way line as they enter and exit the Authority’s right-of-way.

Markers shall identify each utility, its owner, HST milepost, and depth. Markers shall consist of a metal target plate with reflective background mounted on a metal post.

The front face of the target plate shall be marked as indicated in Standard and Directive Drawings for wayside signage and graphics or as required by the utility owner with the

- milepost and with a symbol identifying the type of the facility, both reading vertically from top to bottom.
- The rear face of the target plate shall be marked with the name of utility owner and telephone number for emergency contact, both reading vertically from top to bottom.
- Markers shall be oriented so that the front face is facing the trackway.
- For facility naming conventions, refer to the CHSTP Plan Preparation Manual.

#### 9.5.6.4 Cathodic Protection and Corrosion Control

Cathodic protection and corrosion control measures shall be applied to all relocated and new metallic and reinforced concrete underground utilities. These measures shall include: protective exterior coating, electrical continuity of pipes, electrical isolation of new and existing pipes, cathodic protection, and test facilities as appropriate. Soil and groundwater corrosive characteristics shall be determined and documented during the Baseline Corrosion Survey. Analysis of soil boring samples obtained during geotechnical survey shall supplement on-site measurements and shall be the basis for corrosion control design. Cathodic protection and corrosion control of carrier pipes and casings shall be in accordance with AREMA requirements and the *Corrosion Control* chapter.

In addition, to minimize the possibility that utility pipelines become part of the Traction Power Return System, insulated joints or couplings shall be installed at or adjacent to the shut-off valves or at a similar location if shut-off valves are not required.

#### 9.5.7 Miscellaneous Utilities and Supporting Facilities

##### 9.5.7.1 Railroad Utilities

Signals and communication lines belonging to railroad companies may be affected by construction of the California High-Speed Train Project (CHSTP). The design and rearrangement of such utilities and supporting facilities that conflict with the CHSTP shall be performed by the facility owner in accordance with owner's standards.

Where HST utilities and supporting facilities impact railroad utility facilities or where rearrangement of other utility facilities will impact or potentially affect the operation of railroad utility facilities, the Designer shall coordinate its work with the Authority so that adequate data is furnished to the railroad to complete its work. The Designer shall include the rearrangement of railroad utilities in the utilities plan.

##### 9.5.7.2 Street Lights and Traffic Control Devices

Traffic control devices and street lighting facilities belonging to a third party agency may be affected by construction. Any facility affected or impacted by the construction shall be replaced in-kind, unless otherwise indicated in the agreement between the Authority and the affected

1 agencies. Refer to the current standards of the governing agency having jurisdiction over the  
2 affected traffic control devices or street lighting facility.

### **9.5.7.3 Fire Alarm and Police Communication Systems**

3 Protection, relocation, and support-in-place of the fire alarm and police communication systems  
4 within construction limits shall be in strict conformance with the current standards of the  
5 governing agency or municipality.

6 The Designer shall indicate which emergency communication lines are to be maintained  
7 complete in-place, removed, protected and supported, temporarily relocated and replaced after  
8 construction is complete. The lines to be abandoned or are already abandoned shall also be  
9 indicated.

### **9.5.7.4 Vaults**

10 Private vaults surrounding the project area that do not encroach upon the Authority's right-of-  
11 way shall be protected in place. Vaults that are found to be in conflict with the proposed  
12 construction shall be subject to demolition and/ or relocation.

13 Relocation, abandonment, or other work involving private vaults extending from adjoining  
14 buildings into public space shall be in accordance with rules, regulations, and practices of the  
15 governing agency, which shall include the city building codes, electric codes, plumbing codes,  
16 and National Electrical Safety Code (ANSI C2/NESC). Locations and dimensions of vaults  
17 affected by the project shall be verified. Quantities and types of goods or facilities including  
18 utility services to be removed from the vault shall be verified; the process of deliveries to  
19 properties when existing vault entrances require abandonment; and the construction time  
20 required to make alterations and occupy the vault shall be determined and incorporated into  
21 the design. This information shall be presented to the vault owner, through the Authority,  
22 which will arrange for permission to occupy the vault to make the necessary alterations.

## Chapter 10

### Geotechnical

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
0.1	07 Dec 12	Addendum 07 – CP01
1	June 2013	Revision 1
2	Feb 2014	Revision 2
3	Feb 2016	Revision 3



## Table of Contents

<b>10</b>	<b>GEOTECHNICAL .....</b>	<b>10-1</b>
10.1	Scope .....	10-1
10.2	Regulations, Codes, Standards, and Guidelines .....	10-1
10.3	General Geotechnical Design Requirements .....	10-3
10.4	Subsurface Investigation and Data Analysis .....	10-5
10.5	Geotechnical Reports .....	10-7
10.5.1	Geotechnical Data Report (GDR) .....	10-8
10.5.2	Geotechnical Engineering Design Report (GEDR) <sup>1</sup> .....	10-9
10.5.3	Geotechnical Baseline Report for Construction (GBR-C) .....	10-11
10.6	Bridge, Aerial Structure, and Grade Separation Foundations .....	10-11
10.6.1	Geotechnical Data .....	10-12
10.6.2	Load Modifiers, Load Factors, Load Combinations, and Resistance Factors .....	10-12
10.6.3	Allowable Foundation Settlements for Primary Type 1 Structures .....	10-12
10.6.4	Bridge, Aerial Structure, and Grade Separation Foundation Types .....	10-13
10.6.5	Other Design Considerations .....	10-22
10.7	Station and Miscellaneous At-Grade Structures .....	10-23
10.7.1	Shallow Foundations .....	10-23
10.7.2	Deep Foundations .....	10-24
10.7.3	Miscellaneous At-Grade Structure Foundations .....	10-24
10.8	Retaining Walls and Trenches .....	10-24
10.8.1	Design .....	10-25
10.8.2	Unacceptable Walls .....	10-25
10.8.3	Stability of Retaining Walls .....	10-26
10.8.4	Base Pressure .....	10-28
10.8.5	Hydrostatic Pressure (Buoyancy) .....	10-28
10.8.6	Settlements and Horizontal Deformations .....	10-28
10.8.7	Drainage .....	10-29
10.8.8	Backfill .....	10-29
10.9	Embankments and At-Grade Earthen Structures for HST Trackway (Primary Type 1) 10-29	
10.9.1	Slope Inclination .....	10-30
10.9.2	Safety Factors .....	10-30
10.9.3	Settlements .....	10-30
10.9.4	Benching of Slopes .....	10-34
10.9.5	Particular Requirements .....	10-35
10.9.6	Soil Materials Used for Embankments .....	10-43
10.10	Cut Slopes .....	10-44
10.10.1	Design of Cut Slopes .....	10-45
10.10.2	Drainage (Surface and Subsurface) .....	10-46
10.10.3	Slope Stability Mitigation Methods for Cut Slopes .....	10-46
10.11	Existing Slopes .....	10-47
10.11.1	Protection of Existing Slopes .....	10-47
10.11.2	Drainage (Surface and Subsurface) .....	10-47
10.12	Cut-and-Cover Underground Structures .....	10-48
10.12.1	Structural Systems .....	10-48

10.12.2	Water Pressure (Buoyancy) .....	10-48
10.12.3	Temporary Support of Underground Structures .....	10-49
10.12.4	Temporary Lateral Loading Conditions .....	10-49
10.12.5	Permanent Lateral Loading Conditions .....	10-50
10.12.6	Deformation Limits for Support of Excavation Systems .....	10-50
10.12.7	Dewatering .....	10-51
10.13	Seismic Design .....	10-51
10.13.1	Design Earthquakes .....	10-51
10.13.2	Seismic Hazard and Ground Motions .....	10-51
10.13.3	Liquefaction of Foundation Soils .....	10-52
10.13.4	Underground Structures .....	10-53
10.13.5	Effect of Ground Deformation .....	10-55
10.13.6	Soil-Structure Interaction for Bridges, Aerial Structures, and Grade Separations .....	10-57
10.14	Track Structure .....	10-59
10.14.1	Formation Supporting Ballasted Tracks .....	10-59
10.14.2	Determination of the Thickness of the Trackbed Layers .....	10-60
10.14.3	Design of Formation for Dynamic Loading from HST Operations .....	10-61
10.15	Maintenance of Geo-Structures .....	10-63
10.16	References .....	10-63

## Tables

Table 10-1:	Maximum Allowable Settlement Limits <sup>(4),(5)</sup> for Service 1 and OBE Load Cases 10-13	
Table 10-2:	Pile Load Modifiers, $P_m$ , for Multiple Row Shading .....	10-18
Table 10-3:	Minimum Penetration Depth in Rock .....	10-23
Table 10-4:	Minimum Required Factors of Safety for Embankment Slopes .....	10-30
Table 10-5:	Maximum Residual Settlement Limits .....	10-32
Table 10-6:	Soil Material Suitability for Engineered Fill in Embankments .....	10-44

## Figures

Figure 10-1:	Settlements of Embankments .....	10-31
Figure 10-2:	Different Settlement Parts by Time .....	10-33
Figure 10-3:	Typical Section Earthwork Cut/Fill .....	10-35
Figure 10-4:	Earthwork Embankment in Wet Conditions .....	10-36
Figure 10-5:	Drainage Layer under Embankments in Floodplain / High Water .....	10-36
Figure 10-6:	Transition from Concrete Slab to Embankment .....	10-40
Figure 10-7:	Transition from Cut to Embankment .....	10-41
Figure 10-8:	Transition from Bridge, Aerial Structure, or Grade Separation to Embankment .....	10-42
Figure 10-9:	Tunnel Transverse Ovaling and Racking Response to Vertically Propagating Shear Waves .....	10-54
Figure 10-10:	Tunnel Longitudinal Axial and Curvature Response to Traveling Waves .....	10-54



## **Appendix**

- Appendix 10.A: Guidelines for Geotechnical Investigations
- Appendix 10.B: Guidelines for Geotechnical Earthquake Engineering
- Appendix 10.C: Guidelines for Rock Slope Engineering

## Acronyms

AASHTO	American Association of State Highway and Transportation Officials
Authority	California High-Speed Rail Authority
BDS	Bridge Design Specifications
Caltrans	California Department of Transportation
CBC	California Building Codes
CBR	California Bearing Ratio
CEG	Certified Engineering Geologist
CGS	California Geological Survey
CHSTP	California High-Speed Train Project
CPT	Cone Penetration Test
CPT <sub>u</sub>	Cone Penetration Test with pore pressure measurements
FLH	Federal Lands Highway
FOS	Factor of Safety
FRA	Federal Railroad Administration
GBR	Geotechnical Baseline Report
GBR-B	Geotechnical Baseline Report for Bidding
GBR-C	Geotechnical Baseline Report for Construction
GDR	Geotechnical Data Report
GEDR	Geotechnical Engineering Design Report
GIP	Geotechnical Investigation Plan
GTGM	Geotechnical Technical Guidance Manual
HST	High-Speed Train
IGM	Intermediate Geomaterials
LOTB	Logs of Test Borings
LRFD	Load and Resistance Factor Design
MASW	Multichannel Array of Surface Wave
M-O	Mononobe-Okabe
MCE	Maximum Considered Earthquake
NEHRP	National Earthquake Hazards Reduction Program
OBE	Operating Basis Earthquake
OCS	Overhead Contact System
PDA	Pile Driving Analyzer
PDDM	Project Development Design Manual
PGA	Peak Ground Acceleration
RAM	Reliability, Availability, and Maintainability
SASW	Spectral Analysis of Surface Waves

SSI	Soil-Structure Interaction
USGS	United States Geological Survey



## 10 Geotechnical

### 10.1 Scope

This chapter provides guidance, geotechnical criteria, and requirements for the geotechnical engineering design for earthwork, embankments, and bridges/aerial structures, abutments, underground structures, and culverts for the California High-Speed Train (HST) trackway.

### 10.2 Regulations, Codes, Standards, and Guidelines

Elements of HST infrastructure, based on their importance to HST, shall be classified as Primary Type 1, Primary Type 2, Complex, and Secondary. Definitions of these elements can be found in the *Seismic* chapter. Design of geotechnical work specified in this chapter applies to Primary Type 1, Primary Type 2, and Complex structures, while the Secondary structures shall be subject to the requirements of the governing local jurisdiction.

Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards. Geotechnical design work for Primary Type 1, Primary Type 2, and Complex Structures shall be in accordance with AASHTO LRFD BDS with California Amendments, these geotechnical design criteria, and the requirements of the following standards and guidelines. Use of the LRFD methodologies in some earthquake engineering and geotechnical engineering areas requires careful examination for applicability. However, any variation to the LRFD methodologies is subject to the Design Variance Process as noted in the *General* chapter.

#### Standards

- American Association of State Highway and Transportation Officials (AASHTO)
  - AASHTO Standard Specifications for Highway Bridges
  - AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals
  - AASHTO Guide Specifications for Design and Construction of Segmental Concrete bridges
  - AASHTO Guide Specifications for Thermal Effects in Concrete Bridge Superstructures
- California Department of Transportation (Caltrans)
  - Caltrans Bridge Design Specification – AASHTO LRFD Bridge Design Specifications and California Amendments (to the AASHTO LRFD Bridge Design Specifications), hereafter referred to as “AASHTO LRFD BDS with California Amendments”
  - Caltrans Seismic Design Criteria (CSDC)
  - Soil and Rock Logging, Classification, and Presentation Manual, June 2010

- California Building Code (CBC)
  - International Union of Railways (UIC) Code 719R Earthwork and Trackbed for Rail Lines (2008)
- Guidelines**
- American Society of Civil Engineers (ASCE), Geotechnical Baseline Reports for Construction – Suggested Guidelines, prepared by Essex, 2007
  - Federal Highway Administration (FHWA) Guidelines
    - FHWA Project Development and Design Manual (PDDM), 2011
    - FHWA Geotechnical Technical Guidance Manual (GTGM), 2007
    - Geophysical Methods - Technical Manual (Application of Geophysical Methods to Highway Related Problems, cooperatively with Blackhawk Geosciences), DTFH68-02-P-00083, 2003
    - Soils and Foundations Workshop, NHI Course No. 132012, Volumes I and II FHWA-NHI-06-088, and FHWA-NHI-06-089, 2006
    - Subsurface Investigations – Geotechnical Site Characterization, NHI Course Manual No. 132031, FHWA-NHI-01-031, 2002
    - Evaluation of Soil and Rock Properties, Geotechnical Engineering Circular No. 5, FHWA-IF-02-034, 2002
    - FHWA Drilled Shaft Construction Procedures and LRFD Design Methods, FHWA-NHI-10-016
    - Technical Manual for Design and Construction of Road Tunnels – Civil Elements, FHWA-NHI-10-034
    - FHWA Drilled Shafts: Construction and Procedures and Design Methods, FHWA-IF-99-025
    - FHWA Mechanically Stabilized Earth Walls and Reinforced Soil Slope Design and Construction Guidelines, FHWA-NHI-00-043
    - FHWA Earth Retaining Structures Manual, FHWA-NHI-99-025
    - FHWA Soil Slope and Embankment Designs, FHWA-NHI-01-026
    - FHWA Rock Slopes Reference Manual, FHWA-HI-99-007
    - FHWA Geosynthetics Design and Construction Guidelines, FHWA HI-95-038
    - FHWA Geotechnical Instrumentation, FHWA-HI-98-034
  - National Cooperative Highway Research Program (NCHRP) Report 611; Seismic Analysis and Design of Retaining Walls, Buried Structures, Slopes, and Embankments, Transportation Research Board

### 10.3 General Geotechnical Design Requirements

The geotechnical criteria were developed from operational requirements outlined in the Federal Railroad Administration (FRA) Class 9 Track Safety Standards. While these HST criteria are developed prior to the development of final track and systems design, the design approach is intended to comply with FRA Class 9 standards to result in appropriate infrastructure facilities for the HST trackway construction.

Each Geotechnical Designer shall be a licensed Geotechnical Engineer in the State of California with a minimum of 15 years of design and practical field experience in geotechnical and seismic engineering. For specialized structures, such as mined tunnels and aerial structures, additional experience requirements apply as described below.

Geotechnical Designers for underground structures (tunnels and trenches) shall have served as the geotechnical engineer of record for the design of at least 3 similar structures that have been constructed, each exceeding 20 feet in width or diameter and 5,000 feet in length. Geotechnical Designers for aerial structures shall have served as the geotechnical engineer of record for the design of at least 3 rail or highway bridge projects that have been constructed, each exceeding 1,000 feet in length.

The Geotechnical Designers shall conduct work necessary to perform supplemental geotechnical investigation and complete the design for the California High-Speed Train Project (CHSTP). The Geotechnical Designers shall develop geotechnical designs and construction excavation support systems in accordance with the requirements set forth in this chapter. Elements of the work include, but are not limited to, the following:

- Review of existing geotechnical information, including but not limited to the Geotechnical Baseline Report for Bidding (GBR-B), the preliminary Geotechnical Data Report (GDR), and the preliminary Geotechnical Engineering Design Report (GEDR).
- Evaluate the requirements of the work and perform additional geotechnical explorations, laboratory testing, and geotechnical analyses to supplement the existing data in support of final design and proposed means and method of construction.
- Perform additional field testing to characterize the in situ shear wave velocity ( $V_{s30}$ ) profile and dynamic soil properties at Special Sites<sup>1</sup> along the project alignment. Shear wave velocity profiles and dynamic soil properties shall be obtained at the Special Sites<sup>1</sup>. Measurements of shear wave velocity,  $V_{s30}$ , shall be conducted via downhole PS seismic

<sup>1</sup> Special Sites are defined as locations subject to liquefaction or strong nonlinear site effects such as at water crossings including rivers, creeks, canals, etc., sites underlain by NEHRP site categories E and F, sites which are technically classified as Complex due to Complex Geological Conditions per the Seismic Chapter, and locations with underground structures such as tunnels, below grade stations, cut-and-cover structures, trenches, and below grade U-walls.

suspension logging to a maximum of 500 feet or until reference rock material (minimum  $V_{s30}$  of 2,500 ft/s) is encountered, whichever occurs first.

- Perform site specific site response analyses to develop final time series at the Special Sites using the site specific geotechnical data and using the response spectra and corresponding spectrally matched time series of the referenced rock outcrop motions developed by the Authority. Use the results for final design of the infrastructure systems at the Special Sites.
- Prepare final Geotechnical Data Report (GDR) and Geotechnical Engineering Design Report (GEDR), and Geotechnical Baseline Report for Construction (GBR-C) as stated herein.
- Perform professional engineering support for the final structural design and design of temporary support works.
- Perform construction inspection and provide construction support to the Contractor related to geotechnical related works

The Geotechnical Designers shall prepare the Geotechnical Reports in accordance with the criteria set forth in this chapter. Geotechnical work shall be conducted under the direction of the Geotechnical Designers. Geotechnical reports, calculations, and drawings shall be signed and stamped by the Geotechnical Designers. In addition, the Geotechnical Designers shall be responsible for the following:

- Overseeing geotechnical design and construction support of bridges, embankments, retaining walls, roadways, tunnels, underground stations, roadways, and other geotechnical related facilities
- Determining if more stringent criteria are appropriate and/or required by applicable codes or manuals (in addition to those listed). In situations where conflicts arise between these criteria and other applicable codes or manuals, the more stringent criteria will be used.
- Approving construction under their design control

Land subsidence is well documented in areas along portions of the proposed alignment. Consequently, the design and construction of the high-speed rail facilities shall consider the ongoing land subsidence conditions. Land subsidence shall be studied, analyzed, monitored, and mitigated to reduce its effect on the high-speed operations, passenger comfort, and long term serviceability. Groundwater pumping has been the primary factor responsible for land subsidence. Although halting or limiting the water pumping is an effective mitigation, it shall not be considered as a solution for this contract.

Refer to the scope of work in the contract documents regarding additional requirements for addressing land subsidence.



## 10.4 Subsurface Investigation and Data Analysis

The Geotechnical Designers shall interpret the existing geotechnical data and perform subsurface investigations, field and laboratory testing, fault displacement mapping, and rock slope mapping as may be necessary to satisfy themselves as to the nature of the following:

- Soil, rock, groundwater, and subsurface conditions including gassy or potentially gassy ground, if applicable
- The geologic and seismic hazards (e.g., faults, landslides, rockfall, debris flows, liquefaction, soft ground, swelling or collapsible soil, or otherwise unstable soil) within and in the immediate vicinity of the project site
- Variations in the subsurface and groundwater conditions across the project site and adjacent areas that can potentially impact construction activities or train operations (e.g., ground movements or high-speed train induced ground vibration)

*Appendix 10.A – Guidelines for Geotechnical Investigations* provides guidance for the expected level, frequency, and reporting of geotechnical investigation envisioned as necessary to fully satisfy the requirements of the Project.

Interpretations and necessary investigations and testing shall consider the methods of construction, critical combinations of loading, and other site-specific factors (e.g., drainage, issues, proximity of the alignment and structures to adjacent structures) that may impact final design, construction and operations.

In addition, the Geotechnical Designers shall undertake investigations and data review to assess the potential for adverse conditions posed by areas of soil and/or groundwater contamination, chemically aggressive soil conditions (e.g., high sulfate content), corrosive ground, and regions that may be impacted by stray electrical currents.

For locations where structures containing steel and/or concrete are intended, a site specific corrosion study shall be performed to evaluate corrosive characteristics of soil and groundwater that have negative impact to concrete and steel. In addition to stray currents, the ability of soils to conduct electricity may have a significant impact on the corrosion of buried structures and the design of grounding systems. Accordingly, subsurface investigations shall include conducting appropriate investigations to obtain soil resistivity values. The following criteria are required:

- Soil resistivity readings shall be obtained to evaluate the electric conduction potential of soils at (1) each traction power facility site (supply/paralleling/switching station), which are to be spaced at approximately 5-mile intervals, (2) major structures, such as aerial structures and freeway overpass bridges, and (3) tunnel portal areas.
- Where there is an absence of major structures between traction power facilities, soil resistivity readings shall be obtained to evaluate the electric conduction potential of soils at approximately the midpoint between facilities.

- Where significant differences in soil resistivity values are identified at adjacent locations, additional readings shall be obtained so that an adequate basis is developed for the grounding design.
  - Resistivity measurements shall be obtained in accordance with the Institute of Electrical and Electronics Engineers (IEEE) Standard 81-1983 – IEEE Guide for Measuring Earth Resistivity using the four-point method for determining soil resistivity. IEEE states that the four-point method is more accurate than the two-point method.
- A Geotechnical Investigation Plan (GIP) shall be prepared by the Geotechnical Designer to supplement and update existing subsurface information available for final design of the structures. The investigation shall follow *Appendix 10.A – Guidelines for Geotechnical Investigations*.
- The plan shall include the criteria or rationale used in developing the plan and shall identify locations of explorations, together with their depths, sampling intervals, and a description of both the field methods and laboratory testing program utilized. In addition, the plan shall include a detailed description regarding the investigative methods which shall be optimized to make best use of cone penetration testing, soil/rock borings, monitoring wells and piezometers to efficiently characterize the subsurface conditions along the project alignment. This plan shall be submitted to the Authority for review and acceptance prior to commencing geotechnical investigations.
- The requirements for the field and laboratory investigations to be performed by the Geotechnical Designers shall be the following:
- Perform additional subsurface investigations to supplement existing geotechnical data for the design of elements along the proposed alignment. Specific guidance on subsurface investigation methods that shall be considered for this project is presented in *Appendix 10.A – Guidelines for Geotechnical Investigations* of this chapter.
  - Supervision – Boring and in situ testing and inspection, and laboratory classification and testing, shall be performed by a trained geologist or geotechnical engineer under the supervision of a geotechnical engineer or an engineering geologist licensed in California with a minimum of 10 years of experience in the performance and supervision of geotechnical investigations.
  - Location and Ground Surface Elevation – The Geotechnical Designer shall determine the coordinate location and ground surface elevation for each boring and field investigation site, and shall show the coordinates, and station and offset, and the elevation for each individual boring log or investigation record. Coordinates, stationing and offsets shall be referenced to the Project horizontal control system. Elevations shall be referenced to the CHSTP datum.
  - Laboratories shall be Caltrans certified and equipment used for field testing shall have documentation of calibration within the last year.

- Information obtained using a pocket penetrometer or field torvane shall not be relied upon as the primary means for development of geotechnical parameters.
- Soil samples and rock cores shall be kept and maintained in a readily accessible storage facility within 100 miles of the project site during construction. No disposal of the soil samples and rock cores shall be made until it is instructed by the Authority after completion of the project. These samples shall be available for viewing by the Authority or its designees within 2 business days of a request. Untested samples shall not be disposed of or released to a third party at any time without the written authorization of the Authority.
- For rock slopes, tunnels through rock, and rock excavations at the portals and substructures, oriented cores with down hole camera logging shall be performed to obtain structural geological parameters such as orientations (dip/strike), roughness, infilling, spacing, etc., of structural discontinuities (bedding, joints, fault zones, shear zones, breccias, etc.). At a minimum, detailed geologic information shall be collected to a depth of one tunnel diameter (D) below the invert of the tunnel.
- Borehole Site Cleanup – Backfilling of borings, test pits, Cone Penetration Tests (CPTs), rotasonic holes, wells, and probe holes shall be performed in accordance with the provisions of applicable local, state, or federal laws and regulations, and permit requirements. Restoration of pavement shall be performed in accordance with street use permit requirements.
- Test holes shall be backfilled in a manner that ensures against subsequent settlement or heave of the backfill. Upon completion of field investigations, surplus materials, temporary structures, and debris resulting from the drilling work performed on land and in water shall be removed and disposed of from the site.
- Final boring and rock core logs shall be prepared using gINT Geotechnical and Geo-environmental software.
- No geologic or hydrogeologic data or seismic hazard evaluation results shall be released to a third party without the written approval of the Authority.

## 10.5 Geotechnical Reports

Geotechnical reports including the GDR, GEDR, and GBR-C shall be prepared, signed, and stamped by the Geotechnical Designer. Preliminary documents such as GDR, GEDR, and the GBR-B have been provided to the Contractor to support the bidding process. The preliminary GDR presents the existing geotechnical data for the project. The preliminary GEDR, if available, presents the preliminary geotechnical design elements and analyses for the project and is based on the data included in the GDR. The GBR-B documents baseline subsurface conditions anticipated for the purpose of the bidding process (hence the suffix “B”). The Contractor will conduct additional subsurface investigations and develop the final design and construction documents. These final geotechnical reports include the Final GDR, containing all data collected for the project (preliminary data as well as that collected by the geotechnical designer); the Final

GEDR that documents the design assumptions, design process, geotechnical analyses and their results, and final design recommendations; and the GBR-C that will update the GBR-B based on new information obtained during the investigation period.

### **10.5.1 Geotechnical Data Report (GDR)**

Geotechnical investigation of the subsurface conditions, including laboratory and field testing, shall be performed to describe the geologic features of the project area. A summary of geotechnical data and findings, including a summary of the preliminary design level investigation results along with the results of the final field subsurface investigations including mappings, if any, and laboratory testing data, shall be prepared as the GDR. The GDR shall contain factual information that has been gathered in the preliminary design of subsurface investigations and the final subsurface investigations. The GDR shall contain the following information:

- Project description
- Description of desk study results gathered from existing available data
- Description and discussion of the site exploration program
- Locations and results of subsurface investigations (borings, CPTs, Geophysical Testing, etc.) including photo documentation of core hole core samples and investigation sites
- A detailed description of geological and subsurface conditions (including a description of site stratigraphy, geologic hazards, and groundwater conditions)
- Rock parameters including orientation and nature of jointing, bedding, etc.
- Description of surface water (springs, streams, etc.) and groundwater conditions
- Seismic setting including location of nearby faults
- Boring and rock core logs with soil descriptions and field test results
- Groundwater level measurements from monitoring wells and piezometers
- Vibration propagation characteristics of soils including surface waves such as Rayleigh waves
- Ground movement measurements from inclinometers and others such as Global Positioning System (GPS), Interferometric Synthetic Aperture Radar (InSAR) methods, etc.
- Description and results of field/in situ testing and rock mapping
- Description and results of laboratory tests
- Material properties
- Chloride content, acidity (pH value) and sulfate content of the surface water, groundwater, and soils
- Statistical analysis for test results per geotechnical layer

- 1 • Results of field and laboratory testing
- 2 • Logs of borings, CPTs, seismic cones, trenches, and other site investigations
- 3 • Logs of geophysical testing including MASW, SASW, downhole PS seismic suspension
- 4 logging, cross-hole logging, and others, etc.
- 5 • Standards for laboratory and field testing

### 10.5.2 Geotechnical Engineering Design Report (GEDR)<sup>1</sup>

6 The findings and evaluations of subsurface data along with geotechnical and foundation  
7 analyses and design recommendations shall be documented in the form of a GEDR<sup>1</sup>, which  
8 serves as the basis for final geotechnical design. The GEDR shall include, but is not limited to,  
9 the following:

- 10 • Project description including surface conditions and current use
- 11 • Regional and site geology
- 12 • Regional and site seismicity
- 13 • A summary of subsurface explorations, including field and laboratory testing, and
- 14 locations (map with coordinates) of borings, wells, and other in-situ testing sites
- 15 • Detailed description of geological and subsurface conditions (including a description of site
- 16 stratigraphy) along with geological profile and cross-sections
- 17 • Seismic design criteria including design earthquakes (Operating Basis Earthquake [OBE]
- 18 and Maximum Considered Earthquake [MCE]), magnitudes, and peak ground and bedrock
- 19 accelerations, where applicable. Refer to the *Seismic* chapter for definitions of the design
- 20 earthquakes
- 21 • Evaluation of seismic and geologic hazards including, but not limited to,
- 22 liquefaction/lateral spreading, pre-historic landsliding and land subsidence due to long-
- 23 term pumping of groundwater or withdrawal of petroleum and gas, if any
- 24 • Subsurface material properties
- 25 • Data and complete discussions of geotechnical analyses, designs, and studies
- 26 • Recommended design parameters for soil and rock types
- 27 • Conclusions and recommendations for foundation types for structures (with appropriate
- 28 design parameters), soil and rock cut slopes, fill embankments, retaining walls,
- 29 requirements for backfill materials

---

<sup>1</sup> GEDR is equivalent to design memoranda reference in Essex (2007) and the GBR framework

- 1 • Lateral earth pressures to be used in designing temporary and permanent excavation
- 2 support structures
- 3 • Seismic earth pressure design considerations for embankments and structures
- 4 • Results of site specific site response analysis used for design of deep foundations and
- 5 underground structures
- 6 • Potential groundwater impact and dewatering requirements
- 7 • Instrumentation and monitoring requirements during and after construction
- 8 • Potential settlement/horizontal deflection problems and mitigation measures
- 9 • Potential soil and rock slope and retaining wall stability problems and analysis results
- 10 along with mitigation measures
- 11 • Impact of dynamic train loading on the ballasted tracks and/or non-ballasted tracks in
- 12 terms of residual settlements on the foundation systems and subgrade soils
- 13 • Evaluate the impact of the ground vibration induced by high-speed train operations (i.e.,
- 14 Rayleigh wave) to proposed and existing infrastructures such as bridges, embankments,
- 15 retaining walls, and underground structures, and carry out mitigations as necessary
- 16 • Anticipated ground behavior and categorization of ground during excavation, filling and
- 17 foundation, and retaining structure construction; particular attention shall be paid to
- 18 identifying and mitigating impacts due to excavating near the groundwater table.
- 19 • Blasting and excavation methods as related to the design of cut slopes, including a
- 20 discussion of blast design parameters that are related to the geotechnical conditions
- 21 • Consideration for, discussion of, and rationale for protection of existing structures, water
- 22 bodies, and environmentally or historically sensitive areas
- 23 • Discussion on induced vibration and noise from the selected construction equipment and
- 24 procedures and the effects on adjacent structures and landowners
- 25 • Discussion on studies to evaluate and assess the impact of land subsidence to the
- 26 performance of the HSR systems
- 27 • Evaluation of in situ stress conditions (if applicable)
- 28 • Evaluation of load bearing capacity of the encountered soil/rock types
- 29 • Stability analyses in agreement with applicable codes and standards
- 30 • Evaluation, if excavated material can be used as fill/backfill material
- 31 • Geotechnical recommendations including earthwork/sitework; ground stabilization for
- 32 foundation support; stabilization of unstable soil and rock slopes; mitigation measures to
- 33 reduce land subsidence; and foundation options for aerial structures, underground
- 34 structures, retaining walls, hydraulic structures, and other structures

- Construction considerations given to issues related to construction staging, shoring needs, potential installation difficulties, temporary slopes, earthwork constructability issues, dewatering, etc.
- Long-term and construction monitoring and evaluation needs

### 10.5.3 Geotechnical Baseline Report for Construction (GBR-C)

A Geotechnical Baseline Report for Construction (GBR-C) shall be developed, upon completion of subsurface investigations, to summarize design assumptions and final design results developed in the GEDR, and also to document interpretations and baseline conditions anticipated for Construction. As part of the final design and construction planning process, the Geotechnical Designer shall interpret the various baselines expressed in the GBR-B, and consider those baselines in the development of the design and construction approaches. Based on the data collected and presented in the final GDR and the design process documented in the GEDR, Contractor will further develop and finalize the GBR-C by updating the GBR-B accordingly. An electronic version of the GBR-B shall be used to record modifications or clarifications in the “track changes” mode using a computerized word processing software program. In its completed form, the GBR-C will document the physical baselines established by the Authority and the Contractor as well as the behavioral baselines described by the Contractor consistent with its design approach, equipment, means and methods.

The GBR-C shall include, but is not limited to, the elements listed in the “Geotechnical Baseline Reports for Construction – Suggested Guidelines” prepared by ASCE (Essex, 2007). The GBR-C shall be limited to interpretive discussion and baseline statements, and shall make reference to information obtained in the Geotechnical Data Report (GDR), Geotechnical Baseline Report for Bidding (GBR-B), drawings, and specifications.

## 10.6 Bridge, Aerial Structure, and Grade Separation Foundations

Foundation design shall be based on project-specific information developed for the location(s) and foundation type planned. It shall be carried out in accordance with AASHTO LRFD BDS with California Amendments or other Standards or Codes referred to in Section 10.2 of this chapter provided that these are comparable and equivalent to or complement AASHTO LRFD BDS with California Amendments, and as described below. Some Primary Type 2 structures may also be subject to design criteria of local jurisdictions (e.g., UPRR, Metrolink, Caltrans, etc.). For Primary Type 2 structures that are subject to the jurisdiction of local authorities, soil parameters, such as design bearing and frictional values for foundations, shall not exceed the limits given by the applicable codes, except for deviations as provided for in the codes.

### 10.6.1 Geotechnical Data

- 1 The type and depth of foundations shall be determined from available geotechnical data and
- 2 additional geotechnical investigations at the locations of the foundations. Use of assumed
- 3 values shall not be allowed for final design.
- 4 Foundations to be constructed in rivers, canals, and creeks shall take into consideration flood
- 5 levels and maximum scour depth as determined by the *Drainage* chapter.

### 10.6.2 Load Modifiers, Load Factors, Load Combinations, and Resistance Factors

- 6 The design shall be in accordance with the concepts and general methodology of AASHTO
- 7 LRFD BDS with California Amendments. Refer to the *Structures* chapter for load factors and
- 8 load combinations. Load resistance factors for walls and shafts shall be in accordance with
- 9 AASHTO LRFD BDS with California Amendments.

### 10.6.3 Allowable Foundation Settlements for Primary Type 1 Structures

- 10 Requirements for foundation settlement performance presented herein shall supplement to (or
- 11 apply in addition to) the criteria indicated in AASHTO LRFD BDS with California
- 12 Amendments. Foundation settlements shall be calculated from the Service 1 load combination
- 13 plus any irreversible settlements resulting from the post-earthquake effects of Operating Basis
- 14 Earthquake (OBE) such as those resulting from liquefaction induced down drag, seismic
- 15 compaction, etc. The settlements include components of short-term and long-term settlements
- 16 as well as elastic (reversible) and plastic deformation (irreversible) from dynamic train loading,
- 17 and shall not exceed the values shown in Table 10-1. Transient and temperature loads in the
- 18 Service 1 load combination shall be used to calculate the short- term settlements. Traction and
- 19 braking forces need not be considered.
- 20 Compliance with the settlement limits in Table 10-1 shall be applicable to settlements that occur
- 21 after completion of construction and installation of all superimposed dead loads including the
- 22 trackwork. For approach embankments, the settlements shall be measured at the top of the
- 23 embankment.
- 24 Differential settlement limits in Table 10-1 are required to control the long term changes of track
- 25 geometry within track maintainable tolerances.



**Table 10-1: Maximum Allowable Settlement Limits <sup>(4),(5)</sup> for Service 1 and OBE Load Cases**

Settlement Criteria	Non-Ballasted Track	Ballasted Track
Differential Settlement Between Adjacent Structure Supports <sup>(1), (5)</sup>	$\leq L/1500$ (L = smaller span in inches), but no greater than 3/4 inch	$\leq L/900$ (L=smaller span in inches), but no greater than 1-1/4 inch
Differential Settlement Between Abutment and Approach Embankment <sup>(2)</sup>	$\leq 1/1000$ , but no greater than 3/8 inch	$\leq 1/500$ , but no greater than 3/4 inch
Differential Settlement Between Abutment and Tunnel Portal	$\leq 1/1000$ , but no greater than 3/8 inch	N/A <sup>(3)</sup>
Uniform Settlement at Structure Supports	$\leq 3/4$ inch	$\leq 1-1/4$ inch

**Notes:**

<sup>(1)</sup> The additional forces imposed on the structural system by differential settlements shall be calculated and considered as part of dead load in the design (refer to settlement effects (SE) in Structural chapter)

<sup>(2)</sup> Geostructures shall be instrumented and monitored for a period of at least 12 months following completion of the structure. The Geotechnical Designer shall demonstrate future compliance with the residual settlements (i.e., defined as settlements which are the sum of the remaining native foundation settlement and embankment consolidation settlement estimated to occur after 12 months of completion of embankment construction plus elastic and plastic deformations from dynamic train loading) by comparison of the monitored data and predicted settlement.

<sup>(3)</sup> Not applicable based on the assumption that ballasted track will not be used for tunnels.

<sup>(4)</sup> The settlements calculated from the Service 1 load combination plus any irreversible settlements resulting from the post effects of OBE (such as those resulting from post-liquefaction down drag, seismic compaction, etc.). For approach embankments and aerial structures, the Service 1 settlement limits and OBE load combinations are applicable to settlements that occur after completion of construction.

<sup>(5)</sup> For special conditions, such as a straddle bent adjacent to a single column bent, the allowable Differential Settlement between Adjacent Structure Supports as noted in this table will be reduced by the differential vertical deformations of substructures. Refer to the *Structures* chapter for information on these additional requirements.

Refer to the *Structures* chapter's section on Track-Structure Interaction for additional performance requirements for allowable deformations for the track.

No specific settlement limits are required for the Extreme Event Maximum Considered Earthquake (MCE) loading case, except that the structure shall not collapse and that foundation elements are capacity protected in accordance with the *Seismic* chapter.

#### 10.6.4 Bridge, Aerial Structure, and Grade Separation Foundation Types

Bridge, aerial structure, and grade separation foundations shall be either shallow or deep foundations, depending upon the site specific conditions.

##### 10.6.4.1 Shallow Foundations

Shallow foundations shall be spread footings, combined footings, or mat foundations. They shall be used where there is competent bearing layer near the surface, no highly compressible layers below, and calculated settlements are within the allowable limits outlined in this chapter.

Geotechnical design of abutment and bent/pier shallow foundations shall be carried out in accordance with AASHTO LRFD BDS with California Amendments Articles 10 and 11, and as supplemented in this chapter. Unless otherwise specified, refer to the *Structures* chapter for LRFD load factors and load combinations.

Geotechnical design of retaining wall shallow foundations shall be carried out in accordance with Section 10.8 of this chapter, AASHTO LRFD BDS with California Amendments Articles 10 and 11, and as supplemented in this chapter. Refer to the *Structures* chapter for LRFD load factors and load combinations and this chapter for additional 3 service load conditions.

#### A. Bearing of Soil/Rock

The nominal bearing resistance for shallow foundations shall be determined based on existing available geotechnical data and the geotechnical subsurface conditions of the foundation soil or rock. For all types of shallow soil foundations, the factored uniform bearing stress at the strength limit state, based on the effective footing dimension method in accordance with AASHTO LRFD BDS with California Amendments Articles 10.6.1.3 and 11.6.3.2, shall not be greater than the factored nominal bearing resistance. For all types of shallow rock foundations, the factored bearing stress at the strength limit state, based on the linearly distributed pressure method in accordance with AASHTO LRFD BDS with California Amendments Article 11.6.3.2, shall not be greater than the factored nominal bearing resistance.

For abutment shallow soil/rock foundations, the bearing stress at the Service 1 limit state, based on the linearly distributed pressure method, shall not be greater than the site specific nominal bearing resistance according to AASHTO LRFD BDS with California Amendments.

#### B. Stability

1. Under normal loading<sup>1</sup> conditions, the location of the resultant of reaction forces shall be within the middle one-third of the foundation width.
2. Under exceptional loads<sup>2</sup>, the location of the resultant of reaction forces shall be within the middle one-half of the foundation width.
3. Under ultimate loads<sup>3</sup>, the location of the resultant of the reaction forces shall be within the middle two-thirds of the foundation width. Over-strength plastic hinge demands from MCE shall apply.

---

<sup>1</sup> Normal Loads =  $DC + DW + L + CF + E + WA + LF_2 + 0.6TU$

<sup>2</sup> Exceptional Loads =  $DC + DW + L_1 + CF_1 + LF_1 + E + WA + WS + WL_1$   
=  $DC + DW + L_1 + CF_1 + LF_1 + E + WA + OBE$

<sup>3</sup> Ultimate Loads =  $DC + DW + E + WA$  (buoyancy only) + MCE

For loading definitions of cases 1, 2, and 3 noted above, refer to the *Structures* chapter and as summarized below:

DC	=	Dead load of structural components and permanent attachments
DW	=	Dead load of non-structural components and non-permanent attachments
CF	=	Centrifugal force (multiple trains)
CF <sub>1</sub>	=	Centrifugal force (single train)
E	=	Earth pressures, including EV, EH, and ES
L	=	Multiple trains of LLRR or LLV, whichever governs
L1	=	Single train of LLRR or LLV, whichever governs
LF1	=	Braking forces (apply braking to 1 train) for LLV loading
LF2	=	Acceleration and braking forces (apply braking to 1 train, and acceleration to the other train) for LLV loading
MCE	=	Maximum Considered Earthquake (refer to the <i>Seismic</i> chapter)
OBE	=	Operating Basis Earthquake (refer to the <i>Seismic</i> chapter)
TU	=	Uniform temperature effects
WA	=	Water loads, including stream flow and buoyancy,
WS	=	Wind load on structure
WL <sub>1</sub>	=	Wind load on 1 train

For Primary Type 1 and Complex Structures bridge, aerial structures, or grade separations, a design strategy based on transient foundation uplift or foundation rocking as described in the *Seismic* chapter is not permitted.

#### C. Allowable Foundation Settlements

Settlements and differential settlements of shallow foundations under the service limit state shall not exceed those specified in Table 10-1. Refer to the *Structures* chapter for service limit state load combinations.

#### D. Benching

Where footings are to be constructed on inclined surfaces with slopes greater than 1 Vertical: 10 Horizontal (1V:10H), the surface shall be benched (Section 10.9.4).

#### E. Bottom of Footings

The depth of footings shall be determined based on the characteristics of the foundation materials and in consideration of the possibility of undermining. Footings not exposed to the action of a stream or river current shall be founded such that the top of the footing has a minimum depth of 3 feet below the lowest adjacent finished grade. In areas susceptible to frost development, footings shall be placed on a firm foundation below the frost level, or on a firm foundation that is made frost resistant by over excavation of frost-susceptible material below the frost line and replaced with material that is not frost susceptible, or such that the top of the footing is at least 3 feet below the surface, whichever is deeper. In locations where expansive or

1 collapsible soils are present, deleterious soils should be over excavated and replaced with  
2 suitable foundation material or footings shall be placed at a depth sufficient to eliminate  
3 impacts from swelling or collapsible soils.

4 In cases where spread footings are used in streams and rivers, the following additional design  
5 requirements shall be considered:

- 6 • **Footings on Soils** – The bottom of footings on soils shall be set at least 10 feet below the  
7 river bottom unless otherwise stated in this chapter. The potential shift of the stream or  
8 river channel shall be considered when determining the channel bottom. The top of  
9 footings shall be below the total scour depth determined for the 100-year flood, and the  
10 bottom of footings shall be below the total depth determined for the 500-year flood.
- 11 • **Footings on Rock** – The bottom of footings shall be at least 3 feet below the surface of scour-  
12 resistant rock (i.e., rock not subject to scour attack) with the top of the footings at least  
13 below the rock surface.
- 14 • **Footings on Erodible Rock** – The foundation design of footings on erodible rock shall be  
15 based on the following:
  - 16 – Assess weathered rock or other potentially erodible rock formations for scour.
  - 17 – An analysis of intact rock cores, including rock quality designations and local geology,  
18 hydraulic data, and anticipated structure life.

#### **10.6.4.2 Deep Foundations**

19 Deep foundations shall be bored or driven piles, or Cast-in-Drilled-Hole (CIDH) piles (also  
20 known as drilled shafts). These shall be used when shallow foundations cannot be used to carry  
21 the applied loads safely and economically while also meeting the required settlement criteria.  
22 Alternative deep foundation systems such as micropiles, rammed aggregate piers, augercast  
23 piles, and propriety systems shall not be allowed for support of bridges, aerial structures, or  
24 grade separations.

25 The top of deep foundations, including top of drilled shafts or pile caps where multiple shafts  
26 or piles are employed, shall be a minimum of 3 feet below the lowest adjacent finished grade.

##### **A. Ultimate Pile Load Capacities**

27 The ultimate pile axial capacity shall be determined based on appropriate values of skin friction  
28 plus end bearing developed from the results of site-specific geotechnical investigations, and  
29 shall be verified by test piles and load testing as described herein.

30 The adequacy of the bearing capacity of the drilled shafts and bore or driven piles shall be  
31 verified regarding (1) the factual soil parameters at the respective locations and depth of the  
32 foundations, and (2) the groundwater table. Refer to Section 10.6.4.3 on Test Piles and Load  
33 Tests for verification of assumptions for deep foundation design.

Pile foundations shall be designed in such a way that plastic hinges are not located in the piles or drilled shafts. If below-ground plastic hinging of the piles or drilled shafts is unavoidable, then a design variance shall be submitted per the *General* chapter. In cases where plastic hinges are a necessary part of the design, the design shall include an inspection protocol that does not require excavation to inspect the pile condition.

#### B. Settlements

Settlements of deep foundations shall not exceed those specified in Table 10-1. Design settlement values shall be verified with appropriate calculations in the design process. Piles/drilled shafts and connections to pile caps shall be checked for the estimated deflection from lateral loads.

For deep foundations where soils exhibiting potential liquefaction and lateral spreading in an earthquake are present, ground improvement may be considered to improve the foundation stability. Where ground improvement measures alone are prohibitively costly and impractical, consideration shall be given to designing a combined system composed of improved ground and strengthening of the foundation.

#### C. Lateral Load Capacity

Piles and drilled shafts shall be designed to adequately resist lateral loads transferred to them from the structure without exceeding the deformation which creates a stress outside the allowable stress range of the structure or overstressing the foundation elements. The lateral load resistance of the individual and groups of piles and drilled shafts shall be analyzed. The analysis shall consider nonlinear soil pressure-displacement relationships, soil-structure interaction, group action, groundwater, and static and dynamic load conditions. The performance of the piles and drilled shafts shall include determination of settlements and horizontal deformations, rotation, axial loads, shear, and bending moment for the foundation elements.

The lateral load capacity of piles and drilled shafts shall be verified by means of pile load tests in the field as described in Section 10.6.4.3 Test Piles and Load Tests.

#### D. Wave Equation Analyses

The constructability of a pile design and the development of pile driving criteria shall be performed using a Wave Equation Analysis for Piles (WEAP) computer program in accordance with AASHTO Standard Specifications for Highway Bridges. Analysis shall be conducted for hammers and pile types proposed for use and for each bridge foundation. Wave equation analysis shall not be used as the sole basis for determining pile capacity or pile acceptance.

#### E. Pile Group Effects

Generally for piles or drilled shafts constructed in groups, the spacing of pile centers shall not be less than 2.5 times the pile diameters (or pile size). Piles or drilled shafts in any 1 group shall be of the same diameter. Pile group effects shall be considered with regard to the bearing capacity, settlement, and lateral resistance.

Multiple rows of piles/drilled shafts often have less resistance than the sum of the single individual piles/drilled shafts because of pile-soil-pile interactions that take place in the pile group (also called shadowing effect). The shadowing effect can cause the lateral capacity of the pile group being less than the sum of the lateral capacities of the individual piles comprising the group. Consequently, lateral loaded pile groups often will have group efficiencies less than unity, depending on the pile spacing.

Accordingly, when the P-Y method of analysis is used to evaluate a laterally loaded pile group, reduce the values of P by a multiplier ( $P_m$ ) as shown in Table 10-2.

**Table 10-2: Pile Load Modifiers,  $P_m$ , for Multiple Row Shading**

Pile Center-to-Center Spacing (in direction of loading)	Pile Load Modifiers, $P_m$		
	Row 1	Row 2	Row 3 and Higher
3D	0.75	0.55	0.40
5D	1.0	0.85	0.7
7D	1.0	1.0	0.90

#### F. Down Drag (Negative Skin Friction) Effects

The design of piles and drilled shafts shall take into consideration the effect of negative skin friction as induced by dewatering, liquefaction, construction of embankments, or from pile installation methods. When down drag (negative skin friction) is considered, it shall be treated as an addition to the nominal loads.

The nominal pile resistance available to support the down drag and nominal loads shall be estimated by considering only the positive side and tip resistance below the lowest layer contributing to down drag (i.e., neutral plane<sup>1</sup>). The structure shall also be designed to meet settlement limits resulting from down drag and the applied loads and the structural limits resulting from the combination of down drag plus structure loads.

As noted by Fellenius (2004 and 2006), down drag increases the load developed in the pile at the neutral plane, and thus it is a structural capacity issue for pile design. For soil capacity calculations for pile design, the down drag load does not need to be included for most cases because when the pile is punching into the soil, all of the soil deposit resists the downward pile movement at the ultimate pile load. However, down drag loading should be considered when the soil below the neutral plane is subject to creep deformation or creep rupture.

<sup>1</sup> Neutral plane is the location where the downward acting forces are equal to the upward acting forces and where there is no movement between the pile and the soil. At this location, the pile and the soil settle equally.

1 In the case of soil liquefaction, the effects of soil liquefaction in soil above the neutral plane will  
2 be negligible if the dynamic loads do not raise the neutral plane significantly. The soil above the  
3 neutral plane has already loaded the pile downward under working loads. The loss of soil  
4 strength due to liquefaction in soils above the neutral plane does not change this loading, so the  
5 resulting effects are inconsequential. Observations following earthquakes indicate that pile  
6 foundations with their neutral plane well below liquefiable layers do not settle significantly  
7 (although one must always check that the dynamic loading does not push the neutral plane up  
8 into the liquefiable soils and that the bearing soil/rock materials below the neutral plane are  
9 sufficiently stiff and strong to resist dynamic loads). Methods proposed by Fellenius and Siegel  
10 (2008) should be used for evaluating down drag in deep foundations in liquefiable soils. In  
11 developing pile designs, care shall be taken to incorporate appropriate considerations for  
12 designs of drilled and/or driven pile installations. Driven piles develop residual stresses so that  
13 the neutral plane is located at depth under working loads. Drilled shafts transfer load to the soil  
14 from the top-down, so that under working loads the shaft may be providing all required  
15 resistance and the neutral plane is likely at the ground surface. For these cases, drilled shafts  
16 may settle significantly if the soil along its shaft softens significantly, such as due to soil  
17 liquefaction.

18 If measures are proposed for reducing the effect of negative skin friction by means of a slip  
19 coating (e.g., bitumen, geotextile coating, etc.), then consideration shall be given to the long  
20 term value of residual negative skin friction that may develop. Instrumented pile load tests and  
21 dynamic tests shall be undertaken to verify design assumptions and to estimate the available  
22 nominal resistance to withstand the down drag plus the nominal loads.

#### G. Uplift

23 Friction piles may be designed to resist uplift in non-liquefiable soils in accordance with  
24 recommendations in the GEDR. Resistance factors are per AASHTO LRFD BDS with California  
25 Amendments.

26 Should any loading or combination of loadings produce uplift on any pile, the pile to pile cap or  
27 pile to invert slab connection or footing shall be designed to resist uplift forces and other  
28 tension zones caused by the uplift condition.

#### H. Scour

29 For design of deep foundations to support bridges, aerial structures, or grade separations, in  
30 addition to analyses for current site conditions, geotechnical analyses shall be performed  
31 assuming that the soil above the estimated scour line based on the 100-year flood has been  
32 removed and is not available for bearing or lateral support.

### 10.6.4.3 Test Piles and Load Tests

#### A. Indicator Piles/Test Piles, Method Test Shafts, and Load Test Shafts

An adequate number of indicator piles<sup>1</sup>, test piles<sup>2</sup> and method test shafts<sup>3</sup>/load test shafts<sup>4</sup> shall be specified as described below. These shall include advanced test piles/shafts tested to ultimate load to verify design assumptions. The locations and length of the indicator/test piles and method shafts/load test shafts shall be shown on the plans. Indicator piles/test piles and method test shaft/load test shafts shall be located to cover conditions of pile type, sizes, pile/shaft capacity, and soil conditions that will be encountered. Test piles that pass the load test in an undamaged condition may be utilized as production piles in the work. However, method test shafts/load test shafts shall be considered sacrificial and shall not be used as production drilled shafts.

As a minimum, indicator piles, test piles, and method test shafts shall be located according to the following criteria:

- One indicator pile and 1 test pile per 300 driven piles
- One indicator pile and 1 test pile at each pile location separated by a distance of 500 feet or less from other indicator pile/test pile locations
- One method test shaft per 50 drilled shafts
- One method test shaft and 1 load test shaft at each shaft location separated by a distance of 500 feet or less from other method test shaft/load test locations
- Test programs as indicated elsewhere in this chapter

#### B. Load Tests

An appropriate number of deep foundations (driven piles and drilled shafts) shall be tested to ultimate or design loads to verify design assumptions. The location and length of the test deep foundations shall be such that they will cover conditions of types and capacity of the deep foundations as well as soil conditions that will be encountered. These load tests shall be conducted on test piles, method test shafts, and production piles/drilled shafts.

---

<sup>1</sup> Indicator Pile – An individual pile that is tested and observed to determine its behavior during driving.

<sup>2</sup> Test Pile – An individual pile that is tested and observed under static axial compressive or tension load, under lateral load, and under dynamic load tests.

<sup>3</sup> Method Test Shaft – A drilled shaft that is excavated to verify construction methods so that drilling and support of excavation can be evaluated for each site.

<sup>4</sup> Load Test Shaft – A method test shaft with reinforcing placed, any casing or other excavation support system withdrawn, and full concrete placement, followed by gamma ray testing or crosshole sonic testing to verify concrete placement. Method test shaft is then observed under static axial compressive or tension load, under lateral load and under dynamic load tests.



Load tests, if conducted, may be used to increase the resistance factor that is specified in AASHTO Standard Specifications for Highway Bridges. Loading test methods shall be in accordance with the technical specifications applicable to the Contract. In general, static load test capacity of piles shall be tested for compressive, lateral, and tensile loads in accordance with the following ASTM International Standards:

- ASTM D1143, Test Method for Deep Foundations Under Static Axial Compressive Load
- ASTM D3966, Test Method for Deep Foundations Under Lateral Load
- ASTM D3689, Test Method for Deep Foundations Under Static Tensile Load

Alternative load test methods such as Standard Test Method for High Strain Dynamic Testing of Piles (ASTM D4945), Osterberg Cells, Statnamic Load Test (ASTM D7383), etc., may be used. However, these substitutive test methods shall be verified by a conventional loading test of similar piles or drilled shafts.

**Drilled Shafts** – An adequate number of load tests shall be performed in the following sections. These shall include Load Test Shafts tested to ultimate load (load tests) to verify design assumptions. The locations and length of the test shafts shall be shown on the plans. Method test shafts shall be located to cover the shaft type, shaft capacity, and soil conditions that will be encountered.

The Geotechnical Designer shall perform a test shaft program consisting of method test shafts to (1) confirm adequacy of drilling methodology and equipment, and (2) load tests to verify compressive, lateral, and tensile load capacities per site as described below. A location is considered to be a different site if any of the following are true:

- The location is more than 2,000 feet from the method test shaft location where the subsurface conditions are similar.
- The geologic character of the predominantly bearing formation and side resistance is different.
- At each of the main piers of a long span (more than 300 feet) bridge where there are a large number of drilled shafts (greater than 8) in each pier foundation, particularly where the geology may differ on either side of a natural drainage feature.
- The average calibrated resistance (unit load transfer in side resistance or end bearing) in the zone providing the majority of the axial resistance varies from the test location by a factor of 2 or more.
- Sequence, type of construction, and type of shafts are changed.

Once approval has been given to constructing production drilled shafts, no change shall be permitted in the methods and equipment used to construct the satisfactory method test shaft without production of additional method test shafts and written approval of the Geotechnical Designer.

**Driven Piles** – An indicator pile program consisting of indicator piles, test piles, and load tests shall be conducted at each bridge, aerial structure, or grade separation site where driven piles are to be installed. Perform dynamic monitoring using a Pile Driving Analyzer (PDA) on indicator piles conforming to the requirements of ASTM D4945. Perform static load tests to verify compressive, lateral, and tensile loads of individual piles. Indicator piles may be installed as production piles provided PDA test results demonstrate the required capacity is achieved.

To utilize the increase in capacity due to setup in cohesive soils, PDA measurements shall be recorded using Case Pile Wave Analysis Program (CAPWAP) during restrike of piles to evaluate setup. PDA results and revised criteria for the restrike shall be applied to only the piles in that group. Piles shall be re-struck no sooner than 48 hours after installation.

The Engineer inspecting the PDA testing shall have at least 5 years of experience in the monitoring of the driving of piles with PDA and in performing analyses with CAPWAP in similar type of soil conditions. The Engineer performing PDA related analyses shall be a geotechnical engineer licensed in the State of California.

The Geotechnical Designer shall be on-site during PDA testing of initial and restrike pile installation. The Geotechnical Designer shall evaluate data to establish driving criteria for production pile installation.

#### **C. Integrity Testing**

Integrity testing consisting of gamma-gamma or Crosshole Sonic Logging (CSL) or both shall be performed on drilled shafts larger than 24 inches in diameter. Gamma-gamma and CSL tests shall be reviewed and approved by the Geotechnical Designer as well as any remedial measures or repairs that may be needed. In addition, integrity testing is required on driven piles. ASTM D5882, Test Method for Low Strain Impact Integrity Testing shall be performed on piles and drilled shafts 24 inches in diameter or more.

### **10.6.5 Other Design Considerations**

#### **10.6.5.1 Foundation Cover**

Soil cover over top of foundations of piers or abutments shall have a minimum thickness of 3 feet. In addition, for foundations in and adjacent to rivers and creeks, the soil cover over the foundation top for deep foundations shall be at least 3 feet below the maximum estimated scour depth, and at least 10 feet below the river/creek bottom and a minimum of 3 feet below the maximum estimated scour depth for shallow foundations supported by soils.

In urban areas and adjacent to highways, railroads, transit systems, the elevation at the top of the foundations shall be in compliance with the requirements set forth by the local authorities to allow for adequate depth for utilities and surface drains.

### 10.6.5.2 Foundation Rocking

For Primary Type 2 and Secondary structures, if foundation rocking is the preferred design approach, then it will also be limited to cases where the subsoil is not susceptible to loss of strength under cyclic loading, and the footing can be considered to be supported on a rigid perfectly plastic soil with adequate, uniform compressive capacity,  $q_n$  which is defined as nominal bearing capacity of supporting soil or rock (refer to 10.6.4.1A).

### 10.6.5.3 Foundation Thickness

Spread footings for piers and abutments shall have a minimum thickness of 3 feet. The thickness of a pile cap shall be the larger of 3.5 feet or the depth required to develop the full compressive, tensile, flexural, and shear capacity of the pile reinforcement.

### 10.6.5.4 Piles/Drilled Shafts

Minimum penetration depth of piles and drilled shafts into competent bearing soils shall be 10 feet. In the event that the piles and drilled shafts are embedded in rock, the minimum penetration depth shall vary between 3 feet to 10 feet, varying linearly with the unconfined compressive strength of the rock as follows:

**Table 10-3: Minimum Penetration Depth in Rock**

Rock Unconfined Compressive Strength (psi)	Embedded Depth (feet)
< 75	10
≥ 750	3

End bearing soil below the pile/drilled shaft tip shall be competent materials, having a thickness of at least  $3 \times D$  (where  $D$  is either the pile diameter or drilled shaft width) and shall demonstrate the adequacy of resisting punching shear failure and settlements.

## 10.7 Station and Miscellaneous At-Grade Structures

### 10.7.1 Shallow Foundations

Per AASHTO LRFD BDS with California Amendments Article 10.2 Definitions): “Shallow Foundation – A foundation that derives its support by transferring load directly to the soil or rock at shallow depth.”

Design of shallow foundations, e.g., spread and strip footings in addition to mat foundations, shall be based on site-specific information. Soil and rock engineering properties shall be based on the results of field investigations as presented in the Geotechnical Data Report; use of assumed values shall not be allowed. Designs of shallow foundations supporting rail structures or attached appurtenances shall be as required in AASHTO LRFD BDS with California Amendments Article 10.6, and in accordance with FHWA-SA-02-054 (Geotechnical Engineering

Circular No. 6 Shallow Foundations). Shallow foundations for support of structures under the purview of the California Building Code (CBC), buildings not directly supported off the aerial trackway structure, shall be designed in conformance with the requirements of the California Building Code (CBC) – Footings and Foundations. Shallow foundations shall have a minimum ground cover of 3 feet as measured from the top of footing to finished grade.

As these structures are distinct from bridges, aerial structures, and grade separations addressed in Section 10.6.4, shallow foundations shall be designed to limit total settlement (defined as vertical downward deformation of the shallow foundations for their design life) to no more than 1-inch. Differential settlements shall not exceed either 1/2-inch between adjacent supports or the ratio of the amount of settlement between adjacent supports divided by the distance between the supports (in consistent units) shall be no greater than 1/500, whichever is less.

### **10.7.2 Deep Foundations**

Where shallow foundations cannot be used due to presence of soft, compressible soils, deep foundations such as piling can be considered. Design of deep foundations shall be in accordance with AASHTO LRFD BDS with California Amendments. Differential settlements between adjacent supports and the total settlement shall be the same as those stated for shallow foundations in 10.7.1 above.

### **10.7.3 Miscellaneous At-Grade Structure Foundations**

Design of foundations for miscellaneous structures shall be in accordance with the requirements above for shallow foundations, excepting that presumptive values may be used. These include, but are not limited to miscellaneous structures such as light standards, retaining walls less than 5 feet in height and are not supporting any structures, and other lightly loaded and uninhabited structures. These miscellaneous structures shall be limited to those where settlements are not critical to their service performance.

Cantilever signs and signals shall be supported on drilled shaft foundations. Design for cantilever signals and signs shall be performed in accordance with the AASHTO Standard Specifications for Structural Supports for Highway Signs, Luminaires, and Traffic Signals. Seismic issues related to foundation design such as down drag and lateral spread due to liquefaction shall be addressed as stated in this chapter.

Foundation design for noise barriers shall be conducted in accordance with Caltrans Memo to Designer 22-1, Soundwall Design Criteria. Seismic issues related to foundation design such as down drag and lateral spread due to liquefaction shall be addressed as stated in this chapter.

## **10.8 Retaining Walls and Trenches**

The criteria set forth in this section govern the static/seismic load design of retaining walls and trenches (retaining walls with a continuous base slab between them). The design shall conform

to the applicable requirements set forth in AASHTO LRFD BDS with California Amendments Article 11, FHWA Earth Retaining Structures Manual, and the sections specified in this chapter. For permanent surcharge loads, refer to Section 10.12.5. For design loads of the HST, refer to the *Structures* chapter.

Retaining walls can be classified as either a “fill wall” or a “cut wall.” Acceptable fill walls include standard reinforced concrete cantilever walls, mechanically stabilized earth walls, reinforced soil slope embankment, and modular gravity walls (gabions and crib walls). Acceptable cut walls include soil nail walls, cantilever soldier-pile walls, slurry walls, secant pile/tangent pile walls, and ground anchored walls (other than nail walls).

### 10.8.1 Design

Design of retaining walls shall consider the following conditions of external instability where applicable:

- Sliding in connection with a horizontal displacement of the structure
- Overturning or excessive settlement
- Failure of the structure base (allowable soil pressure exceeded)
- Overall stability behind and under the structure (soil shear failure)
- Liquefaction potential of the supporting ground

For geotechnical design, refer to AASHTO LRFD BDS with California Amendments Article 11 and additional seismic criteria specified in the *Structures* chapter.

Design of mechanically stabilized earth structures and reinforced soil slope embankments shall also be in accordance with the LRFD version of FHWA's manual FHWA-NHI-10-024/25 "Design and Construction of Mechanically Stabilized Earth (MSE) Walls and Reinforced Soil Slopes", Volumes 1 and 2. Embedded metallic strip reinforcing elements, if used, shall meet the requirements of corrosion protection as set forth in the *Corrosion Control* chapter. Design of retained fill shall accommodate future Overhead Contact System (OCS) pole foundations. Refer to the *Structures* chapter for design criteria of the OCS pole foundations over retaining walls.

For MSE walls with metallic strips or wire meshes, a minimum of five (5) retrieval test strips or wire meshes shall be installed and retrieved for corrosion evaluation. The test strips or wires shall be retrieved for inspection in 5, 10, 20, 30, and 50 years after completion of the wall. Details of these retrieval strips and wire meshes shall follow those called out by standard Caltrans drawings and guidelines or as recommended by the MSE wall suppliers.

### 10.8.2 Unacceptable Walls

Unacceptable retaining walls include mortar rubble gravity walls, timber or metal bin walls, “rockery” or masonry walls, and other wall types not specifically listed in Section 10.8.

### 10.8.3 Stability of Retaining Walls

Retaining walls, abutment walls, and basement walls shall be evaluated and designed for internal, external (sliding and overturning), and global stability. In addition to the static loads, the retaining walls shall be designed to resist the dynamic (seismic) earth pressure (ED), hydrodynamic force (WAD) and hydrostatic pressure (water pressure), if submerged or below the groundwater table, under the seismic loading conditions.

Except for abutment walls where they shall be designed using the Service-1 Limit State loads, geotechnical designs for retaining walls and basement walls shall be performed in accordance with AASHTO LRFD BDS with California Amendments. Earth pressures used in design of the walls and abutments shall be selected consistent with the requirement that the wall/abutment movements shall not exceed tolerable displacement and settlement set forth in this chapter.

#### 10.8.3.1 Unrestrained retaining walls

Retaining walls with level backfill that are not restrained from rotation at the top, which are located where Peak Ground Acceleration (PGA) values (i.e., from MCE ground motion) are less than or equal to 0.30g, shall be designed for only active pressures, surcharge loads, other static loads and bearing as appropriate, and inertial forces of the wall itself; additional dynamic (seismic) earth pressures need not be considered.

The no-seismic-load options mentioned above shall be limited to internal and external seismic stability design of the retaining wall for level backfills. For sloping backfill, the no-seismic-load options shall be correspondingly 0.2g for 3H:1V and 0.1g for 2H:1V. All these no-seismic-load options shall be applicable to the condition that no liquefaction and no severe strength loss in sensitive clays occur that can cause wall instability. If the wall is part of a bigger slope, overall seismic stability of the wall and slope combination shall still be evaluated.

For walls with cohesionless backfill and located in areas where PGA values are expected to be greater than 0.30g, seismic active pressures shall be included in the stability analysis. Seismic earth pressures shall be estimated using the Generalized Limit Equilibrium (GLE) Method or Mononobe-Okabe (M-O) Method (Mononobe and Matsuo, 1929). Furthermore, the M-O Method should be used only under the following conditions:

- The material behind the wall can be reasonably approximated as a uniform, cohesionless soil within a zone defined by a 3H:1V wedge from the heel of the wall.
- The backfill is not saturated or in loose enough condition such that it can liquefy during shaking.
- The combination of horizontal acceleration coefficient ( $K_h$ ) and vertical acceleration coefficient ( $K_v$ ) and backslope angle,  $i$ , do not exceed the friction angle of the soil behind the wall as specified by:

$$\phi \geq i + \arctan (K_h / (1 - K_v))$$

For wall geometry, seismic acceleration level, or site conditions for which the M-O Method is not suitable, the Generalized Limit Equilibrium (GLE) Method shall be used to determine seismic active earth pressures.

The horizontal acceleration coefficient ( $K_h$ ) shall be the horizontal seismic coefficient as estimated by using the Bray et al. (2010) method assuming a wall movement of 1 inch for OBE case for retaining walls with level granular backfill. The total earth pressure (active and seismic) shall be of triangular distribution with its resultant acting at 0.33H from the bottom for routine walls (defined as walls that function independently of other systems or structures). For walls that have a critical function and act as part of an overall structure or system such as walls used as part of bridge abutments or part of tunnel portals, the earth pressures shall be separated into the incremental seismic pressures and the active earth pressures in the following manner:

$$\Delta K_{AE} = K_{AE} - K_A$$

Where:

$\Delta K_{AE}$  = Incremental seismic pressure coefficient

$K_{AE}$  = Total seismic pressure coefficient

$K_A$  = Active pressure coefficient

In addition, for walls that have a critical function and act as part of an overall structure or system such as walls used as part of bridge abutments or part of tunnel portals, the incremental dynamic (seismic) earth pressure shall be taken as a triangular distribution with the resultant acting at 0.67H from the bottom (i.e., an inverted triangle). This pressure shall be added to the active earth pressure for the design of the walls.

For walls that retain cohesive soils, the seismic demand may be reduced for retaining wall design (Anderson et al., 2008). For walls with sloping grades, the seismic demand on the wall generally increases and this increase should also be considered in those cases (Anderson et al., 2008). For design of retaining wall with cohesive retained soils or where native soils have a clear cohesive strength component, some effects of the cohesion in the determination of the seismic coefficient can be assumed. However, if the cohesion in the soil behind the wall results primarily from capillary stresses, especially in relatively low fines content soils, no cohesion can be allowed when estimating seismic earth pressure. Unless otherwise stated in this chapter, methods presented in Chapter 7 of the NCHRP Report 611 shall be used for backfill materials consisting of cohesive or cohesive and frictional ( $c-\phi$ ) material.

### 10.8.3.2 Restrained or Non-yielding Walls

For basement walls (i.e., non-yielding or walls restrained against rotation) with level backfill in locations where PGA values (for MCE ground motion) are less than or equal to 0.3g, walls shall be designed for only at-rest pressures, surcharge loads, other static loads and bearing as appropriate, and inertial forces from the wall itself, but additional seismic loads shall not be

considered. For higher PGA values, the higher of the at-rest pressures or the active plus M-O pressures shall be used for the design.

As mentioned above, walls that retain cohesive soils reduce the seismic demand, while the sloping grades behind walls increase the seismic demand required for retaining wall design (Anderson et al., 2008). For design of retaining wall with cohesive retained soils, or where native soils have a clear cohesive strength component, refer to section 10.8.3.1 above for design of the walls.

The no-seismic-load options mentioned above shall be limited to internal and external seismic stability design of the retaining wall for level backfills. For sloping backfill, the no-seismic-load options shall be correspondingly 0.2g for 3H:1V and 0.1g for 2H:1V. All these no-seismic-load options shall be applicable to the condition that no liquefaction and no severe strength loss in sensitive clays occur that can cause wall instability. If the wall is part of a bigger slope, overall seismic stability of the wall and slope combination shall still be evaluated.

#### **10.8.4 Base Pressure**

Soil bearing pressures shall be determined based on the applicable backfilled or native bearing materials. In order to minimize differential settlement and excessive outward tilting of walls, walls shall be proportioned so that the base pressure on soil under the footing is as nearly uniform (within 10 percent) as practical under the design load conditions.

#### **10.8.5 Hydrostatic Pressure (Buoyancy)**

Refer to the *Structures* chapter for design criteria for water loads (hydrostatic pressure) (buoyancy).

The use of tiedowns, tension piles, or other elements specifically designed to resist uplift forces shall be permitted. The use of augercast piles shall not be allowed as an anti-buoyancy hold down structure. The use of tension elements to resist buoyancy shall not compromise waterproofing and shall be designed to prohibit corrosion and be designed with the same design life as the rest of the structure.

#### **10.8.6 Settlements and Horizontal Deformations**

Retaining walls directly supporting HSTs, Primary Type 1, shall be designed not to exceed those residual settlement limits shown in Table 10-1 and Table 10-5 and horizontal deformation of 1/2 inch for service 1 and OBE load cases. These settlement and horizontal deformation limits apply after the structure enters service. For Type 2 and Secondary walls, refer to AASHTO LRFD BDS with California Amendments.

To avoid long-term deflections in the track, track structures (ballasted and non-ballasted) shall not be constructed until the majority (i.e., 75 percent) of estimated retaining wall settlement has already occurred. Use of ground improvement methods may be required to expedite settlement,



mitigate lateral deformations, as well as potential seismic hazards such as liquefaction and seismic instability. For loading associated with the MCE load, the settlement limits shall be evaluated and specified by the structural engineer (wall designer) who will ensure that no collapse criterion applies.

#### 10.8.7 Drainage

Adequate drainage behind retaining walls shall be included in the design and implemented during construction. An exception to this requirement is for trenches and underground structure walls where the top of trackway subgrade is below the groundwater table/flood level. These walls shall be designed to resist full hydrostatic pressures, both laterally and vertically (buoyancy).

#### 10.8.8 Backfill

Backfill behind retaining walls shall be cohesionless and drained. Drainage systems shall be designed to completely drain the entire retained soil volume behind the retaining wall face. If drainage cannot be provided due to site constraints, the abutment or wall shall be designed for loads due to full hydrostatic pressure in addition to earth pressures.

The compaction of the backfill shall meet a minimum of 95 percent degree of compaction using the Modified Proctor Test in accordance with ASTM (D-1557) or AASHTO T180. Care shall be taken not to damage the walls during compaction using light compactor or hand-held tamper.

### 10.9 Embankments and At-Grade Earthen Structures for HST Trackway (Primary Type 1)

For roadway and site embankments, refer to the *Civil* chapter. For design loads, refer to the *Structures* chapter.

Embankments and at-grade earthen structures shall be engineered. Design of embankments and at-grade earthen structures shall focus on settlement of support ground and stability of embankment and at-grade earthen structures. Care shall be taken to avoid possible landslides within the embankment and at-grade earthen structure areas.

At each embankment or at-grade earthen structure, the following shall be evaluated:

- Slope stability
- Liquefaction potential of support ground
- Bearing capacity and plastic flow evaluation
- Construction of embankment shall not lead to reactivation of existing landslides or the formation of new ones
- Creep considerations

- Drainage considerations to avoid eroding the slope, scouring the toe, adversely increasing pore-water pressures in the vicinity of the structure, and clogging the water course
- Impact of Rayleigh-wave vibration induced by the high-speed train on the track-ground system composed of ballast/subballast or non-ballasted track, embankment fill, supporting subgrades, and adjacent structures
- Assessment of prepared subgrade, subballast/bearing base layers, and trackway; in particular (1) high dynamic effects on low embankments (less than 6.5 feet)/foundation soils, and (2) critical speed issues of embankments over soft, compressible foundations with undrained shear strength less than 600 psf.

### 10.9.1 Slope Inclination

**Fill** – 2H:1V or flatter. Steeper slopes may be designed using geosynthetics (geogrids or geofabric) reinforcement to engineer an increased slope inclination.

**Cut** – 2H:1V or steeper if justified by slope stability analyses. Refer to Section 10.10.

### 10.9.2 Safety Factors

The stability of an embankment slope shall be evaluated using the Service-1 limit state. For the Service-1 static slope stability, the resistance factor is simply the inverse of the factor of safety (FOS). Table 10-4 shows the minimum required factors of safety for embankment slopes.

**Table 10-4: Minimum Required Factors of Safety for Embankment Slopes**

Loading Conditions	Factor of Safety
Normal (Permanent) <sup>(1)</sup>	≥1.50
Temporary (open less than 1 year)	≥1.30
Earthquake (OBE and MCE)	≥1.0 <sup>(2)</sup>

**Notes:**

<sup>(1)</sup> The factor of safety shall be in accordance with the requirements set forth by the local agencies.

<sup>(2)</sup> The stability of embankment slopes under earthquakes shall be analyzed by using the pseudo-static analysis, under the following conditions:

$K_h$  depends on allowable slope deformation (Refer to Bray and Travasarou (2009) for estimation of  $K_h$ ). Refer to Section 10.B.2.9.2 of *Appendix 10.B – Guidelines for Geotechnical Earthquake Engineering*.

$K_v = 0$

Where:

$K_h$  = Horizontal seismic coefficient

$K_v$  = Vertical seismic coefficient

### 10.9.3 Settlements

Once the embankments are designed to meet safe allowable bearing pressures and satisfy stability, settlements of the embankments during and after construction shall be evaluated.

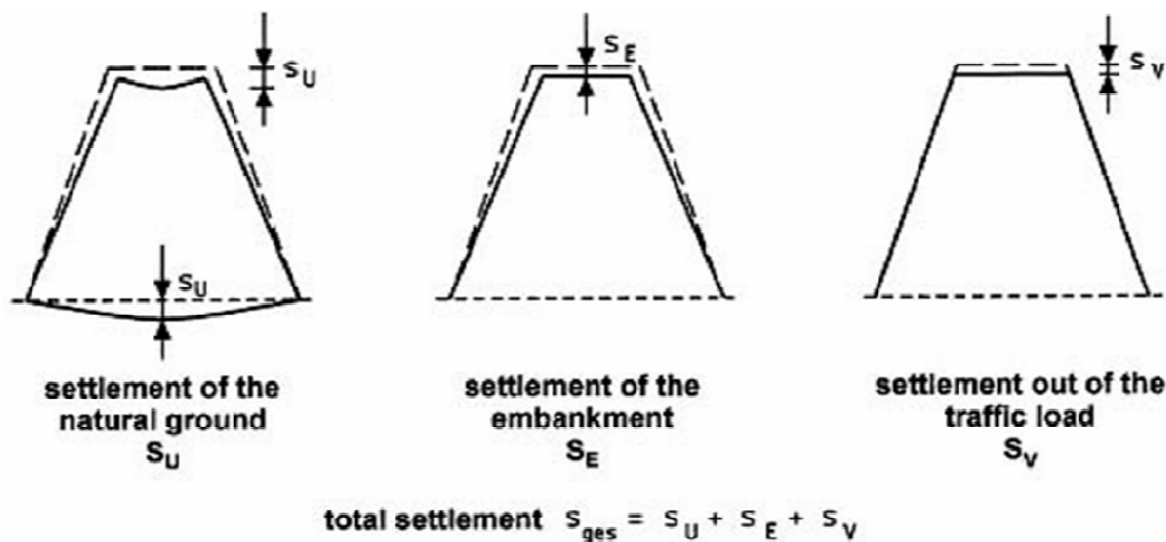
Settlement assessment shall be performed for new and existing embankments with particular emphasis on the following critical areas:

- Approaches to bridge abutments
- Soft and organic layers beneath the embankment

The vertical settlement of an embankment (which also affects overlying trackbed structure) is a combination of the permanent settlement of the foundation on which it is resting, plus permanent settlement of the embankment fill, and elastic and plastic deformations due to dynamic and repeated loading of the high-speed trains as depicted on Figure 10-1. Conventional settlement analyses shall consider 'immediate', 'consolidation', and 'secondary' components of settlement against the requirements of the CHSTP. For analysis of embankments, calculation procedures in the following references shall be used to assess soil settlement:

- Soil Slope and Embankment Design Manual, chapters 4 and 8, FHWA-NHI-05-123, 2005
- Soils and Foundations Reference Manual, chapter 7, FHWA-NHI-06-088 Volume I, 2006

**Figure 10-1: Settlements of Embankments**



Notes:

Reference: Figure no. 21 of UIC-719R (2008)

Geotechnical evaluations for embankments and their foundations shall include the settlement contribution from surcharge/track load, prepare subgrade, high-speed train induced vibration, and additional loading and/or ground deformation due to earthquakes.

Once the embankments are designed based on safe bearing pressures and satisfying stability and constructed in the field, the 'residual' settlement estimates and differential settlements

between locations along the length of the embankments shall be evaluated and estimated through track-earth- structure interaction analyses by the Geotechnical Designer.

**Table 10-5: Maximum Residual Settlement Limits**

Residual Settlement <sup>(1)</sup>	Non-Ballasted Track	Ballasted Track <sup>(4)</sup>
Differential Settlement <sup>(2),(3)</sup>	≤ 3/8 inch	≤ 3/4 inch
Uniform Settlement <sup>(3)</sup>	≤ 5/8 inch	≤ 1-1/8 inch

**Notes:**

- (1) Embankment shall be instrumented and monitored for a period such that the embankment and prepared subgrade-induced settlement levels off following completion of the structure. The Geotechnical Designer shall demonstrate future compliance with the residual settlements in Table 10-5 by extrapolation from the monitored data.
- (2) Differential settlement shall be measured along the track (surface profile uniformity) in the vertical plane of each rail at the mid-point of a 62-foot long chord.
- (3) For Service 1 and OBE load cases.
- (4) For ballasted track, rail geometries will be maintained to meet FRA's guidelines as per normal maintenance.

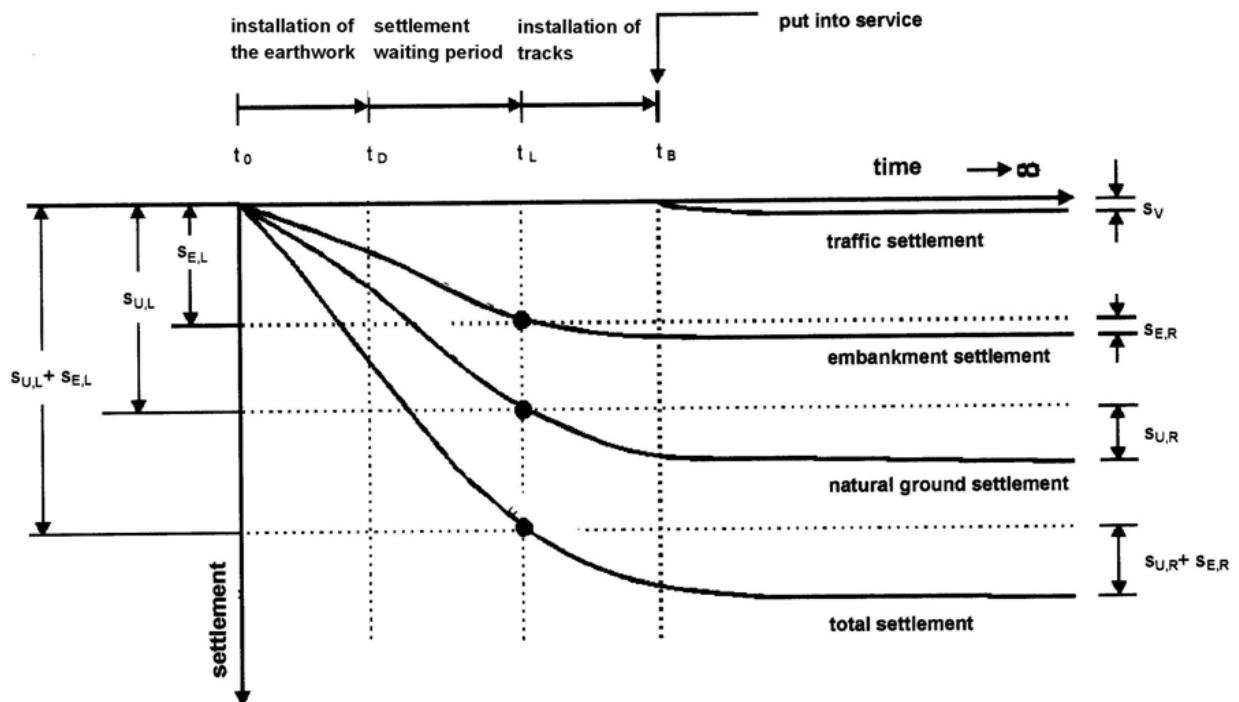
Embankments shall be designed and constructed so as not to exceed the maximum residual settlement set forth in Table 10-5. "Residual" settlements occur over the design life after the track is laid and shall meet these criteria. Geostructures shall be instrumented and monitored for a period such that the embankment and prepared subgrade-induced settlement levels off following completion of the structure (i.e. embankment fill and prepared subgrade are in place). Instrumentation shall remain in place and functioning properly for the Track contractor. The Geotechnical Designer shall demonstrate future compliance with the residual settlements (i.e., defined as settlements that are the sum of the remaining native foundation settlement and secondary embankment consolidation settlement estimated to occur after completion of embankment construction plus elastic and plastic deformations from dynamic train loading) by comparison of the monitored data and predicted settlement. These residual settlements are developed generally based on maintenance, passenger comfort, and track safety requirements. The residual settlements will be field verified by the Track Contractor.

If the predicted differential settlements are excessive and exceed track profile tolerances, then embankment designs shall be modified and ground improvement designed if needed to act as a foundation system. Where predicted settlements and their duration are excessive, consideration shall be undertaken to change the design from an embankment to an aerial structure or other structure.

Settlement of earth structures is time-dependent and will vary by segment. The time duration of the "waiting (leaving) period" such that the embankment/prepared subgrade-induced settlement levels off shall be evaluated and established. This period shall not be greater than the 4 month monitoring period after completion of the fill embankment and prepared subgrade. Upon the embankment/prepared subgrade-induced settlement leveling off, the Contractor shall cover/protect the completed prepared subgrade with asphalt concrete (AC).

An illustration of various settlement parts related to time is depicted on Figure 10-2. To meet CHSTP design and performance requirements, a settlement survey program shall be developed and then implemented by the Track Contractor to monitor settlement at the “acceptance check” timeframe after laying track, and then the long term ‘residual’ settlement as part of the track maintenance program.

**Figure 10-2: Different Settlement Parts by Time**



**Notes:**

Reference: Figure no. 22 of UIC-719R (2008)

Commentary: Per UIC 719R section 2.10.2.2 - Elastic vertical displacement of earthworks under load is usually not a design criterion, as resistance of continuous supporting structure generally implies very low vertical displacement (typically 0.004 to 0.008 inches [or 0.1 to 0.2 mm] on top of supporting structure). However, design criteria may exist to limit elastic deformation to a percentage of deformation of track components to manage the global track stiffness.

Symbols:

$S_{E,L}$  = Embankment Settlement at the End of Waiting Period

$S_{U,L}$  = Native Ground Settlement at the End of Waiting Period

$S_v$  = Settlement due to Traffic

$S_{E,R}$  = Embankment Settlement During and After Installation of Tracks

$S_{U,R}$  = Natural Ground Settlement During and After Installation of Tracks

### 10.9.3.1 Track Subgrade Settlement Analysis

Track subgrade settlement analysis, using finite element methods such as ADINA, ABAQUS, ANSYS, PLAXIS, etc., shall be performed to estimate track-subgrade settlements as a result of dynamic loading of the high-speed trains. Limiting values are presented in Section 10.14.3.1 for ballasted and non-ballasted tracks over earthen structures such as embankments or retaining structures supporting high-speed trains.

### 10.9.3.2 Embankment Foundation Settlement Mitigation and Foundation Modification using Ground Improvement Methods

For track embankment segments or at-grade trackway, including features such as OCS poles, walkway, and ballasted and non-ballasted trackways that do not meet settlement criteria or indicate stability problems, advanced mitigation measures such as pre-loading, over-excavation and replacement, or other ground improvement methods/measures shall be considered for geotechnical design. The selection of mitigation methods/measures shall follow the process described in detail in FHWA's Ground Improvement Reference Manuals Volumes I and II; FHWA-NHI-06-019/020 dated 2006.

A settlement monitoring program shall be developed and implemented during the construction phase for any mitigation method/measure selected. Interferometric Synthetic Aperture Radar (InSAR) techniques shall be considered as possible methods for large scale 'regional' monitoring in addition to ground truth measurements, such as GPS measurements and traditional surveying and use of geotechnical instrumentation during and after construction.

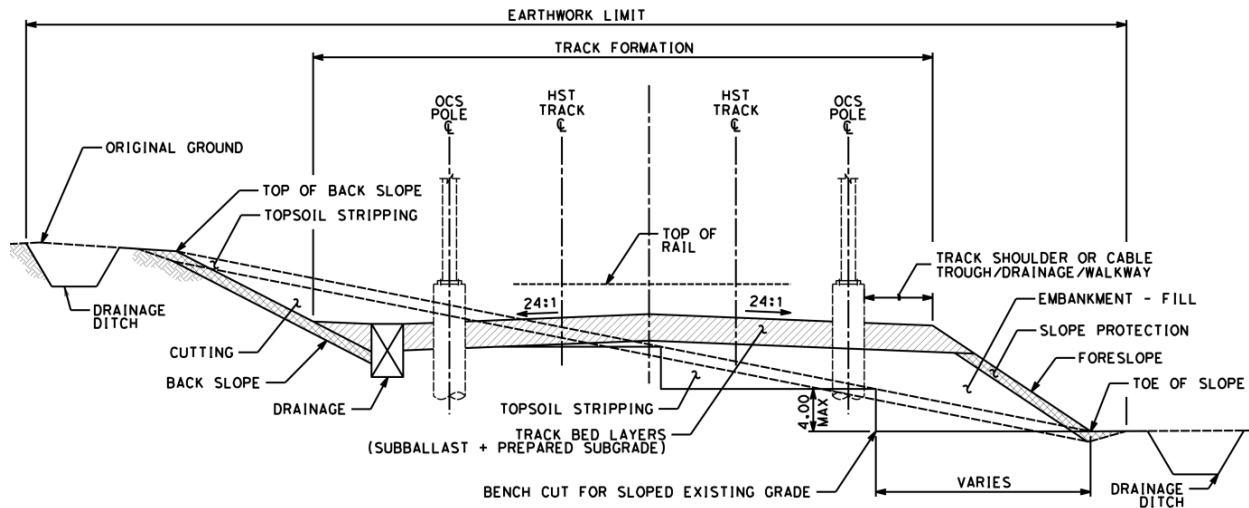
### 10.9.4 Benching of Slopes

For embankments higher than 30 feet (measured from existing ground surface to top of slope), design shall include mid-slope benches to mitigate surface erosion and to facilitate future access for maintenance reasons. Slope benches shall be at least 6 feet wide with a 4 to 6 percent slope towards the low end of the slope with a lined drainage channel. For embankments higher than 30 feet, slope benches shall be designed at every 25 to 30 feet in height connected to the surrounding ground surface for access.

At the top surface of the embankment, transverse cross-slope for drainage shall be 24:1 towards the outer edges of the embankment foreslopes (refer to Figure 10-3).

When an embankment is constructed next to an existing slope, the existing slope shall be benched in steps (typically 5 to 10 feet wide and no greater than 4 feet deep) to ensure the fill embankment is keyed into the existing slope (refer to Figure 10-3). Drainage measures shall be placed on these benches to facilitate and convey groundwater to discharge outlets.

**Figure 10-3: Typical Section Earthwork Cut/Fill**



### 10.9.5 Particular Requirements

#### 10.9.5.1 Foundation Support

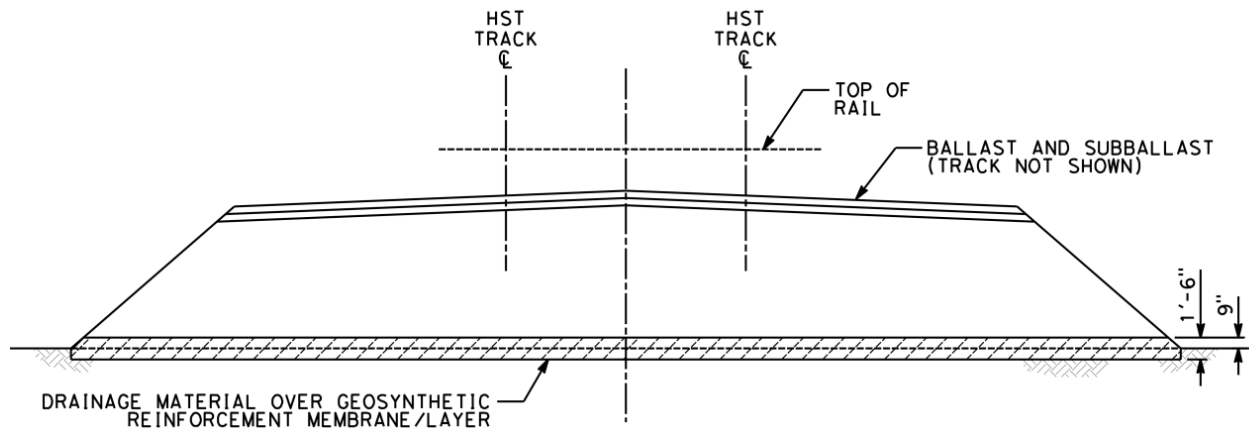
If the height of the embankment is less than or equal to 6.5 feet, as measured from the flat top of the subballast at the side edge of the embankment to the existing ground surface, and the foundation subgrades are loose and soft, compressible soils, they shall be removed and replaced with backfill and compacted to ensure settlement criteria.

For embankment heights greater than 6.5 feet over loose, soft, and compressible subgrade soils, the global stability and settlement induced by the embankment load shall be evaluated and ground improvement implemented, if necessary, to improve stability and achieve settlement criteria.

#### 10.9.5.2 Embankments in Wet Conditions

In case an embankment is located in a wet area where the groundwater table is permanently or periodically at ground level, the embankment shall be constructed on a layer of drainage material as depicted on Figure 10-4. This material shall not swell or deteriorate when immersed in water. It shall be well graded with no more than 10 percent passing the No. 200 sieve. The grading of the drainage material shall be designed according to Sherard's filter criteria (Sherard et al., 1984 a&b). A layer of geosynthetic cloth shall be placed below the drainage material to provide a better support to the drainage material.

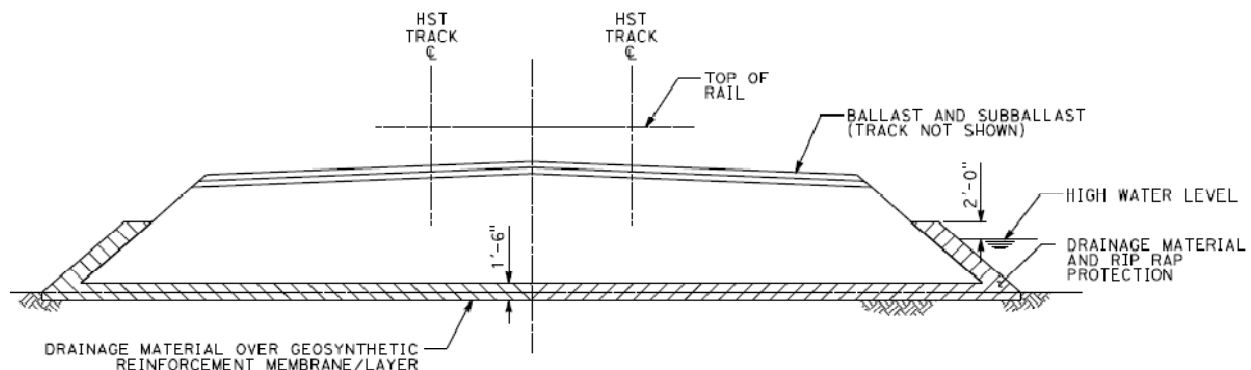
**Figure 10-4: Earthwork Embankment in Wet Conditions**



### 10.9.5.3 Embankment in Flood Plains

Where an embankment is located in a floodplain, the highest flood water level shall be evaluated from the 100-year flood. The embankment shall be, in addition to the drainage layer arrangement in Section 10.9.5.2, designed to protect the slopes within the highest water level with a layer of drainage layer and protection riprap as depicted on Figure 10-5. The drainage material shall be designed to comply with Sherard's filter criteria (Sherard et al., 1984 a&b). This layer shall extend up to the highest flood water level plus 2 feet and be underlain by a layer of geosynthetic membrane.

**Figure 10-5: Drainage Layer under Embankments in Floodplain / High Water**



### 10.9.5.4 Embankments over Active Fault Locations

Where possible, embankments shall be located outside of active fault lines and founded on competent grounds. If this cannot be avoided, the embankments shall be designed with consideration of potential offsets for active fault crossings. Such consideration shall include both potential horizontal and vertical components of potential offset, as well as the relative orientation of this offset with respect to the track or embankment. Approaches to accommodate



offset shall include making embankments wide enough and including designs with layers of geosynthetic cloth, geogrids drain rock at the bottom of embankments, and/or containment earthworks wide enough to accommodate the potential rupture offsets and subsequent re-alignment. Design of embankment over active fault locations shall consider life safety and preventive measures for ease of service restoration.

#### 10.9.5.5 Embankments on Potentially Liquefiable Soils/Compressible Soils

Where embankments are underlain by soft compressible soils or loose saturated soils that indicate high potential of liquefaction under OBE and MCE earthquakes, mitigation shall be required. The following soil improvement methods should be considered to mitigate soil liquefaction and increase the consistency of the foundation subgrade:

- Replacement
  - Excavate and replace with compacted fill
- Vibratory Densification
  - Vibro-compaction
  - Vibro-replacement stone columns (combination of vibration and displacement)
  - Deep dynamic compaction
- Displacement Densification/Reinforcement
  - Compaction grouting
  - Displacement piles
  - Vibro-replacement stone columns (combination of vibration and displacement)
  - Rammed aggregate piers (Replacement or Displacement type)
- Mixing/Solidification
  - Permeation Grouting
  - Deep soil mixing
  - Jet grouting
- Surcharge with wick drains (for soft compressible soils)
- Lime columns for soft compressive clays
- Drainage (only used in combination with other ground improvement methods listed above)
  - Passive or active dewatering systems
  - Pipe Pile Stone Columns (drainage in combination of vibration and displacement)

Ground improvement design shall be in accordance with FHWA Ground Improvements Reference Manual Volumes 1 and 2, FHWA-NHI-06-019 and FHWA-NHI-06-020.

#### 10.9.5.6 High-Speed Train Induced Ground Vibration on Embankments and At-grade Segments

High-speed trains will produce compressive (P) waves, shear (S) waves, and Rayleigh (R) waves, of which, Rayleigh waves (moving parallel to the ground surface) are the primary source of vibrational energy. This vibrational energy could have a substantial destructive and fatiguing effect on the HST track-ground system composed of rails, ballast or ballasted slab, subballast, prepared subgrade, embankments, at grade segments, and foundation subgrades. In addition, ground vibrations generated by high-speed trains are of great concern because of the possible damage they can cause to buildings or other structures near the track and the annoyance to the public living in the vicinity of the track. Particularly in areas of soft, compressible, or loose soils, where the wave speed is comparable to the speed of the trains, a strong increase of the vibration level can occur. The impact of the high-speed train-induced ground vibration on the track-ground system shall be evaluated and mitigated accordingly to avoid long term degradation of the HST track-ground system and all adjacent structures. Mitigation methods are available against excessive ground vibration from high-speed traffic. What method or combination of methods shall be used depends on factors such as (1) the frequency content of the generated ground vibration, (2) overall stiffness of the embankments or at grade segments, and (3) the type, consistency, and layering of the soils at the site. Mitigation measures may consist of replacing soft/loose soils with compacted fill, piled slabs, ground treatment such as dry deep mixing method (lime/cement columns) in soft clays or stone columns in loose sandy soils.

For design purposes, the following shall be required:

- Vibration induced stability of the embankment, at grade segments, and adjacent structures shall be verified.
- Tracks shall be supported by well compacted ballast/subballast, or non-ballasted track.
- Embankments or at grade segments supporting the track shall be adequately compacted.
- Subgrade underlying the embankment or at grade segments shall be competent and firm, and if soft compressible or loose soils are present, they shall be stabilized with ground treatment to increase its overall stiffness with undrained shear strength  $\geq 15$  psi and  $E_{v2} \geq 6,500$  psi.  $E_{v2}$  is the subgrade stiffness (strain modulus) evaluated from the 2<sup>nd</sup> loading of a plate load test according to ASTM D1196, and DIN 18134.

In addition, an instrumentation program shall be devised to investigate the effect of the stabilization measures before and after the measures are conducted.

#### 10.9.5.7 Embankment Prepared Subgrade

Material and thickness of the prepared subgrade for each track type (ballasted and non-ballasted) shall be as noted in the "Thickness of Prepared Subgrade" table of Figure 10-6. For non-ballasted track where the embankment height is low (less than 6.5 feet as measured from the flat top of the subballast at the side edge of the embankment to the existing ground surface), excavation below existing grade is not required to achieve a 6.5-foot thick prepared subgrade if it can be demonstrated by field testing that  $E_{v2}$  of the existing subgrade is  $\geq 11,500$  psi after the

- 1 foundation soil is proof-rolled. In this case, the thickness of the prepared subgrade can be  
2 reduced to the available thickness, but it shall not be less than 14-inch thick and  $E_{v2} \geq 11,500$  psi  
3 shall be provided.

#### **10.9.5.8 Transition of Embankments to Structures**

- 4 Embankments adjacent to the bridge, aerial structure, or grade separation abutments, tunnel  
5 portals, cut-and-cover structures, and cut sections with an abrupt topographic change shall be  
6 designed to minimize the differential settlement and to provide a smooth transition in the  
7 structural stiffness between different infrastructures. Provide a smooth transition by stiffening  
8 the subballast/bearing base layer and the approach fill with soil cement as depicted on Figures  
9 10-6, 10-7, and 10-8.

1 **Figure 10-6: Transition from Concrete Slab to Embankment**

THICKNESS OF PREPARED SUBGRADE		
MATERIAL	TRACK TYPE	THICKNESS
WELL GRADED SOILS CONTAINING 5% TO 15% FINES	BALLASTED TRACK	14"
	NON-BALLASTED TRACK	6'-6"

GRADATION FOR PREPARED SUBGRADE MATERIAL	
GRAIN SIZE (mm)	PERCENTAGE PASSING [D=20 TO 125 (mm)]
P(20)	100
P(DMAX)	100~99
P(D)	99~85
P(D/2)	84~55
P(D/5)	60~31
P(D/10)	49~23
P(D/20)	40~17
P(D/50)	30~11
P(D/100)	22~8
P(D/200)	16~6
P(D/500)	9~3
P(D/1000)	6~2

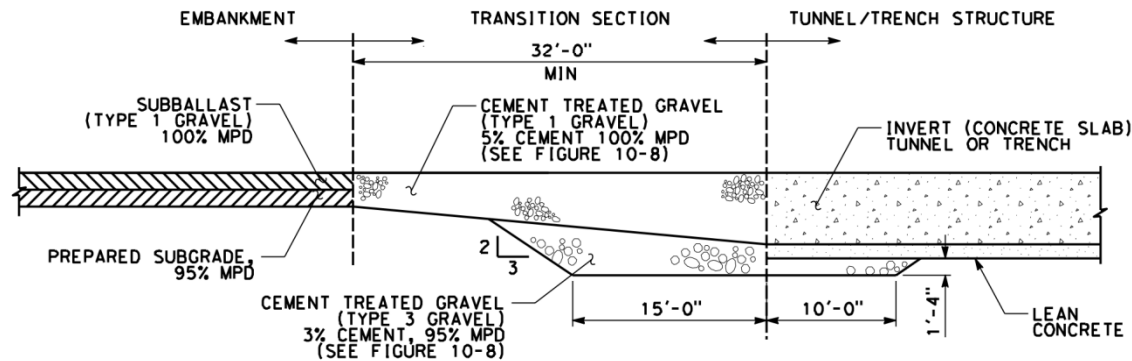
D = NOMINAL GRAIN SIZE  
D<sub>max</sub> = 1.25D IF D ≥ 50 mm;  
D<sub>max</sub> = 1.58D IF D < 50 mm

**NOTES:**

1. TRANSITIONS SHALL BE DESIGNED TO MINIMIZE THE DIFFERENTIAL SETTLEMENT AND TO PROVIDE A SMOOTH TRANSITION IN THE STRUCTURAL STIFFNESS BETWEEN DIFFERENT INFRASTRUCTURES.
2. EMBANKMENTS SHALL BE DESIGNED SPECIFICALLY TAKING INTO ACCOUNT THE CONSTRUCTION SEQUENCE AND THE GEOMETRICAL, GEOLOGICAL AND GEOTECHNICAL CONDITIONS OF THE SITE.
3. THE MINIMUM SUBBALLAST (SUPPORTING BALLASTED TRACK) THICKNESS SHALL BE 9".

**LEGEND:**

MPD MODIFIED PROCTOR DENSITY (AASHTO T180)  
E<sub>v2</sub> DEFORMATION MODULUS OF SECOND LOADING



2

1 **Figure 10-7: Transition from Cut to Embankment**

THICKNESS OF PREPARED SUBGRADE		
MATERIAL	TRACK TYPE	THICKNESS
WELL GRADED SOILS CONTAINING 5% TO 15% FINES	BALLASTED TRACK	14"
	NON-BALLASTED TRACK	6'-6"

GRADATION FOR PREPARED SUBGRADE MATERIAL	
GRAIN SIZE (mm)	PERCENTAGE PASSING [D=20 TO 125 (mm)]
P(20)	100
P(DMAX)	100~99
P(D)	99~85
P(D/2)	84~55
P(D/5)	60~31
P(D/10)	49~23
P(D/20)	40~17
P(D/50)	30~11
P(D/100)	22~8
P(D/200)	16~6
P(D/500)	9~3
P(D/1000)	6~2

D = NOMINAL GRAIN SIZE  
 $D_{max} = 1.25D$  IF  $D \geq 50$  mm;  
 $D_{max} = 1.58D$  IF  $D < 50$  mm

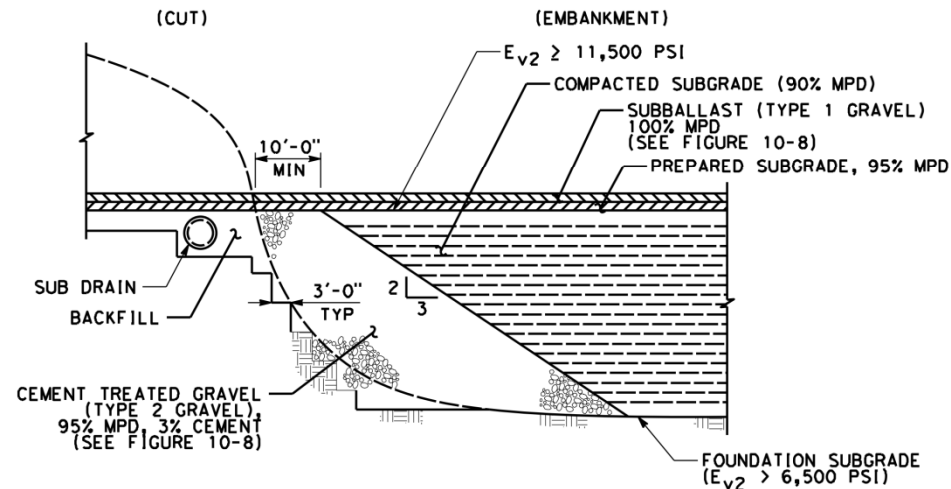
**NOTES:**

1. TRANSITIONS SHALL BE DESIGNED TO MINIMIZE THE DIFFERENTIAL SETTLEMENT AND TO PROVIDE A SMOOTH TRANSITION IN THE STRUCTURAL STIFFNESS BETWEEN DIFFERENT INFRASTRUCTURES.
2. EMBANKMENTS SHALL BE DESIGNED SPECIFICALLY TAKING INTO ACCOUNT THE CONSTRUCTION SEQUENCE AND THE GEOMETRICAL, GEOLOGICAL AND GEOTECHNICAL CONDITIONS OF THE SITE.
3. THE MINIMUM SUBBALLAST THICKNESS SHALL BE 9".

**LEGEND:**

MPD      MODIFIED PROCTOR DENSITY (AASHTO T180)

$E_{v2}$       DEFORMATION MODULUS OF SECOND LOADING



1 **Figure 10-8: Transition from Bridge, Aerial Structure, or Grade Separation to Embankment****TYPE 1 GRAVEL NOTE:**

CEMENT TREATED GRAVEL  
0/31.5. COMPACTION SHALL  
BE GREATER THAN 100% OF  
MAX DRY DENSITY ACCORDING  
TO AASHTO T180 MPD.  
DEFORMATION MODULUS  $E_{v2}$   
SHALL BE GREATER THAN  
17,000 PSI.

TYPE 1 GRAVEL	
SIEVE	% PASSING
1.5 INCH	88~100
1.0 INCH	82~97
3/4 INCH	75~92
3/8 INCH	64~85
NO. 4	53~77
NO. 10	40~68
NO. 40	22~48
NO. 100	10~36
NO. 200	3~22

**TYPE 2 GRAVEL NOTE:**

CEMENT TREATED GRAVEL  
0/20. COMPACTION SHALL  
BE GREATER THAN 95%  
OF MAX DRY DENSITY  
ACCORDING TO AASHTO  
T180 MPD  $E_{v2} > 15,000$   
PSI.

TYPE 2 GRAVEL	
SIEVE	% PASSING
3/4 INCH	83~100
3/8 INCH	53~83
NO. 4	36~65
NO. 10	23~49
NO. 40	12~31
NO. 100	7~19
NO. 200	4~12

**TYPE 3 GRAVEL NOTE:**

GRAVEL 0/60.  
COMPACTION SHALL BE  
GREATER THAN 95% OF MAX  
DRY DENSITY ACCORDING TO  
AASHTO T180 MPD.  
DEFORMATION MODULUS  $E_{v2}$   
SHALL BE GREATER THAN 80  
11,500 PSI.

TYPE 3 GRAVEL	
SIEVE	% PASSING
2.0 INCH	80~100
3/4 INCH	43~72
NO. 4	21~46
NO. 10	14~35
NO. 40	7~19
NO. 100	3~10
NO. 200	2~6

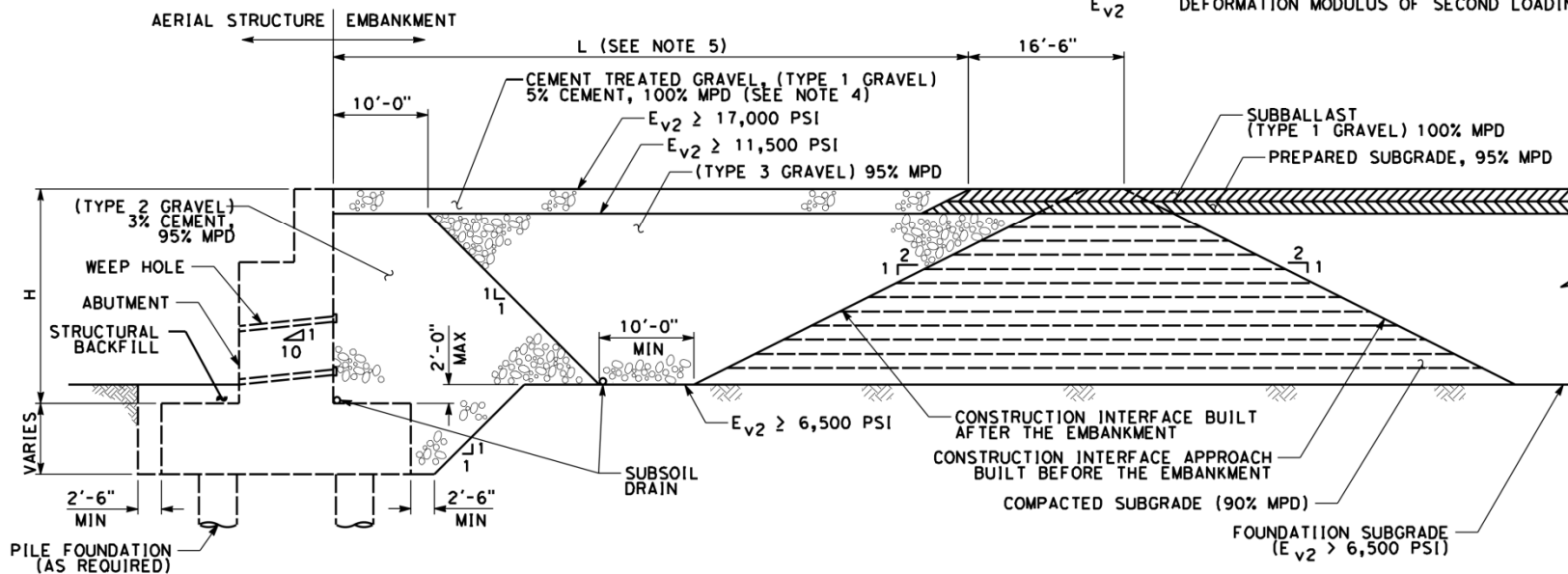
**NOTES:**

1. TRANSITIONS SHALL BE DESIGNED TO MINIMIZE THE DIFFERENTIAL SETTLEMENT AND TO PROVIDE A SMOOTH TRANSITION IN THE STRUCTURAL STIFFNESS BETWEEN DIFFERENT INFRASTRUCTURES.
2. EMBANKMENTS SHALL BE DESIGNED SPECIFICALLY TAKING INTO ACCOUNT THE CONSTRUCTION SEQUENCE AND THE GEOMETRICAL, GEOLOGICAL AND GEOTECHNICAL CONDITIONS OF THE SITE.
3. THE MINIMUM SUBBALLAST THICKNESS SHALL BE 9"
4. THE MINIMUM THICKNESS SHALL BE EQUAL TO THE COMBINED THICKNESS OF THE SUBBALLAST AND THE PREPARED SUBGRADE AND NO LESS THAN 1'-11".
5. LENGTH L, SHALL BE 4H OR 65'; WHICHEVER IS GREATER.
6. PREPARED SUBGRADE THICKNESS IS SHOWN ON FIGURES 10-6 AND 10-7.

**LEGEND:**

MPD MODIFIED PROCTOR DENSITY (AASHTO T180)

$E_{v2}$  DEFORMATION MODULUS OF SECOND LOADING



2

#### 10.9.5.9 Embankments in Cut Sections

Embankment design in cut sections shall include selection of appropriate earthworks for a given setting based on design constraints and potential conflicts, geotechnical subsurface investigations, and surface and groundwater issues. Figure 10-3 depicts a typical embankment in a cut section.

#### 10.9.5.10 Drainage (Surface and Subsurface)

Control of surface and ground water is essential to avoid surface erosion and potential slope instability. In addition to the requirements set forth in the *Drainage* chapter, provision shall be made in the design for an adequate system of surface and subsurface drainage and surface protection that incorporates sufficient capacity for the following:

- Design rainfall run-off to prevent long term erosion
- Build-up of groundwater that could result in slope instability

Notwithstanding the requirements of available relevant standards, consideration shall be given to the long term performance of the drainage and erosion control system for each embankment of fill under local conditions.

Where horizontal drains are to be used, a protective measure shall be devised to protect the drains from freeze/thaw. A long term maintenance program shall be developed by the Geotechnical Designer in order to safeguard the long term functionality of the horizontal drains.

Geotechnical design shall also include evaluation of temporary construction erosion control requirements on cut and fill slopes when integral to geotechnical design or performance. For example, the requirement to provide bench drainage during construction of slopes may be required to ensure construction phase stability.

For secondary structures, the drainage design shall be made according to the requirements set forth in the jurisdiction of the local county, city, or third party such as Caltrans, UPRR, etc.

#### 10.9.6 Soil Materials Used for Embankments

For design purposes, evaluation of soil suitability for re-use within the body of embankments shall be based on the following guidelines:

**Table 10-6: Soil Material Suitability for Engineered Fill in Embankments**

Acceptable <sup>(1)</sup>	Unacceptable <sup>(2)</sup>
A-1-a	A-4 (CBR <10)
A-1-b	A-2-7
A-2-4	A-5
A-2-5	A-6
A-2-6	A-7-5
A-3	A-7-6
A-4 (CBR >10)	*

**Notes:**

Source: Per ASTM D3282 / AASHTO Subgrade Soil Group System

Refer to the *Trackwork* chapter and Standard Specifications for Trackbed layers of subballast and prepared subgrade.

\* Rockfill is not acceptable for track embankment material.

<sup>(1)</sup> In addition to the AASHTO criteria, the maximum soil particle size is limited to 3 inches.

<sup>(2)</sup> Potential embankment fill source materials from groups A-2-7, A-5, A-6, and A-4 (with CBR <10) that can be shown by analysis and testing to meet performance requirements (including strength, stability, settlement/deformation, long-term durability, etc.) shall be submitted for consideration of acceptability on a case-by-case basis. This includes marginal soil types from these groups that can be 'modified' using soil amendments or additives such as cement, lime, hydraulic binders, etc., to be rendered suitable for use provided they meet performance requirements (described above) as demonstrated by analysis and testing programs including laboratory trial batching and field test sections.

Soil suitability evaluations shall also consider potentially detrimental properties as follows:

- **Frost Susceptibility** – soil types susceptible to frost, such as silt or clay, shall not be used for embankments in regions where cold conditions (below freezing temperatures) can occur in order to reduce the potential to cause unacceptable disturbances to track geometry upon freeze/thaw cycles.
- **Corrosivity** – soil suitability shall also consider corrosion potential.<sup>11</sup> Corrosive soils that are potentially detrimental to buried metal and/or concrete features (such as Overhead Contact System (OCS) poles, pipes/culverts, geogrid reinforcement, etc.) shall not be used.
- **Slake Durability of Rock** – based on the slake durability behavior in wetting and drying cycles.

## 10.10 Cut Slopes

Cut slopes include soil, Intermediate Geomaterials (IGM), and rock slopes, and shall be designed per the following sections. Sloped excavations during construction shall be designed

<sup>11</sup> Corrosion potential is the potential of a corroding surface in an electrolyte relative to a reference electrode measured under open-circuit conditions.



and constructed in compliance with local, state, and federal regulations, including but not limited to Occupational Safety and Health Administration (OSHA), and Cal/OSHA requirements.

### 10.10.1 Design of Cut Slopes

Design of cut slopes shall consider the following:

- Impact of slope instability to the HST facility operations and integrity (short term and long term)
- Slopes within existing pre-historic landslide areas
- Locations where liquefaction-related lateral spreading conditions are present
- Rock slopes with adversely oriented and kinematically unstable structural discontinuities such as joints, bedding planes, shear planes, gouges, and faulted zones

At each cut slope location, the following shall be evaluated:

- Locations where evidence of prior landsliding is present
- Slopes composed of quick, sensitive, and expansive clays

At each cut slope, the following shall be evaluated:

- Slope stability (static and seismic)
- Construction of the cut slope shall not lead to reactivation of existing landslides or the formation of new ones

For design of rock slopes, refer to *Appendix 10.C – Guidelines for Rock Slope Engineering*.

#### 10.10.1.1 Design Requirements

Slope Inclination (Typical<sup>12</sup>)

- **Soil cut** – 3H:1V slope or steeper if justified by slope stability analyses
- **IGM cut** – 2H:1V slope or steeper if substantiated by slope stability analyses
- **Rock cut** – 1H:1V slope or steeper if justified by slope stability analyses

#### 10.10.1.2 Safety Factors

For design criteria for stability of cut slopes, refer to Section 10.9.2.

---

<sup>12</sup> The slope inclination design guidelines stated herein do not apply to the cut slopes in pre-historic landslide areas, prior landslide locations, and potential liquefaction related lateral spreading conditions, slopes composed of sensitive, quick, and expansive clays.

### 10.10.2 Drainage (Surface and Subsurface)

Drainage provisions and permanent erosion control facilities to limit erosion (including soil erosion and rock slope degradation) are required for design of cut slopes. Surface drainage shall be accomplished through the use of drainage ditches and berms located above the top of the cut, around the sides of the cut, and at the base of the cut. Erosion control for cut slopes shall be performed similar to those stated in Section 10.9.5.10 and Section 10.11.2. Impermeable coverings with drainage provisions (weeps and geocomposite mats) such as shotcreting (with or without ground reinforcements), stone-pitching, etc., shall be considered to protect rock slopes from degradation and deterioration due to weathering.

Subsurface drainage systems such as cut-off drains, horizontal drains, french drains, etc., shall be designed to permanently lower groundwater table to enhance overall stability of the slopes. For other drainage related design criteria, refer to the *Drainage* chapter for details.

### 10.10.3 Slope Stability Mitigation Methods for Cut Slopes

Where the minimum required factors of safety cannot be achieved or the alignment cannot be relocated away from unstable slopes, the Geotechnical Designer shall design measures to enhance slope stability. Slope stability mitigation measures for cut slopes include the following:

- Soil Cuts
  - Flattening the slopes (if permitted by right-of-way) with vegetation cover
  - Buttressing the toe of the slopes
  - Stabilizing the slope with ground reinforcements such as soil nails and soil anchors with or without shotcrete
  - Covering the slope face with stone pitching, concrete, or shotcreting
  - Debris flow diversion walls
  - Retaining walls such as soldier pile walls, secant pile and tangent piles, gabion walls, etc.
  - Drainage and subdrainage measures
  - Ground improvements such as deep soil cement mixing or jet grouting
  - A combination of any of the above
- Rock Cuts
  - Rock scaling and dentition
  - Rock fall ditches
  - Rock fall retention meshes
  - Rock fall detention fences

- 1       – Rock dowels and anchors
- 2       – Shotcreting
- 3       – A combination of any of the above

## 10.11 Existing Slopes

- 4       The Geotechnical Designer shall evaluate existing slopes for potential instability. At a  
5       minimum, the Geotechnical Designer shall mitigate unstable slopes to ensure that they will not  
6       pose a detrimental impact to the alignment.

### 10.11.1 Protection of Existing Slopes

- 7       The Geotechnical Designer shall be responsible for a design that maintains the stability of  
8       existing slopes during the course of construction. Slope instability that occurs during  
9       construction shall be repaired by the Contractor at its own expense.

### 10.11.2 Drainage (Surface and Subsurface)

- 10       Erosion control and drainage measures shall be evaluated, considered and designed for existing  
11       slopes. Erosion of slopes presents a significant maintenance issue and overall stability concern.  
12       Rock and soil strata that are susceptible to erosion and/or freeze/thaw shall be mapped and  
13       delineated for existing and new fills and cuts. Slope protection measures shall be evaluated on  
14       site-specific conditions, such as surface and subsurface conditions, cut geometry, and  
15       susceptibility of erosion or deterioration. Each cut and fill slope that requires erosion control  
16       and drainage measures shall be evaluated for the following:

#### A. Reduction of water flow across slope

- 17       Where slope revegetation cannot be sufficiently established, reduce the quantity of water  
18       flowing over the slope from upland areas by means of drainage or interceptor ditches across the  
19       top of the slope and down the ends of the slope. At the base of the slope, water shall be directed  
20       to a discharge point. Coordinate discharge point drainage with existing facilities.

- 21       Drainage or interceptor ditches shall be lined or unlined and capable of carrying water  
22       generated from upland areas based on the 100-year storm. Lining materials shall be cast-in-  
23       place concrete, pre-cast concrete, reinforced shotcrete, or asphalt. Rock check dams to slow  
24       flows shall be designed and installed based on flow calculations.

#### B. Slope Revegetation

- 25       Where the slope can be made to support vegetation, local plantings shall be used to establish  
26       root systems to stabilize the surface of the slope and prevent deterioration of the slope. Design  
27       and provide systems of degradable woven blankets to temporarily hold plantings in place and  
28       minimize erosion until vegetation has established a stable root system.

C. Slope Armor

Where slopes will not support vegetation, slope cover/protection or permanent facing shall be used to protect the slope. Such measures as mattress-shaped steel wire mesh containers, gabions, articulated concrete blocks, fabric formed concrete, shotcrete, geosynthetic cells filled with gravel, and rip-rap (crushed stone) placed on a graded filter shall be evaluated, designed and installed. Stone sizes shall be designed based on design water flows.

D. Subsurface Water Control

Design of subsurface water drainage features shall be evaluated as water control measures. Design shall consider the use of horizontal drains, blanket drains, trench drains and geocomposites for both cut and fill slopes. Design shall consider outlet design and address long-term performance and maintenance requirements for the drainage system.

E. Springs and Water Seepage

Any springs and water seepage identified in the field shall be contained by means of drainage systems. Design shall consider long-term performance and maintenance requirements for the drainage system.

For other drainage related design criteria, refer to the *Drainage* chapter for details.

## **10.12 Cut-and-Cover Underground Structures**

The cut-and-cover underground structures include subways, cross-passages, sump pump structures, stations, building basements, vaults, ventilation structures, and other structures of similar nature.

Underground structures shall include waterproofing protection, drainage systems and/or dewatering pumps as needed to prevent water buildup in the underground structures.

### **10.12.1 Structural Systems**

The structural system for cut-and-cover line structures shall be single and/or multi-cell reinforced concrete box structures, with walls and slabs acting one-way in the transverse direction to form a frame. Walls that provide temporary support of excavation shall not be used as part of the permanent structure. Expansion joints are required at locations of major change in structural sections such as from line structure to station. Construction joints shall have continuous reinforcing steel and non-metallic waterstops.

### **10.12.2 Water Pressure (Buoyancy)**

Refer to the *Structures* chapter for water loads (water pressure for design criteria for buoyancy) and requirements for different buoyancy resisting elements.

Refer to Section 10.8 for types of systems to be allowed to resist buoyancy.

### 10.12.3 Temporary Support of Underground Structures

Equivalent static loads and deformations may be used to design temporary support systems such as wales, struts, and braces recognizing the short duration of these systems. These loads shall be provided by the Geotechnical Designer and shall be shown on the shoring design calculations and drawings.

In locations where adjacent buildings and their foundations create an interaction configuration in conjunction with temporary ground support structures that would significantly influence the seismic response of the adjacent buildings themselves, the combined group of temporary ground support and building structural configurations shall also be analyzed as a single structure to confirm seismic response of the buildings. In addition, the effect of stress redistribution onto existing adjacent structures due to the design of temporary support systems shall be considered and mitigated as necessary since it is determined by the means and methods selected by the Contractor.

### 10.12.4 Temporary Lateral Loading Conditions

Refer to the *Structures* chapter for construction loads and definition of temporary structures.

**Soil Pressures** – The Geotechnical Designer shall have the responsibility of determining earth pressures of temporary earth support; however, the earth pressures shall not be less than those calculated assuming the active case. Pressures shall consider the impacts due to compaction. The temporary design of the wall shall not allow for overstressing of the wall.

**Water Pressures** – The temporary earth support system shall be designed to construction term water pressures that are not lower than the existing groundwater level or seepage pressures, with consideration given to the potential of elevated groundwater conditions due to ground water re-injection activities.

**Surcharge Loads** – The earth support system shall include surcharge loads including, but not limited to traffic, construction material and equipment, and building loads.

**Earthquake Loads** – Unless otherwise stated in this chapter, earthquake loads (i.e., seismic earth pressures) shall be considered.

**Temporary Excavation Support Systems** – Excavation and backfill sequence and strut installation and strut removal sequence shall be in accordance with the Designer of Record's design requirements.

Temporary earth support may remain in place or be removed following completion of the structure. Temporary earth support walls left in place shall be cut off at a depth not higher than 5 feet below grade or top of structure whichever is higher. Removal of temporary earth support walls shall be permitted. The settlement analysis shall indicate that removal will not cause settlement and lateral movement of adjacent structures, sidewalks, streets, and utilities. Tiebacks used to retain temporary support walls shall be de-tensioned prior to abandonment.

### 10.12.5 Permanent Lateral Loading Conditions

**Soil Pressures** – Permanent underground structures shall be designed for earth pressures as given in Section 10.8.3. The at-rest pressures shall be used in the design of cut-and-cover underground structures. In addition, hydrostatic pressures and seismic loadings shall also be included in the design of the underground structures.

**Surcharge Loads** – Loads from adjacent building foundations shall be used in the design of cut-and-cover underground structures unless these existing buildings are founded on piles or permanently underpinned at a depth below the zone of influence of the cut-and-cover structures. Horizontal distribution of loads from foundations of existing buildings shall be determined in accordance with AASHTO LRFD BDS with California Amendments Article 3.11.6.

### 10.12.6 Deformation Limits for Support of Excavation Systems

Excavation support systems shall be designed to limit wall deformations that would otherwise lead to ground settlements, resulting in damage to the support systems or any superimposed structures and adjacent structures/utilities. Ground settlement and lateral deformation shall be limited to less than 1 inch and 1/2 inch, respectively. The Geotechnical Designer shall analyze the support of the excavation system taking into account the ground conditions, potential impacts to neighboring or adjacent structures or property, wall stiffness, requirements for wall bracing systems, global stability, and sequence of construction including timing of support installations to evaluate the lateral deformations and settlements for open cut excavation methods. In locations where adjacent structures or property impacts are not significant, more relaxed site-specific criteria may be considered through the design variance process provided that overall stability is maintained.

Ground settlement predictions due to cut-and-cover excavations shall utilize empirical recommendations given by Clough and O'Rourke (1990) or numerical modeling software such as Finite Element Analyses, Plaxis, or Finite Difference modeling software such as FLAC. The Geotechnical Designer shall consider the following:

- The installation and (where appropriate) extraction of the support systems
- Movements (settlement and lateral wall deformation) at all stages of excavation
- Consolidation settlements
- The effects of grouting, piling, soil improvement, dewatering, or any other measures required for the Works that could cause ground settlements
- Seepage analyses shall be carried out for all excavations, and the potential consolidation settlements shall be assessed
- Settlement contour plans associated with excavation of cut-and-cover excavations shall be prepared and shall include immediate and consolidation settlements

### 10.12.7 Dewatering

Concrete placement of a cut-and-cover structure below a groundwater table shall be either by tremie concrete or placed in the dry. When placement in the dry method is chosen, a dewatering/groundwater control system shall be designed to permit placement of all structural elements in the dry. The bearing subgrade shall be kept dry and stable with no flowing, standing and/or piping of the groundwater permitted. Water levels within the limit of excavation shall be maintained a minimum of 5 feet below subgrade. Tremie seals, grouting, and other similar methods shall be permitted as part of dewatering/groundwater control methods.

Design and installation of a groundwater recharge system to protect nearby structures and utilities shall be performed to mitigate excessive ground settlements induced by dewatering. In addition, the dewatering system shall be designed so that the construction dewatering recharge system will not adversely impact existing fresh water aquifers.

### 10.13 Seismic Design

Seismic design requirements are also covered in the *Seismic* chapter and the *Structures* chapter. The geotechnically-focused elements of the seismic design criteria are presented in this section. Structures shall be designed to resist seismically induced forces and deformations due to ground motions resulting from an earthquake, and to meet the performance criteria specified in this document. Foundations shall be designed to address inertial loads from superstructures, liquefaction, lateral spread, and other seismic effects such that they will behave elastically under the design OBE, and no collapse under the design MCE. Earth retaining structures shall be evaluated and designed for seismic stability internally, externally, and globally. Cut slopes in soil and rock, fill slopes, and embankments having impact on the operations of high-speed trains shall be evaluated for instability due to design seismic events and associated geologic hazards.

#### 10.13.1 Design Earthquakes

For seismic design guidelines and performance requirements, refer to the *Seismic* chapter.

#### 10.13.2 Seismic Hazard and Ground Motions

For this project, preliminary design response spectra and ground motions (time histories) matching the site design response spectra has been provided to prospective design bidders for bid. Upon receipt of the site specific subsurface investigation data from the Contractor after the Notice to Proceed (NTP), the preliminary design response spectra and ground motions will be re-evaluated and updated, if necessary, by a seismic specialists team retained by the Authority for use during final design. The seismic hazard levels and new sets of input ground motions at half boundary will be developed by the seismic specialists team and provided to the Contractor for development of site-specific response analyses appropriate for structures to be constructed.

Design of site specific site response analyses and mitigation of the seismic hazard shall be the responsibility of the Contractor.

### 10.13.3 Liquefaction of Foundation Soils

Liquefaction may cause partial or total loss of shear strength of soils, thereby causing foundation instability, flow slides, lateral spreading and ground settlements. The Geotechnical Designer shall evaluate the possibility of ground failures caused by liquefaction, the potential impacts to foundations, structures, and embankments, and develop mitigation measures to satisfy performance requirements.

Liquefaction-triggering evaluations shall be performed for sites that meet the following 2 criteria:

- The estimated maximum groundwater elevation at the site is within 75 feet of the existing ground surface or proposed finished grade, whichever is lower.
- The subsurface profile is characterized in the upper 75 feet as having soils that meet the compositional criteria of soils for liquefaction with a measured Standard Penetration Test (SPT) resistance, corrected for overburden pressure and hammer energy (N<sub>1</sub>)<sub>60</sub>-cs, less than 33 blows/foot, or a cone tip resistance q<sub>c1N</sub>-cs (defined as the normalized cone tip resistance with clean sand equivalence) of less than 185 ton per square feet, or a geologic unit is present at the site that has been observed to liquefy in past earthquakes.

Liquefaction-induced movement/settlement shall be estimated and compared with the allowable deformation values required in this chapter. The Geotechnical Designer shall develop mitigation measures accordingly to meet the allowable deformation values set forth in this chapter.

Guidelines for evaluation of soil liquefaction triggering potential are presented in *Appendix 10.B – Guidelines for Geotechnical Earthquake Engineering*.

Where potential for liquefaction exists under OBE and MCE earthquakes (as confirmed by liquefaction studies by the Geotechnical Designer) and its impact on foundations/structures/embankments is not acceptable, the following remedial measures shall be considered:

- Liquefiable soils shall be removed; or
- Soil improvement techniques shall be used (refer to Section 10.9.5.5); or
- Deep foundations such as piles or drilled shafts shall be used, and shall be designed to resist and accommodate the liquefaction-induced ground movements and force demands, taking into account the reduced soil properties as a result of liquefaction.



### 10.13.3.1 Compositional Criteria for Liquefaction Susceptibility for Soils

#### A. Sandy Soils

Sandy soils with few amounts of fines that meet the above-mentioned 2 criteria shall require liquefaction triggering evaluations.

#### B. Silty and Clayey Soils

Whether silty and clayey soils meet the criteria for liquefaction susceptibility shall be evaluated primarily using the criteria developed by Bray and Sancio (2006) and compared to results by analysis using the methods presented in Idriss and Boulanger (2008). The Modified Chinese Criteria for clayey soils in the Youd et al. (2001) method shall not be used.

For fine-grained soils that do not meet the above criteria for liquefaction, the effect of cyclic softening resulting from seismic shaking shall be evaluated and its impact on foundations/structures shall be analyzed and considered in the design.

Considering the range of criteria currently available in the literature, geotechnical engineers shall consider performing cyclic triaxial or simple shear laboratory tests on undisturbed soil samples to assess cyclic response for critical cases.

#### C. Gravels

Gravel layers shall be considered potentially susceptible to liquefaction, and their liquefaction susceptibility shall be evaluated. A gravel layer that contains sufficient sand to reduce its permeability to a level near that of the sand, even if not bounded by lower permeability layers, shall be considered susceptible to liquefaction and its liquefaction potential shall be evaluated as such.

### 10.13.4 Underground Structures

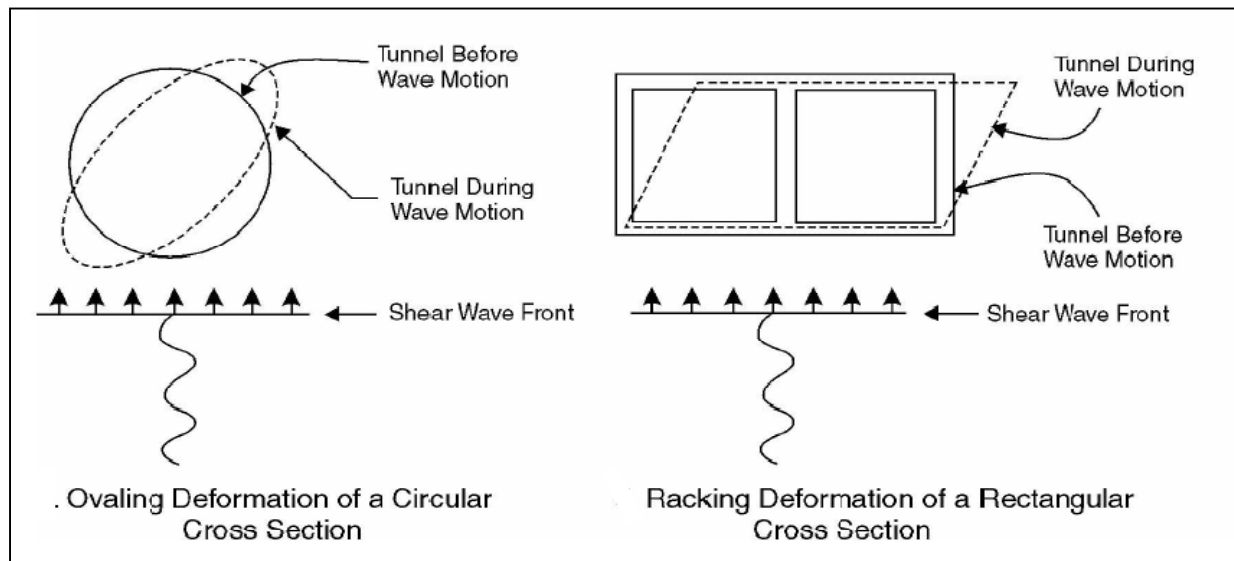
Seismic design of underground structures shall be based primarily on the ground deformation approach specified herein. During earthquakes, underground structures move together with the surrounding soil/rock mass. The structures shall therefore be designed to accommodate the deformations imposed by the ground, taking into consideration the soil-structure interaction effect.

Seismic effects on underground structures take the form of deformations that in general cannot be changed significantly by stiffening the structures. The structures shall instead be designed and detailed to withstand the imposed deformations without losing the capacity to carry applied loads and to meet the performance goals of the structures. Shear capacity degradation and compressive strains shall be evaluated. If necessary, additional confinement reinforcement shall be added to increase ductility and shear capacity.

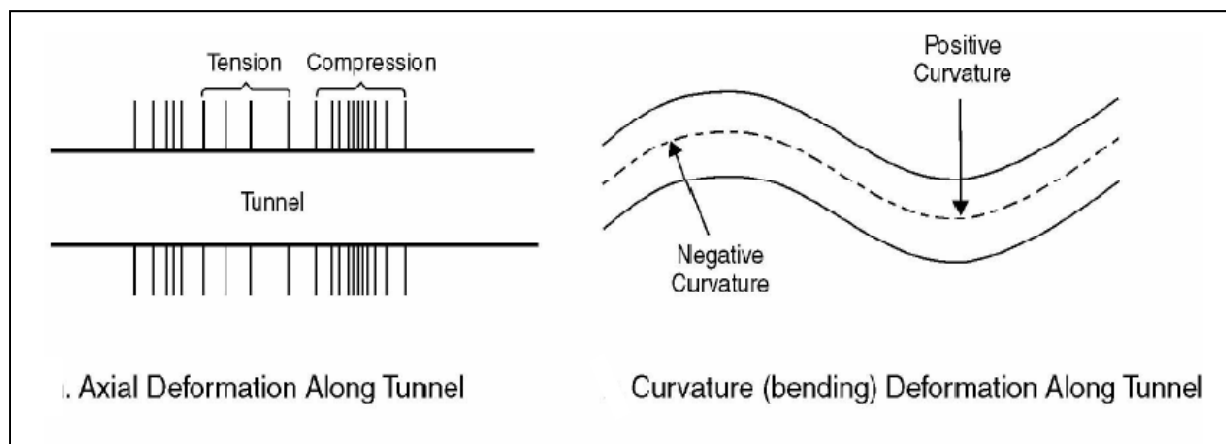
Underground tunnel structures undergo 3 primary modes of deformation during seismic shaking: ovaling/racking, axial, and curvature deformations. The ovaling/racking deformation is caused primarily by seismic waves propagating perpendicular to the tunnel longitudinal axis. Vertically propagating shear waves are generally considered the most critical type of waves for

this mode of deformation (Figure 10-9). The axial and curvature deformations are induced by components of seismic waves that propagate along the longitudinal axis (Figure 10-10).

**Figure 10-9: Tunnel Transverse Ovaling and Racking Response to Vertically Propagating Shear Waves**



**Figure 10-10: Tunnel Longitudinal Axial and Curvature Response to Traveling Waves**



### 10.13.5 Effect of Ground Deformation

#### 10.13.5.1 Transverse Ovaling Deformations

For bored circular tunnels, using either the precast concrete segmental lining or cast-in-place concrete lining, there are 2 general approaches to determining the effects of seismic ovaling deformation.

The first approach is based on closed form solution that accounts for soil-structure interaction effect. The closed form solution is based on the following assumptions: (1) the tunnel is of completely circular shape (without decks or walls inside) with uniform lining section, (2) surrounding soil is uniform, and (3) there is no interaction effect from adjacent tunnels or other structures.

The second approach is a numerical modeling approach that relies on mathematical models of the structures (including adjacent structures if relevant) to account for structural properties, varying soil stratigraphy and properties, loadings and deformations more rigorously. These structural models are generally run on computers with specialized software. If the actual soil-structure systems encountered in the field are more complex than the assumed conditions described above for the closed form solution approach (which could lead to unreliable results), then the numerical modeling approach shall be adopted.

Refer to FHWA-NHI- Report, “Technical Manual for Design and Construction of Road Tunnels”, Chapter 13 for general guidelines on transverse ovaling analysis for bored tunnels.

#### 10.13.5.2 Transverse Racking Deformations

For box type underground structures such as cut-and-cover tunnels and stations, and mined station sections that behave in similar manner as a rectangular structure during earthquake shaking, seismic design of the transverse cross section of the structure shall consider 2 loading components:

- The racking deformations due to the vertically propagating shear waves, which are similar to the ovaling deformations of a circular tunnel lining (refer to Figure 10-9)
- Inertia forces due to vertical seismic motions

There are 2 general approaches to determining the effects of seismic racking deformations:

The first approach is based on semi-closed form solution that has been calibrated with a series of numerical analyses for a number of soil-structure configurations. The semi-closed form solution is based on the following assumptions: (1) the tunnel is of rectangular shape, (2) surrounding soil is reasonably uniform, and (3) there is no interaction effect from adjacent tunnels or other structures.

The second approach is a numerical modeling approach that relies on mathematical models of the structures (including adjacent structures if relevant) to account for structural properties, varying soil stratigraphy and properties, loadings and deformations more rigorously. These

1 structural models are generally run on computers using specialized software. If the actual soil-  
2 structure systems encountered in the field are more complex than the assumed conditions  
3 described above for the semi-closed form solution approach leading to unreliable results, then  
4 the numerical modeling approach shall be adopted.

5 Refer to FHWA-NHI-09-010 Report, “Technical Manual for Design and Construction of Road  
6 Tunnels”, Chapter 13 for general guidelines on transverse racking analysis for box type  
7 structures.

### **10.13.5.3 Longitudinal Axial/Curvature Deformations**

8 The evaluation procedures for the longitudinal response (due to axial/curvature deformations)  
9 of tunnel structures shall be based on the procedures outlined in Section 13.5.2 of the FHWA-  
10 NHI-09-010 Report, “Technical Manual for Design and Construction of Road Tunnels”. The  
11 Free-Field Deformation procedure in section 13.5.2.1 of the Road Tunnel Manual may be used to  
12 evaluate the strains related to axial and longitudinal deformation of the tunnel under seismic  
13 ground motions. Supplement the analysis with Numerical Modeling Approaches similar to  
14 those in Section 13.5.2.3 of FHWA-NHI-09-010 where there are abrupt changes in structural  
15 stiffness or geological properties.

16 For the Free-Field Deformation analysis, the combined axial and bending strains shall be  
17 calculated from the P-Waves (primary waves), S-Waves (shear waves), and R-Waves (Rayleigh  
18 waves) using the formulae given in Section 13.5.2.1 of FHWA-NHI-09-010. The parameters  
19 associated with each class of wave are to be developed and provided by the Geotechnical  
20 Engineer/Seismologist.

21 Numerical modeling approach shall be used to investigate the effects of abrupt changes in  
22 structural stiffness or geological properties. Structural stiffness change locations can include the  
23 tunnel breakouts at the portals; where egress and ventilation shafts may join the tunnel; and  
24 other local hard spots. Geological changes requiring numerical modeling include areas of  
25 abrupt change in soil stiffness along the alignment. These include the interfaces between  
26 liquefiable and non-liquefiable soils and the interfaces between soft soil and rock.

27 The effect of spatial variations of ground motions on long structures resulting from the effects of  
28 wave passage and local soil overburden shall be considered. The wave-passage effect results  
29 from different arrivals of seismic waves at different parts of the structure. The wave-passage  
30 effect can be accounted for by assuming a time lag of the ground-motion time histories between  
31 any 2 locations along the tunnel alignment. This time lag can be estimated by dividing the  
32 distance between the 2 locations by the horizontal wave travelling velocity (in the ground)  $V_H =$   
33 2 km/seconds along the tunnel alignment.

34 The effect of local soil overburden is specified in Section 10.13.4.

#### 10.13.5.4 Site Response Analysis

Variations of local site conditions at different locations along the proposed tunnel alignment will have a major effect on the seismic response of the tunnel structures. The requirements and guidelines for evaluating the local site response effect on design ground motions are defined below.

Site response analyses shall be based on numerical modeling of the soil layering configuration, using site-specific soil properties along the tunnel alignment. Conventional numerical analysis software packages should be used for this process as applicable to the site specific requirements for the response analysis. Examples of commercially available software that may be appropriate include: SHAKE; PROSHAKE; SHAKE2000; DMOD; DEEPSOIL, and FLUSH.

Several analysis methods are available for evaluating the effect of local soil conditions on ground response during earthquakes. The following shall be used:

- The equivalent-linear one-dimensional total stress method
- The non-linear one-dimensional total and effective stress method
- The two- and three-dimensional equivalent-linear total stress methods
- The two- and three-dimensional non-linear total and effective stress methods

The one-dimensional site response analysis described above can be used for developing a ground displacement profile for the evaluation of the ovaling/racking effects on the seismic behavior of a tunnel's transverse section

To evaluate the tunnel's seismic performance in the longitudinal direction, the effect of subsurface variability in soil conditions along the tunnel alignment must be taken into consideration. When the soil/rock strata are highly variable and not horizontally layered, response analysis shall be performed with two-dimensional or three-dimensional modeling techniques.

For any numerical programs to be used (e.g., by finite element or finite difference methods), the Geotechnical Designer shall, prior to final design of any structural elements, verify the accuracy of such programs by a written report and with calculations that explain the theory, the input values, and the results.

#### 10.13.6 Soil-Structure Interaction for Bridges, Aerial Structures, and Grade Separations

For bridges, aerial structures, and grade separations, the following primary soil-structure interaction (SSI) effects shall be considered:

- The influence of foundation stiffness on structural response
- The inertial structural loads imparted to the foundation system – termed as the inertial effect

- The ground displacement loads imparted to the foundation system (resulting from both free-field soil displacement and ground-failure conditions such as lateral spreading or permanent seismically-induced embankment/slope movements if applicable) – termed as the kinematic effect

The soil-foundation-structure interaction problem can be solved using either a coupled or uncoupled analysis. The coupled analysis examines the behavior of the entire soil-foundation-structure system simultaneously in a single, complex model, in which non-linear soil behavior is described by a continuum model and/or non-linear soil springs (e.g., p-y, t-z, and q-z). In the uncoupled analysis, the effect of foundation stiffness on structural response is examined by replacing the foundation in the structural model with a set of springs (or stiffness matrix).

At a minimum, the soil-foundation-structure interaction effects shall be considered using the uncoupled approach using the stiffness matrix approach. In the event that a more detailed representation of the complex interactions between the superstructure, foundation, and the surrounding soil is required, a fully coupled analysis shall be conducted.

#### **10.13.6.1 Pile/Drilled Shaft Design Subject to Ground Displacements**

Ground displacement loading can be divided into 2 categories: (1) free-field ground displacement, and (2) displacement due to unstable ground such as liquefaction induced lateral spread or unstable embankments/slopes. Ground displacements impose forces acting along the length of the piles and pile cap and therefore shall be considered in the design. For the free-field ground displacements, the resulting forces can be estimated by imposing the estimated free-field ground displacement profile on the pile through p-y springs. Proper selection of the non-linear p-y properties of the surrounding soil is crucial for the design. The displacement profile can be estimated from a site response analysis. In competent sites, the free-field ground displacements generally do not govern the pile design because the curvature of the ground displacement is small. This effect, however, has to be considered for piles in soft soils and for sudden changes in soil stiffness with depth. The effect is particularly significant for large diameter piles or drilled caissons in soft soils.

Similarly, seismic soil instability resulting from geotechnical seismic hazards can produce large soil movements adversely affecting the performance of deep foundations. The p-y procedure described above is also applicable for this case. The ground displacements resulting from unstable ground require detailed analysis using site-specific data and shall be provided by the Geotechnical Designer.

The overall evaluation procedure for pile design in liquefied soil deposits would essentially be the same as that described above. However, the choice of p-y characteristics must properly consider liquefaction effects of the soils.

The LPILE computer program has the ability to impose a soil displacement profile against the pile by adjusting the location of the base of the soil springs (p-y). For calculation of loads and deformation demands on bridge foundations and abutment resulting from liquefaction induced

spreading ground, refer to Caltrans Guidelines on Foundation Loading and Deformation Due to Liquefaction Induced Lateral Spreading (2011).

#### 10.13.6.2 Effective Support Motions

Due to the complex interaction between soil, pile, and structures, the effective support motions (i.e., the near field ground motions) at the foundation/structure interface differ from those in the free field. For regular shallow footings and flexible pile-supported footings (relative to the surrounding ground), the use of free-field motions as the support motions in the structure response analysis is reasonable. For very large and stiff foundations, such as large gravity caissons, very stiff battered pile groups, or large diameter drilled shaft foundations, the effective support motions at the foundation/structure interface may differ considerably from the free-field motions. When this situation occurs, a more refined analysis taking into account the presence of the foundation and the soil-pile/shaft kinematic interaction effect shall be performed to derive the effective support motions.

### 10.14 Track Structure

A railway track structure is composed broadly of track-structure and formation. The track structure consists of rails, sleepers, and fastenings or non-ballasted track, while the formation of a track (ballasted track) is typically composed of ballast, subballast, filled/placed soil and the native ground or the subsoil. The filled/placed soil and the subsoil serve as a platform on which the track structure is constructed and are to provide a stable foundation for the subballast and ballast layers. Because of higher train speeds, dynamic forces and axle loads, design of HSR track structure requires higher and more stringent design standards than conventional railway track structure.

The track structure is subjected to cyclic loading due to high-speed train loads. Additionally, the high-speed train load also induces stress due to dynamic effect. The cyclic loading may result in progressive building or pore pressure causing large cumulative strains. The bearing capacity of the track structure has to consider not just a single load application but repeated loading as the allowable stress under repeated loading is much higher than under the static loading.

Design of track structures for trains at speeds greater than 160 mph shall consider the Rayleigh waves induced vibration due to high-speed trains (Section 10.14.3). When the speed of the high-speed train approaches the critical wave velocities in the track-ground (earth) system, large transient movements of the rail and ground will result, causing large rail deflections and formation instability as well as structural vibrations and associated noise in nearby buildings.

#### 10.14.1 Formation Supporting Ballasted Tracks

Formation, for this project, defined as layers comprising subballast, prepared subgrade, and earth fill, provides the base for ballasted track which is composed of rail track and ballast. The formation shall be designed to be safe against shear failure, and accumulated/plastic

deformations under repetitive axle loads of the trains as stated in this chapter. The subballast and prepared subgrade provide support to the ballasted track and bear additional stresses due to static and dynamic effects of moving wheel loads. The load is transmitted through the subballast, prepared subgrade, and earth fill to foundation soils.

The ballast under the rail track serves as a stress disperser. Below the ballast is the subballast overlying the prepared subgrade. This subballast layer (also referred to as the blanket layer in the UIC standards) shall be of adequate thickness to reduce the induced stresses to an acceptable level at the top of prepared subgrade to avoid shear failure. The subballast shall have adequate strength under dynamic loads and vibrations, high resilient modulus, reasonable plastic strain accumulation characteristics under repeated wheel loads, etc. Therefore, the material shall be permeable enough to avoid any positive pore pressure build-up under repeated load. It shall consist of durable particles and should not be sensitive to moisture content. In addition, it shall resist break-down and abrasion from cyclic stresses produced by the train repetitive loading.

- **Subballast** – The subballast shall conform to the following design requirements:
  - It shall be coarse, granular, and well graded as per Standard Specifications.
  - Gap-graded material shall not be permitted.
  - It shall meet the minimum Resistance (R-value), Sand Equivalent and Durability Index requirements set forth in Standard Specifications.
- **Prepared Subgrade** – Below the subballast is the prepared subgrade layer, which in its most complete form, has a cross slope. It shall consist of imported or treated material depending of the quality of the upper part of the embankment or the bottom of the cut. In addition, it shall have a gradation as specified on Figure 10-6. Its deformation modulus,  $E_{v2}$ , from the 2nd loading in the plate load test shall not be less than 11,500 psi.
- **Earth Fill** – Underlying the prepared subgrade is the fill (embankment fill/retaining structure backfill) on top of the existing foundation soils. This earth fill shall be designed against slope failure and settlement/deformation as provided earlier in this chapter.

#### 10.14.2 Determination of the Thickness of the Trackbed Layers

Trackbed layers are composed of ballast and subballast that are placed on top of the prepared subgrade overlying earthfill or existing subgrade. The dimensioning of trackbed layers shall take into account both the following:

- Desirable bearing capacity
- Problems of frost penetration

The total thickness (ballast layer plus sub-ballast layer) varies according to the following:

- Bearing capacity of the prepared subgrade



- Level of frost protection required
  - Type of tie and the tie spacing
  - Traffic characteristics (tonnage supported, axle-load, and speed)
- The thickness of the ballast varies depending on the train types, sleeper types, or whether non-ballasted tracks are used. The minimum thickness of subballast shall be 9 inches. For the prepared subgrade, a minimum thickness of 14 inches is required for ballasted tracks, whereas, a minimum thickness of 6 feet-6 inches of prepared subgrade is required for support of non-ballasted tracks unless otherwise stated in Section 10.9.5.7.

### 10.14.3 Design of Formation for Dynamic Loading from HST Operations

Knowledge of cumulative plastic deformation for foundation soils under repeated loading is essential for the proper design of HST tracks. Excessive foundation soil plastic deformation will produce high maintenance costs and undesirable ride quality.

Design methods of formation, particularly for subballast thickness, are used in different railway systems. They are based on different properties of soil used in embankment construction that governs the behavior of the soil (viz. percentage of fines less than 75 microns) present in the soil, CBR value of the soil, undrained shear strength of the soil, etc. Methods such as the Association of American Railroads (AAR) method (Li and Selig, 1998) may be used for design of the formation.

#### 10.14.3.1 Rail Deflections

Rail deflections as a result of dynamic amplification due to high-speed trains shall be considered. These deflections are a function of (1) axle load of the train, (2) thickness of the embankment fill, (3) elastic properties of the sub-soil/foundation subgrade and the damping in the system, (4) train speed, and (5) both upward and downward rail deflections during the train passages. At certain speeds of the train, “resonance” phenomena may cause rail deflections that are far larger than the static values.

Rail deflections induced by high-speed trains as a result of the dynamic amplification shall not exceed 1/12-inch and 1/6-inch for non-ballasted and ballasted trackways, respectively. These deflections are elastic and reversible after train passage. Plastic deformations induced by repeated high-speed (up to operating speed of 220 mph) train loads for non-ballasted and ballasted trackways shall be limited to 1/8 inch and 1/4 inch, respectively for its design life. These plastic deformations will be irreversible and remain after the train loads are removed. Deformation analyses using numerical modeling such as ADINA, ABAQUS, ANSYS, or PLAXIS, etc. shall be performed to verify the rail deflections are within the required limits. If such limit cannot be achieved, consideration shall be given to increasing the thickness/stiffness of the prepared subgrade, subballast/bearing base layer and/or stabilizing the foundation subgrade.

### 10.14.3.2 Existing Embankments/Retaining Structures over Soft Grounds

In addition to checking against shear/bearing failure, design of high-speed train track formation over existing embankments underlain by soft and compressible ground shall be performed to evaluate the structural integrity of the formation supporting the trackways. As mentioned in Section 10.9.5.6, for high-speed railway, the running train produces compressive (P) waves, shear (S) waves, and Rayleigh (R) waves, of which Rayleigh waves – moving parallel to the surface, are the primary source of vibration energy carried away from the source and are less prone to geometric attenuation than P- and S-waves. The propagation of vibration is dependent on the source frequency and soil properties such as stiffness, depths of strata and damping. Stiff soils have high velocity, high frequency, shorter wave characteristics, while soft soils are the converse of the above. For embankment stability, the Rayleigh wave-induced vibration by the high-speed train is an important factor to be considered for design, especially for existing embankments over soft, compressible grounds.

The velocity of a high-speed train may approach or exceed the characteristic wave velocity of the dynamic system comprising the underlying soft ground, the formation, and the moving load. As the train's velocity reaches some "critical velocity", large deformations may occur. These motions could be dangerous for the train and the integrity of the track structure, and potentially costly in terms of track maintenance and performance. It is therefore vital to design the embankments which provide a dynamic stiffness that will limit track deflections to acceptable levels (refer to Section 10.14.3.1).

For design, the critical velocity of the embankments/retaining structures shall exceed 1.7 that of the design speed of the train.

Analytical methods such as a simple elastic beam model and modern numerical modeling using Finite Element Methods (FEM) such as ADINA, ABAQUS, ANSYS, or PLAXIS shall be used to model train-induced dynamic motion. Of these methods, the Winkler model can be used as a screening process as it is a very prevalent and simple numerical model. If this screening process confirms that the required critical velocity of the embankment or retaining wall meets the design value, then the numerical modeling can be waived. However, sophisticated FEM modeling shall be used for evaluation of high-speed train induced vibration on embankments over soft and compressible grounds.

In the Winkler model, the embankment/rail/foundation material structure is simplified as a beam on an elastic or visco-elastic foundation, represented by a series of discrete springs and dashpots. The solution of the model may be used to calculate the critical velocity ( $V_{cr}$ ) (Kenny, 1954) that is equal to:

$$V_{cr} = \sqrt[4]{\frac{4kEI}{\rho^2}}$$

Where k = Spring constant per unit length of the beam;  
E = Modulus of elasticity of beam;

I = Moment of inertia of beam; and  
 $\rho$  = Mass per unit length of beam

### 10.14.3.3 Drainage of Track and Formation

Drainage requirements for non-ballasted sections of track, as well as surface drainage in general, are described in the *Drainage* chapter.

Water contained in the formation layers cause detrimental conditions in the track. Therefore, it is necessary to contain and reduce water content in the formation layers by the following measures:

- Removal of vegetation growth on surface
- Cleaning ballast bed and establishing cross fall slope at top of formation, subballast, and prepared subgrade/subgrade layers
- Provision of longitudinal drains and drainage outfall facilities
- Arrangement of lateral side drainage facilities

## 10.15 Maintenance of Geo-Structures

For the CHSTP, a Reliability, Availability and Maintainability (RAM) Program has been established. One of the key components of this program is maintainability. The RAM program requires each contractor to establish a Contractor's RAM Program Plan for the Contractor's scope of work. The Contractor's RAM Program Plan goals shall include establishing provisions for safeguarding continual performance of geo-structures including, but not limited to, embankments, retaining walls, slopes, underground (cut-and-cover) structures, trenches, tunnels, culverts, etc. The design of geo-structures shall consider long term maintenance issues including, but not limited to (1) consideration of closing roads for maintenance of retaining walls, bridges, embankments, (2) difficult access for maintenance of elements in a cut-and-cover section, (3) lack of or limited access to remove rock debris from rock slopes, etc. These and similar issues shall be included in the Contractor's RAM Program Plan, considered and addressed in the design, and shall constitute an integral part of the final design of the geo-structures to mitigate aforementioned maintenance and maintenance access concerns.

## 10.16 References

1. Anderson, D. G., G. R. Martin, I. P. Lam, and J. N. Wang. (2008). *Seismic Analysis and Design of Retaining Walls, Slopes and Embankments, and Buried Structures*, NCHRP Report 611. National Cooperative Highway Research Program, Transportation Research Board, National Research Council, Washington, DC.

2. Bray, J. D., Travasarou, T., and Zupan, J. (2010). "Seismic Displacement Design of Earth Retaining Structures." *Proceedings, Earth Retention Conference 3*, Bellevue, Washington, August 1-
3. Clough, G. W. and T. D. O'Rourke, 1990, Construction Induced Movements of In Situ Walls, Design and Performance of Earth Retaining Structures, Proceedings of a Specialty Conference at Cornell University, ASCE, New York, 439-470.
4. Fellenius, B. H. (2004). "Unified Design of Pile Foundations with Emphasis on Settlement Analysis." Honoring George G. Goble - Current Practice and Future Trends in Deep Foundations, Proceedings, Geo-Institute Geo TRANS Conference, ASCE GSP 125, ASCE, Reston, 253-275.
5. Fellenius, B. H. (2006). "Results from Long-term Measurement in Piles of Dragload and Downdrag." *Canadian Geotechnical Journal*, 43(4), 409-430.
6. Fellenius, B. H. and Siegel, T. C. (2008). "Pile Drag Load and Downdrag in a Liquefiable Event." *Journal of Geotechnical and Geoenvironmental Engineering*, ASCE, September, 1412-1416.
7. Kenny, J. T. (1954). "Steady-State Vibrations of Beam on Elastic Foundation for Moving Load." *Journal of Applied Mechanics*, December.
8. Li D., and Selig, E. T. (1998). "Method for Railtrack Foundation Design I: Development," *Journal of Geotechnical/Geoenvironmental Engineering*, Vol. 124, No. 4, pg 316-322.
9. Sherard, J. L., Dunnigan, L. P., and Talbot, J. R. (1984a). "Basic Properties of Sand and Gravel Filters," *Journal of the Geotechnical Engineering Division*, ASCE, June, pp. 684-700.
10. Sherard, J. L., Dunnigan, L. P., and Talbot, J. R. (1984b). "Filters for Clays and Silts," *Journal of the Geotechnical Engineering Division*, ASCE, June, pp. 701-718.

## Appendix 10.A: Guidelines for Geotechnical Investigations

### 10.A.1 Purpose

These guidelines represent the preferred, but not necessarily the only actions required for the development of additional geotechnical investigations. These guidelines convey a minimum standard of care in performing geotechnical investigations and are not intended as prescribed site investigation criteria or checklists.

### 10.A.2 Geotechnical Investigation Guidelines

Geotechnical investigations are to be performed by a Geotechnical Designer in collaboration with an Engineering Geologist, both of which are licensed in the State of California. The level of geotechnical investigation performed shall consider the engineering needs and amount of information necessary to achieve performance criteria, complete the design, and mitigate construction risks. Guidelines for advancing the geotechnical investigations are described in the following sections.

The Geotechnical Designer/Engineering Geologist shall be required to present the investigation results in a Geotechnical Data Report (GDR) document that contains the factual information/data gathered during the geotechnical investigations. The GDR shall minimally contain the following information:

- Summary and reference to separate geologic hazards report
- Description and discussion of the site exploration program, including specific goals and objectives
- Logs of borings, trenches, and other site investigations
- Description and discussion of field and laboratory test programs
- Results of field and laboratory testing

The high cost component of geotechnical investigations is borehole drilling; therefore, planning of the geotechnical investigations shall maximize the use of existing geologic and subsurface data, and optimize the use of geophysical testing and Cone Penetration Tests (CPTs) where warranted in order to minimize the amount and cost of drilling required and still achieve a level of knowledge commensurate with good engineering practice and judicious judgment for similar locations and applications. Geotechnical investigations shall not begin until project specific information is gathered as set forth in the following sections.

#### 10.A.2.1 Standards and Key Geotechnical Investigation Reference Documents

The ASTM test methods, Caltrans Manual, and FHWA manuals are considered the most comprehensive and applicable guideline documents for geotechnical investigation of the

CHSTP as well as federal transportation projects. Chapter 6 of the 2008 FHWA Project Development and Design Manual (PDDM) provides an overview of practice for geotechnical work and direction for understanding policies and standards for geotechnical work performed by the Federal Lands Highway (FLH) Office of FHWA. The PDDM also provides a portal to technical information and presents a high-level source of technical guidance with regard to what needs to be accomplished. The corresponding 2007 FHWA Geotechnical Technical Guidance Manual (GTGM) provides guidance as to how the work shall be done. The GTGM also provides guidance for activities where standards and standard practices do not exist and provides access to and guidance for the use of new technologies. Chapter 3 of FHWA-NHI-09-010 presents good geotechnical investigation techniques and parameters for planning, design, and construction of road tunnels. For soil and rock logging, classifications, and presentation, refer to 2010 Caltrans Soil and Rock Classification, Classification, and Presentation Manual.

### 10.A.2.2 Geotechnical Investigation Goals

The goals of geotechnical investigations project are as follows:

1. Perform additional subsurface investigations to supplement existing geotechnical data for design of structural elements including bridges, retaining walls, at-grade structures, cut-and-cover tunnels, large culverts, mast arm supports (OCS, signals), wayside equipment, and signs along the proposed alignment.
2. Identify the distribution of soil and rock types within the project limits and assess how the material properties will affect the final design and construction of the project elements.
3. Define the groundwater and surface water regimes, especially, the depth, and seasonal and spatial variability of groundwater or surface water within the project limits. The locations of confined water-bearing zones, artesian pressures, and seasonal or tidal variations shall also be identified.
4. Identify and characterize any geologic hazards that may be present within or adjacent to the project limits that may impact construction or operation of the project (e.g., faults, landslides, rockfall, debris flows, liquefaction, soft ground or otherwise unstable soils, seismic hazards). These items are vital pieces of the overall geotechnical exploration process, and the investigators must ensure that these elements are addressed.
5. Assess surface hydrological features (infiltration or detention facilities) that are required for the project, as well as evaluate pond slope angle and infiltration rates to enable estimation of the size and number of those facilities.
6. Identify suitability of onsite materials as fill and/or the suitability of nearby materials sources.

7. For structures including bridges and cut-and-cover tunnels, large culverts, signs, signals, walls, or similar structures, provide adequate subsurface information for final design and construction.
8. For tunnels, trenchless technology, or ground improvement, provide adequate information to evaluate the viability of construction methods and potential impacts to adjacent facilities.
9. For landslides, rockfall areas, and debris flows, provide adequate information to evaluate stabilization or containment methods for design and construction.
10. Develop design soil properties for engineering evaluations, including dynamic analysis to evaluate response associated with rail operations and seismic events.
11. Perform chemical assessment of groundwater and soil for the impact evaluation of existing soil and groundwater on foundation materials.
12. Substantiate the various baselines expressed in the Geotechnical Baseline Report for Bidding (GBR-B), consider those baselines in the development of the design and construction approaches, and fill in any missing information in the GBR-B accordingly to develop a Geotechnical Baseline Report for Construction (GBR-C).

### 10.A.2.3 Sequence of Geotechnical Investigations

Details on performing geotechnical investigations are provided in Section 10.A.2.4 and shall follow the general sequence listed below.

1. Review the scope of project requirements to obtain a clear understanding of project goals, objectives, constraints, values, and criteria. This information may consist of the following:
  - Project location, size, and features
  - Project element type (bridge, tunnel, station, embankment, retaining wall, etc.)
  - Project criteria (alignments, potential structure locations, approximate structure loads, probable bridge span lengths and pier locations, and cut and fill area locations)
  - Project constraints (context-sensitive design issues, right-of-way, environmental and biological assessments and permitting)
  - Project design and construction schedules and budgets
2. Review the available geologic and geotechnical data.
3. Initiate and prepare geotechnical investigation plans. Identify the anticipated required analyses and key engineering input for the final design and construction.
4. Perform field reconnaissance and geological mapping.
5. Finalize the Geotechnical Investigation Plan (GIP) and submit it to the Authority.

6. Obtain permits and rights-of-entry.
7. Perform exploration and laboratory testing for final design.
8. Compile and summarize data for use in performing engineering analyses, and prepare geotechnical data reports, geotechnical engineering reports, and geotechnical baseline report for construction.

#### 10.A.2.4 Planning Geotechnical Investigations

The planning process for geotechnical investigations requires evaluating the appropriate number, depth, spacing, and type of exploration holes, as well as sampling intervals and testing frequencies. The involvement of engineering geologists (supporting the Geotechnical Designer) is critical throughout the investigation process, from initial exploration planning through the characterization of site conditions, to assure consistency for geologic interpretation of subsurface conditions in support of developing parameters for use in phased engineering design and construction.

The geotechnical investigation program shall be carried out in phases, as appropriate to efficiently and cost-effectively characterize the project site(s).

##### 10.A.2.4.1 Desk Study

All relevant available information on the project site shall be reviewed. Available data may consist of reports, maps, journal articles, aerial photographs, historical records of previous investigations by agencies, as-built plans from construction of existing facilities, and communication with individuals with local knowledge. A Preliminary Geologic Hazards Report shall be prepared by a California Certified Engineering Geologist (CEG) in advance of geotechnical investigations. The report shall be reviewed and utilized as a basis for geologic characterization and potential geologic hazards, and for identifying proposed subsurface exploration points. The results of the geologic and seismic hazard evaluation shall be shared with the Geotechnical Designer to inform the final design. Other sources of available information include the California Geological Survey (CGS), the United States Geological Survey (USGS), Caltrans archived Logs of Test Borings (LOTBs), the GIS database developed as part of the CHSTP, and data in individual city and county records and archives.

##### 10.A.2.4.2 Field Reconnaissance

Field reconnaissance shall be conducted jointly by the Geotechnical Designer and the CEG after the desk study is completed. The following factors shall be evaluated by the field reconnaissance:

- **Geologic Report Reviews** – The Geotechnical Designer and Engineering Geologist responsible for the geotechnical investigations shall review and become familiar with geologic site characterizations and any identified geologic hazards provided in geologic hazards evaluation reports.



- 1 • **Environmental Considerations** – Potential impacts the project may have on subsurface  
2 materials, landforms, and the surrounding area shall be identified, and assessed to evaluate  
3 if project areas are governed by special regulations or have protected status.
- 4 • **Explorations** – The type(s) and amount of exploration and the kinds of samples that would  
5 best accomplish the phased project needs shall be evaluated.
- 6 • **Drilling Logistics** – The type, approximate locations, and depths of geotechnical  
7 explorations shall be defined, and approximate routes of access to each exploration location  
8 shall be evaluated. Make note of any feature that may affect the geotechnical investigation  
9 program, such as accessibility, structures, overhead utilities, evidence of buried utilities, or  
10 property restrictions. Evaluate potential water sources for use during borehole drilling  
11 operations. Evaluate potential concerns that may need to be addressed while planning an  
12 exploration program (permits, buried or overhead utilities clearance, equipment security,  
13 private property, etc.).
- 14 • **Permits** – The various types of permits that may be required shall be assessed, and all  
15 applicable jurisdictions shall be considered, which could include partner agencies,  
16 adjoining properties including railroads, Caltrans, regulatory agencies, and state and local  
17 government agencies. Local government agencies requirements could include regulations,  
18 codes, and ordinances from city, county, and departments of public works having  
19 jurisdiction. Permits could include right-of-entry, drilling and well permits, special use  
20 permits, lane closure and traffic control plans, utility clearances, etc.

#### 10.A.2.4.3 General Subsurface Profiles

21 The general subsurface profiles, once developed, will present the overall geologic conditions  
22 along the project site. Profiles should be parallel to the rail alignment, but also perpendicular in  
23 locations of major structures where future project facilities (e.g., stations or ancillary facilities) or  
24 important geologic conditions (e.g., geologic hazards needing definition) extend perpendicular  
25 to the alignment. Evaluation of these areas will allow the Geotechnical Designer (in  
26 collaboration with the Engineering Geologist) to identify the locations of supplementary  
27 explorations for final design and construction.

#### 10.A.2.4.4 Carry Out Geotechnical Investigations In Stages

28 For areas where there are no existing subsurface investigation data, conduct geophysical testing  
29 such as Spectral Analysis of Surface Wave (SASW), Multi-channel Analysis of Surface Wave,  
30 (MASW), Suspension PS Logging, Cross-hole Seismic Logging, seismic refraction tests, seismic  
31 reflection tests, or a combination of the above to measure shear wave and P-wave velocities in  
32 situ and to generalize the subsurface conditions prior to drilling CPTs and borings. The  
33 sequence of site investigation shall be as follows:

- 34 1. **Geophysical testing** – To evaluate the general subsurface conditions for areas with no  
35 available existing geologic data.

2. **CPTs** - To confirm the general subsurface conditions with measurements of pore water pressure and shear wave velocities with depth by means of using a combination of seismic cones, CPT<sub>u</sub>, and CPTs.
3. **Borings** - To refine the general subsurface conditions after CPTs are performed. Install observation wells or piezometers and inclinometers where necessary to confirm groundwater table levels, seasonal fluctuations in groundwater levels, and ground movement in the field. Perform suspension PS logging or cross-hole seismic logging at deep boreholes (180 feet or deeper) where structures will be located over river crossings or unusual geologic conditions<sup>13</sup>, and other boring locations selected by the Geotechnical Designer in collaboration with the engineering geologist.

### 10.A.2.5 Surface Explorations

Standards for surface exploration methods are provided in PDDM Section 6.3.2.2, and technical guidance is provided in GTGM Section 3.2.2. Geologic field mapping of surficial soil and rock units and measurements of rock discontinuities shall begin by observing, measuring, and recording of exposed rock structure data at existing road cuts, drainage courses, and bank exposures, as well as portal locations where profiles transition from underground segments to elevated structures or at-grade reaches. Where rock exposures exist, mapping shall include initial characterization of rock mass rating, weathering, texture, overall quality, and discontinuity orientation, spacing, and physical characteristics.

The objective of these observations and data collection efforts is to confirm the general types of soil and rock present, and topographic and slope features. For rock slopes, stability of slopes and the rockfall history are important indicators of how a new slope in the same material will perform. In addition to plotting data on a site plan or large-scale topographic map, preparation of field-developed cross sections is a valuable field method.

### 10.A.2.6 Subsurface Explorations

Relative advantages (economy, data quality, data collection time) of various methods of subsurface investigation should be considered in selecting the exploration plan. For example, geophysical methods and CPTs, which are relatively cheap and faster in operations, shall be conducted first, then followed by conventional test borings in specific situations.

---

<sup>13</sup> Unusual Geologic Conditions – Structures that are subject to and founded on the following geologic conditions:

- Soft, collapsible, or expansive soil
- High groundwater table (within 5 feet below ground surface)
- Soil having moderate to high liquefaction and other seismically induced ground deformation potential
- Soil of significantly varying type over the length of the structure
- Fault Zones
- Unusual geologic conditions shall be defined within the Geotechnical Reports.

Standards for performing subsurface explorations are provided in PDDM Section 6.3.2.2, and technical guidance is provided in GTGM Section 3.2.2. A guideline for the type of equipment and frequency of use for various types of investigations is presented in GTGM Exhibit 3.2-E. Additional guidance is contained in Caltrans (2007) logging manual.

The scope of investigation shall reflect the anticipated subsurface and surface conditions and the preliminary results presented in the GDR during the bidding phase. Some factors that may impact the prioritization (sequence order ranking), method, number, and depth of subsurface explorations include the potential geologic hazards identified; geology (soil and rock units); landslides; slope stability; rockfall; rip-ability; fill suitability; expansive soils; compressible or collapsible soils; groundwater and hydrogeology; ground-borne vibration and noise transmissivity; erosion; temporary shoring; and excavation slopes. The level of investigation, priority, and scope of work for each component shall be developed in accordance with these geotechnical investigation guidelines.

- **Geophysical Methods** – Spectral Analysis of Surface Wave (SASW), Multi-channel Analysis of Surface Wave (MASW), suspension logging, or cross-hole seismic logging shall be conducted to measure in situ shear wave and primary (P) wave velocities with depth. Shear wave and P-wave velocities are the key dynamic properties for seismic design and shall be measured in situ during geotechnical investigations.

Standards for geophysical methods are provided in PDDM Section 6.3.2.3.2. The primary source supporting the guidance is FHWA DTFH68-02-P-00083 Geophysical Methods Technical Manual (2003). Secondary sources are NHI 132031 and USACE EM 1110-1-1802. Generally, geophysical methods are used as a reconnaissance investigation to cover large areas and/or to supplement information between boreholes. These exploration techniques are most useful in providing a preliminary interpretation on a large spatial scale complementary to information from borings. The methods presented in FHWA (2003) shown as Exhibit 3.2-F of the GTGM are some of the most commonly used. The reliability of geophysical results can be limited by several factors, including the presence of groundwater, non-homogeneity of soil stratum thickness, gradation or density, the range of wave velocities or other geophysical parameters within a particular stratum and the quality of the test and the experience of the testing team.

Subsurface strata that have similar physical properties can be difficult to distinguish with geophysical methods. Geophysical methods are also applicable for testing ground-borne vibration characteristics of subsurface conditions, and assessment of this is considered important for high-speed train systems. The reference document for this testing is titled, "High-Speed Ground Transportation Noise and Vibration Impact Assessment," FRA Report No. 293630-1, December 1998.

**Cone Penetration Test, Seismic Cones, and Piezocone Penetrometer Test** – CPT is a specialized quasi-static penetration test where a cone on the end of a series of rods is pushed into the ground at a constant rate and continuous or intermittent measurements are made of the resistance to penetration of the cone. This test can be used in sands or clays, fibrous peat,

1 or muck that are sensitive to sampling techniques, but not in rock, dense to very dense  
2 sands, or soils containing appreciable amounts of gravel, and cobble. The CPT is relatively  
3 inexpensive in comparison to borings and can be used to supplement borings since boring  
4 samples are obtained for positive identification of soil types. Piezocones are electric  
5 penetrometers that are capable of measuring pore-water pressures during penetration.  
6 When equipped with time-domain sensors, cones can also be used to measure shear wave  
7 velocity.

8 Tests are conducted in accordance with ASTM D 5778 (Standard Test Method for Electronic  
9 Friction Cones and Piezocone Penetration Testing of Soils). References: Guides to CPT  
10 (Robertson, 2010), TRB-NCHRP synthesis report 368 (2007), and FHWA-SA-91-043.

- 11 • **Test Borings** – Guidance for selection of the applicable exploration methods is presented in  
12 PDDM Exhibit 6.3-A (borings). Methods for exploratory borings shall be in accordance with  
13 AASHTO and ASTM standards. Detailed information on drilling and sampling methods is  
14 given in NHI132031 which lists applicable AASHTO and ASTM drilling and sampling  
15 specifications and test methods. Additional references include AASHTO MSI-1, FHWA  
16 GEC-5, FHWA-ED-88-053, National Highway Institute (NHI) 132012, NHI132035, USACE  
17 EM 1110-1-1804, USACE EM 1110-1-1906, FHWA-FL-91-002, and Caltrans (2007).

18 For the rotary wash drilling method, the drilling fluid in boreholes shall be kept above the  
19 groundwater level at all times. Rapid fluctuations in the level of drilling fluids shall be  
20 avoided. The boreholes shall be thoroughly cleaned prior to taking samples. Drill cuttings  
21 shall be collected and disposed of in accordance with applicable regulations.

22 Disturbed samples can be used for determining the general lithology of soil deposits, for  
23 identifying soil components and general classification purposes, and for determining grain  
24 size, Atterberg limits, and compaction characteristics of soils. The most commonly used in-  
25 situ test for surface investigations is the Standard Penetration Test (SPT), AASHTO T206.  
26 The use of automatic hammers for SPT is highly recommended, and drop height and  
27 hammer weight must deliver 60 percent energy so that an energy correction is not required.  
28 The SPT values obtained with non-automatic hammers are discouraged and could be  
29 allowed when calibrated by field comparisons with standard drop hammer methods. The  
30 SPT dynamic analyzer shall be used to calibrate energy of the SPT equipment at the site at  
31 least at the start of the project and bi-weekly for long-duration site investigations. More  
32 frequent use of the SPT dynamic analyzer is encouraged.

33 Undisturbed samples shall be obtained in fine-grained soil strata for use in laboratory  
34 testing to evaluate the engineering properties of those soils. Specimens obtained by  
35 undisturbed sampling methods may be used to develop the strength, stratification,  
36 permeability, density, consolidation, dynamic properties, and other engineering  
37 characteristics of soils. Disturbed and undisturbed samples can be obtained with a number  
38 of different sampling devices, as summarized in Table 7 of FHWA GEC-5 and Table 3-4 of  
39 NHI 132031.

1 It is the responsibility of the Geotechnical Designer to obtain enough testable samples of  
2 rock and soil to complete the laboratory testing program detailed in the GIP accepted by the  
3 Authority. The quantity of each type of test conducted shall be proposed by the geotechnical  
4 investigation consultant to adequately characterize each soil or rock unit encountered.  
5 Adequate subsurface exploration and sampling is necessary to obtain sufficient samples for  
6 adequate subsurface characterization.

7 – **Sandy or Gravely Soils Sampling** – The SPT (split-spoon) samples shall be taken at 5-  
8 foot intervals or at significant changes in soil strata, whichever is more frequent.  
9 Continuous SPT samples with a gap of at least 6 inch between 2 consecutive tests are  
10 recommended in the top 15 feet of borings made at locations where spread footings may  
11 be placed in natural soils. SPT bagged samples shall be sent to lab for classification  
12 testing and verification of field visual soil identification.

13 – **Silt or Clay Soils and Peat Sampling** – The SPT or undisturbed thin wall tube samples  
14 shall be taken at 5-foot intervals or at significant changes in strata of cohesive soils.  
15 Hydraulic (Osterberg) thin-walled piston samplers shall be used in collecting medium  
16 stiff to very soft clays. Take SPT and tube samples in same borings or take tube samples  
17 in separate undisturbed borings. Tube samples shall be sent to lab to allow consolidation  
18 testing (for settlement analysis) and strength testing (for slope and embankment stability  
19 and foundation-bearing capacity analysis). The tube samples shall be retrieved by  
20 pushing soil out in the same direction that it entered the tube (i.e., through the top of the  
21 tube sampler; do not reverse and push it back out of the bottom). Field vane shear  
22 testing is also recommended to obtain in-place shear strength of soft clays, silts, and  
23 peat.

24 – **Rock Sampling** – Continuous cores shall be obtained in rock using double- or triple-  
25 tube core barrels. In structural foundation investigations, core a minimum of 10 feet into  
26 rock to ensure it is bedrock and not a boulder. Core samples shall be sent to the lab for  
27 possible strength testing (unconfined compression) if for foundation investigation.  
28 Percent core recovery and rock quality designation (RQD) value shall be evaluated in  
29 field or lab for each core run and recorded on the boring log. Additional guidelines for  
30 rock coring are described later in this section and in the reference manuals.

31 – **Groundwater in Borings** – Water level encountered during drilling, at completion of  
32 boring, and at 24 hours after completion of boring shall be recorded on the boring log. In  
33 low permeability soils such as silts and clays, a false indication of the water level may be  
34 obtained when water is used for drilling fluid and adequate time is not permitted after  
35 boring completion for the water level to stabilize (more than 1 week may be required). In  
36 such soils, a plastic pipe water observation well shall be installed to allow monitoring of  
37 the water level over a period of time. Seasonal fluctuations of the water table shall be  
38 evaluated where fluctuation will have significant impact on design or construction (e.g.,  
39 borrow source, footing excavation, excavation at toe of landslide). Artesian pressures  
40 and seepage zones, if encountered, shall also be noted on the boring log. In landslide  
41 investigations, slope inclinometer casings can also serve as water observation wells by

1 using leaky couplings (either normal aluminium couplings or PVC couplings with small  
2 holes drilled through them) and pea gravel backfill. The top 1 foot or so of the annular  
3 space between water observation well pipes and borehole wall shall be backfilled with  
4 grout, bentonite, or sand-cement mixture to prevent surface water inflow, which can  
5 cause erroneous groundwater level readings.

- 6 • **Probes, Test Pits, Trenches, and Shafts** – Guidance for selection of the applicable  
7 exploration methods is presented in PDDM Exhibit 6.3-B (probes, test pits, trenches, and  
8 shafts), and GTGM Section 3.2.3.5. The recommended primary reference is NHI 132031.  
9 Additional guidance is contained in AASHTO MSI-1 and Caltrans 2007. Exploration pits  
10 and trenches performed by hand, backhoe, or dozer allow detailed examination of the soil  
11 and rock conditions at shallow depths and relatively low cost. Exploration pits can be an  
12 important part of geotechnical explorations where significant variations in soil conditions  
13 occur (vertically and horizontally), large soil and/or non-soil materials exist (boulders,  
14 cobbles, debris) that cannot be sampled with conventional methods, or buried features  
15 must be identified and/or measured. Upon completion, the excavated test pit shall be  
16 backfilled and compacted with the excavated material or other suitable soil material, and  
17 the surface shall be restored to its previous or approved condition.

- 18 • **Soil Resistivity Testing** – The ability of soils to conduct electricity can have a significant  
19 impact on the corrosion of buried structures and the design of grounding systems.  
20 Accordingly, subsurface investigations shall include conducting appropriate investigations  
21 to obtain soil resistivity values. The following information and methodologies are  
22 recommended.

- 23 – Soil resistivity readings shall be obtained to evaluate the electric conduction potential of  
24 soils at each traction power facility (supply/paralleling/switching station), which are  
25 spaced at approximately 5-mile intervals and at major structures, such as aerial  
26 structures and freeway overpass bridges, and at tunnel portal areas.
- 27 – Where there is an absence of major structures between traction power facilities, soil  
28 resistivity readings shall be obtained to evaluate the electric conduction potential of soils  
29 at approximately the midpoint between facilities.
- 30 – Where significant differences in soil resistivity values are identified at adjacent locations,  
31 additional readings shall be obtained so that an adequate basis is developed for the  
32 grounding design.
- 33 – Resistivity measurements shall be obtained in accordance with Institute of Electrical and  
34 Electronics Engineers (IEEE) Standard 81-1983 - IEEE Guide for Measuring Earth  
35 Resistivity using the four-point method for determining soil resistivity. IEEE states that  
36 the four-point method is more accurate than the 2-point method.

- 37 • **Standards for Boring Layout and Depth** – Standards for boring layout and depth with  
38 respect to structure types, locations and sizes, and proposed earthwork are provided in  
39 these guidelines.

- 1 • **Standards for Sampling and Testing From Borings** – Minimum standards for disturbed  
2 and undisturbed soil and rock are presented in Exhibit 6.3-D of PDDM, and Section 3.2.3.3  
3 of GTGM.
- 4 • **Rock Coring** – Standards for soil and rock classification are provided in PDDM Section  
5 6.3.2.3.4, and technical guidance is provided in GTGM Section 3.2.3.4. The International  
6 Society of Rock Mechanics (ISRM) classification system shall be followed for rock and rock  
7 mass descriptions, as presented in FHWA GEC-5 FHWA-IF-02-034. The primary source  
8 supporting the standards and guidance is NHI 132031, and a secondary source is AASHTO  
9 MSI-1. Because single-tube core barrels generally provide poor recovery rates, the double-  
10 or triple-tube core barrel systems shall be used. To protect the integrity of the core from  
11 damage (minimize extraneous core breaks), a hydraulic ram shall be used to expel the core  
12 from the core barrel. Rock cores shall be photographed in color as soon as possible after  
13 being taken from the bore hole and before laboratory testing.

14 If rock is encountered in boreholes within the planned depth of drilling, continuous rock  
15 coring shall be performed in accordance with the following procedures. Rock coring shall be  
16 performed using a double or triple tube HQ coring system or other larger-diameter, double  
17 or triple-tube coring system. The HQ system produces cores 2.4 inches in diameter. The  
18 advantage of the triple tube system is that a split liner is used to contain the core, which  
19 results in relatively minimal disturbance to the core. Where weak rock zones are  
20 encountered, soil sampling techniques may be used instead of coring to recover samples  
21 that would be relatively undisturbed and suitable for testing. These techniques include the  
22 use of samplers such as the Pitcher or MC samplers. The potential difficulty with these  
23 samplers is that they can be easily damaged by hard, gravel-size particles that are often  
24 mixed with the softer, clay-like matrix of the weathered rock. These difficulties will need to  
25 be considered when planning the exploration program.

26 Rock core samples shall be placed in plastic core bags or double wrapped in plastic wrap  
27 and placed in properly labeled wooden core boxes indicating the run number and depth of  
28 each run with consistent orientation. The core boxes should be transported to a storage  
29 facility at the end of each day. An adequate number of core boxes shall be maintained on  
30 site at all times during field exploration activities. The core shall be digitally photographed,  
31 (at least 10 megapixels) taking at least 1 photo for each core box, and close-ups taken of  
32 special features such as shear zones or other features of special interest. The core box label  
33 shall be clearly visible within the photo. An experienced geologist shall study the core and  
34 edit the borehole log based on their observations. Cores boxes and photos shall be  
35 maintained throughout the design process and construction, with cores that have been  
36 removed for testing duly indicated in the appropriate locations in each box.

37 In some rock slope applications, it is important to understand the precise orientation of rock  
38 discontinuities for the design. Standards for using orienting-recovered rock core are  
39 presented in NHI 132031. In special cases, boreholes can be photographed/imaged to  
40 visually inspect the condition of the sidewalls, distinguish gross changes in lithology, and

1 identify fracture zones, shear zones, and joint patterns by using specialized television  
2 cameras. Refer to AASHTO MSI-1, Section 6.1.2.

- 3 • **Care and Retention of Samples** – Technical guidelines for soil and rock retention are  
4 provided in GTGM Section 3.2.3.7, and geotechnical boring and sample identification,  
5 handling and storage guidelines are provided in each Contract.

### 10.A.2.7 Soil and Rock Classification

6 Standards for soil and rock classification are provided in PDDM Section 6.3.2.4, and technical  
7 guidance is provided in GTGM Section 3.2.4. Soils shall be classified in accordance with the  
8 ASTM Unified Soil Classification System (USCS). Rock and rock mass descriptions and  
9 classification shall follow the ISRM classification system presented in FHWA GEC-5. Material  
10 descriptions are based on the visual-manual method, and materials classifications are based on  
11 laboratory index tests (ASTM D 2487). Additional guidance is contained in Caltrans Soil and  
12 Rock Logging, Classification, and Presentation Manual (2007).

### 10.A.2.8 Exploration Logs

13 Standards for preparing exploration field logs are provided in PDDM Section 6.3.2.5, and  
14 technical guidance is provided in GTGM Section 3.2.5.

- 15 • **Field Logs** – Field logging shall be performed by a geologist or engineer under the direct  
16 supervision of a California registered geotechnical engineer, professional geologist, or CEG.  
17 Logging shall be performed in accordance with ASTM D 5434. The location information  
18 (e.g., station, offset, elevation, and/or state plane coordinates) of all the explorations are to  
19 be recorded on the field logs. Exploration locations shall be located at the time of drilling by  
20 GPS with at least sub-10-foot accuracy. The explorations shall eventually be located by a  
21 licensed land surveyor. Required documentation for test pits shall include a scale drawing  
22 of the excavation, and photographs of the excavated faces and spoils pile. Drilling and  
23 sampling methods and in-situ measurement devices that were used shall also be  
24 documented. The field logs shall contain basic reference information at the top, including  
25 project name, purpose, specific location and elevation, exploration hole, number, date,  
26 drilling equipment, procedures, drilling fluid, person or persons logging the hole, etc. In  
27 addition to the logging descriptions of soil and rock encountered during exploration, the  
28 depth of each stratum contact, discontinuity, and lens shall be recorded. The reason for  
29 terminating an exploration hole and a list/description of instrumentation (if any) or  
30 groundwater monitoring well installed shall be written at the end (bottom) of each  
31 exploration log.
- 32 • **Final Logs** – Exploration logs shall be prepared with the gINT boring/test pit log software  
33 platform, using the formatted boring record template standardized by Caltrans (illustrated  
34 as Figures 5-12 and 5-13 in the Caltrans logging manual, 2007 version). An explanation key,  
35 known as the Boring Record Legend shall always accompany exploration logs whenever  
36 they are presented. The standardized legends to be used for CHSTP are illustrated as figures



5-14 through 5-16 of Caltrans (2007). The final edited log shall be based on the initial field log, visual classification, and the results of laboratory testing. The final log shall include factual descriptions of all materials, conditions, drilling remarks, results of field and lab tests, and any instrumentation. Where groundwater observation wells or piezometers are installed, construction details shall be included (casing size, type of casing, depth of screen length of screen, screen opening, depth and type of filter material, sanitary seal and annular backfill material). Observation wells and piezometer should also be developed by bailing, surging or overpumping to enhance communication with the surrounding strata. For observation wells and piezometers, several measurements are usually necessary within a one-week timeframe following drilling to verify that measured groundwater levels or pressures have achieved equilibrium. Where seasonal fluctuations of groundwater levels are of concern, water level measurements shall be collected on a monthly or quarterly schedule, as appropriate to establish the nature and magnitude of variability. As a minimum, final boring logs shall contain the information shown in NHI132031. AASHTO MSI-1 provides additional guidance regarding documentation for boring logs.

#### 10.A.2.9 In Situ Testing

Standards for performing in situ testing are provided in PDDM Section 6.3.2.6, and technical guidance is provided in GTGM Section 3.2.6. The primary reference is NHI1 32031. In-situ testing is very beneficial for projects where obtaining representative samples suitable for laboratory testing is difficult. Field in-situ borehole tests can be correlation tests, strength and deformation tests and permeability tests. Correlation tests primarily consist of SPTs performed in accordance with ASTM D 1596 and AASHTO T206, and Dynamic CPTs are performed in accordance with ASTM D 3441.

- In-situ soil tests may consist of the following:
  - **Cone Penetration Test (CPT)** – Refer to Section 10.A.2.6.
  - **Pressuremeter Test** – This test measures state of stress in-situ and stress/strain properties of soils by inflating a probe placed at a desired depth in a borehole. Tests are completed in accordance with ASTM D 4719. Reference FHWA-IP-89-008.
  - **Field Vane Shear Test (VST)** – This test is used on very soft to medium stiff cohesive soil or organic deposits to measure the undrained shear strength, remolded strength of the soil and soil sensitivity. Field vane shear test may provide more reliable estimate of peak and residual shear strength in cohesive soils, as disturbance from sampling and testing in laboratory is avoided. Tests are completed in accordance with ASTM D 2573 and AASHTO 223. VST is often regarded as a valuable test to estimate peak and residual shear strength in cohesive soils as disturbance from sampling and testing in the laboratory can be avoided.
  - **Flat-Plate Dilatometer Test** – This test uses pressure readings from an inserted plate at the base of a borehole to evaluate and assess stratigraphy and obtain estimates of at-rest lateral stresses, elastic modulus, and shear strength of loose to medium dense sands

(and to a lesser degree, silts and clays). Tests are completed in accordance with ASTM D 6635. Reference FHWA-SA-91-044. Care and judgment shall be undertaken for this test as it often provides information that is difficult to interpret or relate to parameters needed for engineering design.

- Hydrogeologic testing in-situ may consist of the following:

- **Permeability Tests** – Several in-situ hydraulic conductivity tests exist, with the most commonly used methods being the pumping test and the slug test. The selection of the appropriate aquifer test method for determining hydraulic properties by well techniques is described in ASTM D 4043. In general, refer to NHI1 32031, USBR Geology Manual, and NAVFACDM-7.1.

- **Pumping Test** – The pumping test requires not only a test well to pump from, but also 1 to 4 adjacent observation wells to monitor the changes in water levels as the pumping test is performed. Pumping tests are typically used in large-scale investigations to more accurately measure the permeability of an area for the design of dewatering systems. Refer to ASTM D 4050.

- **Slug Test** – The slug test is quicker to perform and much less expensive, because observation wells are not required; however, this test typically only examines a small volume of the permeable material around the instrument when compared to pumping tests. It consists of affecting a rapid change in the water level within a well by quickly injecting or removing a known volume of water or solid object, known as a slug. The water levels are monitored continuously while the natural flow of groundwater out of or into the well occurs until equilibrium in the water level is stabilized. Refer to ASTM D 4044.

- **Packer Tests** – These tests are performed in a borehole by placing packers above and below the soil/rock zone to be tested. One method is to remove water from the material being tested (Rising Water Level Method). Another method is to add water to the borehole (Falling Water Level Method and Constant Water Level Method). A third method utilizes water under pressure rather than gravity flow. The coefficient of permeability that is calculated provides a gross indication of the overall mass permeability. Refer to FHWA-TS-89-045 and NHI1 32031.

- **Open Borehole Seepage Tests** – Methods include "Falling Water Level," "Rising Water Level," and "Constant Water Level" and are selected based on the relative permeability of the subsurface soils and groundwater conditions. Further detail is provided in Chapter 6 of NHI1 32031.

- **Infiltration Tests** – Two types of infiltrometer systems are available: sprinkler type and flooding type. Sprinkler types attempt to simulate rainfall, while the flooding type is applicable for simulating runoff conditions. Applications for these tests include the design of subdrainage and dry well systems. The most common application is the falling

head test, performed by filling (flooding) a test pit hole and monitoring the rate at which the water level drops. Refer to ASTM D 4043.

Handling and disposal (or permitted discharge to storm sewer system) of water generated from hydrogeologic field testing shall be the responsibility of the Geotechnical Designer conducting the investigation work.

If the Geotechnical Designer intends to use field tests not covered in the current ASTM or referenced standards, the proposed test methods shall be submitted to the Authority for acceptance prior to start of testing.

#### 10.A.2.10 Laboratory Testing of Soil and Rock

Standards for performing laboratory testing are provided in PDDM Section 6.3.2.7 and technical guidance is provided in GTGM Section 3.2.7. Sufficient laboratory testing shall be performed to represent in-situ conditions. Exhibit 3.2-J of the GTGM provides a guideline for estimating laboratory test requirements for the different types of geotechnical analysis. Chapters 7 through 10 of NHI 132031, GEC-5, and Chapters 2 and 3 of NHI 132012 provide overviews of testing and correlations, as well as criteria to consider when planning the scope of testing programs. Additional references include AASHTO MSI-1, NHI 132012, NHI 132035, USACE EM 1110-2-1906, FHWA-FL-91-002; and Kulhawy and Mayne (1990). Exhibits 3.2-K (soil) and Exhibit 3.2-L (rock) of GTGM present a summary of the predominant laboratory tests. The proposed work plans for laboratory testing programs shall be submitted for review. Testing shall be done at a Caltrans approved facility.

If the Geotechnical Designer proposes to use laboratory tests not covered in the current ASTM or referenced standards, a variance of test methods shall be submitted to the Authority for acceptance prior to commencement.

#### 10.A.2.11 Instrumentation and Monitoring

Standards for installing and monitoring geotechnical instrumentation are provided in PDDM Section 6.3.2.8, and technical guidance is provided in GTGM Section 3.2.8. Instrumentation is used to augment standard investigation practices and visual observations where conditions would otherwise be difficult to evaluate or quantify due to location, magnitude, or rate of change. The quantity and locations of proposed geotechnical instrumentation shall be selected to suit the anticipated conditions consistent with project objectives and design requirements. The geotechnical exploration work plan shall include instrumentation work detailing locations, installation procedures, and methods to be used. The work plan shall be submitted to the Authority for acceptance prior to commencement. Additional information about inclinometers and piezometers are presented in Cornforth (2005).

### 10.A.3 Project Features Requiring Geotechnical Investigations

#### 10.A.3.1 General

The CHSTP will require geotechnical investigations of the various project features. The referenced standards and technical guidance documents shall be utilized, in addition to the primary and secondary references, where listed. Guidelines for the approximate number and depth of various exploration methods are included. In addition to the general guidelines, the scope of the investigation for the various project features shall also reflect the anticipated subsurface and surface conditions, as well as the design phase level (whether preliminary or final). Some factors that may impact the method, number, depth, and prioritization of subsurface explorations include: the type of soil or rock; presence of landslides or unstable slopes; the presence of rockfalls; rock rippability; fill suitability; presence of expansive or collapsible soils; presence of compressible soils; occurrence of groundwater and hydrogeologic features; potential for ground-borne vibrations; erosion; engineering design needs; temporary shoring; and excavation slopes.

The scope of investigation work for each component shall be developed in accordance with the guidelines contained in this section. The quantity, locations, and depths of proposed geotechnical exploration shall be selected to suit the anticipated conditions consistent with phased project objectives and design requirements. The geotechnical exploration work plan shall include information detailing methods to be used and proposed schedule. The work plan shall be submitted to the Authority for acceptance prior to commencement. If the Geotechnical Designer proposes to use exploration methods or frequencies that differ from the guidelines set forth herein or are not covered in the current reference standards, a variance for the proposed alternate exploration plans shall be submitted to the Authority for acceptance prior to commencement.

The geophysical testing and CPTs provide advantages over conventional test borings under specific situations and should be considered first.

#### 10.A.3.2 Rail Alignment and Earthwork

Standards for investigations for the at-grade rail alignment and earthwork are provided in PDDM Section 6.3.1.2.1, and technical guidance is provided in GTGM Section 3.1.2.1. Explorations are made along the proposed at-grade rail alignment for the purpose of defining the geotechnical properties of materials. This information is used to:

- Design cut and fill slopes
- Assess material suitability for embankment construction
- Define the limits of potential borrow materials
- Assess the suitability of foundation materials
- Evaluate settlement or slope stability problems

- Quantify the depths of topsoil and volumes of material to be removed
- Design remedial measures in areas of poor materials
- Aid the designer of the rail roadbed subgrade section
- Identify geologic hazards such as liquefaction and landslides
- Evaluate train induced vibrations and their impact on the embankment and adjacent structures

For cuts and fills, test borings and/or CPTs shall be advanced at least every 200 feet (for erratic or rapidly changing conditions) to 400 feet (for uniform conditions) along the project alignment where cuts or fills are anticipated. For large cuts or fills (e.g., 30 feet or more in height) an additional boring near the top of the proposed cut and toe of the proposed fill to evaluate cut/fill feasibility and overall stability may be necessary. Depths of the borings shall be at least 3 times the vertical height of the fill (or 40-foot minimum depth) and at least 15 feet below the base of the cut. If soft or poor soils are encountered, additional depth to competent material or 10 feet into rock will be needed to define the subsurface conditions.

### 10.A.3.3 Structures

Standards for structures and geotechnical hazards are provided in PDDM Section 6.3.1.2.3, and technical guidance is provided in GTGM Section 3.1.2.3 and Exhibit 3.1-B Guideline “Minimum Boring” Criteria. Structures and geotechnical hazards will primarily consist of the following:

- Bridges, aerial structures, and grade separations
- Stations
- Buildings
- Retaining walls
- Tunnels and portals
- Large culverts
- Mast-arm supports (OCS, signals, message signs)
- Landslides
- Faults

For bridges, 1 boring shall be drilled at the substructure unit under 100 feet in width and 2 borings per substructure unit over 100 feet in width, both drilled to a depth of 20 feet below pile/shaft tip elevation or 2 times maximum pile group dimension, whichever is greater or to a depth of a minimum of 10 feet into bedrock. In addition, at least 1 seismic cone, suspension PS logging, or SASW shall be conducted at each bridge to measure shear wave and P-wave velocities in situ, each to a depth of 100 feet or deeper. The number of the seismic cones, suspension loggings, and SASW shall increase if the bridge is of multiple long spans (greater

1 than 350 feet) and/or if the bridge is located in erratic soil conditions with soft, compressible and  
2 loose saturated soils.

3 For buildings and stations, 1 boring shall generally be made at each corner and 1 in the center.  
4 This may be reduced for small buildings. For extremely large buildings and stations or highly  
5 variable site conditions, 1 boring shall be taken at each support location. Refer to building  
6 foundation manuals and CBC (codes) for additional guidance in planning geotechnical  
7 investigations. In addition, areas of influence of the building/station and/or of surrounding  
8 geologic or geotechnical issues shall be considered in defining the extent of explorations.

9 For retaining walls, the minimum site exploration will be 1 boring or 1 CPT (or both) at 100 to  
10 200 foot intervals, each drilled to a depth of 0.75 to 1.5 times wall height or to a competent  
11 stratum if potential deep stability or settlement is a problem. The boring and CPT can be  
12 interchangeable and located both at the front of and back of the wall face.

13 Due to the extreme variability of conditions under which tunnels are constructed and the  
14 complexity of the projects, it is difficult to provide specific recommendations for tunnel  
15 investigation criteria. In general, boring footage is typically on the order of 1.5 to 3.0 linear feet  
16 of borehole per route foot of tunnel, and site exploration budgets are typically on the average of  
17 3 percent of the estimated tunnel cost. To characterize the rock in a proposed tunnel zone, rock  
18 borings should be advanced to depths such that they extend at least 1.5 to 2 times the tunnel  
19 diameter below the tunnel invert elevation. Criteria shall be established for each project reach  
20 on an individual basis and be based on the complexity of the geology and the length and depth  
21 of the tunnel. FHWA-IF-05-023 and U. S. National Committee on Tunneling (USNCTT, 1995)  
22 shall be considered the primary references.

23 For culverts, a minimum of 1 boring per major culvert drilled to a competent stratum or to a  
24 depth of twice the culvert height, whichever is less.

25 Standard foundations for sign bridges, cantilever signs, cantilever signals, and strain pole  
26 standards are based on allowable lateral bearing pressure and angle of internal friction of the  
27 foundation soils. The determination of these values may be estimated by SPT and CPT. One  
28 CPT or 1 boring shall be made at each designated location. CPT soundings shall be drilled to at  
29 least 50 feet into firm ground. Borings shall extend 50 feet into suitable soil or 5 feet into  
30 competent rock. Deeper borings may be required for posts with higher torsional loads or if large  
31 boulders are anticipated. Other criteria are the same as for bridges.

32 In addition to the above structures, any structure such as signage or other design features shall  
33 be addressed with regard to their potential influence and evaluated, as needed.

#### **10.A.3.4 Landslides and Slope Instability**

34 Standards for investigations for landslides are provided in PDDM Section 6.3.1.2.4, and  
35 technical guidance is provided in Section 3.1.2.4 and Exhibit 3.1-B of the GTGM. A minimum of  
36 3 borings shall be advanced along a line perpendicular to centerline or planned slope face to

1 establish geologic cross sections for stability analysis. The number of cross sections depends on  
2 the extent of the slope stability problem. For active slides, place at least 1 boring each above and  
3 below the sliding area. The borings shall be extended to an elevation below active or potential  
4 failure surfaces and into hard stratum, or to a depth for which failure is unlikely because of  
5 geometry of the cross section. If slope inclinometers are used to locate the depth of an active  
6 slide, they must extend to a depth below the base of the slide. Observation wells and/or  
7 piezometers at selected depths will also be required to evaluate the groundwater table in the  
8 soil/rock mass.

### 10.A.3.5 Faults

9 At locations where active faulting is suspected to be coincident with or within the area of  
10 CHSTP operations and facilities, a geologic reconnaissance will be required to ground-truth  
11 mapped fault traces. This reconnaissance shall be carried out by means of interpretations of  
12 aerial photos, LiDAR data, satellite imagery, and topographic information. The locations shall  
13 be reviewed in the field to assess the presence of geomorphic features associated with faulting  
14 such as escarpments, pressure ridges, sag ponds, seeps/springs, vegetation contrasts, or  
15 deflected drainages. All such features shall be documented on a geologic field map. If sufficient  
16 field data is available to document that the fault or fault zone is outside the footprints of the  
17 high-speed train operations, no further fault evaluation is required. Otherwise, a site specific  
18 investigation including paleo-seismic trenching will be necessary.

19 If existing paleo-seismic trenching data is available, it may be reviewed and used as a basis for  
20 locating the fault and providing its rupture characteristics for final design; however, if either a  
21 known active fault or suspected active fault is located near or at the location of a project facility,  
22 exploratory trenching across the fault will be required to assess its rupture characteristics for  
23 input to final design. Additional guidance will be provided when characterizing active faults  
24 that may produce surface rupture.

### 10.A.3.6 Construction Material Sources

25 Standards for investigations for construction material sources are provided in PDDM Section  
26 6.3.1.2.2, and technical guidance is provided in Section 3.1.2.2 and Exhibit 3.1-B of the GTGM.  
27 Borings shall be spaced every 100 to 200 feet. The depth of exploration shall extend 5 feet  
28 beyond the base of the deposit, or to a depth required to provide the needed quantity of borrow  
29 material. These investigations shall evaluate the quality and quantity of materials available at  
30 existing and prospective sources within the vicinity of a project. These materials could include  
31 gravel base, crushed surfacing materials, pavement and concrete aggregates, riprap, wall  
32 backfill, borrow excavation, and select backfill materials. The evaluation may consider existing  
33 government-owned material sources, existing commercial material sources, expansion of  
34 existing sources, and development of new material sources.

### 10.A.3.7 Hydrological Features – Infiltration and Detention Facilities

For surface hydrological features (infiltration or detention facilities) that may be needed, at least 1 boring per site shall be obtained to assess feasibility and define groundwater conditions. Boring depths will depend on the nature of the subsurface conditions encountered and the depth of influence of the geotechnical feature. Borings shall extend at least 20 feet below the likely base elevation of the facility, or five times the maximum anticipated ponded water depth, whichever is greater. Observation wells and/or piezometers shall be installed and monitored for at least 1 year to assess yearly highs and lows for the groundwater.

### 10.A.3.8 Pavement

Pavements are not a significant component of the HST trackway alignment design but will be an extensive design element for station areas, access roads, grade separations, and surface road reconstruction. Standards for investigations for pavement subgrade are provided in PDDM Chapter 6, Section 6.3.1.2.5 and Chapter 11, and technical guidance is provided in GTGM Section 3.1.2.5. Other sources supporting investigation standards and guidance are NHI 132031, AASHTO MSI-1, and FHWA GEC-5. For design of pavement, refer to *Civil* chapter for details.

### 10.A.4 References

1. American Association of State Highway and Transportation Officials (AASHTO)
  - Manual on Subsurface Investigations, MSI-1, 1988.
  - Standard Recommended Practice for Decommissioning Geotechnical Exploratory Boreholes, AASHTO R 22-97, standard Specifications, 2005.
  - Specification for Transportation Materials and Methods of Sampling and Testing, Part II: Tests, HM-28-M, 2008.
2. American Society of Civil Engineers (ASCE) reference titled “Geotechnical Baseline Reports for Construction – Suggested Guidelines”, ASCE 2007.
3. American Railway Engineering and Maintenance of Way Association (AREMA) – Manual for Railway Engineering, 2008 Edition.
4. ASTM, Annual Book of ASTM Standards, 2008 Edition.
5. Caltrans, Soil and Rock Logging, Classification, and Presentation Manual, June 2010.
6. Cornforth, D.H., Landslides in Practice: Investigations, Analysis, and Remedial/Preventive Options in Soils, Chapter 4, John Wiley & Sons 2005.
7. Federal Highway Administration (FHWA):
  - Geotechnical Technical Guidance Manual, May 2007.
  - Project Development and Design Manual (Draft) – Chapter 6 - Geotechnical, April 2011.



- 1 – Checklist and Guidelines for Review of Geotechnical Reports and Preliminary Plans and  
2 Specifications, FHWA-ED-88-053, 1988, revised February 2003.
- 3 – Road Tunnel Design Guidelines, FHWA-IF-05-023, 2004.
- 4 – Geophysical Methods - Technical Manual (Application of Geophysical Methods to  
5 Highway Related Problems, cooperatively with Blackhawk Geosciences), DTFH68-02-P-  
6 00083, 2003.
- 7 – Soils and Foundations Workshop, NHI Course No. 132012, Volumes I and II FHWA-  
8 NHI-06-088, and FHWA-NHI-06-089, 2006.
- 9 – Subsurface Investigations – Geotechnical Site Characterization, NHI Course Manual No.  
10 132031, FHWA-NHI-01-031, 2002.
- 11 – Evaluation of Soil and Rock Properties, Geotechnical Engineering Circular No. 5,  
12 FHWA-IF-02-034, 2002.
- 13 8. Federal Railroad Administration (FRA), High-Speed Ground Transportation Noise and  
14 Vibration Impact Assessment, FRA Report No. 293630-1, December 1998.
- 15 9. ISRM, Suggested Methods for the Quantitative Description of Discontinuities in Rock  
16 Masses, 1981.
- 17 10. Kulhawy, F.H. and Mayne, P.W., Manual on Estimating Soil Properties for Foundation  
18 Design, EPRI Report EL-6800, 1990.
- 19 11. U. S. Army Corps of Engineers (USACE), Geotechnical Investigations, Engineering Manual,  
20 EM 1110-1-1804, Department of the Army, 2001.
- 21 12. U. S. Army Corps of Engineers (USACE), Soil Sampling, Engineering Manual, EM 1110-1-  
22 1906, Department of the Army, 1996.



## Appendix 10.B: Guidelines for Geotechnical Earthquake Engineering

### 10.B.1 Purpose

These guidelines represent a preferred, but not necessarily the only required actions needed for a particular design feature associated with earthquake engineering. These guidelines convey a minimum standard of care in performing earthquake engineering design. These are not intended as a prescribed design criteria or checklist.

### 10.B.2 Seismic Design Criteria

Seismic design criteria for geotechnical earthquake engineering have been established in terms of 2 levels of project performance criteria: No Collapse Performance Level (NCL) and Operability Performance Level (OPL) as noted in the *Seismic* chapter of the Design Criteria.

Geotechnical seismic design shall be consistent with the philosophy for structural design for the 2 performance levels. The performance objective shall be achieved at a seismic risk level that is consistent with the seismic risk level required for that seismic event. Slope instability and other seismic hazards such as liquefaction, lateral spread, post-liquefaction pile down drag, and seismic movement/settlement may require mitigation to ensure that acceptable performance is obtained during a design seismic event. The Geotechnical Designer shall evaluate the potential for differential movement/settlement between mitigated and non-mitigated soils. Additional measures may be required to limit differential movement/settlements to tolerable levels both for static and seismic conditions. The foundations shall be designed to address liquefaction, lateral spread, and other seismic effects to prevent collapse. All earth-retaining structures shall be evaluated and designed for seismic stability internally and externally. Cut slopes in soil and rock, fill slopes, and embankments, especially those that could have significant impact on high-speed train operation, shall be evaluated for instability due to design seismic events and associated geologic hazards.

#### 10.B.2.1 Liquefaction Triggering and Consequences

Evaluation of soil liquefaction triggering potential shall be performed in 2 steps. The first step involves evaluating whether the soil meets the compositional criteria necessary for liquefaction. These compositional criteria are presented in Section 10.12.2.

For soils meeting the compositional criteria, the next step is to evaluate whether the design level ground shaking is sufficient to trigger liquefaction given the soil's in-situ penetration resistance. If it is assessed that liquefaction will be triggered, the engineering consequences of liquefaction shall be evaluated. In addition to triggering for liquefaction, the Geotechnical Designer shall consider the allowable deformation and the long-term, post-construction performance requirements for earth and fill conditions.

1 For fine-grained soils (especially soils that are potentially sensitive) that do not meet the  
2 compositional criteria for liquefaction, the impact of cyclic softening resulting from seismic  
3 shaking shall be evaluated. Considering the range of criteria currently available in the literature,  
4 the Geotechnical Designer shall consider performing cyclic triaxial or simple shear laboratory  
5 tests on undisturbed soil samples to assess cyclic response for critical cases.

6 For gravels, field investigation methods appropriate for soil layers containing gravels include  
7 the Becker Hammer Penetration Test (BPT), Large Sampler Penetration Test (LPT), and small  
8 interval SPT. Seed et al. (2003) discusses different methods for performing liquefaction analysis  
9 in coarse and gravelly soils.

### **10.B.2.2 Liquefaction Triggering Evaluations**

10 Liquefaction-triggering evaluations shall be performed for sites that meet the 2 design criteria  
11 established in the *Geotechnical* chapter:

12 CPT and/or CPT<sub>u</sub> (with pore water pressure measurement) shall be used as the primary method  
13 of field investigation for liquefaction analysis where it can be advanced without premature  
14 refusal. Where CPT data are unavailable, SPT values can be used as the liquefaction evaluation  
15 method where borings are performed. LPT, shear wave velocity (Vs), or BPT shall be used in  
16 soils difficult to test using SPT and CPT methods, such as gravelly soils. In addition, small  
17 interval SPT (blow counts measured for every 1 inch) shall be used in gravelly soils. More  
18 rigorous, nonlinear, dynamic, effective stress computer models may be used for site conditions  
19 or situations that are not modeled well by the simplified methods.

#### **10.B.2.2.1 Simplified Procedures**

20 All 3 simplified methods by Youd et al. (2001), Seed et al. (2003), and Idriss and Boulanger  
21 (2008) shall be used for liquefaction-triggering analysis for each boring and/or CPT. Results in  
22 terms of FOS shall be reported. Results of these analyses shall be interpreted according to the  
23 following. If the FOS values between the 3 methods are within 20 percent of each other, an  
24 average FOS shall be reported for that particular boring and/or CPT. If the FOS values from  
25 these 3 methods vary by more than 20 percent and use of the more conservative results for  
26 design would have significant cost consequences, some additional evaluations may be  
27 warranted. The additional evaluations shall include an assessment of which method best  
28 applies to this specific case, additional soil-specific field and laboratory testing, and/or review  
29 by an expert panel.

30 The potential consequences of liquefaction and (if necessary) liquefaction hazard mitigation  
31 measures shall be evaluated if the FOS against liquefaction is less than 1.1.

#### **10.B.2.2.2 Liquefaction-Induced Movement/Settlement**

32 Both dry and saturated deposits of loose granular soils tend to densify and settle during and/or  
33 following earthquake shaking. Methods to estimate movement/settlement of unsaturated

granular deposits are presented in Section 10.B.2.8. Liquefaction-induced total ground settlement of saturated granular deposits shall be estimated using Zhang et al. (2002) and at least 1 of the following methods: Ishihara and Yoshimine (1992), Idriss and Boulanger (2008), and Cetin et al. (2009). If a laboratory-based analysis of liquefaction-induced settlement is needed for fine-grained soils, laboratory cyclic triaxial shear or cyclic simple shear testing may be used to evaluate the liquefaction-induced vertical settlement in lieu of empirical SPT- or CPT-based criteria. Even when laboratory-based volumetric strain test results are obtained and used for design, the empirical methods shall be used to qualitatively check the reasonableness of the laboratory test results.

It should be noted that all of these estimates are free-field settlements, and structural movement/settlements resulting from soil liquefaction are more important in most of the cases (Bray and Dashti, 2010). Structural movement/settlements may also result from shear-induced movements. Hence, methods that are used for estimating shear-induced ground movements may be required.

#### 10.B.2.2.3 Liquefied Residual Strength Parameters

Unless soil-specific laboratory performance tests are conducted as described later in this section, residual strengths of liquefied soil shall be evaluated using at least 2 of these procedures: Seed and Harder (1990), Idriss and Boulanger (2008), Olson and Stark (2002), and Kramer and Wang (2011). Design liquefied residual shear strengths shall be based on weighted average of the results; Ledezma and Bray (2010) may be used as a reference to select a reasonable weighting scheme.

Results of laboratory cyclic triaxial shear or cyclic simple shear testing may be used to evaluate the residual strength for fine-grained soils that can be sampled with minimal disturbance in lieu of empirical SPT- or CPT-based criteria. Even when laboratory based test results are obtained and used for design, 2 of the above empirical methods shall be used to qualitatively check the reasonableness of the laboratory test results. It shall be noted that SPT N fines content corrections for residual strength calculations are different than corrections for liquefaction triggering and settlement.

#### 10.B.2.2.4 Surface Manifestations

The assessment of whether surface manifestation of liquefaction (such as sand boils, ground fissures, etc.) will occur during earthquake shaking at a level-ground site that is not within a few hundred feet of a free face shall be made using the method outlined by Ishihara (1985) and shall be compared against results by the method presented in Youd and Garris (1995). It is emphasized that settlement may occur, even with the absence of surface manifestation. The Ishihara (1985) method is based on the thickness of the potentially liquefiable layer (H2) and the thickness of the non-liquefiable crust (H1) at a given site. In the case of a site with stratified soils containing both potentially liquefiable and non-liquefiable soils, the thickness of a potentially liquefiable layer (H2) shall be estimated using the method proposed by Ishihara (1985) and

1 Martin et al. (1991). If the site contains potential for surface manifestation, then use of mitigation  
2 methods shall be evaluated.

### 10.B.2.3 Evaluation of Lateral Spreading and Consequences

3 Lateral spreading shall be evaluated for a site if liquefaction is expected to trigger within 50 feet  
4 of the ground surface, and either a ground surface slope gradient of 0.1 percent or more exists,  
5 or a free face conditions (such as an adjacent river bank) exists. Use Shamoto et al. (1998) as a  
6 method to assess the maximum distance from the free face where lateral spreading  
7 displacements could occur. Historic and paleoseismic evidence of lateral spreading is valuable  
8 information that shall also be reviewed and addressed. Such evidence may include sand boils,  
9 soil shear zones, and topographic geometry indicating a spread has occurred in the past.

#### 10.B.2.3.1 Methodologies for Predicting Lateral Spreading

10 If there is a free face condition, the post-liquefaction flow failure FOS of an earth slope or  
11 sloping ground shall be estimated per Section 10.B.2.9.1 before estimating liquefaction-induced  
12 lateral movements. If the post-liquefaction stability FOS is less than 1.0 then empirical or  
13 analytical methods cannot generally be used to reliably predict the amount of ground  
14 movement.

15 In order to predict the permanent deformations resulting from the occurrence of lateral  
16 spreading during earthquake loading, several methods of analyses are available. These methods  
17 of analyses can be categorized into 2 general types: Empirical Methods and Analytical Methods.

18 **Empirical Methods** – The most common empirical methods to estimate lateral displacements  
19 are Youd et al. (2002), Bardet et al. (1999), Zhang et al. (2004), Faris et al. (2006) and Idriss and  
20 Boulanger (2008). Analysts shall be aware of the applicability and limitations of each method.  
21 Lateral displacements shall be evaluated using the Zhang et al. (2004) method and at least 1 of  
22 the other methods described above.

23 Empirical methods shall be used as the primary means to estimate deformations due to lateral  
24 spreading. Multiple models shall be considered, and the range of results shall be reported.

25 **Analytical Methods** – For cases where slope geometry, structural reinforcement, or other site-  
26 specific features are not compatible with the assumptions of the empirical methods, the  
27 Newmark sliding block analyses shall be used. Newmark analyses shall be conducted similar to  
28 that described in the seismic slope stability section, except that estimation of the yield  
29 acceleration ( $k_y$ ) shall consider strength degradation due to liquefaction. In addition, numerical  
30 methods using finite elements and/or finite difference approach may be used.

31 The Geotechnical Designer shall compare the estimated lateral spread values with the allowable  
32 deformation values and develop mitigation plans described in Section 10.B.2.4, if necessary. The  
33 Geotechnical Designer shall consider the long-term, post-construction performance  
34 requirements for earth-and-fill conditions.

#### 10.B.2.4 Analysis for Design of Liquefaction Mitigation

During the liquefaction evaluation, the engineer shall evaluate the extent of liquefaction and potential consequences such as bearing failure, slope stability, and/or vertical and/or horizontal deformations. Similarly, the engineer shall evaluate the liquefaction hazard in terms of depth and lateral extent affecting the structure in question. The lateral extent affecting the structure will depend on whether there is potential for large lateral spreads toward or away from the structure and the influence of liquefied ground surrounding mitigated soils within the perimeter of the structure.

Large lateral spread or flow failure hazards may be mitigated by the implementation of containment structures, removal or treatment of liquefiable soils, modification of site geometry, structural resistance, or drainage to lower the groundwater table.

Where liquefiable clean sands are present, geotechnical evaluations for design shall consider an area of softening due to seepage flow occurring laterally beyond the limit of improved ground a distance of two-thirds of the liquefiable layer thickness, as described in studies by Iai et al. (1988). To calculate the liquefiable thickness, similar criteria shall be used as that employed to evaluate the issue of surface manifestation by the Ishihara (1985) method. For level ground conditions where lateral spread is not a concern or the site is not a water front, this buffer zone shall not be less than 15 feet and it is likely not to exceed 35 feet when the depth of liquefaction is considered as 50 feet, and the entire soil profile consists of liquefiable sand.

The performance criteria for liquefaction mitigation, established during the initial investigation, shall be in the form of a minimum and average penetration-resistance value associated with a soil type (fines content, clay fraction, USCS classification, CPT soil behavior type index  $I_c$ , normalized CPT friction ratio), or a tolerable liquefaction settlement as calculated by procedures discussed earlier. The choice of mitigation methods will depend on the extent of liquefaction and the related consequences. In general, options for mitigations are divided into 2 categories: ground improvement options and structural options.

#### 10.B.2.5 Ground Improvement Options

Refer to Section 10.9.5.5.

#### 10.B.2.6 Structural Options

Structural mitigation involves designing the structure to withstand the forces and displacements that result from liquefaction. In some cases, structural mitigation for liquefaction effects may be more economical than soil improvement mitigation methods. However, structural mitigation may have little or no effect on the soil itself and may not reduce the potential for liquefaction. With structural mitigation, liquefaction and related ground deformations will still occur. The structural mitigation shall be designed to produce acceptable structural performance (consistent with the requirements for the 2 design earthquakes) in terms of liquefaction/lateral spread-induced displacements and structural damage. The appropriate

means of structural mitigation may depend on the magnitude and type of liquefaction-induced soil deformation or load.

Depending on the type of structure and amount and extent of liquefaction, common structural options to be considered are as follows:

- Piles or caissons extending to non-liquefiable soil or bedrock below the potentially liquefiable soils
- Post-tensioned slab foundation (appropriate only for small, lightly loaded structures)
- Continuous spread footings having isolated footings interconnected with grade beams
- Mat foundation (appropriate only for small, lightly loaded structures)

Details, applicability, and limitations of these techniques can be found in Martin and Lew (1999). Additional requirements for design of piles in liquefied soil are presented in Section 10.B.2.7.

#### **10.B.2.7 Seismic Considerations for Lateral Design of Piles in Liquefiable Soils**

Seismic considerations for lateral design of pile/shaft design in soils include the effects of liquefaction on the lateral response of piles/shafts and designing for the additional loads due to lateral spread and/or slope failures. Effects of liquefiable soils shall be included in the lateral analysis of piles/shafts by using appropriate p-y curves to represent liquefiable soils. Liquefied soil p-y curves shall be estimated using the static API sand model reduced by a p-multiplier using the method of Brandenberg, et al. (2007) and Boulanger, et al. (2007).

The displacement-based approach for evaluating the impact of liquefaction-induced lateral spreading loads on deep foundation systems that shall follow Caltrans' "Guidelines on Foundation Loading and Deformation Due to Liquefaction Induced Lateral Spreading," dated February 2011. However, the liquefaction susceptibility and triggering analyses performed as part of this procedure shall be based on Section 10.B.2.1 and Section 10.B.2.2, respectively. Similarly, the lateral spread estimates shall be based on Section 10.B.2.3. The Geotechnical Designer shall compare the estimated lateral spread values with the allowable deformation values and develop mitigation plans described in Section 10.B.2.4, if necessary. The Geotechnical Designer shall also consider the long-term, post-construction performance requirements for earth-and-fill conditions.

Numerical methods incorporating finite element and/or finite difference techniques may be used to assess pile response in laterally spreading soils.

#### **10.B.2.8 Seismic Settlement of Unsaturated Soils**

Seismically induced settlement of unsaturated granular soils (dry sands) shall be estimated using procedures provided by Tokimatsu and Seed (1987). Estimated values in terms of total and differential settlements shall be reported.



1 The Geotechnical Designer shall compare the estimated settlement values with the allowable  
2 deformation values and develop mitigation plans described in Section 10.B.2.4, if necessary. The  
3 Geotechnical Designer shall also consider the long-term, post-construction performance  
4 requirements for earth-and-fill conditions.

### 10.B.2.9 Seismic Slope Stability and Deformation Analyses

5 Instability of slopes during seismic loading could be due to liquefaction or due to inertial  
6 loading, or a combination of both. In this section, instability of both the natural existing slopes  
7 and embankment slopes is addressed.

8 The Geotechnical Designer shall compare the estimated deformation values with the allowable  
9 deformation values and develop mitigation plans described in Section 10.B.2.4, if necessary. The  
10 Geotechnical Designer shall also consider the long-term, post-construction performance  
11 requirements for earth-and-fill conditions.

#### 10.B.2.9.1 Liquefaction-Induced Flow Failure

12 Liquefaction leading to catastrophic flow failures driven by static shearing stresses that result in  
13 large deformation or flow shall also be addressed by the Geotechnical Designer. These flow  
14 failures may occur near the end of strong shaking or shortly after shaking and shall be  
15 evaluated using conventional limit equilibrium static slope stability analyses. The analysis shall  
16 use residual undrained shear strength parameters for the liquefied soil assuming seismic  
17 coefficient to be zero (i.e., performed with  $K_h$  and  $K_v$  equal to zero). The residual strength  
18 parameters estimated using the method presented in Section 10.B.2.2.3 shall be used. In addition,  
19 strength reduction due to cyclic degradation versus strength increase due to the effects of rate of  
20 loading shall be considered for normally consolidated clayey layers and non-liquefiable sandy  
21 layers. Chen et al. (2006) have discussed the effects of different factors on the dynamic strength  
22 of soils. The analysis shall look for both circular and wedge failure surfaces. If the limit  
23 equilibrium FOS is less than 1.1, flow failure shall be considered likely. Liquefaction flow failure  
24 deformation is usually too large to be acceptable for design of structures, and some form of  
25 mitigation will likely be needed. However, structural mitigation may be acceptable if the  
26 liquefied material and any overlying crust flow past the structure and the structure and its  
27 foundation system can resist the imposed loads.

28 If the FOS for this decoupled analysis is greater than 1.1 for liquefied conditions,  $k_y$  shall be  
29 estimated using pseudo-static slope stability analysis. The same strength parameters as used  
30 during the flow failure analysis shall be used. A new critical failure plane shall be searched  
31 assuming both circular and non-circular failure surfaces. Yield acceleration is defined as the  
32 minimum horizontal acceleration in a pseudo-static analysis for which FOS is 1.0. Using the  
33 estimated  $k_y$  values, deformations shall be estimated using simplified methods such as Makdisi  
34 and Seed (1978) and Bray and Travarasrou (2007). Other methods such as Newmark time history  
35 method or more advanced methods involving numerical analysis may be used, but shall be  
36 checked against the simplified methods.

1 For pseudo-static analyses to estimate  $k_y$  values, residual strengths for the liquefied layers and  
2 reduced strengths for normally consolidated clayey and saturated sandy layers with excess pore  
3 water pressure generation (as described earlier) shall be used. This is generally a conservative  
4 approach but is appropriate for initial engineering design. For final design more advanced  
5 methods involving numerical analyses may be used to better characterize the initiation of  
6 liquefaction and pore pressure generation and subsequent reduction in strength.

#### 10.B.2.9.2 Slope Instability Due to Inertial Effects

7 Pseudo-static slope stability analyses shall be used to evaluate the seismic stability of slopes and  
8 embankments due to inertial effects. The pseudo-static analysis consists of conventional limit  
9 equilibrium slope stability analysis with horizontal seismic coefficient ( $K_h$ ) that acts upon the  
10 critical failure mass. A horizontal seismic coefficient ( $K_h$ ) estimated using Bray and Travararou  
11 (2009) and a vertical seismic coefficient,  $K_v$ , equal to zero shall be used for the evaluation of  
12 seismic slope stability. The Bray and Travararou (2009) method requires an estimate of  
13 allowable deformation to compute  $K_h$ . Therefore, the allowable deformation set forth in the  
14 *Geotechnical* chapter shall be used. For MCE case, the allowable deformation of 4 inches may be  
15 assumed. For these conditions, the minimum required FOS is 1.0. Alternately, pseudo-static  
16 analyses may be performed to estimate  $K_y$  values. A new failure plane shall be searched for the  
17 pseudo-static analysis. The analysis shall look for both circular and non-circular failure surfaces.

#### 10.B.2.10 Seismic Slope Deformations

18 Deformation analyses shall be performed where an estimate of the magnitude of seismically  
19 induced slope deformation is required, and the pseudo-static slope stability FOS is less than 1.0.  
20 Acceptable methods of estimating the magnitude of seismically induced slope deformation  
21 include Newmark sliding block (time history) analysis, simplified displacement charts and  
22 equations based on Newmark-type analyses Makdisi and Seed (1978), Bray and Travararou  
23 (2007), and Rathje and Saygili (2008), or dynamic stress-deformation models. These methods  
24 shall not be employed to estimate displacements if the post-earthquake static slope stability FOS  
25 using residual strengths is less than 1.0, since the slope will be unstable against static gravity  
26 loading and large displacements would be expected.

#### 10.B.2.11 Downdrag Loading (Drag Load) on Structures Due to Seismic Settlement

27 Downdrag loads on foundations shall be evaluated in accordance with AASHTO LRFD BDS  
28 with California Amendments Article 3.11.8, and as specified herein. AASHTO LRFD BDS with  
29 California Amendments Article 3.11.8 recommends the use of the non-liquefied skin friction in  
30 the non-liquefied layers above and between the liquefied zone(s), and a skin friction value as  
31 low as the residual strength within the soil layers that do liquefy, to calculate down drag loads  
32 for the extreme event limit state.

### 10.B.3 References

13. American Association of State Highway and Transportation Officials (AASHTO) LFRD Bridge Design Specifications with California (Caltrans) Amendments
  - Section 3 “Loads and Load Factors”
  - Section 10 “Foundations”
  - Section 11 “Abutments Piers and Walls”
14. American Association of State Highway and Transportation Officials (AASHTO), Manual on Subsurface Investigations, MSI-1, 1988.
15. American Association of State Highway and Transportation Officials (AASHTO) LFRD Bridge Design Specifications, 4th Edition, 2007.
16. American Concrete Institute, Building Code Requirements for Reinforced Concrete, ACI 318.
17. American Institute of Steel Construction, Steel Construction Manual, Thirteenth Edition.
18. ASTM International.
19. American Railway Engineering and Maintenance of Way Association (AREMA) Manual for Railway Engineering.
20. American Society of Civil Engineers, (ASCE) (2006) ASCE/SEI 7-05, “Minimum Design Load for Buildings and Other Structures Chapter 21.” ASCE, Reston, VA.
21. Bardet, J.-P., O'Rourke, T.D., and Hamada, M. (1999). "Large-scale modeling of liquefaction-induced ground deformation, Parts I: A four-parameter MLR model." Proceedings of the 7th U.S.-Japan Workshop on Earthquake Resistance Design of Lifeline Facilities and Countermeasures Against Soil Liquefaction, Technical Report MCEER-99-0019, Multidisciplinary Center for Earthquake Engineering Research, pp. 155-174.
22. Barton, N.R., Lien, R. and Lunde, J. (1974). “Engineering Classification of Rock Masses for the Design of Tunnel Support.” International Journal of Rock Mechanics and Mining Sciences, 6(4), 189-239.
23. Blake, T.F. et al. (2002). “Recommended Procedures for Implementation of DMG Special Publication 117 Guidelines for Analyzing and Mitigating Landslide Hazards in California.” ASCE Los Angeles and SCEC (downloadable at:  
<http://www.scec.org/resources/catalog/hazardmitigation.html#land>)
24. Boulanger, R.W. and Idriss, I.M. (2011). “Cyclic Failure and Liquefaction: Current Issues.” Proceedings, 5th International Conference on Earthquake Geotechnical Engineering, Santiago, Chile, January 10-13.
25. Boulanger, R.E., Change, D., Brandenberg, S.J., Armstrong, R.J., Kutter, B.L. (2007). “Seismic Design of Pile Foundations for Liquefaction Effects”, Chapter 12 in Earthquake Geotechnical Engineering, Springer, The Netherlands.

26. Brandenburg, S. J., Boulanger, R. W., Kutter, B. L., and Chang, D., (2007). "Static Pushover Analyses of Pile Groups in Liquefied and Laterally Spreading Ground in Centrifuge Tests." ASCE Journal of Geotechnical and Geoenvironmental Engineering, Vol. 133, No. 9, pp. 1055-1066.
27. Bray, J. D., Travasarou, T., and Zupan, J. (2010). "Seismic Displacement Design of Earth Retaining Structures." Proceedings, Earth Retention Conference 3, Bellevue, Washington, August 1-4.
28. Bray, J. D. and Dashti, S. (2010). "Liquefaction-Induced Movements of Buildings With Shallow Foundations." Proceedings, Fifth International Conference on Recent Advances on Geotechnical Earthquake Engineering and Soil Dynamics and Symposium in Honor of Professor I.M. Idriss, San Diego, CA, May 24-29.
29. Bray, J.D. and Travasarou, T. (2009). "Pseudostatic Coefficient for Use in Simplified Seismic Slope Stability Evaluation," ASCE Journal of Geotechnical and Geoenvironmental Engineering, Vol. 135, No. 9, p. 1336-1340.
30. Bray, J. D., and Travasarou, T. (2007). "Simplified Procedure for Estimating Earthquake-Induced Deviatoric Slope Displacements." ASCE Journal of Geotechnical and Geoenvironmental Engineering, Vol. 133, No. 4, pp. 381-392.
31. Bray, J.D. and Sancio, R. B. (2006). "Assessment of Liquefaction Susceptibility of Fine-Grained Soils," ASCE Journal of Geotechnical and Geoenvironmental Engineering, Vol. 132, No. 9, pp. 1165-1177.
32. Bridge Welding Code ANSI/AASHTO/AWS D1.5-95
33. The California Building Code.
34. California Department of Transportation (Caltrans) Bridge Design Manuals, latest edition
  - Bridge Design Specification (BDS) - AASHTO LRFD Bridge Design Specification, 2005, with Caltrans Interim Revisions
  - Bridge Memo to Designers Manual (MTD)
  - Bridge Design Practices Manual (BPD)
  - Bridge Design Aids Manual (BDA)
  - Bridge Design Details Manual (BDD)
  - Standard Specifications
  - Standard Plans
  - Seismic Design Memorandum
  - Seismic Design Criteria (CSDC) ver. 1.4
35. California Department of Transportation (Caltrans), "California Amendments to the AASHTO LRFD Bridge Design Specifications (Fourth Edition)", September 2010.

36. California Department of Transportation (Caltrans), "Guidelines on Foundation Loading and Deformation Due to Liquefaction Induced Lateral Spreading," February 2011.
37. California Geological Survey (2003), Fault Rupture Hazard Zones, Special Publication 42, 47 pages, <http://www.conservation.ca.gov/CGS/rghm/ap/Pages/Index.aspx>
38. California Geological Survey (1996), Guidelines for evaluating the hazard of surface fault rupture, CGS Note 49, 4 pages, [http://www.conservation.ca.gov/cgs/information/publications/cgs\\_notes/note\\_49/Document/s/note\\_49.pdf](http://www.conservation.ca.gov/cgs/information/publications/cgs_notes/note_49/Document/s/note_49.pdf)
39. California High-Speed Train Project Program EIR/EIS, Task 1.11.a – Tunneling Issues Report, by Parsons Brinckerhoff, Inc., January, 2004.
40. Cetin, K.O., Bilge, H.T., Wu, J., Kammerer, A.M., Seed, R.B. (2009a). "Probabilistic Models for Cyclic Straining of Saturated Clean Sands." ASCE Journal of Geotechnical and Geoenvironmental Engineering, Vol. 135, No. 3, p. 371-386.
41. Cetin, K.O., Bilge, H.T., Wu, J., Kammerer, A.M., Seed, R.B. (2009b). "Probabilistic Model for the Assessment of Cyclically Reconsolidation (Volumetric) Settlements." ASCE Journal of Geotechnical and Geoenvironmental Engineering, Vol. 135, No. 3, p. 387-398.
42. Chen, W., Bray, J.D., and Seed, R.B (2006). "1-G Small Scale Shaking Table Experiments – Seismic Slope Responses and Deformations." Proceedings, 8th US National Conference on Earthquake Engineering, April 18-22, San Francisco, California, Paper No. 1322.
43. Coppersmith and Youngs (1992), Modeling Fault Rupture Hazard for the Proposed Repository at Yucca Mountain, Nevada
44. Dawson, E.M., W.H. Roth, A. Drescher (1999). "Slope stability analysis by strength reduction," Geotechnique, 49(6), 835-840.
45. Empelmann, Whittaker, Los and Dorgarten, "Taiwan High Speed Rail Project – Seismic Design of Bridges Across the Tuntzuchiaio Active Fault", 13th World Conference on Earthquake Engineering, Paper No. 140, Vancouver, B.C., Canada, 2004
46. European Standard EN 1991-2:2003 Traffic Loads on Bridges
47. European Standard EN 1990 annex A2: Application to Bridges
48. Faris, A.T., Seed, R.B., Kayen, R.E. and Wu, J. (2006). "A semi-empirical model for the estimation of maximum horizontal displacement due to liquefaction-induced lateral spreading," Proceedings, 8th U.S. International Conference on Earthquake Engineering; Paper No. 1323, San Francisco, CA; April 18-26; 10 p.
49. FIB (Federation Internationale du Beton) Task Group 7.4, Seismic Design and Assessment Procedures for Bridges, Chapter 8: Design for Active Fault Crossings, primary author Ian Billings.

50. Federal Highway Administration (FHWA) Publications:
- FHWA and Oregon Department of Transportation (2001). "Rockfall Catchment Area Design Guide." Final Report SPR-3 (032).
  - FHWA-CFL/TD-06-002, "InSAR Applications For Highway Transportation Projects." April 2006.
  - FHWA-ED-88-053, "Checklist and Guidelines for Review of Geotechnical Reports and Preliminary Plans and Specifications." 1988, Revised Feb. 2003.
  - FHWA-IF-02-034, "Geotechnical Engineering Circular No. 5 (GEC5), Evaluation of Soil and Rock Properties." April 2002.
  - FHWA-NHI-05-123 "Soil Slope and Embankment Design Reference Manual." Sept. 2005.
  - FHWA-NHI-00-044, "Corrosion/Degradation of Soil Reinforcements for Mechanically Stabilized Earth Walls and Reinforced Soil Slopes." Sept. 2009.
  - FHWA-NHI-05-052, "Development of Geotechnical Resistance Factors and Downdrag Load Factors for LRFD Foundation Strength Limit State Design Reference Manual." Feb. 2005.
  - FHWA-NHI-06-019/020, "Ground Improvement Methods Reference Manual, Volumes I and II." August 2006.
  - FHWA-NHI-06-088/089, "Soils and Foundations Reference Manual, Volumes I and II." December 2006.
  - FHWA-NHI-07-071, "Earth Retaining Structures Reference Manual." June 2008.
  - FHWA-HI-99-007, "Rock Slopes Reference Manual." 1999.
51. Idriss, I.M. and Boulanger, R.W. (2008). "Soil Liquefaction During Earthquakes." Earthquake Engineering Research Institute, MNO-12.
52. Ghasemi, H., "Bolu Viaduct: Damage Assessment and Retrofit Strategy", The 36th Joint Meeting US-Japan Panel on Wind and Seismic Effects (UJNR), Session 1, National Institute of Standards and Technology, Gaithersburg, Maryland, USA, 2004
53. Ishihara, K. (1985). "Stability of Natural Deposits During Earthquakes." Proc. 11th International Conference on Soil Mechanics and Foundation Engineering, San Francisco, Vol. 2. pp. 321-376.
54. Ishihara, K. and Yoshimine, M. (1992). "Evaluation of Settlements in Sand Deposits Following Liquefaction During Earthquakes." Soils and Foundations, JSSMFE, Vol. 32, No. 1.
55. Kenny, J. T. (1954). "Steady-State Vibrations of Beam on Elastic Foundation for Moving Load." Journal of Applied Mechanics, December.
56. Kramer, S. L., and Wang, C. H. (2011). "A hybrid model for estimation of the residual strength of liquefied soil." Submitted for review.

- 1 57. Iai, S. Noda, S. and Tsuchida, H. (1988). "Basic considerations for designing the area of the  
2 ground compaction as a remedial measure against liquefaction," Proc. U.S.-Japan Joint  
3 Workshop of Remedial Measures for Liquefiable Soils, UJNR, Jackson, Wyoming, May 11-  
4 13.
- 5 58. Ledezma, C. and Bray, J.D. (2010). "Probabilistic Performance-Based Procedure to Evaluate  
6 Pile Foundations at Sites with Liquefaction-Induced Lateral Displacement," ASCE Journal of  
7 Geotechnical and Geoenvironmental Engineering, Vol. 136, No. 3, p. 464-476.
- 8 59. Li D., and Selig, E. T. (1998). "Method for Railtrack Foundation Design I: Development,"  
9 Journal of Geotechnical/Geoenvironmental Engineering, Vol. 124, No. 4, pg 316-322.
- 10 60. Mahin, Sakai, and Jeong, "Use of Partially Prestressed Reinforced Concrete Columns to  
11 Reduce Post-Earthquake Residual Displacements of Bridges", Paper No. B25, 5th National  
12 Seismic Conference on Bridges & Highways, San Francisco, CA, Sept 18-20, 2005.
- 13 61. Makdisi, F.I. and Seed, H.B. (1978). "Simplified Procedure for Estimating Dam and  
14 Embankment Earthquake-Induced Deformations." ASCE Journal of the Geotechnical  
15 Engineering Division, Vol. 104, No. GT7, July, 1978, pp. 849-867.
- 16 62. Martin, G.R., Tsai, C-F, Arulmoli, K. (1991). "A Practical Assessment of Liquefaction Effects  
17 and Remediation Needs." Proceedings 2nd International Conference on Recent Advances in  
18 Geotechnical Earthquake Engineering and Soil Dynamics, St. Louis, Missouri, March 11-15.
- 19 63. Martin, G.R. and Lew, M (1999). "Recommended Procedures for Implementation of DMG  
20 Special Publication 117 – Guidelines for Analyzing and Mitigating Liquefaction in  
21 California." Southern California Earthquake Center.
- 22 64. Matasovic, N., Athanasopoulos-Zekkos, A., and Zekkos, D. (2008). "Validation of generic  
23 municipal solid waste material properties for seismic design of landfills." 4th Geotechnical  
24 Earthquake Engineering and Soil Dynamics Conference, Sacramento, CA, May 18-22, 2008.
- 25 65. National Cooperative Highway Research Program (NCHRP),  
26 – Transportation Research Board (2004). "Load and Resistance Factor Design (LRFD) for  
27 Deep Foundations." NCHRP Report 507.
- 28 – Transportation Research Board (2006). "Riprap Design Criteria, Recommended  
29 Specifications, and Quality Control," Report 568.
- 30 – "Seismic Analysis and Design of Retaining Walls, Buried Structures, Slopes and  
31 Embankments," Report 611, Chapter 7.
- 32 66. Priestley, M.J.N. and Calvi, G.M., "Strategies for Repair and Seismic Upgrading of Bolu  
33 Viaduct 1, Turkey", Journal of Earthquake Engineering, Vol. 6, Special Issue 1, pp. 157-184,  
34 2002.
- 35 67. Rathje, E.M. and Saygili, G. (2008). "Probabilistic Seismic Hazard Analysis for the Sliding  
36 Displacement of Slopes: Scalar and Vector Approaches," Journal of Geotechnical and  
37 Geoenvironmental Engineering, ASCE, 134(6), 804-814.

68. Rollins, K. M., Gerber, T. M., Lane, J. D., and Ashford, S. A. (2005). "Lateral Resistance of a Full-Scale Pile Group in Liquefied Sand." *ASCE Journal of Geotechnical and Geoenvironmental Engineering*, Vol. 131, No. 1, pp. 115-125.
69. Saygili, G., and Rathje, E. M. (2008). "Empirical Predictive Models for Earthquake-Induced Sliding Displacements of Slopes." *ASCE Journal of Geotechnical and Geoenvironmental Engineering*, Vol. 134, No. 6, pp. 790-803.
70. Seed, R.B., and Harder, L.F., Jr. (1990). "SPT-based analysis of cyclic pore pressure generation and undrained residual strength." *Proceedings of the H. Bolton Seed Memorial Symposium*, May Vol. 2.
71. Seed, R.B., K. O., Cetin, R.E.S., Moss, A., Kammerer, J., Wu, J.M., Pestana, M.F., Riemer, R.B., Sancio, J.D., Bray, R.E., Kayen, R.E., Faris, A. (2003). "Recent Advances in Soil Liquefaction Engineering: A Unified and Consistent Framework." Keynote Address, 26th Annual Geotechnical Spring Seminar, Los Angeles Section of the GeoInstitute, American Society of Civil Engineers, H.M.S. Queen Mary, Long Beach, California, USA.
72. Shamoto, Y., Zhang, J., and Tokimatsu, K. (1998). "New Charts for Predicting Large Residual Post-Liquefaction Ground Deformation." *Soil Dynamics and Earthquake Engineering* 17, p. 427-438.
73. Sherard, J. L., Dunnigan, L. P., and Talbot, J. R. (1984a). "Basic Properties of Sand and Gravel Filters," *Journal of the Geotechnical Engineering Division, ASCE*, June, pp. 684-700.
74. Sherard, J. L., Dunnigan, L. P., and Talbot, J. R. (1984b). "Filters for Clays and Silts," *Journal of the Geotechnical Engineering Division, ASCE*, June, pp. 701-718.
75. *Structural Welding Code, Steel*, 1996 ANSI/AWS D1.1-96.
76. Taiwan High Speed Rail (THSR) Corporation: Volumes 1 and 3.
77. Taylor, D.W. (1937). "Stability of earth slopes." *Journal of the Boston Society of Civil Engineers*, 24(3).
78. Tokimatsu, K. and Seed, H.B. (1987). "Evaluation of Settlements in Sand Due to Earthquake Shaking." *Journal of Geotechnical Engineering, ASCE* Vol. 113, No. 8, pp. 861-878.
79. United States Army Corps of Engineers (USACE), EM-1110-2-1902, "Engineering and Design – Slope Stability." Oct. 2003.
80. United States Army Corps of Engineers (USACE), EM-1110-2-2901, "Engineering and Design - Tunnels and Shafts in Rock." May 1997.
81. Wells, D., and Coppersmith K., "New empirical relationships among magnitude, rupture length, rupture width, rupture area, and surface displacement", *Bulletin of the Seismological Society of America*, Vol. 84, pages 974-1002, 1994.



- 1 82. Wesnousky, Steven, 2008, "Displacement and Geometrical Characterization of Earthquake  
2 Surface Rupture: Issues and Implications for Seismic Hazard Analysis and the Process of  
3 Earthquake Rupture", Bull. Of the Seismological Society of America, vol. 98, No. 4, p. 1609.
- 4 83. Youd, T.L., and Garriss, C.T. (1995). "Liquefaction-Induced Ground Surface Disruption."  
5 Journal of Geotechnical Engineering, ASCE, Vol. 121, No. 11.
- 6 84. Youd, T.L.; Hansen, C.M. and Bartlett, S.F. (2002). "Revised Multilinear Regression  
7 Equations for Prediction of Lateral Spread Displacement." ASCE Journal of Geotechnical  
8 and Geoenvironmental Engineering, Vol. 128, No. 12, pp. 1007-1017.
- 9 85. Youngs et al., (2003), A Methodology for Probabilistic Fault Displacement Hazard Analysis  
10 (PFDHA), Earthquake Spectra, Vol. 19, No. 1, p 191-219.
- 11 86. Zhang, G. Robertson P.K., Brachman, R.W.I. (2002). "Estimating Liquefaction-Induced  
12 Ground Settlements from CPT for Level Ground," Canadian Geotechnical Journal, Vol. 39,  
13 pp 1168-1180.
- 14 87. Zhang, G., Robertson, P. K., Brachman, R. W. I. (2004). "Estimating Liquefaction-Induced  
15 Lateral Displacements Using the Standard Penetration Test or Cone Penetration Test."  
16 ASCE Journal of Geotechnical and Geoenvironmental Engineering, Vol. 130, No. 8, pp. 861-  
17 871.



## Appendix 10.C: Guidelines for Rock Slope Engineering

### 10.C.1 Purpose

- 1 These guidelines convey a minimum standard of care for performance of rock slope engineering
- 2 design, mapping, and construction.

### 10.C.2 Design

3 Rock slopes are typically composed of heterogeneous rock masses with structural anisotropic  
4 systems of relatively regular discontinuities in the form of joint sets, bedding, fissures, or  
5 foliation. The strength and slope stability of these types of rock masses are typically controlled  
6 by the discontinuities. Analytical techniques for rock slope stability assessment shall consider  
7 the kinematic stability of blocks or groups of blocks sliding upon the discontinuities, toppling,  
8 or in terms of wedge failure. Limit equilibrium methods that calculate a factor of safety shall be  
9 used. These analyses shall consider blocks that are kinematically permissible as evaluated by  
10 the Markland (1972) method, block theory (Goodman and Shi 1985), or rock slope engineering  
11 techniques described by Hoek and Bray (1981) and Wyllie and Mah (2004). If computer software  
12 is used for rock slope stability analyses, it shall be well validated and widely accepted.

13 For rock mass consisting of homogeneous and isotropic rock masses with irregular and/or  
14 closely spaced discontinuities that do not have well defined systematic planes of weakness, the  
15 evaluation of the stability of these types of slopes shall be based on the non-circular limit  
16 equilibrium techniques described above for soil, except that a suitable rock strength model shall  
17 be used such as General Hoek-Brown criterion (Hoek et al. 2002; Wyllie and Mah, 2004; Hoek,  
18 2010).

19 Where rock slopes exist upslope of HST facilities and have the potential to shed rock pieces over  
20 time, an evaluation of the rock fall hazard shall be performed in accordance with the procedures  
21 outlined in the FHWA and Oregon DOT (2001) Rockfall Catchment Area Design Guide.  
22 Computer programs that model rockfall physics such as the Colorado Rockfall Simulation  
23 Program (CRSP III) or RocFall (by RocScience), or other equivalent software, may be used in  
24 conjunction with the FHWA procedures. Rockfall catchment basin width and inclination shall  
25 be designed to retain 100 percent of fallen rocks. If right-of-way is not available to size  
26 catchment basins to achieve 100 percent rockfall retention, additional mitigation measures such  
27 as rockfall protection walls, wire mesh, cable drape, or catchment fences shall be used in the  
28 design. In areas where rock fall is a critical problem, a railway slide fence with electronic  
29 warning system shall be installed in conjunction with an appropriate catchment ditch and rock  
30 fall retention system described above. Other warning systems for rockfall events that may be  
31 considered are as follows:

- 32 • Acoustic sensing
- 33 • Electromagnetic sensing

- Seismic sensing
- Visual sensing, using cameras

Input data and parameters used in rock slope stability analyses shall take into consideration geology, groundwater and rainfall, and proposed geometry/topography. Rock engineering parameters shall be developed for use in slope stability analyses.

When available, empirical or historical data and direct observation within the geologic unit or the past performance of similar slopes shall be considered in slope stability evaluations. In particular, when assessing existing landslides, shear strength parameters back-calculated from previous failures shall be considered.

Drained shear strength parameters shall be selected, depending upon the rate of loading, and permeability characteristics of the rock. In the analysis of existing landslides, residual shear strengths shall be used for existing landslide slip planes. FHWA (2005) Section 4 shall be consulted for additional guidance on the selection of shear strength parameters.

### **10.C.3 Rock Slope Mapping and Condition Survey Requirements**

The results of the mapping and condition surveys shall be used by the Geotechnical Designer to develop design and construction recommendations for treatment of exposed rock slopes and design of new rock cut slopes.

Under supervision of the Geotechnical Designer, qualified personnel trained in geology or engineering geology shall supervise and perform the rock slope mapping activities and data collection. A Certified Engineering Geologist (CEG) licensed in the State of California with at least 5 years of experience in rock slope design shall conduct slope condition surveys and rock mapping. Prior to mapping, the CEG shall be familiar with the local and regional geology. The mapping teams shall be knowledgeable of the rock units and structural and historical geologic aspects of the areas to be mapped.

### **10.C.4 Rock Slope Mapping**

Procedures for mapping shall follow those given in the Rock Slopes Reference Manual, FHWA-HI-99-007, 1998, "Appendix I, Geologic Mapping," Parts 1, 2, and 3. At each mapping window, the CEG shall prepare a detailed section of the exposed cut.

Field observation data shall be recorded on approved forms similar to the 1 depicted on Figure AI-9a and b of the Rock Slope Reference Manual, FHWA-HI-99-007 and in field notebooks. Parameters described in the Rock Slope Reference Manual, FHWA-HI-99-007 (pages AI-3 to AI-14) shall be recorded. The following methods/assessments shall be used in the rock slope mapping:

- Use Project stationing to describe the location of rock mapping or rock slope condition observations. Record observation locations to within plus or minus 3 feet of actual Project

stations. Also, designate observation locations with a sequential numbering system. Orientation data shall be referenced to Project north (as shown on the plans).

- Color digital photographs ( at least 10 megapixels) shall be taken of each mapping area and window. A scale shall be included in the window mapping photograph. Photographs shall be mounted on an 8 1/2 x 11-inch sheet and labeled.
- Feature specific photographs shall be taken, with a minimum of 1 photograph per window, and labeled.
- After the geologic mapping for a window has been completed, evaluate the rock slope at each mapping window using the Rock Slope Hazard Rating System presented in Chapter 10 of the Rock Slope Reference Manual.

### 10.C.5 Rock Excavation

Rock excavation surfaces shall be mapped to ensure that the final excavation surfaces are examined and to aid in the discovery of unanticipated adverse geologic conditions. The mapping shall serve as a permanent record of the geologic conditions encountered during construction.

For rocks that are prone to weathering and deterioration when exposed by excavation processes, the Geotechnical Designer in collaboration with the CEG shall develop measures to protect the rock surfaces to preserve the strength and character of the material.

Rock excavation may be done either by mechanical equipment; by using explosives in drill-and-blast operations, or both. However, blasting shall not be allowed in urban areas unless otherwise permitted per local building ordinances. If permitted, blasting of rock shall be undertaken by controlled blasting techniques (cushion [trim], pre-splitting, smooth-wall blasting, and line drilling). The Geotechnical Designer in collaboration with the CEG shall select the rock excavation method to minimize vibration, over-breaks, fly rock and air blast. A pre-blast survey shall also be undertaken. The Contractor shall repair any blast and vibration induced damage.

#### 10.C.5.1 Quality Assurance During Blasting Operation

The Geotechnical Designer in collaboration with the CEG shall do the following:

- Obtain copies of applicable codes, standards, regulations, and ordinances, and keep readily accessible copies at the project field office at all times.
- Retain a blasting specialist who shall be responsible for supervision of field blasting operations and personnel, and have a minimum 15 years of blasting experience with 10 years experience in responsible charge of blasting operations. Such a blasting specialist shall possess required federal, state and local licenses and/or permits.

- 1 • Prepare a blasting plan for the areas to be excavated by means of controlled blasting. The  
2 plan shall describe the necessary items to excavate the rock using the controlled blasting  
3 techniques selected by the Geotechnical Designer.
- 4 • The Blasting Plan shall be prepared and signed by the blasting specialist.

#### 10.C.5.2 Damage Repair

5 Damage to existing structures or property caused by the blasting shall be repaired by the  
6 Contractor.

7 The Geotechnical Designer shall notify the Authority immediately of any blasting-induced  
8 damage.

#### 10.C.5.3 Fly Rock Control

9 The Contractor shall control fly rock at all times during construction.

#### 10.C.5.4 Notification

10 The Contractor shall notify each adjoining property owner, the Authority, local agencies where  
11 applicable, in writing, prior to each blast. Indicate the date and time of the proposed blast, and  
12 include any safety precautions required of the adjoining property owner.

#### 10.C.5.5 Photography

13 Photographs shall be taken before, during, and at the end of construction of excavated surfaces.  
14 Photos shall be properly labeled with date, subject, direction of view, vantage point, and  
15 photographer.

#### 10.C.6 References

- 16 1. FHWA-HI-99-007, 1999, Rock Slopes Reference Manual.
- 17 2. Goodman, R.E. and Shi, G. (1985). *Block Theory and its Application to Rock Engineering*,  
18 Prentice-Hal, Englewood Cliffs, N.J.
- 19 3. Hoek, E. and Bray, J.W. (1981). "Rock Slope Engineering, Revised 3rd edition." The  
20 Institution of Mining and Metallurgy, London.
- 21 4. Hoek, E. (2010). "Practical Rock Engineering." in Rocscience 2010.
- 22 5. Hoek, E., Carranza-Torres, C.T., and Corkum, B. (2002). "Hoek-Brown failure criterion –  
23 2002 edition." *Proc. North American Rock Mechanics Society meeting in Toronto in July 2002*.
- 24 6. Markland, J.T. (1972). "A Useful Technique if Estimating the Stability of Rock Slopes When  
25 the Rigid Wedge Sliding Type of Failure is Expected." *Imperial College Rock Mechanics*  
26 *Research Report No. 19*. London, Imperial College Press.

- 1 7. Wyllie, D.C and C.W. Mah (2004). "Rock Slope Engineering: Civil and Mining." 4th Edition,  
2 Spon Press.

## Chapter 11

### Seismic

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
0.1	07 Dec 12	Addendum 7 – CP01
1	June 2013	Revision 1
2	Feb 2014	Revision 2
3	Feb 2016	Revision 3





## Table of Contents

<b>11</b>	<b>SEISMIC .....</b>	<b>11-1</b>
11.1	Scope .....	11-1
11.2	Regulations, Codes, Standards, and Guidelines .....	11-1
11.3	Seismic Design and Analysis Plan .....	11-3
11.4	Design Classifications .....	11-4
11.4.1	General Classifications .....	11-4
11.4.2	Technical Classification.....	11-6
11.5	Seismic Design Approach .....	11-7
11.5.1	Seismic Performance Criteria .....	11-7
11.5.2	Design Earthquakes .....	11-9
11.5.3	Retrofit of Existing Structures.....	11-11
11.5.4	Seismic Requirements for Temporary Construction Structures.....	11-11
11.6	Seismic Design Requirements .....	11-12
11.7	Bridges, Aerial Structures, and Grade Separations.....	11-12
11.7.1	Design Codes.....	11-12
11.7.2	Seismic Design Approach .....	11-13
11.7.3	Seismic Demands on Structural Components .....	11-15
11.7.4	Seismic Capacities of Structural Components.....	11-27
11.7.5	Seismic Performance Evaluation .....	11-30
11.8	Passenger Stations and Building Structures .....	11-32
11.8.1	Design Codes.....	11-33
11.8.2	Seismic Design Approach .....	11-33
11.8.3	Seismic Demands on Structural Components .....	11-34

## Tables

Table 11-1:	Performance Objectives/Acceptable Damage for NCL.....	11-8
Table 11-2:	Performance Objectives/Acceptable Damage for OPL.....	11-9
Table 11-3:	Seismic Design Requirements.....	11-12
Table 11-4:	Applicable Bridge, Aerial Structure and Grade Separation Design Codes ...	11-13
Table 11-5:	Applicable Passenger Station and Building Structure Design Codes .....	11-33

## Acronyms

AASHTO	American Association of State Highway and Transportation Officials
ASCE	American Society of Civil Engineers
Authority	California High-Speed Rail Authority
BDS	Bridge Design Specifications
Caltrans	California Department of Transportation
CBC	California Building Code
CBDM	Caltrans Bridge Design Manuals
CQC	Complete quadratic combination
CSDC	Caltrans Seismic Design Criteria
D/C	Demand/Capacity
ELTHA	Equivalent linear time history analysis
ESA	Equivalent static analysis
HST	High-Speed Train
LDP	Linear Dynamic Procedure
LRFD	Load and Resistance Factor Design
MCE	Maximum Considered Earthquake
NCL	No Collapse Performance Level
NDP	Non-linear Dynamic Procedure
NSP	Non-linear Static Procedure
NLTHA	Non-linear time history analysis
OBE	Operating Basis Earthquake
OPL	Operability Performance Level
RSA	Response Spectrum Analysis
SDAP	Seismic Design and Analysis Plan
SRSS	Square root of the sum of the squares
SSI	Soil-structure interaction

Note: Additional Acronyms are found in Section 11.2 of this chapter.

# 11 Seismic

## 11.1 Scope

This chapter provides design classifications used to determine seismic design objectives for California High-Speed Train (HST) infrastructure design. Two design earthquakes and classification specific seismic performance objectives and acceptable damage are defined in this chapter.

The design classifications, design earthquakes, and performance objectives are used and referenced by the *Structures* and *Geotechnical* chapters.

This chapter provides seismic design criteria for the following infrastructure elements: bridges, aerial structures, grade separations, passenger stations and building structures.

Seismic design criteria for earthen or soil supporting structures, tunnels and underground structures are within the *Geotechnical* chapter.

## 11.2 Regulations, Codes, Standards, and Guidelines

Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards. Design shall meet applicable portions of the general laws and regulations of the State of California and of respective local authorities.

The provisions within this chapter shall govern seismic design. The following current documents are either referenced by this chapter, or shall be considered as guidelines when sufficient criteria are not provided by this chapter.

- American Concrete Institute (ACI)
  - ACI 318: Building Code Requirements for Structural Concrete
  - ACI 350: Code Requirements for Environmental Engineering Concrete Structures and Commentary
- American Welding Society (AWS) Codes
  - AWS D1.1/D1.1M: Structural Welding Code-Steel
  - AWS D1.8/D1.8M: Structural Welding Code-Seismic Supplement
- American Association of State Highway and Transportation Officials (AASHTO)
  - AASHTO/AWS D1.5M/D1.5: Bridge Welding Code
  - AASHTO Guide Specifications for LRFD Seismic Bridge Design

- 1       – AASHTO Guide Specifications for Seismic Isolation Design
- 2       – AASHTO LRFD Bridge Construction Specifications
- 3       • California Building Code (CBC)
- 4       • American Railway Engineering and Maintenance-of-Way Association (AREMA)
- 5       – Manual for Railway Engineering
- 6       • American Society of Civil Engineers (ASCE)
- 7       – ASCE 7: Minimum Design Loads for Buildings and Other Structures
- 8       – ASCE 41: Seismic Rehabilitation of Existing Buildings
- 9       • American Institute of Steel Construction (AISC)
- 10      – Steel Construction Manual
- 11      • California Occupational Safety and Health Administration (Cal/OSHA) Department of
- 12      Industrial Relations
- 13      • California Department of Transportation (Caltrans) Bridge Design Manuals (CBDM)
- 14      – Caltrans Bridge Design Specification – AASHTO LRFD Bridge Design Specifications and
- 15      California Amendments (to the AASHTO LRFD Bridge Design Specifications), hereafter
- 16      referred to as “AASHTO LRFD BDS with California Amendments”
- 17      – Caltrans Bridge Memo to Designers Manual (CMTD)
- 18      – Caltrans Bridge Design Practices Manual (CBDP)
- 19      – Caltrans Bridge Design Aids Manual (CBDA)
- 20      – Caltrans Bridge Design Details Manual (CBDD)
- 21      – Caltrans Seismic Design Criteria (CSDC)
- 22      – Office of Special Funded Projects (OSFP) Information and Procedures Guide
- 23      • Code of Federal Regulations (CFR)
- 24      • United States Department of Transportation Federal Highway Administration; Technical
- 25      Manual for Design and Construction of Road Tunnels – Civil Elements; Publication No.
- 26      FHWA-NHI-09-010

### 11.3 Seismic Design and Analysis Plan

The Designer shall develop a Seismic Design and Analysis Plan (SDAP) for infrastructure elements including:

- bridges, aerial structures, and grade separations
- passenger stations and building structures
- earthen or soil supporting structures
- tunnels and underground structures

The SDAP shall define the following:

- the General Classification as Primary Type 1, Primary Type 2, or Secondary, as defined in Section 11.4.1
- the Technical Classification as Complex, Standard, or Non-Standard, as defined in Section 11.4.2

The SDAP shall contain detailed commentary on seismic analysis for each design earthquake per Section 11.5.2, including analysis during Operating Basis Earthquake (OBE) events as required in the Track-Structure Interaction section of the *Structures* chapter.

The SDAP shall be consistent with the Track-Structure Interaction Design and Analysis Plan (TSIDAP) required per the *Structures* chapter.

The SDAP shall indicate the analysis software to be used, modeling assumptions and techniques to be employed.

The SDAP shall contain commentary as to the suitability of linear versus nonlinear analysis, considering geological hazards, the severity of design ground motions, induced strains in the soil and structure, expected nonlinearities, and expected inelastic behavior.

The SDAP shall define the pre-determined mechanism for seismic response (i.e.: plastic hinging, foundation rocking, sacrificial shear keys, etc.) and the regions of targeted inelastic response.

The SDAP shall contain detailed commentary on the site-specific geological conditions identified during subsurface investigations required by the *Geotechnical* chapter. For Special Sites as defined in the *Geotechnical* chapter, the SDAP shall clearly define the approach for site response analysis as required per Section 11.5.2.2. The SDAP shall indicate the site response analysis software to be used and address potential soil nonlinearities.

For retrofit of existing structures, the SDAP shall provide a detailed discussion of the extent of retrofit and the proposed methodology to verify seismic performance. The required amount of retrofit will be determined by the Authority on a case by case basis.

The SDAP shall justify all seismic related design variances submitted per the *General* chapter.

1 Examples of seismic related design variances include:

- 2 • Exceedance of allowable material strain limits as given in Section 11.7.4.5 to 11.7.4.8. The  
3 SDAP shall specifically justify the reasons for exceeding the allowable strain limits.
- 4 • Exceedance of allowable deformation or rail stress limits during Operating Basis Earthquake  
5 (OBE) events as required in the Track-Structure Interaction section of the *Structures* chapter.  
6 The SDAP shall specifically justify the reasons for exceeding the allowable deformation or  
7 rail stress limits.
- 8 • The use of energy dissipation, seismic response modification, or base isolation systems. The  
9 SDAP shall discuss in detail the proposed use of any such system, including the nonlinear  
10 response, and the capacity under service (i.e., braking and acceleration, wind, etc.) loads and  
11 OBE events in order to meet criteria in the Track-Structure Interaction section of the  
12 *Structures* chapter.
- 13 • The use of any design spectra, ground motion time histories, and fault displacements other  
14 than those provided by the Authority. The SDAP shall specifically justify the use of such  
15 alternate items.
- 16 • Any expected in-ground flexural plastic hinging. The SDAP shall provide mitigating  
17 measures such as reduced ductility demand, and reference the reduced strain limits  
18 provided in Section 11.7.4.6.

## 11.4 Design Classifications

19 Infrastructure elements will provide a broad range of functions for the HST system.

20 General and technical classification provides a method to differentiate between seismic design  
21 objectives for the various elements of HST infrastructure.

### 11.4.1 General Classifications

22 Infrastructure elements, based on their importance to HST service, shall be generally classified  
23 as Primary Type 1, Primary Type 2, or Secondary.

24 **Primary Type 1** – Primary Type 1 elements are those that directly support HST track, including,  
25 but not limited, to the following:

- 26 • Bridges, aerial structures, and grade separations that directly support HST track.
- 27 • Tunnels and underground structures that directly support HST track.
- 28 • Passenger stations and building structures that directly support HST track.
- 29 • Earthen or soil supporting structures, such as retaining walls, embankments, cut and  
30 existing slopes, and reinforced earth structure, that directly support HST track.

**Primary Type 2** – Primary Type 2 elements are those that do not directly support HST track, but have the potential to affect HST track or service, including, but not limited, to the following:

- Highway, roadway, railway, and pedestrian structures that span over HST track.
- Train control, traction power, communication, operation, control, and equipment facilities essential for HST service.
- Tunnels and underground structures near HST track, where potential damage could affect HST track or service.
- Building structures near HST track, where potential damage could affect HST track or service.
- Earthen or soil supporting structures near HST track or facilities, such as retaining walls, embankments, cut and existing slopes, and reinforced earth structure, where potential damage could affect HST track or service.
- Structures or infrastructure not supporting HST track, but essential for HST service, including, but not limited, to:
  - Train control, communication and operation facilities
  - Traction power facilities
  - Other equipment facilities essential for HST service

Primary Type 2 structures owned by the Authority and Third Parties shall be subject to the seismic criteria in this chapter.

For retrofit of existing Primary Type 2 structures, see Section 11.5.3.

**Secondary** – Secondary elements are those not designated as Primary Type 1 or Primary Type 2, including, but not limited, to:

- Highway, roadway, railway, and pedestrian structures that do not span over HST track, where potential damage would not affect HST track or service.
- Tunnels and underground structures removed from HST track, where potential damage would not affect HST track or service.
- Building structures removed from HST track, where potential damage would not affect HST track or service, including, but not limited to:
  - Administrative buildings
  - Shop and maintenance buildings
  - Storage facilities
  - Parking structures
  - Training facilities



- Other buildings or facilities not essential for HST service.
  - Earthen or soil supporting structures removed from HST track, such as retaining walls, embankments, cut and existing slopes, and reinforced earth structure, where potential damage would not affect HST track or service.
- Secondary structures owned by the Authority shall be subject to the seismic criteria in this chapter.
- Secondary structures owned by Third Parties shall be subject to the seismic criteria of the governing local jurisdiction.
- For retrofit of existing Secondary structures, see Section 11.5.3.

#### 11.4.2 Technical Classification

Structures shall be technically classified, in order to determine the scope of seismic design requirements.

**Complex Structures** – Structures that have complex response during seismic events are considered Complex. Complex structural features include:

- **Irregular Geometry** – Structures that include multiple superstructure levels, variable width or bifurcating superstructures, tight horizontal curves (inside radius of curvature < 400 feet), large subtended horizontal angles (angle > 30°), or adjacent frames with corresponding transverse or longitudinal fundamental periods of vibration varying by greater than 25%.
- **Unusual Framing** – Structures with straddle, outrigger, or C-bent supports, or unbalanced mass and stiffness distribution not complying with CSDC's balanced stiffness and balanced frame geometry requirements.
- **Short Columns** – Structures with concrete columns having a ratio of clear height to greatest cross sectional dimension (H/D) less than 2.5. The clear height (H) is the visible length of column above grade and shall not include any embedded portion. The clear height (H) may be increased by the use of isolation casings extending below grade, provided that the casings allow access for column inspection.
- **Pier Walls** – Structures consisting of a wall on a footing or piles having a ratio of clear height to maximum wall width (H/W) less than 2.5. This is not applicable to seat type abutments with sacrificial transverse shear keys, refer to Section 11.7.5.9.
- **Tall Columns** – Structures with concrete columns having a ratio of clear height to least cross sectional dimension (H/D) > 10 in single curvature, or > 15 in double curvature.
- **Long Span Structures** – Structures that have spans greater than 300 feet.
- **Skewed Structures** – Primary Type 1 Structures with skewed bents or abutments > 15 degrees. Primary Type 2 or Secondary Structures with skewed bents or abutments > 30 degrees.

- **Lightweight Concrete** – Structures that consist of lightweight concrete. Lightweight concrete shall not be used for ductile earthquake resisting elements.
- **Energy Dissipation, Seismic Modification, or Base Isolation Devices** – Structures using energy dissipation, seismic response modification, or base isolation devices.
- **Unusual Foundation Systems** – Structures with foundations other than spread footings, caissons, piles, or drilled shafts.
- **Complex Geologic Conditions** – Structures that are subject to complex geologic conditions identified during subsurface investigation required by the *Geotechnical* chapter. Examples of complex geologic conditions include:
  - Soft, collapsible, or expansive soils
  - Soils having moderate to high liquefaction or other seismically induced ground deformation potential
  - When significantly varying types of soil occurs over the length of the structure
- **Tunnels or Underground Structures**
- **Structures at or in Close Proximity to Hazardous Faults** – Refer to Section 11.5.2.1 for hazardous fault definition.
- Standard Structures** – Structures that are not Complex Structures and have the following features:
  - **Superstructure** – Simply supported prestressed concrete box girders or series of parallel concrete girders supporting a concrete deck, without skew
  - **Substructure** – Single or multiple column bents having a ratio of clear height to the greatest cross sectional dimension (H/D) greater than 2.5.
  - **Earthquake-Resisting System** – Above ground column flexural plastic hinging
- Non-Standard Structures** – Structures that are not Complex or Standard Structures.

## 11.5 Seismic Design Approach

The goal of these criteria is to safeguard against loss of life and major structural failures due to the Maximum Considered Earthquake (MCE), and interruption of HST operations due to structural or track damage and derailment caused by the Operating Basis Earthquake (OBE).

### 11.5.1 Seismic Performance Criteria

Two levels of seismic performance criteria are defined:

- **No Collapse Performance Level (NCL)** – For the NCL, Table 11-1 states the performance objectives and acceptable damage for Maximum Considered Earthquake (MCE) design.

- 1 • **Operability Performance Level (OPL)** – For the OPL, Table 11-2 states the performance  
2 objectives and acceptable damage for Operating Basis Earthquake (OBE) design.

**Table 11-1: Performance Objectives/Acceptable Damage for NCL**

Performance Level & Design Earthquake	Performance Objectives	Acceptable Damage
No Collapse Performance Level (NCL)  Maximum Considered Earthquake (MCE)	<u>Primary Type 1</u>	
	<ul style="list-style-type: none"> <li>Structures shall be designed for MCE ground motion with no collapse.</li> <li>Occupants not on trains shall be able to evacuate the structure safely.</li> <li>Damage and collapse due to train derailment shall be mitigated per the <i>Rolling Stock and Vehicle Intrusion Protection</i> and <i>Structures</i> chapters.</li> <li>If derailment occurs, train passengers and operators shall be able to be evacuated from derailed trains safely per the <i>System Safety and Security</i> and <i>Structures</i> chapters.</li> <li>Access for post-earthquake emergency services shall be provided for within design per the <i>System Safety and Security</i> chapter.</li> <li>Extensive repairs or complete replacement of some system components may be required before train operation may resume.</li> <li>For underground structures, no flooding or mud inflow.</li> </ul>	Significant yielding of reinforcement steel or structural steel. Minor fracturing of secondary and redundant steel members or rebar, with no collapse.
		Extensive cracking and spalling of concrete, with minimal loss of vertical load carrying capability.
	<u>Primary Type 2</u>	
	<ul style="list-style-type: none"> <li>Structures shall be designed for MCE ground motion with no collapse.</li> <li>Occupants shall be able to evacuate the structure safely.</li> <li>Damage and collapse due to train derailment shall be mitigated per the <i>Rolling Stock and Vehicle Intrusion Protection</i> and <i>Structures</i> chapters.</li> </ul>	Large permanent offsets, without collapse
	<u>Secondary (owned by Authority)</u>	
	<ul style="list-style-type: none"> <li>Structures shall be designed for MCE ground motion with no collapse.</li> <li>Occupants shall be able to evacuate the structure safely.</li> </ul>	Extensive damage to HST track, track support, and rail fasteners

**Table 11-2: Performance Objectives/Acceptable Damage for OPL**

Performance Level	Performance Objectives	Acceptable Damage
Operability Performance Level (OPL)  Operating Basis Earthquake (OBE)	<u>Primary Type 1</u> <ul style="list-style-type: none"> <li>Essentially elastic structural response</li> <li>Occupants not on trains shall be able to evacuate the structure safely.</li> <li>No derailment, trains shall be able to safely brake from the maximum design speed to a safe stop.</li> <li>Train passengers and operators shall be able to evacuate stopped trains safely per the <i>System Safety and Security</i> and <i>Structures</i> chapters.</li> <li>Structure and track designed to comply with Track-Structure Interaction section of the <i>Structures</i> chapter.</li> <li>Minimal disruption of service for all systems supporting HST operations.</li> <li>Resumption of HST operations within a few hours with the possibility of reduced speeds.</li> <li>For underground structures, no flooding or mud inflow</li> </ul>	Minor inelastic behavior, no spalling.
		No damage to HST track, track support, and rail fasteners
	<u>Primary Type 2</u> <ul style="list-style-type: none"> <li>Essentially elastic response of structural components where inelastic response (i.e.: cracking or spalling of concrete) could potentially affect HST operations.</li> <li>Occupants shall be able to evacuate the structure safely.</li> <li>Minimal disruption of service for all systems supporting HST operations.</li> <li>Resumption of HST operations within a few hours with the possibility of reduced speeds.</li> </ul> <u>Secondary (owned by Authority)</u> <ul style="list-style-type: none"> <li>OPL does not apply</li> </ul>	Negligible permanent deformation of substructure and superstructure components.

### 11.5.2 Design Earthquakes

Two design earthquakes, the Maximum Considered Earthquake (MCE) and the Operating Basis Earthquake (OBE), are defined:

- Maximum Considered Earthquake (MCE)** – Ground motions corresponding to greater of (1) a probabilistic spectrum based upon a 10% probability of exceedance in 100 years (i.e., a return period of 950 years); and (2) a deterministic spectrum based upon the largest median response resulting from the maximum rupture (corresponding to  $M_{max}$ ) of any fault in the vicinity of the structure.
- Operating Basis Earthquake (OBE)** – Ground motions corresponding to a probabilistic spectrum based upon an 86% probability of exceedance in 100 years (i.e., a return period of 50 years).

### 11.5.2.1 Hazardous Faults

1 All hazardous fault screening, characterization and displacement design values will be  
2 provided by the Authority.

3 The use of any fault characterization or displacements other than those provided by the  
4 Authority shall be subject to a design variance per the *General* chapter. The SDAP shall  
5 specifically justify the use of such alternate items.

### 11.5.2.2 Design Spectra and Ground Motion Time Histories

6 Design spectra and ground motion time histories will be provided by the Authority.

7 Each set of ground motion time histories provided by the Authority will consist of 2 orthogonal  
8 horizontal components, and 1 vertical component.

9 Where applicable, the specific directionality of the 2 orthogonal horizontal components (i.e.:  
10 fault-normal and fault-parallel directions) will be provided by the Authority including the fault  
11 and bearing angle. The specific directionality of the motions shall be retained by applying the  
12 motions to structural models along the given bearing angles directly, or transforming the  
13 motions to align with specific global or local coordinate systems of the structural models.

14 Where no specific directionality of the 2 orthogonal horizontal components exists, the motions  
15 shall be applied to the structural models directly, without transformation, along the global or  
16 local coordinate systems of the structural models. The Contractor may use engineering  
17 discretion as to which direction to apply each horizontal component, however all sets of  
18 motions shall be applied to structural models in a consistent and documented manner.

19 Refer to Section 11.7.3.3 for details regarding the application of the vertical component of  
20 motion.

21 At Special Sites, as defined in the *Geotechnical* chapter, referenced rock outcrop spectra and  
22 motions will be provided by the Authority. Site response analysis shall be used to determine the  
23 ground motions to be used for design.

24 The use of any design spectra or ground motion time histories other than those provided by the  
25 Authority shall be subject to a design variance per the *General* chapter. The SDAP shall  
26 specifically justify the use of such alternate items.

### 11.5.2.3 Time History Analysis for Final Design

27 When time history analysis is used for final design, then seven (7) sets of motions shall be used,  
28 and the average value of each response parameter (e.g., force or strain in a member,  
29 displacement or rotation at a particular location) shall be used for design.

30 For bridges, aerial structures, and grade separations, this shall apply to:

- ELTHA = Equivalent Linear Time History Analysis per Section 11.7.3.18

- NLTHA = Nonlinear Time History Analysis per Section 11.7.3.19

For passenger stations and building structures, this shall apply to:

- LDP = Linear Dynamic Procedure per Section 11.8.3.2
- NDP = Nonlinear Dynamic Procedure per Section 11.8.3.4

### 11.5.3 Retrofit of Existing Structures

See the *Structures* chapter for design considerations for Primary Type 2 structures, which addresses the retrofit of existing structures.

Existing Primary Type 2 structures shall be subject to NCL/MCE design.

Select components of existing Primary Type 2 structures shall be subject to OPL/OBE design, limited to those components where response to the OBE could potentially affect HST track or service. Examples of such response include the following:

- permanent damage which directly impedes HST safe passage (such as concrete spalls on tracks, component collapse on tracks, or loss of critical systems)
- elastic or inelastic deformations (temporary or permanent) that interfere with HST right of way and clearance envelopes
- damage resulting in significant interruption of HST service to facilitate inspection/repair

For example, OPL/OBE design would apply only to select components of existing Primary Type 2 highway, roadway, railway, and pedestrian structures that span over HST track, such as the superstructure above HST track or substructures adjacent to HST track. Components of Primary Type 2 structures removed from HST track, where response to OBE will not affect HST track or service, shall not be subject to OPL/OBE design.

Existing Secondary structures owned by the Authority shall be subject to NCL/MCE design only.

Existing Secondary structures owned by Third Parties shall be subject to the seismic criteria of the governing local jurisdiction.

A detailed discussion on the retrofit of existing structures shall be defined in the SDAP, per Section 11.3.

### 11.5.4 Seismic Requirements for Temporary Construction Structures

Temporary construction structures include new temporary structures and the temporary shoring and underpinning of existing structures. See the *Structures* chapter for design requirements for support and underpinning of structures.

For seismic requirements for temporary construction structures, the following design spectra and motions:

- 125% of the OBE design spectra and motions (to approximate a return period of 75 years), or
  - a scaled OBE spectra so the scaled peak ground acceleration (PGA) equals 0.1g
- whichever governs, shall apply.

For temporary construction structures, the performance requirements for NCL/MCE as given in Table 11-1 for Secondary Structures shall apply.

## 11.6 Seismic Design Requirements

For each general classification, Table 11-3 defines seismic design requirements for each seismic performance level and design earthquake.

Note that for Primary Type 1 structures, TSI/OBE refers to track and structure seismic performance during OBE events as defined in the Track-Structure Interaction section of the *Structures* chapter.

**Table 11-3: Seismic Design Requirements**

General Classification		
Primary Type 1	Primary Type 2	Secondary
NCL/MCE	NCL/MCE	NCL/MCE
OPL/OBE	OPL/OBE	--
TSI/OBE	--	--

## 11.7 Bridges, Aerial Structures, and Grade Separations

All Primary Type 1 and Primary Type 2 bridges, aerial structures, and grade separations shall be subject to both NCL/MCE and OPL/OBE seismic criteria herein.

All Secondary bridges, aerial structures, and grade separations owned by the Authority shall be subject to the NCL/MCE seismic criteria herein.

All Secondary bridges, aerial structures, and grade separations owned by Third Parties shall be subject to the seismic criteria of the governing local jurisdiction.

### 11.7.1 Design Codes

For NCL/MCE design, current Caltrans performance based design methods as given in CBDM form the basis of design. Certain criteria herein exceed those of CBDM. For items not specifically addressed in this or other Design Criteria chapters, CBDM shall be used. See the *Structures* chapter for the load combination including MCE events.

For OPL/OBE design, current Caltrans force based design methods as given in the AASHTO LRFD BDS with California Amendments form the basis of design. Certain criteria herein exceed those of the AASHTO LRFD BDS with California Amendments. See the *Structures* chapter for the load combinations including OBE events.

Table 11-4 summarizes the applicable seismic design code for each General Classification.

**Table 11-4: Applicable Bridge, Aerial Structure and Grade Separation Design Codes**

Performance/ Design Earthquake	General Classification		
	Primary Type 1	Primary Type 2	Secondary
NCL/MCE	CBDM	CBDM	CBDM
OPL/OBE	AASHTO LRFD BDS with California Amendments	AASHTO LRFD BDS with California Amendments	--
TSI/OBE	<i>Structures</i> chapter	--	--

### 11.7.2 Seismic Design Approach

The seismic design approach differs depending upon the design earthquake.

#### 11.7.2.1 NCL/MCE Design Approach

For NCL/MCE design, the approach shall be:

- The structure shall have a clearly defined and pre-determined mechanism for seismic response, with targeted regions for inelastic response.
- Inelastic behavior shall be limited to columns and abutments, at above soil or water surface locations.
- Regions adjacent to inelastic behavior shall be capacity protected and perform as essentially elastic. The requirement for capacity protection does not depend on the seismic demand and shall apply regardless of elastic or inelastic column behavior under MCE loading.
- Seismic design and detailing requirements per CSDC shall be satisfied.

Allowable pre-determined mechanisms for NCL/MCE design include:

- Flexural Plastic Hinging (see Section 11.7.2.3)
- Foundation Rocking (see Section 11.7.2.1)
- Energy dissipation, seismic response modification, or base isolation systems (see Section 11.7.2.5)

#### 11.7.2.2 OPL/OBE Design Approach

For OPL/OBE design, the approach shall be:



- The structure shall respond as essentially elastic.
  - For Primary Type 1 structures, the structure and track seismic performance during OBE events shall comply with the Track-Structure Interaction section of the *Structures* chapter.
- OPL/OBE demands shall be compared versus force-based capacities calculated per AASHTO LRFD BDS with California Amendments, Article 11.7.5.3.

#### **11.7.2.3 Flexural Plastic Hinging**

Flexural plastic hinging shall be limited to the columns. The location of plastic hinges shall be at above soil or water surface locations accessible for inspection and repair.

Non-fusing or capacity protected members shall be designed to prevent brittle failure mechanisms, such as footing shear, joint shear, column shear, tensile failure at the top of concrete footings, and unseating of girders. Non-fusing or capacity protected members shall be designed as essentially elastic, with 120% over-strength factor on the column plastic moment and shear applied. Consideration shall be made to column plastic hinging about all potential axes in the design of the foundation. The requirement for capacity protection does not depend on the seismic demand and shall apply regardless of elastic or inelastic column behavior under MCE loading

Modeling, analysis and design shall conform to CBDM and CSDC.

#### **In-ground flexural plastic hinging**

In-ground flexural plastic hinging may be unavoidable in special cases such as:

- Short column foundations
- Pier wall foundations in the transverse direction
- Complex geologic conditions, refer to Section 11.4.2

For cases involving in-ground hinging, a design variance shall be submitted per the *General* chapter, since such damage cannot be easily inspected. All seismic related design variances shall be identified and justified in the SDAP, as required in Section 11.3.

The design variance and SDAP shall specifically address in-ground hinging by providing mitigating measures, such as reduced in-ground ductility demand, and reference the reduced strain limits provided in Section 11.7.4.6.

#### **11.7.2.4 Foundation Rocking**

Foundation rocking is a design strategy based upon deliberately proportioning spread footing foundations to allow rocking, or transient uplift and separation of the foundation from the subsoil. Foundation rocking shall only apply to soils are not susceptible to loss of strength under the imposed cyclic loading.

Foundation rocking analysis is an iterative procedure which accounts for a lengthening of the structural period, increased damping, and subsequently reduced demands. The goal of foundation rocking is to limit column damage at the expense of significantly higher displacement demands at the superstructure.

Refer to Section 11.7.3.16 for foundation rocking methodology.

Foundation rocking shall not be allowed for Primary Type 1 structures.

Foundation rocking shall be allowed for NCL/MCE design for Primary Type 2 or Secondary Structures, except those designated as Complex per Section 11.4.2.

#### **11.7.2.5 Energy Dissipation, Seismic Response Modification, or Base Isolation Systems**

Energy dissipation, seismic response modification, or base isolation systems can be used to minimize damage, reduce seismic demands on substructures, and reduce foundation costs.

If energy dissipation, seismic response modification, or base isolation systems are proposed, then the use of such systems shall be subject to a design variance per the *General* chapter, and identified in the SDAP per Section 11.3.

For seismic isolation design, Caltrans implemented AASHTO Guide Specifications for Seismic Isolation Design shall apply.

Energy dissipation, seismic response modification, or base isolation systems shall contain sufficient capacity under service (i.e., braking and acceleration, wind, etc.) loads and OBE events, in order to meet criteria in the Track-Structure Interaction section of the *Structures* chapter.

#### **11.7.3 Seismic Demands on Structural Components**

In increasing order of complexity, analysis techniques include equivalent static analysis (ESA), response spectrum analysis (RSA), equivalent linear time history analysis (ELTHA), and nonlinear time history analysis (NLTHA).

For NCL/MCE design of Complex structures, NLTHA shall apply. For NCL/MCE design of Standard or Non-Standard structures, the appropriate analysis technique will depend upon the site specifics and structure.

For OPL/OBE design of the structure, a linear elastic analysis technique may be used. For Primary Type 1 structures, due to non-linear rail fastener slippage, NLTHA shall apply for rail-structure interaction analysis including the OBE. Refer to the *Structures* chapter.

##### **11.7.3.1 Force Demands ( $F_u$ ) for OPL/OBE**

For OPL/OBE design, the ultimate force demand,  $F_u$ , shall be determined for all structural components.

1 For the structure, the loading combination shall be as specified in the *Structures* chapter.

#### 11.7.3.2 Displacement Demands ( $\Delta_D$ ) for NCL/MCE

2 For NCL/MCE design, the global displacement demand,  $\Delta_D$ , at the center of mass of the  
3 superstructure for each bent shall be determined, and compared versus the displacement  
4 capacity,  $\Delta_C$ .

5 The loading combination shall be as specified in the *Structures* chapter.

#### 11.7.3.3 Vertical Earthquake Motions

6 Where the MCE peak rock acceleration is 0.6g or greater, an equivalent static vertical load per  
7 CSDC shall be applied to the superstructure for design in order to estimate the effects of vertical  
8 acceleration.

9 For structures at or in close proximity to hazardous faults, as defined in Section 11.5.2.1, vertical  
10 motions shall be considered. This applies to the structural loading combinations as specified in  
11 the *Structures* chapter, and the loading combinations for OBE events as specified in the Track-  
12 Structure Interaction section of the *Structures* chapter.

#### 11.7.3.4 Effective Sectional Properties

13 For NCL/MCE design, cracked bending and torsional moments of inertia for ductile  
14 substructure, and superstructure concrete members shall be per CSDC.

15 When moment-curvature analysis of concrete members is used, elemental cross sectional  
16 analysis shall be performed which considers the effects of concrete cracking, the degree of  
17 confinement, reinforcement yield and strain hardening, in accordance with CMTD and CSDC.

18 For structural steel sections, either moment-curvature analysis shall be performed which  
19 considers the stress-strain relationship of the structural steel, or effective section properties  
20 derived based upon the degree of nonlinearity shall be used. Effective section properties for  
21 structural steel components shall be consistent with AASHTO LRFD BDS with California  
22 Amendments and AISC Manual of Steel Construction.

23 For OPL/OBE design, effective bending moments of inertia for concrete column members shall  
24 consider the maximum moment demand,  $M_a$ , and the cracking moment,  $M_{cr}$ , in accordance with  
25 AASHTO LRFD BDS with California Amendments, Article 5.7.3.6.2. When using this method,  
26 the cracked moment of inertia,  $I_{cr}$ , shall be per CSDC. Alternatively, OBE effective sectional  
27 properties shall be directly found through the use of moment-curvature analysis.

#### 11.7.3.5 Mass

- 1 Both elemental and lumped mass shall be used in analysis.
- 2 Translational and rotational elemental mass is based upon the mass density, length and cross
- 3 sectional properties of discrete elements within the analytical model.
- 4 Translational and rotational lumped mass shall be based upon engineering evaluation of the
- 5 structure, and consider items modeled as rigid (i.e., pile and bent caps), or items not explicitly
- 6 modeled (i.e., non-structural mass).
- 7 Where applicable, train mass shall be considered per Section 11.7.3.12.

#### 11.7.3.6 Material Properties for Demands

- 8 For NCL/MCE design, expected material properties shall be used in calculating the structural
- 9 seismic demands in conformance with CSDC for concrete members and AASHTO LRFD BDS
- 10 with California Amendments for structural steel members.
- 11 For OPL/OBE design, nominal material properties shall be used in calculating the structural
- 12 seismic demands.

#### 11.7.3.7 Flexural Plastic Hinging

- 13 Where flexural plastic hinging is used as the NCL/MCE seismic response mechanism of the
- 14 structure, the analysis shall conform to CSDC methods and procedures.

#### 11.7.3.8 Assessment of Track-Structure Interaction

- 15 For assessment of train and track-structure interaction, including requirements and load
- 16 combinations which include OBE events, refer to the Track-Structure Interaction section of the
- 17 *Structures* chapter.

#### 11.7.3.9 Foundation Stiffness, Mass and Damping

- 18 Where applicable, foundation stiffness, mass and damping, including the effects of soil
- 19 structure interaction, shall be considered for seismic analyses per the *Geotechnical* chapter.

#### 11.7.3.10 Boundary Conditions

- 20 In cases where the structure is adjacent to or connected to other structures which are not
- 21 included in the model (e.g.: adjacent spans or abutments), the model shall also contain
- 22 appropriate elements at its boundaries to capture mass and stiffness effects of the adjacent
- 23 structures.
- 24 For NCL/MCE abutment design, longitudinal and transverse response shall be similar to CSDC,
- 25 considering the effects of cement treated gravel backfill, where used.
- 26 For OPL/OBE design of Primary Type 1 structures, abutment response shall be elastic.

1 After completion of static or dynamic analysis, a check shall be performed to verify that the  
2 boundary conditions and element properties are consistent with initial modeling assumptions.

#### 11.7.3.11 Continuous Welded Rail

3 For structures that carry continuously welded rail, there may be benefits to structural  
4 performance during a seismic event provided by the rail system. The rails may serve as  
5 restrainers at the expansion joints tying adjacent frames together under seismic loading.  
6 However, this is complex behavior, which shall be substantiated and validated through  
7 analysis. The use of continuously welded rail to benefit structural performance shall be limited  
8 to OBE design only. Such use shall only be allowed for MCE design upon approval of a design  
9 variance. All seismic related design variances shall be identified and justified in the SDAP, as  
10 required in Section 11.3.

11 Since the rail system seismic response at the expansion joints is highly nonlinear, linear elastic  
12 analysis is not appropriate. Instead NLTHA, in accordance with Section 11.7.3.19, shall be  
13 performed which considers track-structure interaction.

14 The Track-Structure Interaction section of the *Structures* chapter contains details of the rail-  
15 structure interaction modeling methodology. The rail-structure interaction shall include the  
16 rails and fastening system, modeled to consider fastener slippage and rail stiffness. The fastener  
17 non-linear longitudinal restraint, transverse stiffness, and uplift capacity shall be accounted for  
18 in the analysis. Without these rail-structure interaction considerations, any structural  
19 performance benefits provided by continuous welded rail shall be ignored.

#### 11.7.3.12 Train Mass and Live Load

20 For NCL/MCE design, train mass and live load shall not be considered.

21 For OPL/OBE design, train mass and live load shall be considered per load combinations  
22 defined in the *Structures* chapter. For all OBE load combinations, both strength and track-  
23 structure interaction per the *Structures* chapter, train loads may be modeled as equivalent static  
24 distributed loads, and train mass as stationary mass. Although equivalent distributed loads are  
25 used in the analysis, local design shall account for any effects due to actual concentrated axle  
26 loads.

27 For OBE strength load combinations, the following train effects shall be considered  
28 simultaneously:

##### 29 Single track structures:

- 30 • One train vertical live load + impact
- 31 • One train longitudinal braking force
- 32 • Mass of 1 train, applied at the center of train mass

##### 33 Multiple track structures:

One-half of trains potentially occupying the structure shall be considered. Where an odd number of trains potentially occupy the structure, round down to the nearest whole number of trains (example: for 3 trains, use  $1/2(3) = 1.5$  and round down to 1).

- 1/2 of the trains live load + impact
- 1/2 of trains longitudinal braking force (note: longitudinal braking force for multiple trains shall be applied in the same or opposite directions, whichever governs design)
- Mass of 1/2 of the trains, applied at the center of train mass

For OBE track-structure interaction load groups per the *Structures* chapter, the following train effects shall be considered simultaneously:

- One train vertical live load + impact
- One train longitudinal braking force, where applicable (Rail-Structure Interaction Only)
- Mass of 1 train, applied at the center of train mass

#### 11.7.3.13 P-Δ Effects

For flexural plastic hinging, P-Δ effects shall be considered and conform to the requirements in CSDC.

For foundation rocking response, P-Δ effects shall be considered per Section 11.7.3.16.

#### 11.7.3.14 Displacement Demand Amplification Factor

When ESA or RSA is used for NCL/MCE design, the displacement demand,  $\Delta_D$ , obtained shall be multiplied by an amplification factor,  $C$ , as follows:

$$\text{For } T^*/T_i > 1: C = [(1 - 1/\mu_d)/(T_i/T^*)] + (1/\mu_d)$$

$$\text{For } T^*/T_i \leq 1: C = 1.0$$

Where:

$T_i$  = fundamental period of structure (including foundation stiffness)

$T^*$  = the site-specific characteristic ground motion period supplied by the Authority

$\mu_d$  = the target displacement ductility demand per CSDC

The amplification factor,  $C$ , shall be applied separately in both orthogonal directions prior to obtaining the orthogonal combination of seismic displacement.

The amplification factor,  $C$ , shall not apply to foundation rocking analysis.

### 11.7.3.15 Equivalent Static Analysis

- 1 Equivalent Static Analysis (ESA) may be used to determine earthquake demands, E:
- 2
  - 3 • For NCL/MCE design, the Displacement Demand,  $\Delta_D$ , at the center of mass of the superstructure.
  - 4 • For OPL/OBE design, the Force Demands,  $F_u$
- 5 where the structure can be characterized as a single-degree-of-freedom (SDOF) system, and
- 6 more sophisticated dynamic analysis will not add significantly more insight into behavior.
- 7 For NCL/MCE and OPL/OBE design, ESA shall apply to Standard or Non-Standard structures
- 8 with the following characteristics:
- 9
  - No skew.
  - 10 • Single column piers or multiple column bents where most of the structural mass is
  - 11 concentrated at a single level.
  - 12 • The fundamental mode of vibration is uniform translation.
  - 13 • Well defined lateral force distribution due to balanced spans, and bents with approximately
  - 14 equal stiffness.
- 15 ESA shall not apply to Complex structures.
- 16 ESA earthquake demands shall be determined from horizontal spectra by either of 2 methods:
- 17
  - 18 • Method 1 – Earthquake demand,  $E = (E_L^2 + E_T^2)^{1/2}$ , where  $E_L$  and  $E_T$  are the responses due to
  - 19 longitudinal and transverse direction earthquake motions as defined below. The application
  - 20 of ground motion shall be along the principal axes of individual components.
  - 21 • Method 2 – Earthquake demand, E, by using the 100%-30% rule, for 2 cases:
  - 22 Case 1:  $E = 1.0E_L + 0.3E_T$
  - 23 Case 2:  $E = 0.3E_L + 1.0E_T$
- 24 For calculation of ESA earthquake demands for both Methods 1 and 2:
- 25 Longitudinal:  $E_L = C * S_a^L * W$
- 26 Transverse:  $E_T = C * S_a^T * W$
- 27 Where:
- 28
  - 29 C = the amplification factor, C, given in Section 11.7.3.14, for NCL/MCE design only
  - $S_a^L$  = longitudinal acceleration response spectral value at period  $T_L$

- 1  $T_L$  = fundamental period of structure in the longitudinal direction  
2 (including foundation stiffness)
- 3  $S_a^T$  = transverse acceleration response spectral value at period  $T_T$
- 4  $T_T$  = fundamental period of structure in the transverse direction (including  
5 foundation stiffness)
- 6  $W$  = tributary dead load + superimposed dead load for NCL/MCE design
- 7  $W$  = tributary dead load + superimposed dead load + train mass and live  
8 load per Section 11.7.3.12 for OPL/OBE design.

9 Effective sectional properties shall be used per Section 11.7.3.4. Material properties shall be used  
10 per Section 11.7.3.6.

11 An equivalent linear representation of foundation stiffness shall be used. Iteration shall be  
12 performed until the equivalent linear foundation stiffness converges (i.e., the assumed stiffness  
13 is consistent with the calculated response).

14 For both NCL/MCE and OPL/OBE design, the 5% damped response spectra shall be used to  
15 determine  $S_a$ .

#### 11.7.3.16 Foundation Rocking Analysis

16 Where foundation rocking is allowed per Section 11.7.2.4, the procedure presented in AASHTO  
17 Guide Specifications for LRFD Seismic Bridge Design, Appendix A: Foundation-Rocking  
18 Analysis shall be used for design. Design for P- $\Delta$  effects and column plastic hinging  
19 requirements are included within this reference.

20 For NCL/MCE design, should column plastic hinging occur concurrent with foundation rocking  
21 response, then all non-fusing or capacity protected members including the foundation, if  
22 applicable, shall be designed as essentially elastic, with 120% over-strength factor on the column  
23 plastic moment and shear applied.

24 No constraints such as tracks, ballast, significantly large soil overburden, concrete slabs or other  
25 infrastructure shall be placed over foundations designed for rocking. When determining the  
26 rocking response, consideration shall be given to possible future conditions, such as a change in  
27 depth of the soil cover above the footing or other loads that may affect rocking response.

28 Foundation rocking is limited to cases where the subsoil is not susceptible to loss of strength  
29 under cyclic loading, and the footing can be considered to be supported on a rigid perfectly  
30 plastic soil with uniform compressive capacity. Special detailing may be required to ensure that  
31 plastic soil deformations do not reduce the effective length of contact between the footing and  
32 soil. Where applicable, the Geotechnical Reports required by the *Geotechnical* chapter shall  
33 provide design parameters for foundation rocking analysis.



### 11.7.3.17 Response Spectrum Analysis

Response Spectrum Analysis (RSA) shall be used to determine earthquake demands, E:

- For NCL/MCE design, the Displacement Demand,  $\Delta_D$ , at the center of mass of the superstructure
- For OPL/OBE design, the Force Demands,  $F_u$

where ESA provides an unrealistic estimate of the dynamic behavior.

For NCL/MCE and OPL/OBE design, RSA shall apply to Standard or Non-Standard structures with the following characteristics:

- Skewed bents or abutments  $\leq 15$  degrees
- Single column pier or multiple column bents
- Response primarily captured by the fundamental structural mode shapes containing a minimum of 90% mass participation in the longitudinal and transverse directions

For NCL/MCE design, RSA shall not apply to Complex structures.

For OPL/OBE design, RSA may apply to Complex structures, upon approval of the SDAP per Section 11.3.

RSA involves creating a linear, three dimensional dynamic model of the structure, with appropriate representation of all material properties, structural stiffness, mass, boundary conditions, and foundation characteristics. The dynamic model is used to determine the fundamental structural mode shapes. A sufficient number of mode shapes shall be included to account for a minimum of 90% mass participation in the longitudinal and transverse directions.

Care shall be taken to ensure 90% mass participation for long aerial structure models. The Designer shall examine the mode shapes to ensure that they sufficiently capture the behavior of the structure.

A linear elastic multi-modal spectral analysis shall be performed using the 5% damped response spectra. The modal response contributions shall be combined using the complete quadratic combination (CQC) method.

For NCL/MCE and OPL/OBE design, modal damping shall be 5%.

For NCL/MCE design, RSA based on design spectral accelerations will likely predict forces in some elements that exceed their elastic limit, the presence of which indicates nonlinear behavior. The Designer shall recognize that forces generated by RSA could vary considerably from the actual force demands on the structure. Sources of nonlinear response not captured by RSA include the effects of surrounding soil, yielding of structural members, opening and closing of expansion joints, plastic hinging, and nonlinear restrainer and abutment behavior.

Where there is a change in soil type along the bridge alignment, consideration shall be made to the possibility that out-of-phase ground displacements at 2 adjacent piers may increase the computed demand on expansion joints, rails or columns. This effect is not explicitly considered in RSA. In such cases, more sophisticated time history analyses shall be used.

Appropriate linear stiffness shall be assumed for abutments and expansion joints similar to CSDC, considering the effects of cement treated gravel backfill, where used. Analyses shall be performed for compression models (abutments engaged, gaps between frames closed) and for tension models (abutments inactive, gaps between frames open), to obtain a maximum response envelope. If analysis results show that soil capacities are exceeded at an abutment, iterations shall be performed with decreasing soil spring constants at the abutment per CSDC recommendations.

For calculation of differential displacements at expansion joints and for calculation of column drift, the analysis shall either explicitly compute these demands as modal scalar values or assume that the displacements and rotations combine to produce the highest or most severe demand on the structure.

RSA demands shall be determined from horizontal spectra by either of the 2 following methods:

- Method 1 – Earthquake demand,  $E = (E_L^2 + E_T^2)^{1/2}$ , where  $E_L$  and  $E_T$  are the responses due to longitudinal and transverse earthquake spectra as defined below. The application of ground motion shall be along the principal axes of individual components.
- Method 2 – Earthquake demand,  $E$ , by using the 100%-30% rule, for 2 cases:

$$\text{Case 1 : } E = 1.0E_L + 0.3E_T$$

$$\text{Case 2 : } E = 0.3E_L + 1.0E_T$$

For calculation of RSA earthquake demands:

$$\text{Longitudinally: } E_L = C * (\text{RSA demands from longitudinal earthquake spectra})$$

$$\text{Transversely: } E_T = C * (\text{RSA demands from transverse earthquake spectra})$$

Where:

$$C = \text{the amplification factor per Section 11.7.3.14, for NCL/MCE design only}$$

Effective sectional properties shall be used per Section 11.7.3.4. Material properties shall be used per Section 11.7.3.6. An equivalent linear representation of foundation stiffness shall be used. Iteration shall be performed until the equivalent linear foundation stiffness converges (i.e., the assumed stiffness is consistent with the calculated response).

For NCL/MCE design, dead and superimposed dead loads shall be applied as an initial condition.

1 For OPL/OBE design, in addition to dead and superimposed dead loads, train mass and live  
2 load shall be considered per Section 11.7.3.12.

3 After completion of each RSA, the Designer shall verify that structural members which are  
4 modeled as elastic do remain elastic and satisfy strength requirements.

#### **11.7.3.18 Equivalent Linear Time History Analysis**

5 Equivalent Linear Time History Analysis (ELTHA) shall be used to determine earthquake  
6 demands, E:

- 7 • For NCL/MCE design, the Displacement Demand,  $\Delta_D$ , at the center of mass of the  
8 superstructure
- 9 • For OPL/OBE design, the Force Demands,  $F_u$

10 where ESA or RSA provides an unrealistic estimate of the dynamic behavior.

11 For NCL/MCE and OPL/OBE design, ELTHA shall apply to Standard or Non-Standard  
12 structures with the following characteristics:

- 13 • Skewed bents or abutments  $\leq 15$  degrees
- 14 • Single column pier or multiple column bents

15 For NCL/MCE design, ELTHA shall not apply to Complex structures.

16 For OPL/OBE design, ELTHA may apply to Complex structures, upon approval of the SDAP  
17 per Section 11.3.

18 ELTHA involves creating a three dimensional dynamic model of the structure, with appropriate  
19 representation of all material properties, structural stiffness, mass, boundary conditions, and  
20 foundation characteristics.

21 For NCL/MCE and OPL/OBE design, motions consistent with the 5% damped response spectra  
22 shall be used. Consideration of vertical earthquake motions shall be considered per Section  
23 11.7.3.3.

24 For NCL/MCE and OPL/OBE design, structural damping shall be 5%.

25 Should Rayleigh damping be used for ELTHA, it requires the calculation of both stiffness and  
26 mass proportional coefficients anchored at 2 structural frequencies, which shall envelope all  
27 important modes of structural response. The lower anchoring frequency (i.e., longest period)  
28 shall be determined using effective section properties per Section 11.7.3.4 and by reducing the  
29 resulting lowest natural frequency by 10%. The higher anchoring frequency (i.e., shortest  
30 period) shall be chosen such that a minimum of 90% mass participation in the longitudinal,  
31 transverse directions are mobilized.

Effective sectional properties shall be used per Section 11.7.3.4. Material properties shall be used per Section 11.7.3.6.

Appropriate linear stiffness shall be assumed for abutments and expansion joints similar to CSDC, considering the effects of cement treated gravel backfill, where used. Analyses shall be performed for compression models (abutments engaged, gaps between frames closed) and for tension models (abutments inactive, gaps between frames open), to obtain a maximum response envelope. If analysis results show that soil capacities are exceeded at an abutment, iterations shall be performed with decreasing soil spring constants at the abutment per CSDC recommendations.

An equivalent linear representation of foundation stiffness shall be used. Iteration shall be performed until the equivalent linear foundation stiffness converges (i.e., the assumed stiffness is consistent with the calculated response).

For NCL/MCE design, dead and superimposed dead loads shall be applied as an initial condition.

For OPL/OBE design, in addition to dead and superimposed dead loads, train mass and live load shall be considered per Section 11.7.3.12.

After completion of each ELTHA, the Designer shall verify that structural members which are modeled as elastic do remain elastic and satisfy strength requirements.

#### 11.7.3.19 Nonlinear Time History Analysis

Nonlinear Time History Analysis (NLTHA) shall be used to determine earthquake demands, E:

- For NCL/MCE design, the Displacement Demand,  $\Delta_D$ , at the center of mass of the superstructure
- For OPL/OBE design, the Force Demands,  $F_u$

where ESA, RSA or ELTHA provides an unrealistic estimate of the dynamic behavior, provides overly conservative demands, or where nonlinear response is critical for design.

For NCL/MCE design, NLTHA shall apply to Complex structures.

For OPL/OBE design, NLTHA, ELTHA, or RSA may apply to Complex structures, upon approval of the SDAP per Section 11.3.

For TSI/OBE design of Primary Type 1 structures, due to required track and structure seismic performance during OBE events per the Track-Structure Interaction section of the *Structures* chapter, NLTHA shall be used.

NLTHA involves creating a three dimensional dynamic model of the structure, with appropriate representation of all material properties, structural stiffness, mass, boundary conditions, and foundation characteristics. This dynamic model is used to determine the

dynamic characteristics of the structure by including selected nonlinear representations of structural and foundation elements.

For NCL/MCE and OPL/OBE design, motions consistent with the 5% damped response spectra shall be used. Consideration of vertical earthquake motions shall be considered per Section 11.7.3.3.

For NCL/MCE and OPL/OBE design, structural damping shall be 5%.

Should Rayleigh damping be used for NLTHA, it requires the calculation of both stiffness and mass proportional coefficients anchored at 2 structural frequencies, which shall envelop all important modes of structural response. The lower anchoring frequency (i.e., longest period) shall be determined using effective section properties per Section 11.7.3.4 and by reducing the resulting lowest natural frequency by 10%. The higher anchoring frequency (i.e., shortest period) shall be chosen such that a minimum of 90% mass participation in the longitudinal, transverse directions are mobilized.

Effective sectional properties or moment-curvature analysis shall be used per Section 11.7.3.4. Material properties shall be used per Section 11.7.3.6.

Appropriate linear stiffness may be assumed for abutments and expansion joints similar to CSDC, considering the effects of cement treated gravel backfill, where used. Analyses shall be performed for compression models (abutments engaged, gaps between frames closed) and for tension models (abutments inactive, gaps between frames open), to obtain a maximum response envelope. If analysis results show that soil capacities are exceeded at an abutment, iterations shall be performed with decreasing soil spring constants at the abutment per CSDC recommendations. Otherwise, nonlinear representations of abutment and expansion joint characteristics shall be used.

Where applicable, an equivalent linear representation of foundation stiffness shall be used, and iteration shall be performed until the equivalent linear foundation stiffness converges (i.e., the assumed stiffness is consistent with the calculated response). Otherwise, nonlinear representations of foundation characteristics shall be used.

For NCL/MCE design, dead and superimposed dead loads shall be applied as an initial condition.

For OPL/OBE design, in addition to dead and superimposed dead loads, train mass and live load shall be considered per Section 11.7.3.12.

After completion of each NLTHA, the Designer shall verify that structural members which are modeled as elastic do remain elastic and satisfy strength requirements.

## 11.7.4 Seismic Capacities of Structural Components

### 11.7.4.1 Force Capacities ( $\Phi F_N$ ) for OPL/OBE

For OPL/OBE design, LRFD force capacities,  $\Phi F_N$ , for all structural components shall be found in accordance with AASHTO LRFD BDS with California Amendments. Nominal material properties shall be used when determining OBE capacities.

### 11.7.4.2 Displacement Capacity ( $\Delta_c$ ) for NCL/MCE

For NCL/MCE design employing flexural plastic hinging using ESA, RSA, and ELTHA demands, the displacement capacity,  $\Delta_c$ , shall be determined by nonlinear static pushover analysis, as described in Section 11.7.4.3. The displacement capacity shall be defined as the controlling structure displacement that occurs when any element of targeted inelastic response reaches its allowable capacity in the pushover analysis. The allowable capacity is reached when the concrete or steel strain of any element of targeted inelastic response meets the allowable strains specified in Sections 11.7.4.5 to 11.7.4.8.

If moment curvature representation of plastic hinging is used for NLTHA, then the curvature demands shall be converted to concrete or steel strains, and verified versus allowable strains specified in Sections 11.7.4.5 to 11.7.4.8.

The displacement capacity,  $\Delta_c$ , shall include all displacements attributed to flexibility in the foundations, bent caps, and other elastic and inelastic member responses in the system. The assumptions made to determine the displacement capacity,  $\Delta_c$ , shall be consistent with those used to determine the displacement demand,  $\Delta_D$ .

All capacity protected structural members and connections shall satisfy requirements in Section 11.7.5.5.

### 11.7.4.3 Nonlinear Static Pushover Analysis

For NCL/MCE design employing flexural plastic hinging, the following procedure shall be followed to determine the displacement capacity,  $\Delta_c$ , using nonlinear static pushover analysis.

Dead and superimposed dead load shall be applied as an initial step.

Incremental lateral displacements shall be applied to the system. A plastic hinge shall be assumed to form in an element when the internal moment reaches the idealized yield limit in accordance with Section 11.7.3.7. The sequence of plastic hinging through the frame system shall be tracked until an ultimate failure mode is reached. The system capacity shall then be determined in accordance with CSDC.

### 11.7.4.4 Plastic Hinge Rotational Capacity

Plastic moment capacity of ductile flexural members shall be calculated by moment-curvature ( $M-\phi$ ) analysis and shall conform to CSDC for concrete members and AASHTO LRFD BDS with California Amendments for structural steel members.

The rotational capacity of any plastic hinge is defined as the product of the plastic hinge length, as defined by CSDC for concrete members and AASHTO LRFD BDS with California Amendments for structural steel members, and the curvature (from M- $\phi$  analysis) when the element of targeted inelastic response first reaches the allowable strains in Sections 11.7.4.5 to 11.7.4.8.

#### 11.7.4.5 Strain Limits for Ductile Reinforced Concrete Members

For NCL/MCE design, the following reinforcing steel (A706/Grade 60) allowable tensile strain limits ( $\epsilon_{su}^a$ ) shall apply for ductile reinforced concrete members:

$$\text{NCL/MCE: } \epsilon_{su}^a \leq 2/3 \epsilon_{su}$$

Where:  $\epsilon_{su}$  = ultimate tensile strain per CSDC

For NCL/MCE design, the following allowable confined concrete compressive strain limits ( $\epsilon_{cu}^a$ ) shall apply for ductile reinforced concrete members:

$$\text{MCE: } \epsilon_{cu}^a \leq \epsilon_{cu}$$

Where:  $\epsilon_{cu}$  = ultimate compressive strain as computed by Mander's model for confined concrete.

#### 11.7.4.6 Reduced Strain Limits for Ductile Reinforced Concrete Caissons, Piles, and Drilled Shafts

For cases involving in-ground hinging, a design variance shall be submitted per the *General* chapter, since such damage cannot be easily inspected. All seismic related design variances shall be identified and justified in the SDAP, as required in Section 11.3.

Where in-ground hinging is allowed by variance, the following reduced strain limits for ductile reinforced concrete caissons, piles, and drilled shafts apply:

For NCL/MCE design, the following reinforcing steel (A706/Grade 60) allowable tensile strain limit ( $\epsilon_{su}^a$ ) shall apply for ductile reinforced concrete caissons, piles, and drilled shafts:

$$\text{NCL/MCE: } \epsilon_{su}^a \leq \epsilon_{sh}$$

Where:  $\epsilon_{sh}$  = tensile strain at the onset of strain hardening per CSDC

For NCL/MCE design, the following allowable confined concrete compressive strain limits ( $\epsilon_{cu}^a$ ) shall apply for ductile reinforced concrete caissons, piles, and drilled shafts:

$$\text{MCE: } \epsilon_{cu}^a \leq \text{lesser of } 0.008 \text{ or } 0.33\epsilon_{cu} \text{ or } 1.5 \epsilon_{cc}$$

Where:  $\epsilon_{cu}$  = ultimate compressive strain as computed by Mander's model for confined concrete

$\epsilon_{cc}$  = strain at maximum concrete compressive stress as computed by Mander's model for confined concrete.

#### 11.7.4.7 Strain Limits for Unconfined Concrete

Unconfined compressive strain limits shall be applied to concrete members without sufficient lateral reinforcement to be considered confined. If the lateral reinforcement does not meet the requirements of CBDM for confinement, the section shall be considered unconfined.

For NCL/MCE design, the following allowable concrete unconfined compressive strain limit ( $\epsilon_{cu}^a$ ) applies:

$$\epsilon_{cu}^a = 0.005, \text{ for above and below ground concrete}$$

For NCL/MCE design, there are no allowable strain requirements for unconfined cover concrete.

#### 11.7.4.8 Strain Limits for Structural Steel Elements

For NCL/MCE design, the following structural steel allowable tensile strain limits ( $\epsilon_{su}^a$ ) apply:

$$\text{NCL/MCE: } \epsilon_{su}^a \leq 2/3 \epsilon_{su}$$

Where:  $\epsilon_{su}$  = ultimate tensile strain

Structural steel allowable compressive strain limits shall be determined based upon governing local or global buckling in accordance with AASHTO LRFD BDS with California Amendments, using expected material properties.

#### 11.7.4.9 Foundation Rocking Capacity

For NCL/MCE design, where foundation rocking is allowed per Section 11.7.2.4, the foundation rocking capacity shall be determined per Section 11.7.3.16.

#### 11.7.4.10 Material Properties for Capacities

For NCL/MCE design, the plastic moment capacity of all ductile concrete members shall be based upon expected material properties. Where brittle failure is a concern, such as shear, joint shear or torsion, capacities shall be based upon nominal material properties. Expected material properties shall conform to CSDC for concrete members and AASHTO LRFD BDS with California Amendments for structural steel members.

For OPL/OBE design, nominal material properties shall be used in calculating all capacities.

#### 11.7.4.11 Shear Capacity

For NCL/MCE design, the shear capacity of ductile components shall conform to CSDC for concrete members and AASHTO LRFD BDS with California Amendments for structural steel members.



#### 11.7.4.12 Joint Internal Forces

Continuous force transfer through the column/superstructure and column/footing joints shall conform to CSDC. These joint forces require that the joint have sufficient over-strength to ensure essentially elastic behavior in the joint regions based on the capacity of the adjacent members.

### 11.7.5 Seismic Performance Evaluation

#### 11.7.5.1 Definition of Essentially Elastic

For both NCL/MCE over-strength and OPL/OBE general design, “essentially elastic” is defined as when the LRFD force capacities ( $\Phi F_N$ ) exceed the over-strength or factored demands.

#### 11.7.5.2 Foundation Rocking

For NCL/MCE design, where foundation rocking is allowed per Section 11.7.2.4, seismic performance evaluation shall be per Section 11.7.3.16.

#### 11.7.5.3 Force Based Design for OPL/OBE

For OPL/OBE design, the maximum force based Demand/Capacity (D/C) Ratio shall be:

$$F_U / \Phi F_N \leq 1.0$$

Where:

$F_U$  = the force demand, as defined in Section 11.7.3.1.

$\Phi F_N$  = the LRFD force capacity, as defined in Section 11.7.4.1.

in order to satisfy the OPL performance objectives specified in Section 11.5.1.

#### 11.7.5.4 Displacement Based Design for NCL/MCE

For NCL/MCE design, the maximum displacement Demand/Capacity Ratio shall be:

$$\Delta_D / \Delta_C \leq 1.0$$

Where:

$\Delta_D$  = the displacement demand, as defined in Section 11.7.3.2.

$\Delta_C$  = the displacement capacity, based on strain limits, as defined in Section 11.7.4.2.

in order to satisfy the NCL performance objectives specified in Section 11.5.1.

#### 11.7.5.5 Capacity Protected Element Design

In order to limit the inelastic deformations to the prescribed element of targeted inelastic response, the plastic moments and shears of the element of targeted inelastic response shall be

used in the demand/capacity analysis of any adjacent capacity-protected elements of the structure.

Component 120% over-strength factors for the evaluation of capacity-protected elements shall be applied as specified in CSDC for concrete members and AASHTO LRFD BDS with California Amendments for structural steel members.

#### 11.7.5.6 Soil Improvement

The Geotechnical Reports required by the *Geotechnical* chapter shall provide information and design parameters regarding soil improvement.

#### 11.7.5.7 Design of Shallow Foundations

The Geotechnical Reports required by the *Geotechnical* chapter shall provide information and design parameters regarding the design of shallow foundations.

Shallow foundations shall be designed as capacity protected structural elements under any loading or combination of loadings, including seismic loads. When designing for footing shear, column-to-footing joint shear, and moments in footings, the column plastic moment and shear shall be used with 120% over strength factors applied.

#### 11.7.5.8 Design of Deep Foundations

The Geotechnical Reports required by the *Geotechnical* chapter shall provide information and design parameters regarding the design of deep foundations, such as bored or driven piles, drilled shafts, or caissons.

Deep foundations shall be designed as capacity protected structural elements under any loading or combination of loadings, including seismic loads. When designing for pile/drilled shaft cap shear, column-to-pile/drilled shaft cap joint shear, and moments in pile/drilled shaft cap, the column plastic moment and shear shall be used with 120% over-strength factors applied.

If below ground plastic hinging is unavoidable in caissons, piles or drilled shafts, such as at potentially liquefiable or exceptionally soft soil sites, then a design variance shall be submitted per the *General* chapter. All seismic related design variances shall be identified and justified in the SDAP, as required in Section 11.3.

The design of deep foundations shall be in accordance with CBDM.

#### 11.7.5.9 Expansion Joints, Seat Width, Restrainers, and Shear Keys

Structural expansion joints shall provide free movement (i.e.: no pounding) for OPL/OBE design. Local damage to structural expansion joints is allowed for NCL/MCE design, in accordance with CBDM.

1 Relative expansion joint displacements (longitudinal, transverse, and vertical) for load cases  
2 including OBE shall comply with limits contained within the Track-Structure Interaction section  
3 of the *Structures* chapter.

4 Adequate seat width at expansion joints and abutments shall be provided to prevent unseating  
5 of the structure, and shall comply with CSDC.

6 Expansion joint restrainers may be designed to limit relative longitudinal expansion joint  
7 displacements for OPL/OBE response; design shall be in accordance with CBDM.

8 Sacrificial components, such as seat type abutment shear keys, are not subject to capacity  
9 protection under NCL/MCE design. Additional restraint shall be considered if stability is  
10 questionable after shear keys are severely damaged.

11 For Primary Type 1 structures, seat type abutment shear keys shall be designed as essentially  
12 elastic for OPL/OBE design.

13 For NCL/MCE design, when excessive longitudinal or transverse seismic displacement must be  
14 prevented, non-sacrificial shear keys shall be provided and designed as capacity-protected  
15 elements.

#### **11.7.5.10 Columns**

16 Columns shall satisfy the detailing requirements for ductile structural elements as specified in  
17 CSDC. Ductile detailing requirements apply to all columns, even those designed to be  
18 essentially elastic due to foundation rocking or energy dissipation, seismic response  
19 modification, or base isolation systems.

20 The use of lightweight concrete is not allowed in columns.

21 The column reinforcement ratio shall be kept below 4% to reduce congestion due to added joint  
22 reinforcement. Column reinforcement shall not be adjusted for drain pipes or other utilities in  
23 potential plastic hinge zones. For column flare design and detailing, CSDC shall apply.

#### **11.7.5.11 Superstructures and Bent Caps**

24 Superstructures and bent caps shall be designed as capacity protected elements and shall  
25 conform to the requirements of CSDC.

#### **11.7.5.12 Structural Joints**

26 Structural joints (e.g.: column/superstructure, column/bent cap, or column/footing) shall  
27 conform to the requirements of CSDC.

### **11.8 Passenger Stations and Building Structures**

28 All Primary Type 1 passenger stations and building structures shall be subject to the seismic  
29 criteria for Bridges, Aerial Structures, and Grade Separations per Section 11.7.

- 1 All Primary Type 2 passenger stations and building structures shall be subject to both
- 2 NCL/MCE and OPL/OBE seismic criteria herein.
- 3 All Secondary passenger stations and building structures owned by the Authority shall be
- 4 subject to the NCL/MCE seismic criteria herein.
- 5 All Secondary passenger stations and building structures owned by Third Parties shall be
- 6 subject to the applicable Third Party seismic criteria.

### 11.8.1 Design Codes

- 7 For NCL/MCE design of Primary Type 2 structures, ASCE 41 shall apply. Although ASCE 41 is
- 8 a document originally issued for seismic rehabilitation of existing buildings, it is applicable in
- 9 absence of a similar performance based code for the seismic design of new buildings. Certain
- 10 criteria herein might exceed those of ASCE 41. If items are not specifically addressed in this or
- 11 other chapters, ASCE 41 shall be used.
- 12 For OPL/OBE design of Primary Type 2 structures, current CBC force based design methods
- 13 shall apply. Note that the OPL/OBE load combination, as given in the *Structures* chapter, is a
- 14 strength load combination. No seismic response modification factors shall apply to the OBE
- 15 demands.
- 16 For NCL/MCE design of Secondary structures owned by the Authority, current CBC seismic
- 17 design shall apply, including applicable use of seismic response modification factors.
- 18 Table 11-5 summarizes the applicable seismic design code for each General Classification.

**Table 11-5: Applicable Passenger Station and Building Structure Design Codes**

Performance/ Design Earthquake	General Classification		
	Primary Type 1	Primary Type 2	Secondary
NCL/MCE	CBDM <sup>1</sup>	ASCE 41	CBC (Seismic Design)
OPL/OBE	AASHTO LRFD BDS with California Amendments <sup>1</sup>	CBC (Strength Design)	--
TSI/OBE	<i>Structures</i> chapter	--	--

Notes:

<sup>1</sup> as amended by Section 11.7

### 11.8.2 Seismic Design Approach

- 21 The seismic design approach differs depending upon the design earthquake.

### 11.8.2.1 NCL/MCE Design Approach

For Primary Type 2 passenger station and building structures, NCL/MCE design shall include the following:

- A “weak beam - strong column” design, plastic hinges shall form in the beams and not in the columns. Proper detailing shall be implemented to avoid any kind of nonlinearity or failure in the joints. The formation of a plastic hinge shall take place in the beam element at a distance not less than twice the beam depth away from the face of the joint by adequate detailing.
- The structure shall have a clearly defined mechanism for response to seismic loads with clearly defined load path and load carrying systems.
- Each component shall be classified as deformation-controlled (ductile) or force-controlled (non-ductile). The station or building shall be provided with at least 1 continuous load path to transfer seismic forces, induced by ground motion in any direction, from the point of application to the final point of resistance. All components shall be capable of resisting deformation and force actions within the applicable criteria.
- Ductile detailing and proportioning requirements shall be satisfied. No brittle failures shall be allowed.

If energy dissipation, seismic response modification, or base isolation systems are used, then a design variance shall be submitted per the *General* chapter. In addition, the use of such systems shall be identified in the SDAP, as discussed in Section 11.3.

### 11.8.2.2 OPL/OBE Design Approach

For Primary Type 2 structures, for OPL/OBE design, the approach shall be:

- The station or building shall respond as essentially elastic.

## 11.8.3 Seismic Demands on Structural Components

### 11.8.3.1 Analysis Techniques – General

The station or building shall be modeled, analyzed, and evaluated as a three-dimensional assembly of elements and components. SSI shall be considered in the modeling and analysis.

Structures shall be analyzed using Linear Dynamic Procedure (LDP), Nonlinear Static Procedure (NSP), or Nonlinear Dynamic Procedure (NDP).

Unless it is shown that the conditions and requirements for LDP or NSP can be satisfied, all structures shall be analyzed using NDP.

### 11.8.3.2 Linear Dynamic Procedure

LDP shall be used in accordance with the requirements of ASCE 41. This can be either a response spectrum method or time-history method as applicable. Buildings shall be modeled

with linear elastic stiffness and equivalent viscous damping values consistent with the behavior of the components responding at or near yield level, as defined in ASCE 41.

When LDP response spectrum method is used, modal combination shall be performed using the CQC approach, while spatial combination shall be performed using the square root of the sum of the squares (SRSS) technique.

When LDP time history method is used, input ground motions shall be applied to a three-dimensional model of the structure. Where the relative orientation of the ground motions cannot be determined, the ground motion shall be applied in the direction that results in the maximum structural demands.

For buildings that have 1 or more of the following conditions, LDP shall not be used:

- In-Plane Discontinuity Irregularity, unless it is shown that the building remains linear elastic per requirements of Section 2.4.1.1.1 of ASCE 41
- Out-of-Plane Discontinuity Irregularity, unless it is shown that the building remains linear elastic per requirements of Section 2.4.1.1.2 of ASCE 41
- Weak Story Irregularity, unless it is shown that the building remains linear elastic per requirements of Section 2.4.1.1.3 of ASCE 41
- Torsional Strength Irregularity, unless it is shown that the building remains linear elastic per requirements of Section 2.4.1.1.4 of ASCE 41
- Building structures subject to potential foundation sliding, uplift and/or separation from supporting soil (near field soil nonlinearity)
- Building structures that include components with nonlinear behavior such as, but not limited to, buckling, expansion joint closure
- When energy dissipation, seismic response modification, or base isolation systems are used
- When the building site is at or in close proximity to hazardous faults, as defined in Section 11.5.2.1, or for ground motions with near-field pulse-type characteristics, a time history analysis shall be used.

After completion of each LDP, the Designer shall verify that structural members which are modeled as elastic do remain elastic and satisfy strength requirements.

### 11.8.3.3 Nonlinear Static Procedure

For NSP, a mathematical model directly incorporating the nonlinear load-deformation characteristics of individual components and elements of the building shall be developed and subjected to monotonically increasing lateral loads representing inertia forces in an earthquake until a target displacement is exceeded. Mathematical modeling and analysis procedures shall comply with the requirements of ASCE 41. The target displacement shall be calculated by the procedure described in ASCE 41. At least 2 types of lateral load pattern shall be considered as described in ASCE 41. The pushover analysis shall be performed in 2 principal directions

independently. Force-controlled actions shall be combined using SRSS, while deformation-controlled action shall be combined arithmetically. Due to soil properties, the embedded and underground building structures may have different behavior when they are pushed in opposite directions. In these cases the NSP shall include pushover analysis in 2 opposite directions (for a total of 4 analyses for 2 principal directions). When the response of the structure is not primarily in 1 of the principal directions, the pushover analysis shall consider non-orthogonal directions to develop a spatial envelope of capacity.

For buildings that have 1 or more of the following conditions, NSP shall not be used:

- For buildings that the effective modal mass participation factor in any 1 mode for each of its horizontal principal axes is not 70% or more
- If yielding of elements results in loss of regularity of the structure and significantly alters the dynamic response of the structure
- When ignoring the higher mode shapes has an important effect on the seismic response of the structure
- When the mode shapes significantly change as the elements yield
- When 1 of the structure's main response is torsion
- When energy dissipation, seismic response modification, or base isolation systems are used

After completion of each NSP, the Designer shall verify that structural members which are modeled as elastic do remain elastic and satisfy strength requirements.

#### 11.8.3.4 Nonlinear Dynamic Procedure

For NDP, a mathematical model directly incorporating the nonlinear load deformation characteristics of individual components and elements of the building shall be subjected to earthquake shaking represented by ground motion time histories in accordance with these design criteria. Mathematical modeling and analysis procedures shall comply with the requirements of ASCE 41

When NDP is used, input ground motions shall be applied to a three-dimensional model of the structure. Where the relative orientation of the ground motions cannot be determined, the ground motion shall be applied in the direction that results in the maximum structural demands.

As a minimum, NDP shall comply with the following guidelines:

- Dead and required live loads shall be applied as an initial condition.
- In case of embedded building structures, hydrostatic pressure, hydrodynamic pressure, earth pressure, and buoyancy shall be applied along with dead and required live loads. Where these loads result in reducing other structural demands, such as uplift or

overturning, the analyses shall consider lower and upper bound values of these loads to compute reasonable bounding demands.

- For the deformation-controlled action members the deformations shall be compared with the strain limits for each performance level as specified in this document.
- For force-controlled action members the force demand shall be resisted by capacities calculated per ASCE 41, ACI, and AISC.

After completion of each NDP, the Designer shall verify that structural members which are modeled as elastic do remain elastic and satisfy strength requirements.

#### **11.8.3.5 Local Detailed Finite Element Model**

Local detailed finite element models shall be considered as tools to better understand and validate the behavior of the structure when it cannot be obtained from the global model.

#### **11.8.3.6 Floor Diaphragm**

Mathematical models of buildings with stiff or flexible diaphragms shall account for the effects of diaphragm flexibility by modeling the diaphragm as an element with in-plane stiffness consistent with the structural characteristics of the diaphragm system.

When there is interest in the response of equipment installed on the floor diaphragm, proper modeling of the floor shall be made to capture vertical vibration modes of the floor.

#### **11.8.3.7 Building Separation**

Buildings shall be separated from adjacent structures to prevent pounding per requirements specified in Section 2.6.10.1 of ASCE 41. Exempt conditions described in Section 2.6.10.2 of ASCE 41 shall not be permitted.

#### **11.8.3.8 Material Properties for Demands**

Concrete and steel material properties for demands shall be per Section 11.7.3.6. For other material types (e.g.: aluminum, masonry, wood, and others), properties for demands shall be per the CBC.

#### **11.8.3.9 Effective Sectional Properties**

Effective sectional properties shall be per Section 11.7.3.4.

#### **11.8.3.10 Foundation Stiffness, Mass and Damping**

Where applicable, foundation stiffness, mass and damping, including the effects of soil structure interaction, shall be considered for seismic analyses per the *Geotechnical* chapter.

Below grade structures shall be modeled as embedded structures to incorporate and simulate proper soil properties and distribution in the global model. The near field (secondary non-linear) and far field (primary non-linear) effects shall be incorporated in the model. The far field effect shall be modeled with equivalent linear elastic soil properties (stiffness, mass and damping), while the near field soil properties shall represent the yielding behavior of the soil



1 using classic plasticity rules. Input ground motions provided by the Authority shall be used as  
2 appropriate: the application of the motions to the global model shall be illustrated in the SDAP,  
3 as required in Section 11.3.

4 At-grade and above-grade buildings shall be connected to the near field soil with nonlinear  
5 properties when the soil behavior is expected to be subjected to high strains near the structure.  
6 Input ground motions, as supplied by the Authority, shall be applied to the ground nodes of the  
7 soil elements.

#### **11.8.3.11 Boundary Conditions**

8 In cases where the building is adjacent to or connected to other structures that are not included  
9 in the model, the model shall contain appropriate elements at its boundaries to capture mass  
10 and stiffness effects of the adjacent structures.

11 After completion of static or dynamic analysis, a check shall be performed to verify that the  
12 boundary conditions and element properties are consistent with initial modeling assumptions.

#### **11.8.3.12 Multidirectional Seismic Effects**

13 The ground motions shall be applied concurrently in 2 horizontal directions and vertical  
14 direction per ASCE 41. In the demand and capacity assessment of deformation-controlled  
15 actions, simultaneous orthogonality effects shall be considered.

#### **11.8.3.13 Load and Load Combinations**

16 Seismic loads and load combinations shall comply with the requirements of the *Structures*  
17 chapter. For embedded and underground buildings hydrostatic pressure, hydrodynamic  
18 pressure, earth pressure and buoyancy shall be included in addition to dead load and live load.  
19 Differential settlement shall be included for buildings.

#### **11.8.3.14 Accidental Horizontal Torsion**

20 In a three-dimensional analysis, the effect of accidental torsion shall be included in the model.  
21 Accidental torsion at a story shall be calculated as the seismic story force multiplied by 5% of  
22 the horizontal dimension at the given floor level measure perpendicular to the direction of  
23 applied load. Torsion needs not be considered in buildings with flexible diaphragms.

#### **11.8.3.15 P-Δ Effects**

24 Geometric nonlinearity or P-Δ effects shall be incorporated in the analysis.

#### **11.8.3.16 Overturning**

25 Structures shall be designed to resist overturning effects caused by seismic forces. Each vertical-  
26 force-resisting element receiving earthquake forces due to overturning shall be investigated for  
27 the cumulative effects of seismic forces applied at and above the level under consideration. The  
28 effects of overturning shall be evaluated at each level of the structure as specified in ASCE 41.  
29 The effects of overturning on foundations and geotechnical components shall be considered in  
30 the evaluation of foundation strength and stiffness as specified in ASCE 41.

#### **11.8.3.17 Seismic Capacities of Structural Components**

- 1 The component capacities shall be computed based on methods given in Chapters 5 and 6 of
- 2 ASCE 41 for steel and concrete structures, respectively. However, strain limits described in
- 3 Sections 11.7.4.5 and 11.7.4.8 of this document shall be used.

#### **11.8.3.18 Material Properties for Capacities**

- 4 Concrete and steel material properties for capacities shall be per Section 11.7.4.10. For other
- 5 material types (e.g.: aluminum, masonry, wood, and others), properties for capacities shall be
- 6 per the CBC.

#### **11.8.3.19 Capacity of Members with Force-Controlled Action**

- 7 Axial force, bending moment and shear capacities shall be computed in accordance with the
- 8 requirement of ASCE 41.

#### **11.8.3.20 Capacity Protected Element Design**

- 9 For NCL/MCE design, pre-determined structural components may undergo flexural plastic
- 10 hinging, and 120% over strength factors shall be applied to capacity protected members to
- 11 protect against brittle failure mechanisms. All other structural components not pre-determined
- 12 for flexural plastic hinging shall be designed to remain elastic under the MCE.
- 13 For OPL/OBE design, the structure shall respond as essentially elastic.

## Chapter 12

### Structures

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	13 Mar 12	Initial Release, R0
0.1	07 Dec 12	Addendum 7 – CP01
1	June 2013	Revision 1
2	Feb 2014	Revision 2
3	Feb 2016	Revision 3



## Table of Contents

<b>12</b>	<b>STRUCTURES.....</b>	<b>12-1</b>
12.1	Scope .....	12-1
12.2	Regulations, Codes, Standards, and Guidelines .....	12-1
12.3	Types of Structures .....	12-3
12.4	Structural Design Requirements.....	12-3
12.4.1	Structural Design Parameters.....	12-3
12.4.2	Seismic Design.....	12-4
12.5	Permanent and Transient Loads and Load Combinations for Primary Structures ....	12-4
12.5.1	Permanent Loads.....	12-4
12.5.2	Transient Loads.....	12-8
12.5.3	Miscellaneous Loads.....	12-27
12.5.4	Load Factors and Load Modifiers.....	12-32
12.6	Track-Structure Interaction .....	12-37
12.6.1	Track-Structure Interaction Design and Analysis Plan.....	12-38
12.6.2	Design Parameters.....	12-40
12.6.3	Frequency Analysis.....	12-41
12.6.4	Track Serviceability Analysis.....	12-44
12.6.5	Rail-Structure Interaction Analysis .....	12-54
12.6.6	Dynamic Structural Analysis .....	12-62
12.6.7	Dynamic Vehicle-Track-Structure Interaction Analysis .....	12-66
12.6.8	Modeling Requirements .....	12-69
12.7	Structural Design of Stations, Surface Facilities, Buildings and Ancillary Structures ..	12-77
12.7.1	Load Requirements for Stations, Surface Facilities, Buildings, and Ancillary Structures ..	12-77
12.7.2	Foundations for Equipment Enclosures .....	12-84
12.7.3	Foundations for Utility Equipment .....	12-84
12.8	Design Considerations for Primary Type 1 Bridges, Aerial Structures, and Grade Separations .....	12-84
12.8.1	General Design Requirements.....	12-85
12.8.2	Design Loads and Effects .....	12-89
12.8.3	Foundations.....	12-89
12.8.4	Steel Structures.....	12-90
12.8.5	Concrete Structures .....	12-90
12.8.6	General Bridge, Aerial Structure, and Grade Separation Features .....	12-92
12.8.7	Complex and Non-Standard Structures .....	12-100
12.8.8	Emergency Access/Egress Points .....	12-100
12.8.9	OCS and Traction Power Facility Gantry Pole Supports .....	12-100
12.8.10	Maintenance of Primary Type 1 Bridges, Aerial Structures, and Grade Separations....	12-101
12.9	Design Considerations for Primary Type 2 and Secondary Structures .....	12-102
12.9.1	Primary Type 2 Structures .....	12-102
12.9.2	Design Considerations for Secondary Structures .....	12-103
12.10	Design Considerations for Earth Retaining Structures .....	12-103
12.10.1	Retaining Walls .....	12-104

12.10.2	MSE Walls .....	12-104
12.10.3	Trenches .....	12-104
12.10.4	Earth Retaining Structure Intrusion Protection .....	12-104
12.10.5	Struts .....	12-105
12.10.6	Trench Drainage .....	12-105
12.10.7	Trench Emergency Access/Egress Points .....	12-105
12.11	Cut-and-Cover Structures .....	12-105
12.11.1	Structural System .....	12-105
12.11.2	Loads and Forces .....	12-106
12.11.3	Waterproofing of Underground Station Structures .....	12-112
12.11.4	Water Holding and Conveyance Structures .....	12-114
12.11.5	Shoring Support Systems .....	12-114
12.11.6	Structural Fire Resistance .....	12-115
12.12	Support and Underpinning of Structures .....	12-115
12.12.1	Depth of Support Structures .....	12-115
12.12.2	Methods .....	12-116
12.13	Areas of Potential Explosion .....	12-117
12.14	Structure Interface Issues .....	12-117
12.14.1	Cable Trough .....	12-117
12.14.2	Grounding and Bonding .....	12-117
12.14.3	Drainage .....	12-117
12.14.4	Conduit Risers .....	12-118
12.14.5	Embedded Conduits .....	12-118
12.14.6	Trackside Equipment .....	12-118
12.14.7	Emergency Access/Egress Stairs .....	12-118
12.14.8	Concrete Anchors .....	12-118
12.14.9	Utilities .....	12-118
12.14.10	Field-drilled Anchors for OCS Pole and OCS Support Structures .....	12-119

## Tables

Table 12-1:	Unit Weight of Common Materials .....	12-5
Table 12-2:	Loads and Load Combinations for Design of Overhead Contact System Pole Foundations .....	12-28
Table 12-3:	Loads and Load Combinations for Design of Traction Power Facility Gantry Pole Supports on Aerial Structures .....	12-29
Table 12-4:	Load Combinations for Design of Structures .....	12-34
Table 12-5:	Loading Definitions Used in Table 12-4 .....	12-35
Table 12-6:	Load Factors for Permanent Loads, $\gamma_P$ .....	12-36
Table 12-7:	Load Factors for Permanent Loads due to Superimposed Deformations, $\gamma_P$ .....	12-36
Table 12-8:	Track-Structure Interaction Analysis Goals .....	12-38
Table 12-9:	Vertical Deflection Limits: Group 1a .....	12-46
Table 12-10:	Vertical Deflection Limits: Group 1b .....	12-46
Table 12-11:	Transverse Deflection Limits .....	12-47
Table 12-12:	Rotation about Transverse Axis and Relative Displacement at the Level of the Rail Limits .....	12-49

Table 12-13:	Rotation about Vertical Axis and Relative Displacement at Outermost Rail Limits	12-52
Table 12-14:	Relative Vertical Displacement at Expansion Joints Limits – Track Serviceability	12-53
Table 12-15:	Relative Transverse Displacement at Expansion Joints Limits – Track Serviceability.....	12-54
Table 12-16:	Deck Twist Limits .....	12-54
Table 12-17:	Relative Longitudinal Displacement at Expansion Joints Limits .....	12-59
Table 12-18:	Relative Vertical Displacement at Expansion Joints Limits.....	12-60
Table 12-19:	Relative Transverse Displacement at Expansion Joints Limits.....	12-61
Table 12-20:	Minimum Factor of Safety for Uplift on Direct Fixation Fasteners.....	12-62
Table 12-21:	Axial Rail Stress Limits .....	12-62
Table 12-22:	Dynamic Track Safety Limits .....	12-68
Table 12-23:	Damping Values for Dynamic Model .....	12-71
Table 12-24:	Rail Section Properties for RSI Analysis.....	12-72
Table 12-25:	Minimum Recommended Track Extension and Boundary Spring Properties... 12-76	
Table 12-26:	Joint Location Limitations at Low-Speed Turnouts .....	12-96
Table 12-27:	Joint Location Limitations at High-Speed Turnouts .....	12-97

## Figures

Figure 12-1:	Cooper E-50 Loading (LLRR) .....	12-8
Figure 12-2:	Equivalent Loads $q_{1k}$ for Simple Vertical Surfaces Parallel to the Track for Speeds Less than 125 mph .....	12-16
Figure 12-3:	Equivalent Loads $q_{1k}$ for Simple Vertical Surfaces Parallel to Track for Speeds Greater than 125 mph.....	12-17
Figure 12-4:	Equivalent Loads $q_{2k}$ for Simple Horizontal Surfaces Above Track for Speeds Less than 125 mph .....	12-18
Figure 12-5:	Equivalent Loads $q_{2k}$ for Simple Horizontal Surfaces Above Track for Speeds Greater than 125 mph.....	12-19
Figure 12-6:	Equivalent Loads $q_{3k}$ for Simple Horizontal Surfaces Adjacent to Track .....	12-20
Figure 12-7:	Definition of the Distances $\min a_g$ and $\max a_g$ from Centerline of Track .....	12-21
Figure 12-8:	Derailment Case I .....	12-24
Figure 12-9:	Derailment Case II .....	12-25
Figure 12-10:	LLRM Loading .....	12-40
Figure 12-11:	Recommended Lower Bound Threshold of Vertical Frequency .....	12-43
Figure 12-12:	Transverse Span Deformation Limits .....	12-47
Figure 12-13:	Rotation about Transverse Axis at Deck Ends .....	12-49
Figure 12-14:	Rotation about Vertical Axis at Deck Ends – Global View .....	12-51
Figure 12-15:	Rotation about Vertical Axis at Deck Ends – Local View .....	12-51
Figure 12-16:	Relative Vertical Displacement at Expansion Joints – Track Serviceability..	12-52
Figure 12-17:	Relative Transverse Displacement at Expansion Joints – Track Serviceability	12-53

Figure 12-18:	Deck Twist Diagram.....	12-54
Figure 12-19:	$\delta_{LF}$ Definition .....	12-57
Figure 12-20:	$\delta_{LLRM+I}$ Definition.....	12-57
Figure 12-21:	$\delta_{OBE}$ Definition .....	12-58
Figure 12-22:	Structural Thermal Unit ( $L_{TU}$ ) Definition .....	12-59
Figure 12-23:	Relative Vertical Displacement at Expansion Joints .....	12-60
Figure 12-24:	Relative Transverse Displacement at Expansion Joints .....	12-61
Figure 12-25:	Type 1 .....	12-63
Figure 12-26:	Type 2 .....	12-63
Figure 12-27:	Type 3 .....	12-64
Figure 12-28:	Type 4 .....	12-64
Figure 12-29:	Type 5 .....	12-64
Figure 12-30:	Characteristic Wheel Spacing, $d$ .....	12-65
Figure 12-31:	Rail-Structure Interaction Model.....	12-71
Figure 12-32:	Non-Ballasted Track with Direct Fixation Fasteners: Bi-linear Coupling Springs	12-73
Figure 12-33:	Ballasted Track with Concrete Ties and Elastic Fasteners: Bi-linear Coupling Springs	12-73
Figure 12-34:	Collision Loads for Each Group of Columns .....	12-81
Figure 12-35:	Protection Device.....	12-81
Figure 12-36:	Joint Location Limitations at Low-Speed Turnouts .....	12-95
Figure 12-37:	Joint Location Limitations at High-Speed Turnouts .....	12-96



## Acronyms

AASHTO	American Association of State Highway and Transportation Officials
ASCE	American Society of Civil Engineers
Authority	California High-Speed Rail Authority
Caltrans	California Department of Transportation
CBC	California Building Code
CSDC	Caltrans Seismic Design Criteria
CWR	Continuous Welded Rail
HST	High-Speed Train
LRFD	Load and Resistance Factor Design
MCE	Maximum Considered Earthquake
NSFC	Non-Standard Fastener Configuration
NUFC	Non-Uniform Fastener Configuration
OBE	Operating Basis Earthquake
OCS	Overhead Contact System
REJ	Rail Expansion Joints
RLD	Relative Longitudinal Displacement
RSI	Rail-Structure Interaction
RSIDAP	Rail-Structure Interaction Design and Analysis Plan
RVD	Relative Vertical Displacement
SDAP	Seismic Design and Analysis Plan
SEJ	Structural Expansion Joints
TCL	Track Centerline
TOR	Top of Rail
TSI	Track-Structure Interaction
TSIDAP	Track-Structure Interaction Design and Analysis Plan
VTSI	Vehicle-Track-Structure Interaction
VTSIDAP	Vehicle-Track-Structure Interaction Design and Analysis Plan

1 Note: Additional Acronyms are found in Section 12.2 and Table 12-5 of this chapter.



## 12 Structures

### 12.1 Scope

1 This chapter provides design criteria for Primary Type 1, Primary Type 2 and Secondary  
 2 structures supporting California High-Speed Train (HST) service including but not limited to  
 3 bridges, aerial structures, grade separations, earth retaining structures, cut-and-cover  
 4 underground structures, station structures, surface facilities and buildings.

### 12.2 Regulations, Codes, Standards, and Guidelines

5 Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.  
 6 Design shall meet applicable portions of the general laws and regulations of the State of  
 7 California and of respective local authorities.

8 The provisions within this chapter shall govern structural design. The following current  
 9 documents are either referenced by this chapter, or shall be considered as guidelines when  
 10 sufficient criteria are not provided by this chapter.

11 American Concrete Institute (ACI)

12     - ACI 318: Building Code Requirements for Structural Concrete

13     - ACI 350: Code Requirements for Environmental Engineering Concrete Structures and  
 14         Commentary

15     • American Welding Society (AWS)

16         - AWS D1.1/D1.1M: Structural Welding Code-Steel

17         - AWS D1.8/D1.8M: Structural Welding Code-Seismic Supplement

18     • American Association of State Highway and Transportation Officials (AASHTO)

19         - AASHTO/AWS D1.5M/D1.5: Bridge Welding Code

20         - AASHTO Guide Specifications for LRFD Seismic Bridge Design

21         - AASHTO Guide Specifications for Seismic Isolation Design

22         - AASHTO LRFD Bridge Construction Specifications

23         - AASHTO LRFD Guide Specifications for the Design of Pedestrian Bridges

24     • California Building Code (CBC)

- American Railway Engineering and Maintenance-of-Way Association (AREMA)
    - Manual for Railway Engineering
  - American Society of Civil Engineers (ASCE)
    - ASCE 7: Minimum Design Loads for Buildings and Other Structures
    - ASCE 41: Seismic Rehabilitation of Existing Buildings
  - American Institute of Steel Construction (AISC)
    - Steel Construction Manual
    - Seismic Design Manual
  - California Occupational Safety and Health Administration (Cal/OSHA) Department of Industrial Relations
  - California Department of Transportation (Caltrans) Bridge Design Manuals (CBDM)
    - Caltrans Bridge Design Specification – AASHTO LRFD Bridge Design Specifications and California Amendments (to the AASHTO LRFD Bridge Design Specifications), hereafter referred to as “AASHTO LRFD BDS with California Amendments”
    - Caltrans Bridge Memo to Designers Manual (CMTD)
    - Caltrans Bridge Design Practices Manual (CBDP)
    - Caltrans Bridge Design Aids Manual (CBDA)
    - Caltrans Bridge Design Details Manual (CBDD)
    - Caltrans Seismic Design Criteria (CSDC)
    - Office of Special Funded Projects (OSFP) Information and Procedures Guide
  - Code of Federal Regulations (CFR)
  - United States Department of Transportation Federal Highway Administration; Technical Manual for Design and Construction of Road Tunnels – Civil Elements; Publication No. FHWA-NHI-09-010
- Other international standards are used in the development of these criteria, including the following:
- European Standard EN 1991-2:2003 Actions on Structures – Part 2: Traffic Loads on Bridges
  - European Standard EN 1990:2002 +A1 Basis of Structural Design annex A2: Application to Bridges
  - International Federation for Structural Concrete (FIB) Model Code for Concrete Structures, 1990 (For Time Dependent Behavior of Concrete)

## 12.3 Types of Structures

Elements of HST infrastructure, based on their importance to HST, shall be generally classified as Primary Type 1, Primary Type 2, or Secondary. For the definitions of these general classifications, refer to the *Seismic* chapter.

Some examples of structures supporting HST service are:

- Bridges – HST trackway structures crossing rivers, lakes, or other bodies of water
- Aerial Structures – elevated HST trackway structures including bridges, viaducts and HST grade separations
- Grade Separations – structures separating trackways from railroad, highway, or pedestrian usage
- Earth Retaining Structures – including U-walls, trenches, and retaining walls
- Cut-and-Cover Underground structures – including cut-and-cover line structures
- Bored Tunnels
- Mined Tunnels
- Stations, Surface Facilities, Buildings and Ancillary Structures – including station buildings, station parking structures, sound walls, fences, gates and miscellaneous structures
- Underground Ventilation Structures
- Underground Passenger Stations
- Equipment and Equipment Supports

## 12.4 Structural Design Requirements

Structures shall be designed for specified limit states to achieve the objectives of constructability, safety, and serviceability, with due consideration to inspectability and maintainability, as specified in AASHTO LRFD BDS with California Amendments unless otherwise modified in this chapter.

### 12.4.1 Structural Design Parameters

- Structures shall be designed for the appropriate loadings and shall comply with the HST structure gauge per the *Trackway Clearances* chapter.
- The design life for structures shall be as defined in the *General* chapter. For elements such as expansion joints and bearings that will need to be replaced during the life of the structures, specific replacement procedures shall be developed that will show how the element can be replaced within the non-operation hours of the HST service.

- Requirements for noise and vibration suppression are defined in the environmental documents including materials and specific locations and measurements.
- Permanent and temporary structures including falsework shall be designed in accordance with clearance requirements defined in the *Trackway Clearances* chapter. Falsework clearance requirements are only applicable when falsework is erected over an operational road or railway.
- Design of structures shall consider loads and effects due to erection equipment, construction methods, and sequence of construction.
- Design and construction of HST facilities shall comply with the approved and permitted environmental documents.
- Only non-flammable materials are allowed for permanent structural elements supporting HST operations. Timber is allowed in construction of temporary falsework.

#### 12.4.2 Seismic Design

For seismic design criteria for Primary Type 1, Primary Type 2, and Secondary structures, refer to the *Seismic* chapter.

### 12.5 Permanent and Transient Loads and Load Combinations for Primary Structures

This section specifies the permanent and transient loads, load factors and load combinations for Primary Type 1 structures including bridges, aerial structures, grade separations and earth retaining structures. Where applicable, this section shall apply for Primary Type 2 structures.

Facility loads for stations, surface facilities, buildings and ancillary structures are specified in Section 12.7.

Loads and forces for cut-and-cover structures are specified in Section 12.11.

For structures carrying highway loads, AASHTO LRFD BDS with California Amendments shall apply with supplementary provisions herein.

#### 12.5.1 Permanent Loads

##### 12.5.1.1 Dead Load (DC, DW)

The dead load shall include the weight of structure components, appurtenances, utilities attached to the structure, earth cover, finishes, and permanent installations such as tracks, ballast, conduits, piping, safety walkways, walls, sound walls, electrification and utility services.

- In the absence of more precise information, the unit weights specified in Table 12-1 shall be used for dead loads.
- DC refers to the dead load of structural components and permanent attachments supported by the structure including tracks, cable troughs, parapet walls, sound walls, overhead contact system (OCS), etc.
- DW refers to the dead load of non-structural attachments that are permanent or non-permanent attachments including utilities, ballast, plinths, cables, finishes, etc.
- If applicable, dead load shall be applied in stages to represent the sequence required to construct the structure. Analysis shall consider the effect of the maximum and minimum loading imposed on the structure during construction or resulting from placement or removal of earth cover.

**Table 12-1: Unit Weight of Common Materials**

Item	Unit Weight	Reference
Electrification (OCS and fastenings)	100 pounds per foot of track	CHSTP (Refer to note 2)
OCS poles and support	Refer to Section 12.5.3.1	CHSTP
Cable trough including walkway surface without OCS pole	1400 pounds per foot each	CHSTP
Ballast	140 pcf	AASHTO LRFD BDS with California Amendments
Ballasted track not including rail and fastener systems	4200 pounds per foot per track, including ties, (add 1000 pounds per foot per track in superelevated zones)	CHSTP
Parapet wall	800 pounds per foot each side	CHSTP
Rails and fasteners (no ties) including special trackwork	200 pounds per foot of track	AREMA
Non-ballasted track and non-ballasted track base not including rail and fastener systems	2500 pounds per foot per track, (add 1000 pounds per foot per track in superelevated zones)	CHSTP
Soils	See Geotechnical reports described in the <i>Geotechnical</i> chapter	—
Sound wall (clear, 1 inch thick)	125 pounds per foot for 14-foot height from TOR	CHSTP
Systems cables in trough	200 pounds per foot of track	CHSTP

**Notes:**

- For materials not listed, refer to AASHTO LRFD BDS with California Amendments or CBC as applicable.
- CHSTP refers to the weights of internal systems requirements necessary for HST operations.

### 12.5.1.2 Downdrag Force (DD)

Possible development of downdrag on piles or shafts shall be considered. Recommended negative skin friction values shall be as provided for the particular site in the geotechnical reports described in the *Geotechnical* chapter, or as a minimum refer to AASHTO LRFD BDS with California Amendments Article 3.11.8.

### 12.5.1.3 Earth Pressure (EV, EH)

Substructure elements shall be proportioned to withstand earth pressure. Recommended soil parameters, vertical and lateral earth pressure loads, and surcharge loads shall be as provided for the particular site in the geotechnical reports described in the *Geotechnical* chapter.

#### A. Vertical Earth Pressure (EV)

Depth of cover shall be measured from the ground surface or roadway crown, or from the street grade, whichever is higher, to the top of the underground structure. Saturated densities of soils shall be used to determine the vertical earth pressure. For recommended values, refer to the *Geotechnical* chapter.

#### B. Lateral Static Earth Pressure (EH)

For lateral static earth pressures, refer to the *Geotechnical* chapter.

### 12.5.1.4 Earth Surcharge (ES)

Surcharge loads (ES) are vertical or lateral loads resulting from loads applied at or below the adjacent ground surface. For procedures for determining surcharge loads, refer to the *Geotechnical* chapter.

### 12.5.1.5 Earth Settlement Effects (SE)

Earth settlement effects (SE) are forces or displacements imposed on a structure due to either uniform or differential settlement under sustained loading. For settlement calculation, refer to the geotechnical reports described in the *Geotechnical* chapter.

Structures shall be designed to accommodate earth settlement effects. Uniform and differential foundation settlements shall be subject to the allowable limits as given in the *Geotechnical* chapter. Refer to Section 12.8.6.18 for additional requirements.

At and near water crossings, scour potential shall also be considered for earth settlement effects.

### 12.5.1.6 Creep Effects (CR)

For Primary Type 1 structures, the CEB-FIP Model Code shall be used for determining effects due to creep of concrete (CR). For Primary Type 2 and secondary structures, the requirements in AASHTO LRFD BDS with California Amendments Article 5 shall be used for determining effects due to creep of concrete (CR).

Rail-structure interaction forces due to the constraint of structural movement to creep effects shall be considered as specified in Section 12.5.3.4.



#### 12.5.1.7 Shrinkage Effects (SH)

For Primary Type 1 structures, the CEB-FIP Model Code shall be used for determining effects due to shrinkage of concrete (SH). For Primary Type 2 and secondary structures, the requirements in AASHTO LRFD BDS with California Amendments Article 5 shall be used for determining effects due to shrinkage of concrete (SH).

Rail-structure interaction forces due to the constraint of structural movement to shrinkage effects shall be considered as specified in Section 12.5.3.4.

#### 12.5.1.8 Secondary Forces from Prestressing (PS)

Secondary forces from prestressing (PS) effects shall be accounted for in design. Such secondary forces arise during prestress of statically indeterminate structures, which produce additional internal forces and support reactions.

Rail-structure interaction forces due to the constraint of structural movement due to secondary forces from prestressing shall be considered as specified in Section 12.5.3.4.

#### 12.5.1.9 Locked-in Construction Forces (EL)

Locked-in construction force effects (EL) resulting from the construction process shall be considered. Such effects include, but are not limited to, jacking apart adjacent cantilevers during segmental construction.

#### 12.5.1.10 Water Loads (WA)

The effects of ground or surface water hydrostatic force, including static pressure of water, buoyancy, stream pressure, and wave loads (WA) shall be considered using the requirements in AASHTO LRFD BDS with California Amendments Article 3.7. Recommended values given in the geotechnical reports described in the *Geotechnical* chapter shall be used.

Adequate resistance to flotation shall be provided to resist uplift on structure foundations based upon larger of either the maximum probable height of the water table defined in the geotechnical reports described in the *Geotechnical* chapter, or the maximum flood condition described in the hydrology report. For the completed structure, uplift resistance shall consist of the dead load of the completed structure and applicable permanent loads.

Hydrostatic pressure shall be applied normal to surfaces in contact with groundwater with a magnitude based on the maximum probable height of water table and the applicable water density.

The change in foundation condition due to scour shall be investigated per AASHTO LRFD BDS with California Amendments Article 3.7.5.

## 12.5.2 Transient Loads

### 12.5.2.1 Live Loads (LLP, LLV, LLRR, LLH, LLS)

- 1 Live loads are due to high-speed trains, other trains such as freight, Amtrak, passenger rail, and
- 2 shared-use rail trains, highway loads, construction equipment, and pedestrians.

#### A. Floor, Roof, and Pedestrian Live Loads (LLP)

- 3 For the force effects due to floor and roof live loads (LLP), refer to Section 12.7. Section 12.7
- 4 includes provisions for aerial trackways supporting service walkways.

#### B. High-Speed Train Live Loads (LLV)

- 5 The project specific rolling stock has not yet been determined. Once the project specific rolling
- 6 stock is determined, LLV criteria will be updated. In the interim, the representative trainsets
- 7 shown in Section 12.6.6.1 shall be used as LLV.

#### C. Shared-Use Track Train Live Loads (LLRR)

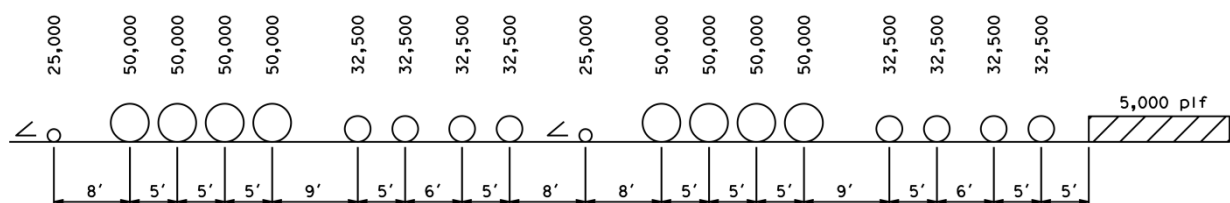
- 8 Structures that will support shared service with another railroad system such as Amtrak,
- 9 Caltrain, Metrolink, UPRR, BNSF, etc., have specific criteria that must be followed in addition to
- 10 the requirements provided herein for high-speed trains.

- 11 Amtrak loading is described in Section 12.5.2.1-E. Additionally, design shall meet the
- 12 requirements described in the *Seismic* chapter and Section 12.6.

#### D. Maintenance and Construction Train Live Loads: Cooper E-50 Loading (LLRR)

- 13 Structures shall be designed to support maintenance and construction trains, vertical loads are
- 14 defined as the Cooper E-50 in the AREMA Specification, refer to Figure 12-1.

#### 15 **Figure 12-1: Cooper E-50 Loading (LLRR)**



- 16
- 17 For the case of multiple tracks on the bridge, LLRR shall be as follows:

- 18 • For 2 tracks, full live load on 2 tracks.
- 19 • For 3 tracks, full live load on 2 tracks and 1/2 on the other track.
- 20 • For 4 tracks, full live load on 2 tracks, 1/2 on one track, and 1/4 on the remaining 1.
- 21 • For more than 4 tracks, to be considered on an individual basis.

- 22 The tracks selected for loading shall be those tracks that will produce the most critical design
- 23 condition on the member under consideration.

E. Amtrak Live Loads

Designated segments of the HST alignment are required to be designed to provide for Amtrak service. These segments shall be designed to support Cooper E-50 loads as described in the AREMA Manual. These structure segments shall also be designed to meet the requirements for structures supporting HSTs, including Section 12.6, and requirements described in the *Seismic* chapter.

These structures shall be designed to resist 2 axles weighing 75 kips each with a longitudinal spacing of 9.0 feet. This additional loading is required to account for local effects of Amtrak locomotives.

F. Highway Live Loads (LLH)

Facilities required to support highway loads over HST shall be designed to the requirements of AASHTO LRFD BDS with California Amendments Article 3.6.1. For facilities intended to support highway permit loads, Caltrans guidelines shall be followed for the routing and sizes of the permit vehicles.

G. Live Load Surcharge (LLS)

Live load surcharge (LLS) shall be applied at the ground surface both over and adjacent to underground structures, as applicable, to account for presence of surface live load. Live load surcharge shall consider the presence of LLRR, LLV, LLH, possible future roadways, sidewalk live loads, and construction live loads.

Methods for lateral distribution of live load surcharge due to rail loading shall be in accordance with AREMA. Lateral distribution of highway surcharge shall be in accordance with AASHTO LRFD BDS with California Amendments Article 3.11.6.4.

No impact factors apply to LLS for walls.

Recommended coefficients for lateral surcharge loading shall be as recommended in the geotechnical reports described in the *Geotechnical* chapter.

H. Live Loading for Fatigue Assessment

For structures carrying high-speed trains, the project specific rolling stock (LLV) plus dynamic impact (I) shall be used for fatigue assessment of structures.

Refer to Section 12.5.2.1B for LLV loading, and Section 12.6.6.3 for determination of dynamic impact (I). The methods of AASHTO LRFD BDS with California Amendments Article 3.6.1.4 shall be used to evaluate fatigue loads.

The fatigue assessment shall be performed for structural elements that are subjected to fluctuations of stress. For structures supporting multiple tracks the loading shall be applied to a minimum of 2 tracks in the most unfavorable positions. The fatigue damage shall be assessed over the required structural life of the structure. For fatigue assessment of structures, use 2.8 million axle loads per track per year.

### 12.5.2.2 Vertical Impact Effect (I)

Moving trains and vehicles impart dynamic loads to bridges, which are considered through a dynamic impact factor. The static effects of the design train and vehicle loads, other than centrifugal, traction, braking, nosing and hunting shall be increased by the percentages specified herein.

Vertical impact effect (I) applies to the following:

- Superstructure, including steel or concrete supporting columns, steel towers, legs of rigid frames, and generally those portions of the structure that extend down to the main foundation
- The portion above the ground line of concrete or steel piles that support the superstructure directly
- Buried components where the depth of fill is less than 8 feet

Vertical impact effect (I) does not apply to the following:

- Retaining walls, wall-type piers, and piles except those described above
- Floor, roof, and pedestrian live loads (LLP)

#### Vertical Impact Effect (I) for LLV

Dynamic analysis is required for structures carrying HSTs (LLV) in order to determine impact effects. Refer to Section 12.5.2.1B for LLV loading, and Section 12.6.6.3 for determination of dynamic impact (I).

For determining impact factors (I) associated with maintenance and construction train loading (LLRR) on ballasted track, AREMA Specifications shall be used as follows:

Ballasted track:

- Reinforced or prestressed concrete bridges:

$$I = 60\% \quad \text{where } L \leq 14 \text{ feet}$$

$$I = \frac{225}{\sqrt{L}} \quad \text{where } 14 \text{ feet} < L \leq 127 \text{ feet}$$

$$I = 20\% \quad \text{where } L > 127 \text{ feet}$$

- Steel bridges:

$$I = 40 - \frac{3L^2}{1600} \quad \text{where } L < 80 \text{ feet}$$

$$I = 16 + \frac{600}{L - 30} \quad \text{where } L \geq 80 \text{ feet}$$

L = span length

For determining impact factors (I) associated with maintenance and construction train loading (LLRR) for direct fixation on concrete non-ballasted track with spans less than and equal to 40 feet, European Standard EN 1991-2 shall be used as modified below. For spans longer than 40 feet, AREMA ballasted track impact factors shall be used.

Direct fixation on concrete non-ballasted track:

$$I = 100 \left( \frac{2.16}{\sqrt{0.305L} - 0.2} - 0.27 \right) \leq 100\% \quad \text{where } L \leq 40 \text{ feet}$$

L = Span length for member under consideration (main girder, bridge deck, etc.)

The calculated value shall be applied at top of rail (TOR) as a percentage of live load.

An additional  $\pm 20$  percent imbalance of live load shall be applied to each rail as a vertical force to model the couple caused by potential rocking of the train. The couple shall be applied on each track in the direction that will produce the most unfavorable effect in the member under consideration.

#### Vertical Impact Effect (I) for LLH

For determining impact factors (I) associated with highway loading (LLH), dynamic load allowance, IM as defined in AASHTO LRFD BDS with California Amendments shall be used.

#### Vertical Impact Effect on Buried Components

A reduction of impact for buried components shall be applicable as specified in AASHTO LRFD BDS with California Amendments Article 3.6.2, with the 33 percent base impact value modified as applicable to LLRR or LLV, as given herein.

#### **12.5.2.3 Centrifugal Force (CF)**

For tracks on a curve, centrifugal force (CF) shall be considered as a horizontal load applied toward the outside of the curve. Multiple presence factors specified in Section 12.5.2.1D shall apply to centrifugal forces. Refer to the *Track Geometry* chapter for the range of radius values.

For centrifugal forces from carrying vehicular traffic, refer to AASHTO LRFD BDS with California Amendments.

The centrifugal force (CF) is a function of the train live load (LLRR or LLV), speed, and horizontal radius of curvature:

$$CF = (LLRR \text{ or } LLV) \times [0.0668 \cdot V^2 \cdot f / R]$$

CF acts at 6 feet above TOR

Where:

V = train speed (mph)

R = horizontal radius of curvature (feet)

f = reduction factor, not to be taken less than 0.35:

f = 1, for LLRR, for  $V \leq 75$  mph

$f = 1 - [(V - 75)/621.4] \times [506/V + 1.75] \times [1 - (9.45/L)^{1/2}] \geq 0.35$ , for LLRR,  $V > 75$  mph

f = 1, for LLV, all speeds

L = length in feet of the loaded portion of curved track on the bridge, for the specific structural element under consideration.

If the maximum line speed at the site is in excess of 75 mph, the centrifugal force for LLRR shall be determined as the maximum calculated under the following conditions:

- At 75 mph with a reduction factor of  $f = 1.0$
- At the maximum line speed with a reduction factor calculated such that  $f < 1.0$

The effect of superelevation shall be considered when present. The superelevation effect shifts the centroid of the train laterally producing an unequal transverse distribution between rails. Consideration shall be given to both a moving train condition, and an at rest train condition.

#### **12.5.2.4 Traction and Braking Forces (LF)**

##### **A. LLRR**

Traction and braking forces (LF) for passenger trains, freight trains, maintenance and construction trains (LLRR) are from AREMA Section 2.2.3:

Traction force =  $N(25\sqrt{L})$  kips, acting 3 feet above TOR

Braking force =  $N(45 + 1.2L)$  kips, acting 8 feet above TOR

Where:

L = length in feet of portion of bridge under consideration

N = ratio of Cooper train load to Cooper E80 loading for the sizes of trains that will use the structure (i.e., for Cooper E50,  $N = 0.625$ )

The LF loads for LLRR are to be distributed over the length of portion of bridge under consideration up to the maximum length of train. Multiple presence factors specified in Section 12.5.2.1D shall apply.

**B. LLV**

For traction and braking forces (LF) from high-speed trains (LLV) taken from European Standard Eurocode EN 1991-2, Article 6.5.3:

Traction force = 2.26 kips per linear foot or 25 percent of train load (if known), with a maximum value of 225 kips, acting at TOR

Braking force = 1.37 kips per linear foot or 25 percent of train load (if known), with a maximum value of 1350 kips, acting at TOR

Multiple presence factors specified in Section 12.5.2.1D shall apply. Traction and braking forces will be reviewed and confirmed when the project specific rolling stock is selected.

**C. LLH**

For braking forces (LF) from highway loading (LLH), AASHTO LRFD BDS with California Amendments Article 3.6.4 shall be used.

**12.5.2.5 Nosing and Hunting Effects (NE)**

Lateral forces, also called nosing and hunting effects (NE) of the wheels contacting the rails, shall be accounted by a 22 kip horizontal force applied to the top of rail, perpendicular to the track centerline (TCL) at the most unfavorable position.

For load combinations with LLRR and LLV loadings, NE shall be applied simultaneously with centrifugal force (CF).

**12.5.2.6 Wind Loads (WS, WL)**

Wind Load on Structures (WS) and Wind Load on Trains (WL) shall be calculated in accordance with requirements in AASHTO LRFD BDS with California Amendments Article 3.8 with the following modifications:

- The effective wind area shall include the exposed area of all bridge elements, OCS poles, and catenary. For parapets and barriers, shielding of downwind elements from those upwind shall not be considered (i.e., the exposed area shall include the summation of parapets on the bridge).
- The base lateral load for Wind Load on Vehicles (WL) shall be revised to 0.3 klf perpendicular to the train acting 8 feet above the TOR. Refer to AASHTO LRFD BDS with California Amendments Table 3.8.1.3-1: Wind Components on Live Load for skewed angles of incidence shall be revised proportionally to reflect the modified base lateral load.
- For structures that utilize sound walls or wind walls capable of effectively shielding the train from wind loading, consideration may be given to a reduction of WL. The reduction

may be taken as the fractional height of train that is shielded by the wall. This reduction shall not exceed 50 percent of WL.

Local design elements such as parapets or components on structures shall be designed to wind loading and slipstream effects. Wind loading shall be calculated per CBC. The wind importance factor shall equal 1.15.

Wind loading for non-conventional bridge types or long-spans will require special attention (e.g., dynamic effects).

Wind loads (WS) on building and station structures are detailed in Section 12.7 – Structural Design of Surface Facilities and Buildings.

Wind loads (WS, WL) on highway structures shall be per AASHTO LRFD BDS with California Amendments.

#### **12.5.2.7 Slipstream Effects (SS)**

##### **A. Aerodynamic Actions from Passing Trains**

The passing of high-speed trains subjects structures situated near the track to transient pressure waves. This action may be approximated by equivalent loads acting at the front and rear of the train.

Aerodynamic actions from passing trains shall be taken into account when designing structures adjacent to railway tracks.

The passing of rail traffic subjects any structure situated near the track to a traveling wave of alternating pressure and suction (refer to Figures 12-2 to 12-7). The magnitude of the action depends mainly on the following:

- Square of the speed of the train
- Aerodynamic shape of the train
- Shape of the structure
- Position of the structure, particularly the clearance between the vehicle and the structure

The actions may be approximated by equivalent loads at the ends of a train when checking strength and service limit states and fatigue. Equivalent loads are given in Sections 12.5.2.7-B to 12.5.2.7-G.

In Sections 12.5.2.7-B to 12.5.2.7-G, the Maximum Design Speed V [mph] shall be taken as the Maximum Line Speed at the site.

For aerodynamic actions inside of tunnels, refer to the *Tunnels* chapter.



At the start and end of structures adjacent to the tracks, for a length of 16.4 feet from the start and end of the structure measured parallel to the tracks, the equivalent loads in Sections 12.5.2.7-B to 12.5.2.7-G shall be multiplied by a dynamic amplification factor of 2.0.

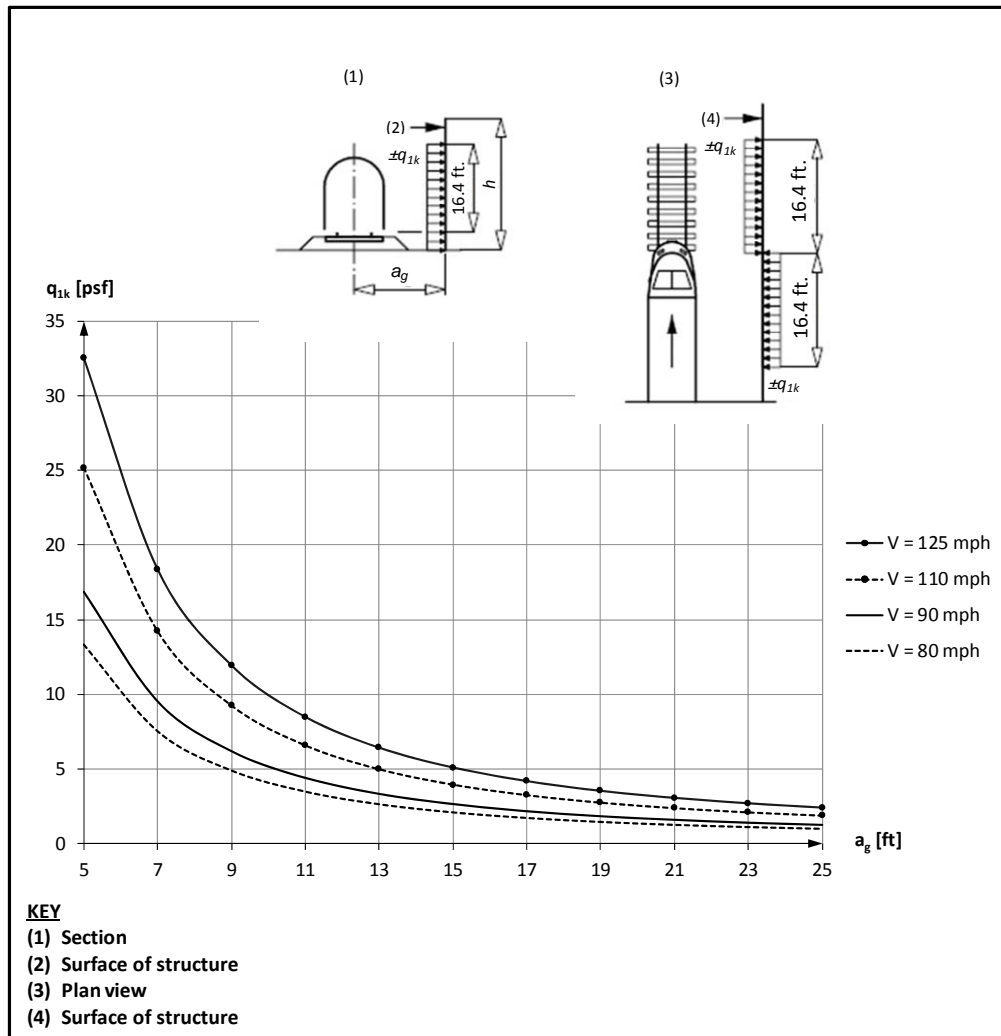
Note that for dynamically sensitive structures, the dynamic amplification factor may be insufficient and may need to be determined by a special study. The study shall take into account dynamic characteristics of the structure including support and end conditions, speed of the adjacent rail traffic and associated aerodynamic actions, and the dynamic response of the structure including the speed of a deflection wave induced in the structure. In addition, for dynamically sensitive structures a dynamic amplification factor may be necessary for parts of the structure between the start and end of the structure.

Simple is defined hereafter as smooth, without projections, ribs, or other obstruction.

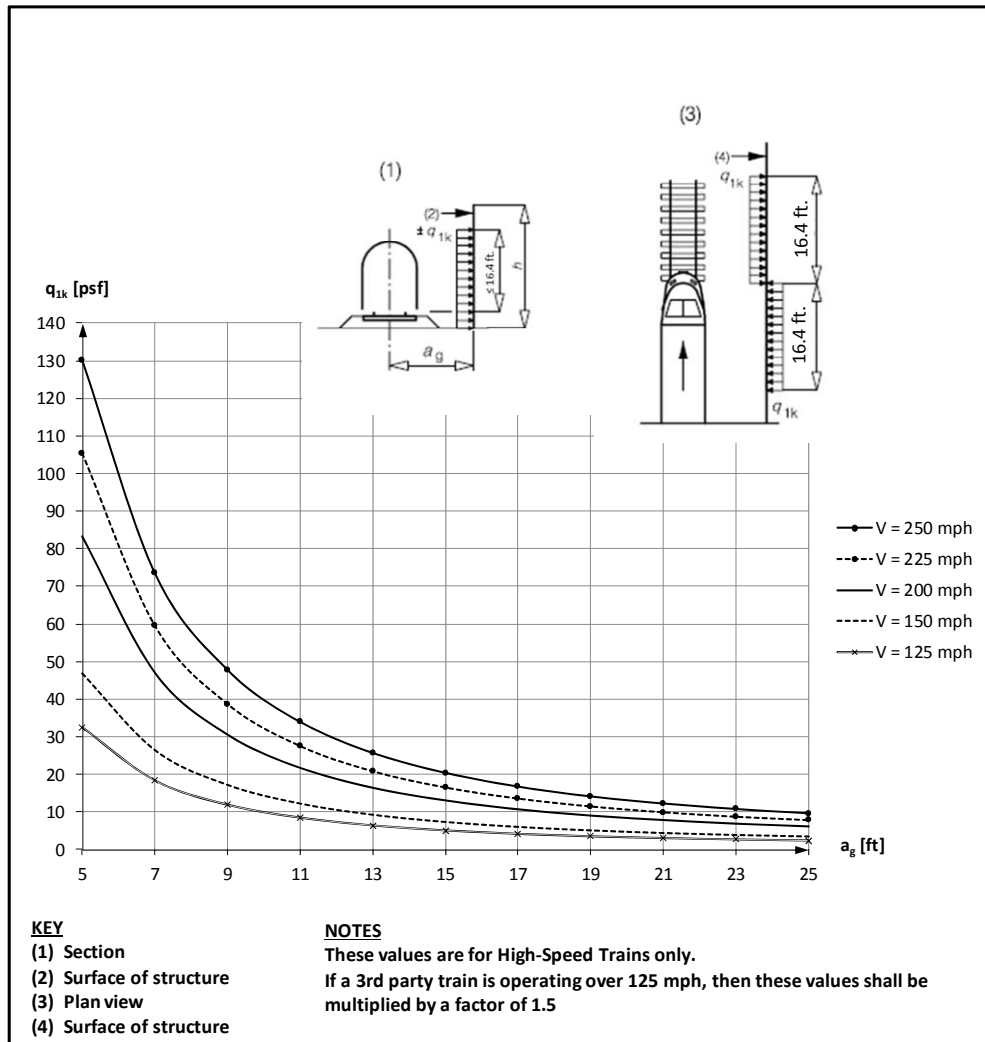
**B. Simple Vertical Surfaces Parallel to the Track**

For simple vertical surfaces parallel to the track, equivalent loads,  $\pm q_{1k}$ , shall apply as given in Figure 12-2 and Figure 12-3.

**Figure 12-2: Equivalent Loads  $q_{1k}$  for Simple Vertical Surfaces Parallel to the Track for Speeds Less than 125 mph**



**Figure 12-3: Equivalent Loads  $q_{1k}$  for Simple Vertical Surfaces Parallel to Track for Speeds Greater than 125 mph**



The equivalent loads apply to trains with an unfavorable aerodynamic shape and may be reduced by the following factors:

- A factor  $k_1 = 0.85$  for trains with smooth sided rolling stock
- A factor  $k_1 = 0.6$  for streamlined rolling stock (e.g., ETR, ICE, TGV, Eurostar or similar)

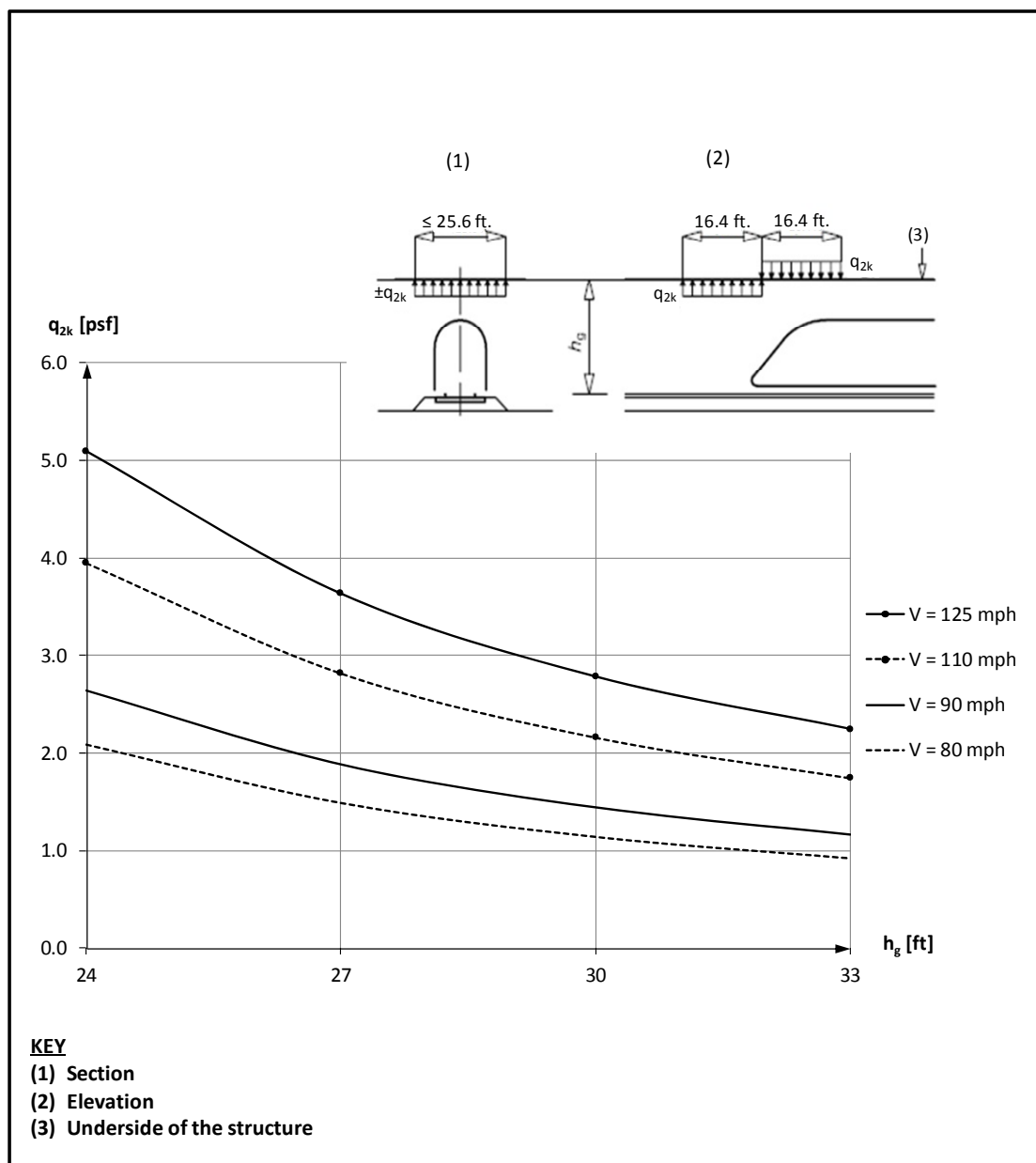
If a small part of a wall with a height  $\leq 3$  feet and a length  $\leq 8$  feet is considered (e.g., an element of a wall), the actions  $q_{1k}$  shall be increased by a factor  $k_2 = 1.3$ .

For surfaces perpendicular to the train, the actions  $q_{1k}$  shall be taken from Figure 12-2 and Figure 12-3 for the distance indicated from TCL modified as described in the previous items.

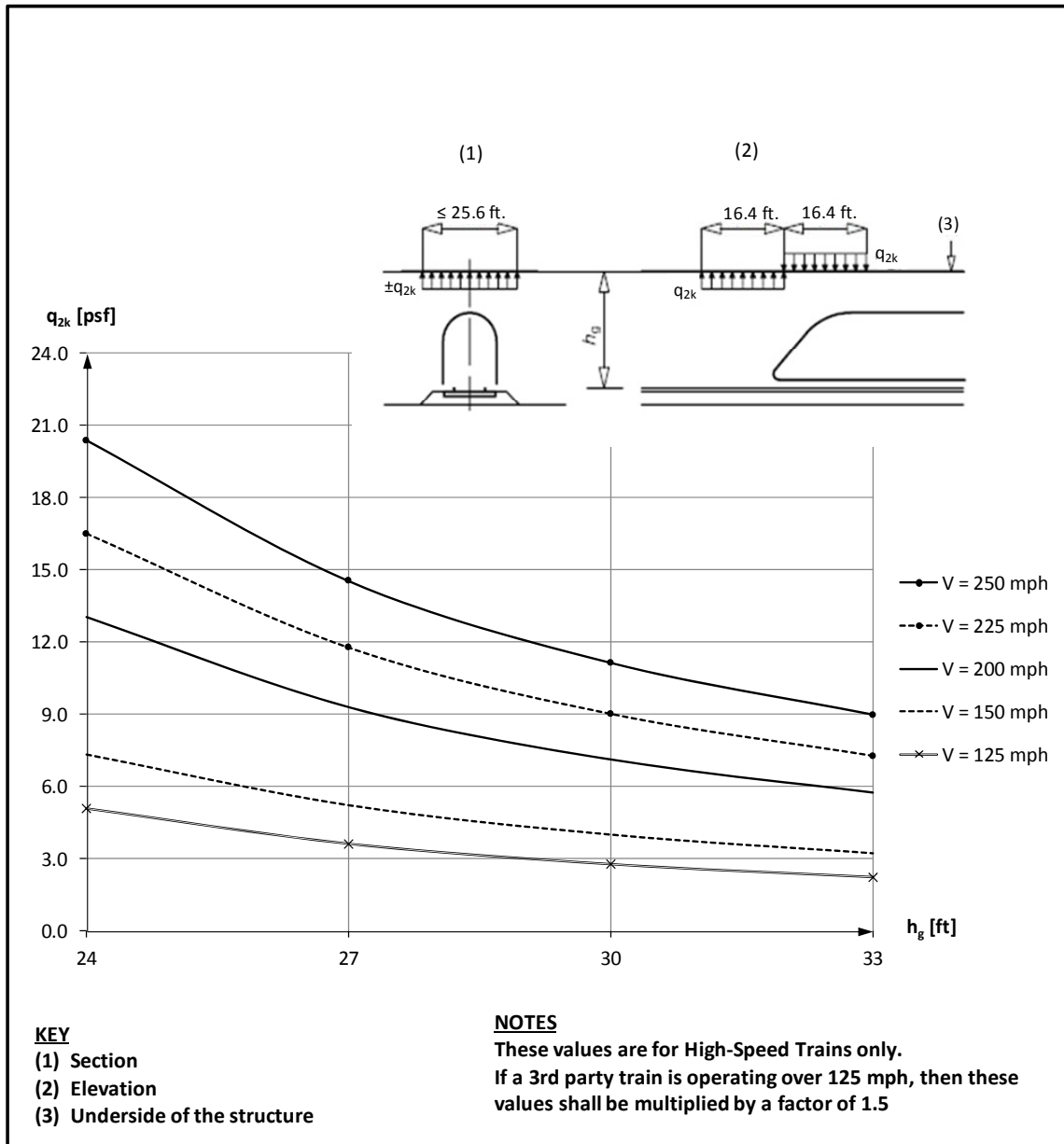
### C. Simple Horizontal Surfaces Above the Track

- 1 For simple horizontal surfaces above the track, such as overhead protective structures,
- 2 equivalent loads,  $\pm q_{2k}$ , shall apply as given in Figure 12-4 and Figure 12-5. The loaded width
- 3 shall extend to 32.8 feet on each side of the TCL.

4 **Figure 12-4: Equivalent Loads  $q_{2k}$  for Simple Horizontal Surfaces Above Track for**  
5 **Speeds Less than 125 mph**



**Figure 12-5: Equivalent Loads  $q_{2k}$  for Simple Horizontal Surfaces Above Track for Speeds Greater than 125 mph**



For trains passing in opposite directions, the actions shall be added. The loading from trains on only 2 tracks shall be considered.

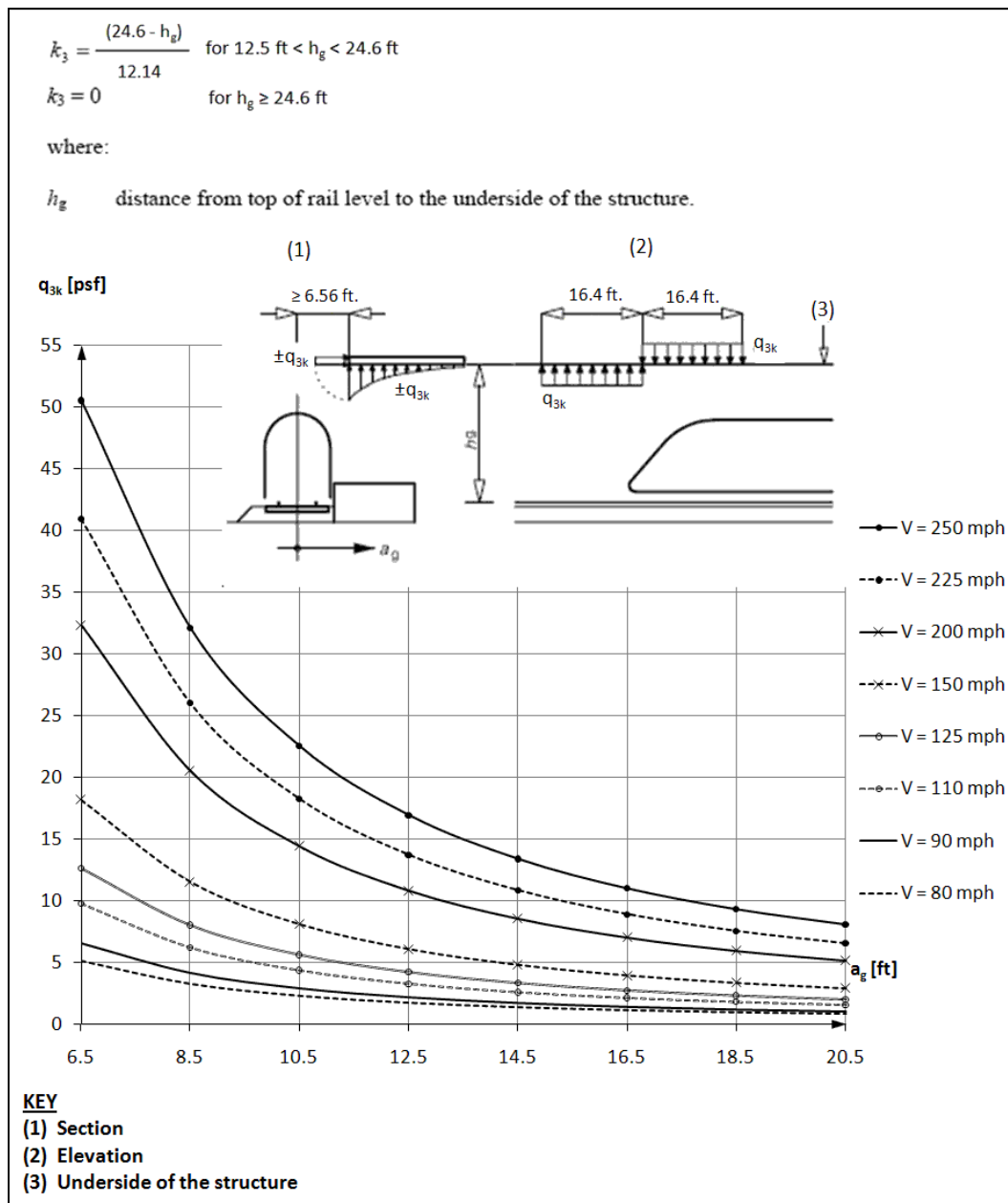
The actions  $q_{2k}$  may be reduced by the factor  $k_1$  as defined in Section 12.5.2.7-B.

The actions acting on the edge strips of a wide structure (greater than 32.8 feet) crossing the track may be multiplied by a factor of 0.75 over a width up to 16.4 feet.

D. Simple Horizontal Surfaces Adjacent to the Track

- 1 For simple horizontal surfaces adjacent to the track, such as platform canopies with no vertical
- 2 walls, equivalent loads,  $\pm q_{3k}$ , shall apply as depicted on Figure 12-6 irrespective of the
- 3 aerodynamic shape of the train.

4 **Figure 12-6: Equivalent Loads  $q_{3k}$  for Simple Horizontal Surfaces Adjacent to Track**



For every position along the structure to be designed,  $q_{3k}$  shall be determined as a function of the distance  $a_g$  from the nearest track. The actions shall be added if there are tracks on either side of the structural member under consideration.

If the distance  $h_g$  exceeds 12.5 feet the action  $q_{3k}$  may be reduced by the factor  $k_3$  as given in Figure 12-6.

**E. Multiple-Surface Structures Alongside the Track with Vertical and Horizontal or Inclined Surfaces**

For multiple-surface structures alongside the track with vertical and horizontal or inclined surfaces, such as noise barriers and platform canopies with vertical walls, equivalent loads,  $\pm q_{4k}$ , shall apply as given in Figure 12-7. The actions shall be taken from the graphs in Figure 12-2 and Figure 12-3 adopting a track distance the lesser of:

$$a'_g = 0.6 * (\min a_g) + 0.4 * (\max a_g) \text{ or } 20 \text{ feet}$$

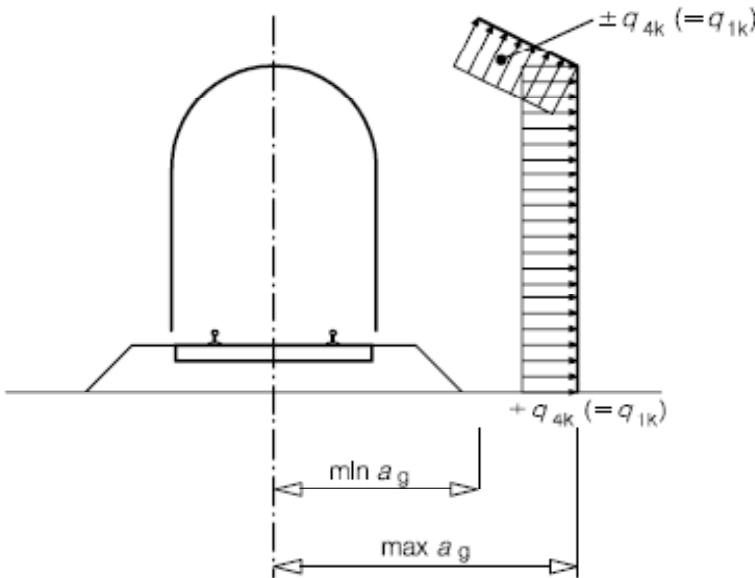
Where:

distances  $\min a_g$  and  $\max a_g$  are shown in Figure 12-7

If  $\max a_g > 20$  feet the value  $\max a_g = 20$  feet shall be used.

The factors  $k_1$  and  $k_2$  defined in Section 12.5.2.7-B shall apply.

**Figure 12-7: Definition of the Distances  $\min a_g$  and  $\max a_g$  from Centerline of Track**



F. Surfaces Enclosing the Structure Gauge of the Tracks over a Limited Length (< 65 feet)

For surfaces enclosing the structure gauge of the tracks over a limited length  $\leq 65$  feet, such as horizontal surfaces above the tracks with at least 1 vertical wall or scaffolding and temporary structures, equivalent loads shall apply irrespective of the aerodynamic shape of the train:

- To the full height of the vertical surfaces:

$$\pm k_4 q_{1k}$$

Where:

$q_{1k}$  is determined according to Section 12.5.2.7-B

$$k_4 = 2$$

- To the horizontal surfaces:

$$\pm k_5 q_{2k}$$

Where:

$q_{2k}$  is determined according to Section 12.5.2.7-C for only 1 track,

$$k_5 = 2.5 \text{ if 1 track is enclosed}$$

$$k_5 = 3.5 \text{ if 2 tracks are enclosed}$$

G. Surfaces Perpendicular to or Above the Tracks over a Limited Length

For surfaces perpendicular to or above the tracks over a limited length, such as wayside equipment and signs oriented normal to the track, equivalent loads shall apply as given in Figure 12-2 and Figure 12-3 for vertical surfaces, and Figure 12-4 and Figure 12-5 for horizontal surfaces.

**12.5.2.8 Thermal Load (TU, TG)**

For uniform (TU) and gradient (TG) temperature effects of the structure, the requirements in AASHTO LRFD BDS with California Amendments Article 3.12 shall be used. Consideration shall be given to the maximum and minimum ambient temperatures.

Rail-structure interaction forces due to the constraint of structural movement to uniform and gradient temperature effects shall be considered as specified in Section 12.5.3.4.

**12.5.2.9 Frictional Force (FR)**

The force due to friction (FR) shall be established on the basis of extreme values of the friction coefficient between sliding surfaces (i.e., at bearing pads). Where appropriate, the effects of moisture, degradation, and contamination of sliding or rotating surfaces upon the friction coefficient shall be considered.



Where applicable, recommended frictional values per AASHTO LRFD BDS with California Amendments shall be used.

#### 12.5.2.10 Seismic Loads

Detailed, project specific seismic design criteria are presented in the *Seismic* chapter. The *Seismic* chapter defines seismic design philosophies, seismic analysis/demand methodologies, and structural capacity evaluation procedures for the 2 levels of design earthquakes.

#### 12.5.2.11 Hydrodynamic Force (WAD)

Hydrodynamic pressure effects acting on submerged portions of structures due to dynamic motion shall be computed using the method of Goyal and Chopra, or by equivalent means.

For possible additional hydrodynamic force effects, refer to the geotechnical reports described in the *Geotechnical* chapter.

#### 12.5.2.12 Dynamic Earth Pressures (ED)

Dynamic earth pressure due to seismic motion acting on retaining structures shall be computed using the methods presented in the *Geotechnical* chapter.

#### 12.5.2.13 Derailment Loads (DR)

##### A. LLRR and LLV

In the event of derailment, damage to bridges, aerial structures, or grade separations shall be minimal. Overturning or collapse of the structure shall not be allowed.

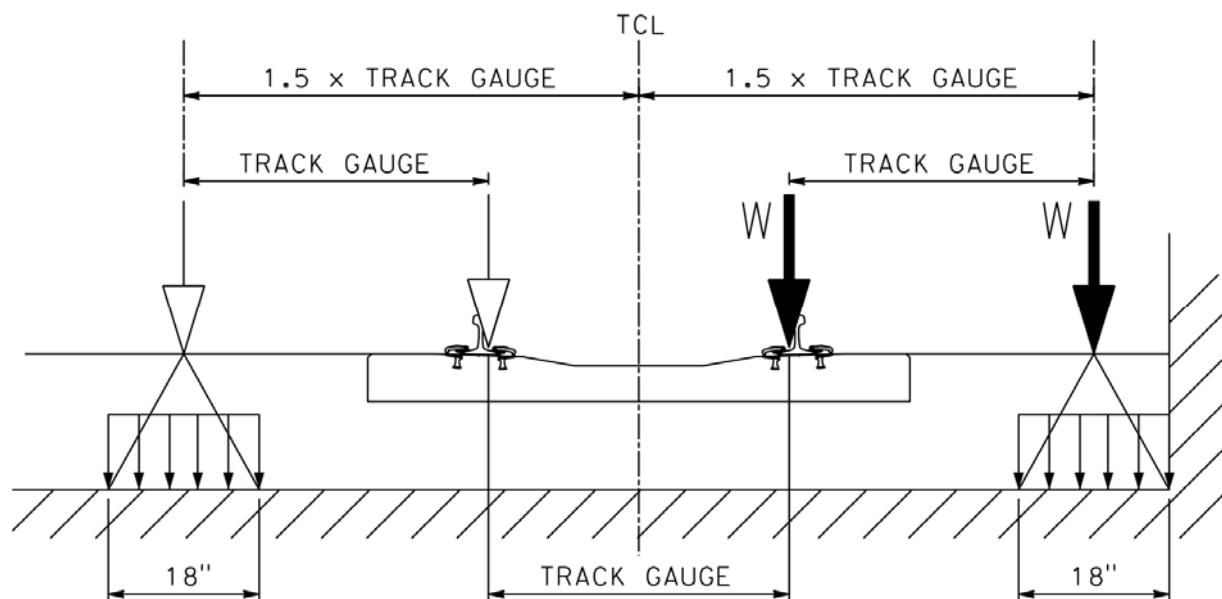
The following design situations shall be considered:

- **Case I** – The derailed vehicles remaining in the track area on the bridge deck with vehicles retained by the adjacent rail or a trackside cable trough wall, refer to Figure 12-8.
- **Case II** – The derailed vehicles balanced on the edge of the bridge and loading the edge of the superstructure (excluding non-structural elements such as walkways), refer to Figure 12-9.
- **Case III** – Derailment of a steel wheel impacting the bridge deck between trackside cable trough wall shall be evaluated using the heaviest axle loads that potentially use the structure with a minimum of the Cooper E-50. In shared use corridors, 2 axles weighing 75 kips each with a longitudinal spacing of 9.0 feet shall be considered. A 100 percent impact factor shall be applied. This force is used to design the concrete deck slab. Refer to item B of this section.
- **Case IV** – Derailment of railway vehicles on a through or semi-through type bridge, aerial structure, or grade separation shall be designed such that the sudden rupture of 1 vertical or diagonal member of the main truss, the sudden rupture of 1 top flange of the main girder or the sudden rupture of 1 hanger of the main arch shall not cause collapse of the structure.

For Case I, collapse of any part of the structure is not permitted. Minor local damage is permitted. The structure shall be designed for the following design loads in the Extreme Loading Combination:

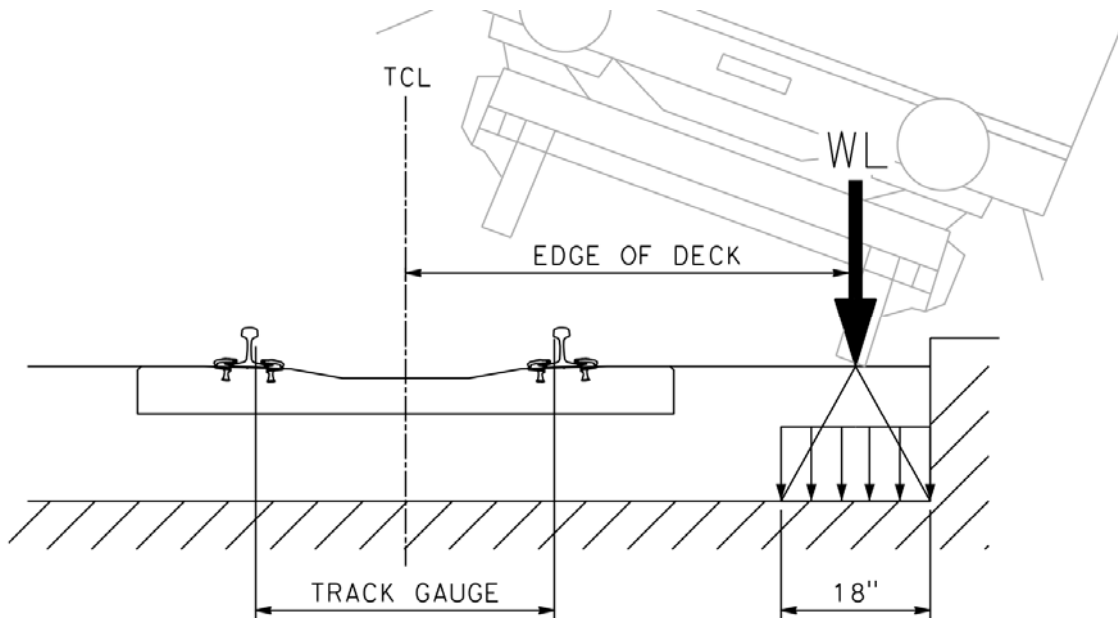
- Cooper E-50 loading, (both point loads  $W$  and uniformly distributed loading  $w$ ) parallel to the track in the most unfavorable position inside an area of width 1.5 times the track gauge on either side of the TCL, or as limited by trackside cable trough walls. If a trackside cable trough wall is used for containment of the train within 1.5 times the track gauge, a coincident horizontal load perpendicular to the track direction shall be used. This horizontal load shall be applied at the top of the trackside cable trough wall.

**Figure 12-8: Derailment Case I**



For Case II, the bridge shall not overturn or collapse. For the determination of overall stability, a maximum total length of 65 feet of Cooper E-50 uniform load shall be taken as a single uniformly distributed vertical line load,  $WL$ , acting on the edge of the structure under consideration. For structures with trackside cable trough wall, this load shall be applied at the wall face.

**Figure 12-9: Derailment Case II**



Cases I and II shall be examined separately. A combination of these loads need not be considered.

For ballasted track, lateral distribution of wheel load may be applied, as shown on Figure 12-8 and Figure 12-9.

For Cases I and II, other rail traffic actions shall be neglected for the track subjected to derailment actions. When the structure under consideration carries more than 1 track, only 1 train shall be considered to have derailed, with other tracks containing a vehicle without impact if producing an unfavorable action. Multiple live load presence factors specified in Section 12.5.2.1D shall apply in this case.

No dynamic factor needs to be applied to the derailment loads for Case 1 and Case II. However, the loads shall be multiplied by the load factor within load combinations. A load factor of 1.0 shall apply to Case III and Case IV.

#### **B. Trackside Cable Trough Wall**

Trackside cable troughs on mainline bridges, aerial structures, grade separations, trenches and tunnel structures shall be designed to resist a transverse horizontal concentrated load of 35 kips applied at top of the wall at any point of contact. The load shall be distributed over a longitudinal length of 1 foot. A load factor of 1.4 shall be applied to the 35 kip load. The height of the wall shall be minimum 8 inches above the level of the adjacent low rail elevation. These walls shall terminate at the back of abutment backwalls or at the termination of the trench or tunnel base slab.

### 12.5.2.14 Collision Loads (CL)

Collision loads in Sections 12.5.2.14-A, 12.5.2.14-B, and 12.5.2.14-C apply to train impact loads (LLRR, LLV). Section 12.5.2.14-D applies to highway collision loads (LLH). Refer to Section 12.7 for collision loads on columns or divider walls of stations or platforms adjacent to the HST tracks.

#### A. Collision Loads on Piers, Columns, Abutments and Retaining Walls

Unprotected piers, columns, abutments and retaining walls shall be designed to resist train collision forces of 900 kips parallel and 350 kips perpendicular to the adjacent TCL. The loads shall be applied to a strip 6 feet in width at a height 6 feet above TOR if unprotected structural members are within 25 feet of the adjacent TCL, or at a height 6 feet above grade if otherwise. Forces shall not be applied simultaneously.

The performance of unprotected Primary Type 2 structural members to this loading shall be a no collapse requirement. The performance of unprotected Primary Type 1 structural members to this loading shall be subject to the following allowable strain and displacement limits:

$$\epsilon_{su}^a \leq \epsilon_{sh} \quad (\text{Equation 12.5.2.14-1})$$

$$\epsilon_{cu}^a \leq 0.003 \quad (\text{Equation 12.5.2.14-2})$$

$$\Delta_u^a \leq \frac{1}{2} \text{ inches} \quad (\text{Equation 12.5.2.14-3})$$

Where:  $\epsilon_{su}^a$  = allowable reinforcing steel tensile strain limit

$\epsilon_{sh}$  = reinforcing steel tensile strain at the onset of strain hardening per CSDC

$\epsilon_{cu}^a$  = allowable concrete compressive strain limit

$\Delta_u^a$  = allowable lateral permanent offset at the top of superstructure of unprotected Primary Type 1 piers, columns and abutments or at the top of unprotected Primary Type 1 retaining walls

Nominal material properties shall be used in calculating the structural demands and capacities.

#### B. Collision Loads on Intrusion Protection and Pier Protection Walls

Refer to the *Rolling Stock and Vehicle Intrusion Protection* chapter for dimensions of intrusion protection and pier protection walls.

Intrusion protection and pier protection walls shall be designed to resist train collision forces of 900 kips parallel and 350 kips perpendicular to the adjacent TCL. The loads shall be applied to a strip 6 feet in width at a height 6 feet above adjacent grade for intrusion protection walls or at a height 6 feet above TOR for pier protection walls. Forces shall not be applied simultaneously. The performance of intrusion protection and pier protection walls to this loading shall be a no collapse requirement. Nominal material properties shall be used in calculating the structural demands and capacities.

C. Structures in Areas beyond Track Ends

Overrunning of rail traffic beyond the end of a track (for example at a terminal station) shall be considered as an accidental design situation when the structure or its supports are located in the area immediately beyond the track ends.

The measures to manage the risk shall be based on the utilization of the area immediately beyond the track end and take into account any measures taken to reduce the likelihood of an overrun of rail traffic.

Members supporting structures shall not be located in the area immediately beyond the track ends.

Where structural supporting members are required to be located near to track ends, an end impact wall shall be provided within 20 feet of the track ends in addition to any buffer stop.

End impact walls shall be designed to resist train collision forces of  $F_{dx} = 1125$  kips for passenger trains and  $F_{dx} = 2250$  kips for freight trains or heavy engines pulling conventional passenger cars. The loads shall be applied horizontally to a strip 6 feet in width at a height 4 feet above TOR. The performance of structural supporting members near the track end to this loading shall be a no collapse requirement. Nominal material properties shall be used in calculating the structural demands and capacities.

D. Highway Vehicle Collision Loads (LLH)

Highway collision load shall be as per AASHTO LRFD BDS with California Amendments Article 3.6.5.

**12.5.3 Miscellaneous Loads**

**12.5.3.1 Loads and Load Combinations for Design of the Surrounding Area of the Embedded Sleeves of Overhead Contact System Pole Foundation**

The embedded sleeves as specified in the Table 12-2 for the OCS pole and down guy anchors shall be installed in the outside compartment of the cable trough on both sides of structural deck at an equal spacing of not more than 30 feet in each span along the aerial structure and the longitudinal offset distance from the centerline of the pier to the centerline of sleeve pattern shall be equal to 1/2 of the equal spacing. The center of embedded sleeves shall not be located within 5 feet to the centerline of bridge expansion joint. For the transverse offset distance from the centerline of track, refer to the Standard and Directive Drawings. The embedded sleeves on each side of the aerial structure shall be directly opposite each other.

At the special tracks such as the crossover and turnout tracks on the aerial structures, the embedded sleeves shall be installed in the outside compartment of the cable trough on both sides of structural deck at a 10 feet equal spacing. This requirement shall apply to a 300 feet distance from the point of switch (PS) on both sides for a total of 600-foot length.

- 1 At the aerial structures adjacent to the station platforms, the embedded sleeves shall be installed
- 2 at the center between the through track and the station track and installed at a 30 feet equal
- 3 longitudinal spacing.
- 4 Provisions shall be provided for all sleeves to prevent water leakage through the sleeves and
- 5 allow for future OCS anchor bolt installation.
- 6 The loads, load combinations, and limit states specified in Table 12-2 shall be investigated for
- 7 design of the surrounding area of the embedded sleeves at every OCS foundation to properly
- 8 transfer the loads to the bridge deck.

**Table 12-2: Loads and Load Combinations for Design of Overhead Contact System Pole Foundations**

Load Combination / Limit State	Location	Load Type	V1 (lbs)	V2 (lbs)	P (lbs)	M1 (lb-ft)	M2 (lb-ft)	Load Factor	
Strength I	OCS pole	Dead	1,500	1,500	-22,000	31,500	31,500	1.25	
		Wind	Refer to note 4						1.40
Strength II	OCS pole	Dead	1,500	1,500	-22,000	31,500	31,500	1.25	
		Slipstream Effects	Refer to note 4						1.75
Strength III	OCS pole	Dead	1,500	1,500	-22,000	31,500	31,500	1.25	
		Wind	Refer to note 4						0.65
		Slipstream Effects	Refer to note 4						1.35
Strength IV	Down guy anchor	Dead	14,000	1,000	14,000	1,000	14,000	1.25	
Extreme I	OCS pole	Accident	14,000	1,500	-8,000	31,500	294,000	1.0	
		Slipstream Effects	Refer to note 4						1.0
Sleeve Pattern and Plate Size									
Anchor Bolt		Bolt Circle		Sleeve Size		Plate Size			
4-2.25" Dia.		24"		2.5"		24" x 24"			

Notes:

1. V<sub>1</sub> denotes shear force parallel to track alignment; V<sub>2</sub> denotes shear force perpendicular to the track alignment.
2. P denotes vertical force, with positive values for tension and negative values for compression.
3. M<sub>1</sub> denotes bending moment about axis parallel to the track alignment; M<sub>2</sub> denotes bending moment about axis perpendicular to the track alignment.
4. Wind load shall be determined according to Section 12.5.2.6; Slipstream effects load shall be determined according to Section 12.5.2.7. Wide flange shape with width of 15 inches, depth of 15 inches, and height of 27 feet shall be used in the calculations of OCS pole foundation loads transferred from OCS pole wind load and slipstream effects load.
5. Loads are assumed at the TOR.

**12.5.3.2 Loads and Load Combinations for Design of Traction Power Facility Gantry Pole Supports on Aerial Structures**

The embedded sleeves for the gantry pole foundations shall be installed in the outside compartment of the cable trough on both sides of structural deck at an equal spacing of not

- more than 10 feet in the designated 2 spans located adjacent to Paralleling Stations, Switch Stations, and Sub Stations. The center of embedded sleeves shall not be located within 5 feet to the centerline of bridge expansion joint. For the transverse offset distance from the centerline of track, refer to the Standard and Directive Drawings. The embedded sleeves on each side of the aerial structure shall be directly opposite each other.
- On single track girders, the gantry pole foundations shall be in line with the OCS poles on the same side of the single track girder.
- Provisions shall be provided for all sleeves to prevent water leakage through the sleeves and allow for future gantry pole anchor bolt installation.
- The loads, load combinations, and limit states specified in Table 12-3 shall be investigated for design of the surrounding areas of the embedded sleeves at every gantry pole foundation to properly transfer the loads to the bridge deck.
- The pattern and size of embedded sleeves and plate shall be determined by the Designer and provided on the drawings.

**Table 12-3: Loads and Load Combinations for Design of Traction Power Facility Gantry Pole Supports on Aerial Structures**

Load Combination / Limit State	Load Type	V1 (lbs)	V2 (lbs)	P (lbs)	M1 (lb-ft)	M2 (lb-ft)	Load Factor
<b>Strength I</b>	Dead	--	5,800	-14,500	230,000	--	1.25
	Wind	Refer to note 4					1.40
<b>Strength II</b>	Dead	--	5,800	-14,500	230,000	--	1.25
	Slipstream Effects	Refer to note 4					1.75
<b>Strength III</b>	Dead	--	5,800	-14,500	230,000	--	1.25
	Wind	Refer to note 4					0.65
	Slipstream Effects	Refer to note 4					1.35
<b>Strength IV</b>	Dead	--	5,800	-14,500	230,000	--	1.25
	Slipstream Effects	Refer to note 4					0.5
	OBE	Refer to note 5					1.0
<b>Extreme I</b>	Dead	--	5,800	-14,500	230,000	--	1.0
	MCE	See note 5					1.0

**Notes:**

- V<sub>1</sub> denotes shear force parallel to track alignment; V<sub>2</sub> denotes shear force perpendicular to the track alignment.
- P denotes vertical force, with positive values for tension and negative values for compression.
- M<sub>1</sub> denotes bending moment about axis parallel to the track alignment; M<sub>2</sub> denotes bending moment about axis perpendicular to the track alignment.
- Wind load shall be determined according to Section 12.5.2.6; Slipstream effects load shall be determined according to Section 12.5.2.7. Wide flange shape W24x117 with height of 40 feet, with 100 percent and 300 percent wind load area increases in along track and transverse directions respectively to account for the cross beams and attachments, shall be used in the calculations of gantry pole foundation loads transferred from gantry wind load and slipstream effects load.

5. Operating Basis Earthquake (OBE) and Maximum Considered Earthquake (MCE) shall be investigated according to the *Seismic* chapter. The pole shall be considered as cantilever, with weight of 11,000 pounds and the center of mass at 27 feet above the TOR.
6. Loads are assumed at the TOR.

### 12.5.3.3 Construction Loads and Temporary Structures

#### A. Temporary Structure Classification

Temporary structures are divided into the following classifications:

- **Type A** – Temporary structures or permanent structures under temporary conditions that carry or will carry HSTs and/or pass over routes carrying HSTs. Subsequent articles herein apply to Type A structures.
- **Type B:** – Temporary structures or permanent structures under temporary conditions that do not carry HSTs and do not pass over routes carrying HSTs. These structures shall be designed in accordance with the requirements of the owning/operating agency (e.g., AASHTO LRFD BDS with California Amendments and CMTD). Structures such as haul bridges used temporarily shall be designed in accordance with CMTD 15-14.

#### B. Construction Load Combinations

Type A temporary structures or permanent structures under temporary conditions shall be designed to adequately resist conditions at all stages of construction, including applicable construction loads. Construction load combinations, in addition to requirements shown in Table 12-4, shall include the following:

- Applicable strength load combinations: Dead load factors shall not be taken less than 1.25, with construction dead loads taken as permanent loads. Construction transient live load factors shall not be taken less than 1.5. Wind load factors may be reduced by 20 percent.
- Service 1, as applicable, refer to AASHTO LRFD BDS with California Amendments Article 3.4.2.
- For seismic requirements of temporary structures, refer to the *Seismic* chapter.

In the absence of specific criteria, a construction live load of 20 psf shall be assumed on the bridge deck.

#### C. Segmental Construction and Specialized Equipment

Construction load combinations per AASHTO LRFD BDS with California Amendments Article 5.14.2 “Segmental Construction” shall be considered. The temporary seismic load event as described in the *Seismic* chapter shall be added to the construction load combination at Strength 5 limit state; however a 1.25 load factor shall be used for dead and live loads. The temporary seismic event need not be combined with the dynamic construction load impact due to segment drop or equipment impact. For balanced cantilever construction methods an additional 2 percent of dead load shall be applied to reflect eccentric conditions at the time of a potential seismic event.



#### D. Temporary Support of Excavation

Temporary supports for excavations include structural elements such as struts, braces, wales, soldier piles, walls and the like. Provisions shall be made for analysis and design of these structural elements, so as not to impose any temporary or permanent adverse effects on adjacent structures and ground surfaces. Soil conditions, earth pressures and distribution of earth pressures, and soil resistance shall be taken from geotechnical reports described in the *Geotechnical* chapter.

#### **12.5.3.4 Rail-Structure Interaction Forces**

Consideration shall be made for forces caused by the constraint of structural movement due to continuous welded rail (CWR). Rail-structure interaction (RSI) can alter the load path distribution, modifying the magnitude and direction of forces applied to the structure or the rail. The constraint of structural movement due to the following effects shall be considered for design:

- Creep (CR)
- Shrinkage (SH)
- Secondary forces from prestressing (PS)
- Uniform temperature (TU)
- Temperature gradient (TG)

RSI modeling technique as specified in Section 12.6.8 shall be used for determining the increased demand forces for the structure. RSI analysis shall consider a specific construction method and construction schedule as well as time-dependent deformations of CR, SH and PS as specified in AASHTO LRFD BDS with California Amendments Articles 4 and 5. Load factors for the increased demand forces due to CR, SH and PS shall be as defined in Table 12-7. Load factors for the increased demand force due to TU and TG shall be as defined in Table 12-4. A temperature differential ( $T_D$ ) of  $\pm 40^\circ\text{F}$  between rails and deck, applied to the superstructure, may be used for determining the increased demand force due to TU. These increased demand forces shall be included in appropriate load combinations for each limit state to satisfy equation 12.5.4-1.

It is critical that rail stress caused by RSI be controlled to minimize probability of rail fracture. Design guidance and structural requirements to limit rail stress are provided in Section 12.6.

#### **12.5.3.5 Blast Loading**

Blast loadings and measures are not specified at this time. Refer to Section 12.13 for general requirements. Refer to AASHTO LRFD BDS with California Amendments Article 3.15 for additional requirements.

#### 12.5.4 Load Factors and Load Modifiers

Regardless of the type of analysis used, the Equation 12.5.4-1 shall be satisfied for specified factored force effect and load combinations for each limit state unless otherwise specified in this chapter:

$$\sum \eta_i \gamma_i Q_i \leq \Phi R_n = R_r \quad (\text{Equation 12.5.4-1})$$

Where:

$\gamma_i$  = load factor applied to force effects (refer to Tables 12-4, 12-6 and 12-7)

$\Phi$  = resistance factor applied to nominal resistance (refer to AASHTO LRFD BDS with California Amendments Article 1.3.2.1)

$\eta_i$  = load modifier relating to ductility, redundancy and importance (refer to AASHTO LRFD BDS with California Amendments Article 1.3.2.1)

$Q_i$  = force effect

$R_n$  = nominal resistance

$R_r$  = factored resistance,  $\Phi R_n$

For loads in which a maximum value of “ $\eta_i$ ” produces an unfavorable action, the value of “ $\eta_i$ ” shall be equal to 1.05 to account for the design life of the facility. The load modifier is applicable to strength limit load combinations only.

##### 12.5.4.1 Design Load Combinations

The load combinations to be used for structures are shown in Table 12-4. The description of the load combinations are as follows:

- “Strength 1” is the basic load combination for normal use.
- “Strength 2” is the load combination for the structure when exposed to wind.
- “Strength 3” is the load combination for very high dead load to live load force effect ratios.
- “Strength 4” is the load combination for normal use when exposed to wind.
- “Strength 5” is the load combination for normal use when designing columns for OBE.
- “Extreme 1” is the load combination for derailment.
- “Extreme 2” is the load combination for collision.
- “Extreme 3” is the load combination for extreme seismic events (MCE).
- “Service 1” is the basic service load combination for normal use with wind.
- “Service 2” is the service load combination intended to control yielding of steel structures and slip of slip-critical connections due to train load.

- "Service 3" is the service load combination relating to tension in prestressed concrete superstructures with the objective of crack control and principal tension in the webs of segmental concrete girders.
  - "Buoyancy at Dewatering Shutoff" is a service load for evaluation of uplift with a minimum weight structure.
  - "Fatigue" is the fatigue and fracture load combination relating to repetitive vertical train loading.
- Note that for each load combination, physically achievable subsets (i.e., omitting loads by setting load factor  $\gamma_i = 0$ ) that may govern design shall be considered.
- Note that other load cases for train and track structure interaction are contained within Section 12.6.
- Note that the rail-structure interaction forces specified in Section 12.5.3.4 shall be considered in the design load combinations.

**Table 12-4: Load Combinations for Design of Structures**

Load Combinations and Load Factors, $\gamma_i$ Load Combination/ Limit State	DC DW DD EV EH ES EL PS CR SH	LLP LLV + I LLRR + I LLH + I LLS LF NE CF SS	WA FR	WS	WL	TU	TG	SE	DR	CL	OBE WA D ED	MCE WA D ED
Strength 1	$\gamma_P$	1.75	1.00	--	--	0.50/ 1.20	--	$\gamma_{SE}$	--	--	--	--
Strength 2	$\gamma_P$	--	1.00	1.40	--	0.50/ 1.20	--	$\gamma_{SE}$	--	--	--	--
Strength 3	$\gamma_P$	--	1.00	--	--	0.50/ 1.20	--	--	--	--	--	--
Strength 4	$\gamma_P$	1.35	1.00	0.65	1.00	0.50/ 1.20	--	$\gamma_{SE}$	--	--	--	--
Strength 5	$\gamma_P$	$\gamma_{EQ}$	1.00	--	--	--	--	--	--	--	1.0	--
Extreme 1	1.00	1.00	1.00	--	--	--	--	--	1.40	--	--	--
Extreme 2	1.00	0.50	1.00	--	--	--	--	--	--	1.00	--	--
Extreme 3	1.00	$\gamma_{EQ}$	1.00	--	--	--	--	--	--	--	--	1.00
Service 1	1.00	1.00	1.00	0.45	1.00	1.00/ 1.20	$\gamma_{TG}$	$\gamma_{SE}$	--	--	--	--
Service 2	1.00	1.30	1.00	--	--	1.00/ 1.20	--	--	--	--	--	--
Service 3	1.00	1.00	1.00	--	--	1.00/ 1.20	$\gamma_{TG}$	$\gamma_{SE}$	--	--	--	--
Buoyancy @ Dewatering Shutoff	0.80	--	1.10	0.45	--	--	--	--	--	--	--	--
Fatigue	--	1.00	--	--	--	--	--	--	--	--	--	--

**Notes:**

- Additional load combinations are found in Section 12.6.
- Additional loads and load combinations for cut-and-cover construction are found in the Technical Manual for Design and Construction of Road Tunnels – Civil Elements; FHWA-NHI-09-010, March, 2009, Chapter 5.
- $\gamma_{TG}$  is equal to 1.0 when live load is not considered and 0.50 when live load is considered.
- $\gamma_{EQ}$  is equal to 0.0 for MCE.  $\gamma_{EQ}$  is equal to 0.50 for OBE, for a 2 track system, 1 train is used. For other track configurations, refer to the *Seismic* chapter.
- $\gamma_{SE}$  is equal to 1.0, in absence of better criteria. For specific areas where settlement values are uncertain, or if otherwise justified, a larger value should apply.
- $\gamma_{TU}$  is equal to the larger value for deformations, and the lesser value for force effects.
- Deraillment load factor taken greater than unity to account for absence of dynamic impact. Refer to Section 12.5.2.13- A.
- WS load factors for Service I and Strength 4 are larger than the AASHTO LRFD BDS with California Amendments to account for a higher wind speed under train operations. Operation of trains is assumed to cease at a wind speed of 67 mph.

**Table 12-5: Loading Definitions Used in Table 12-4**

<b>Permanent Loads</b>	
DC	dead load of structural components and permanent attachments
DW	dead load of non-structural and non-permanent attachment
DD	downdrag force
EV	vertical earth pressure
EH	lateral static earth pressure
ES	surcharge loads
SE	earth settlement effects
EL	locked-in construction forces
PS	secondary forces from prestressing
CR	creep effects
SH	shrinkage effects
WA	water loads and stream pressure
<b>Transient Loads</b>	
LLP	floor, roof, and pedestrian live loads
LLV	high-speed train live loads
LLRR	maintenance and construction train live loads
LLH	highway live loads
LLS	live load surcharge
I	vertical impact effect
LF	traction or braking forces
NE	nosing and hunting effects
CF	centrifugal force
DR	derailment loads
CL	collision loads
WS	wind load on structure
WL	wind load on live load
SS	slipstream effects
TU	uniform temperature effects
TG	gradient temperature effects
FR	frictional force
MCE	Maximum Considered Earthquake
OBE	Operating Basis Earthquake
WAD	hydrodynamic force
ED	dynamic earth pressures

**Table 12-6: Load Factors for Permanent Loads,  $\gamma_P$**

Type of Load, Foundation Type, and Method Used to Calculate Downdrag		$\gamma_P$ Load Factor	
		Maximum	Minimum
DC: Components and Attachments		1.25	0.90
DC: Strength 3 only		1.50	0.90
DD: Downdrag	Piles: $\alpha$ Tomlinson Method	1.40	0.25
	Piles: $\lambda$ Method	1.05	0.30
	Drilled Shafts: O'Neill and Reese (1999) Method	1.25	0.35
DW: Non-structural dead load and non-permanent attachments		1.50	0.65
EH: Horizontal Earth Pressure			
• Active		1.50	0.90
• At-Rest		1.35	0.90
• AEP for Anchored Walls		1.35	N/A
EL: locked-in construction forces		1.00	1.00
EV: Vertical Earth Pressure			
• Overall Stability		1.00	N/A
• Retaining Walls and Abutments		1.35	1.00
• Rigid Buried Structures		1.30	0.90
• Rigid Frames		1.35	0.90
• Flexible Buried Structures other than Metal Box Culverts		1.95	0.90
• Flexible Metal Box Culverts		1.50	0.90
ES: Surcharge Loads		1.50	0.75

1

**Table 12-7: Load Factors for Permanent Loads due to Superimposed Deformations,  $\gamma_P$**

Bridge Component	PS	CR, SH
Superstructures - Segmental Concrete Substructures supporting Segmental Superstructures (Refer to AASHTO LRFD BDS with California Amendments Articles 3.12.4 and 3.12.5)	1.00	see $\gamma_P$ for DC, Table 12-4 and Table 12-6
Concrete Superstructures – non-segmental	1.00	1.00
Substructures supporting non-segmental superstructures		
• Using $I_{gross}$	0.50	0.50
• Using $I_{effective}$	1.00	1.00
Steel Substructures	1.00	1.00

2

#### 12.5.4.2 Resistance Factors

3 For resistance factors  $\Phi$ , refer to AASHTO LRFD BDS with California Amendments.

## 12.6 Track-Structure Interaction

Analysis of track-structure interaction (TSI) shall apply to Primary Type 1 structures (as defined in the *Seismic* chapter) including, but are not limited to: bridges, aerial structures, grade separations, culverts, and aerial stations supporting HST tracks. These Primary Type 1 structures, which are critical for maintaining track performance, are hereafter referred to as “TSI-critical structures”. Span arrangement criteria in Section 12.8.6.13 supplement the deformation and dynamic criteria in this section.

TSI-critical structures are subject to the following design requirements: structural frequency recommendations, track serviceability limits, rail-structure interaction (RSI) limits, dynamic structural analysis limits, and dynamic vehicle track-structure interaction (VTSI) analysis limits.

These requirements are concerned with limiting deformations and accelerations of TSI-critical structures, since the structure response can be dynamically magnified under high-speed moving trains. Excessive deformations and accelerations can lead to unacceptable changes in vertical and horizontal track geometry, excessive rail stress, reduction in wheel contact, dynamic amplification of loads, and passenger discomfort.

Specific criteria limits are provided for structural expansion joints spanned by continuous welded rail (CWR) to reduce the risk of unacceptable track geometry changes and excessive rail stress. For a given TSI-critical structure, all structural discontinuities capable of relative movement (including transitions to other geotechnical elements or tunnels supporting HST tracks) shall be considered to be structural expansion joints and therefore subject to the following applicable relative displacement limits.

Refer to the *Geotechnical* chapter for criteria regarding earthen structures that support HST tracks such as embankments, abutments, retaining walls, and soil subgrades. Refer to the *Tunnels* chapter for criteria regarding tunnels that support HST tracks.

Design assumptions related to track properties (i.e., fasteners, rail section, etc.) found herein are for infrastructure design only and shall not preclude the use of specific track components. Track analysis assumptions not consistent with those provided in this section shall require an approved design variance and shall be provided on the plans.

The following criteria is developed assuming uniform longitudinal rail restraint for structures with maximum structural thermal units (i.e., the maximum distance from a fixed point of thermal expansion to an adjacent fixed point of thermal expansion on a structure) limited to 330 feet. Rail expansion joints are not permitted without an approved design variance.

Table 12-8 summarizes the analysis requirements, including model type, train model/speed, result, and relevant subsections.

**Table 12-8: Track-Structure Interaction Analysis Goals**

Analysis Goal	Model Type	Train model	Train speed	Result	Subsections
Frequency Analysis	Dynamic	--	--	Frequency Evaluation	12.6.3.1 to 12.6.3.3
Track Serviceability Analysis	Static, For OBE: Static or Dynamic	Single or Multiple Tracks of Modified Cooper E50	--	Deformation Limits	12.6.4.1 to 12.6.4.10
Rail-Structure Interaction Analysis	Static (linear or non-linear), For OBE: Static or Dynamic	Single or Multiple Tracks of Modified Cooper E50	--	Deformation and Rail Stress Limits	12.6.5.1 to 12.6.5.6
Dynamic Structural Analysis	Dynamic	Single Tracks of High-Speed Passage	90 mph to 1.2 Line Speed (or 250 mph whichever is less)	Dynamic Impact Factor, Vertical Deck Acceleration	12.6.6.1 to 12.6.6.4
Dynamic Vehicle-Structure Interaction Analysis	Dynamic (Structure and Trainset)	Single Track of High-Speed Passage (with Vehicle Suspension)	90 mph to 1.2 Line Speed (or 250 mph whichever is less)	Dynamic Track Safety and Passenger Comfort Limits	12.6.7.1 to 12.6.7.3

Frequency analysis, track serviceability analysis, rail-structure interaction (RSI) analysis, and dynamic structural analysis, shall apply for all TSI-critical structures.

Dynamic vehicle-track-structure interaction (VTSI) analysis shall apply only to those TSI-critical structures not in compliance with the deformation and acceleration requirements in Sections 12.6.4 through 12.6.6 Alternatively, VTSI can be required as determined by the Authority for those critical structures departing from service-proven concepts.

### 12.6.1 Track-Structure Interaction Design and Analysis Plan

The Designer shall develop a Track-Structure Interaction Design and Analysis Plan (TSIDAP) for each TSI-critical structure.

The TSIDAP shall define the following:

- General Classification as Primary Type 1, Primary Type 2, or Secondary, as defined the Seismic chapter
- Technical Classification as Complex, Standard, or Non-Standard, as defined in the Seismic chapter
- Track Type (ballasted or non-ballasted track)



- Track Configuration (number of tracks, station components, special track features)
  - The Track Fastener properties assumed for analysis (i.e., longitudinal, vertical, and lateral stiffness)
  - The approach used to determine the model boundaries for RSI analysis per Section 12.6.8.7
  - The approach for developing vertical and lateral track stiffness properties at adjacent at-grade track per Section 12.6.8.8
  - Maximum Operating Speed and Design Speed
  - The span arrangement layout, in compliance with Section 12.8.6.13
  - All thermal unit lengths ( $L_{TU}$ ), defined as the point of thermal fixity to the next adjacent point of thermal fixity as depicted on Figure 12-22
  - Locations and extents for all required alternative track solutions such as non-standard fastener configuration (NSFC), non-uniform fastener configuration (NUFC), or rail expansion joints (REJ) as described in Sections 12.6.8.5 and 12.6.8.6
- The TSIDAP shall be consistent with the Seismic Design and Analysis Plan (SDAP) required per the *Seismic* chapter.
- The TSIDAP shall contain detailed commentary on track-structure interaction analysis for all applicable analysis goals presented in Table 12-8, indicating the analysis software to be used, the modeling assumptions, and techniques to be employed.
- Per Section 12.6.8, the TSIDAP shall include an outline of analysis modeling requirements including mass and stiffness variations, presence of continuous welded rail (CWR), and live load configurations. A detailed approach for development of model boundaries at foundations, embankments, and continuous welded rail model boundaries shall also be provided.
- For dynamic structural analysis per Section 12.6.6, the TSIDAP shall summarize the approach for determination of resonance speeds, including the design iteration approach for any structures not consisting entirely of simple spans. Techniques for determining dynamic impact factors and vertical deck accelerations shall be included.
- The TSIDAP shall discuss the approach for determining the rail-structure interaction forces caused by creep, shrinkage, prestressing, and temperature effects per Section 12.5.3.4. The approach for implementation of results into Strength and Service Load combinations in Table 12-4 shall be provided.
- To meet RSI criteria per Section 12.6.5, the TSIDAP may include proposals for alternative track solutions (e.g., NSFC, NUFC, REJs, etc.) through the design variance approval process. The design variance shall be supplemented with a special RSI analysis per Section 12.6.8.6.
- As determined by the Authority, advanced supplemental plans may be required as part of the TSIDAP. These advanced supplemental plans are to be required as part of conditional approval

for design variance requests or for those critical structures that depart from current service-proven design concepts. Advanced supplemental plans include, but are not limited to:

- Rail-Structure Interaction Design and Analysis Plan (RSIDAP) per Section 12.6.8.6
- Vehicle-Track-Structure Interaction Design and Analysis Plan (VTSIDAP) per Section 12.6.7

Track-structure interaction related design variances shall be submitted per the *General* chapter. The TSIDAP shall justify all track-structure interaction design variances related to track performance, rail-structure interaction, or dynamic structural response.

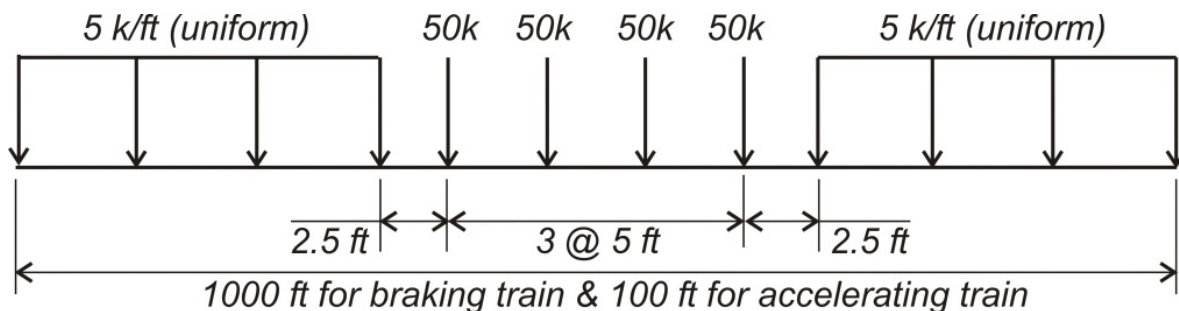
### 12.6.2 Design Parameters

The following defines the loading that shall be used for track serviceability and rail-structure interaction analyses.

#### 12.6.2.1 Modified Cooper E-50 Loading (LLRM)

Modified Cooper E-50 loading (LLRM) shown in Figure 12-10 shall be used for track serviceability and rail-structure interaction (RSI) analyses. LLRM loading is on a per track (i.e., 2 rail) basis.

**Figure 12-10: LLRM Loading**



#### 12.6.2.2 Vertical Impact Effect (I)

The vertical impact effect (I) used with Modified Cooper E-50 loading (LLRM) shall be vertical impact effect from LLRR per Section 12.5.2.2.

Dynamic vertical impact effects ( $I_{LLV}$ ) caused by high-speed trainsets (LLV) shall be per Section 12.6.6.3.

#### 12.6.2.3 Centrifugal Force (CF)

The centrifugal force (CF) used with Modified Cooper E-50 loading (LLRM) shall be determined per Section 12.5.2.3. The maximum CF calculated for LLRR and LLV shall be used, whichever governs.

#### 12.6.2.4 Traction and Braking Force (LF)

The longitudinal traction and braking forces (LF) used with Modified Cooper E-50 loading (LLRM) shall be determined using the approach for LLV loading per Section 12.5.2.4-B.

#### 12.6.3 Frequency Analysis

Frequency recommendations are established for the fundamental mode shapes of TSI-critical structures, in order to promote well-proportioned structures and minimize resonancy effects.

Recommended frequency thresholds in this section are for guidance only, and serve as a preliminary assesment of dynamic performance. As outlined by the TSI Analysis goals in Table 12-8, other more detailed evaluations of dynamic performance are required for all structures.

For each structure, the primary vertical, lateral, and torsional frequencies shall be provided on the plans. Plans shall show primary frequencies for each required analysis condition as defined in this section.

Upper and lower bound mass and stiffness assumptions shall be evaluated per the modeling requirements as given in Section 12.6.8.

##### 12.6.3.1 Recommended Threshold of Vertical Frequency of Span

The recommended vertical frequency lower bound threshold is known to favorably resist high-speed train resonance actions. It is recommended that structures be proportioned to fall above this lower bound threshold.

Vertical frequency analysis shall consider the flexibility of superstructure, bearings, shear keys, columns, and foundations.

For vertical frequency analysis, 2 conditions shall be investigated:

- **Condition #1** – a lower bound estimate of stiffness and upper bound estimate of mass
- **Condition #2** – an upper bound estimate of stiffness and lower bound estimate of mass

Condition #1 will govern the lower bound threshold. Condition #2 is required for future structural assessment.

Modeling requirements for lower and upper bound estimates of stiffness and mass are given in Section 12.6.8.

The recommended threshold for the first natural frequency of vertical deflection,  $\eta_{\text{vert}}$  [Hz], primarily due to bending of the span is the following:

$$\eta_{\text{vert}} \geq \eta_{\text{lower}}$$

Where:

$$\eta_{\text{lower}} = 313.09L^{-0.917} \text{ for } L \leq 330 \text{ feet}$$

where L = effective length of span (feet)

For simple spans, L shall be the span length.

For continuous spans, L shall be the following:

$$L = k(L_{\text{average}})$$

Where:

$$L_{\text{average}} = \frac{(L_1 + L_2 + \dots + L_n)}{n} = \text{the average span length}$$

$n$  = the number of spans

$$k = \left(1 + \frac{n}{10}\right) \leq 1.5$$

For portal frames and closed frame bridges, L shall be the following:

- Single span: consider as 3 continuous spans, with the first and third span being the vertical length of the columns, and the second span the girder length
- Multiple spans: consider as multiple spans, with the first and last span as the vertical length of the end columns, and the interior spans the girder lengths

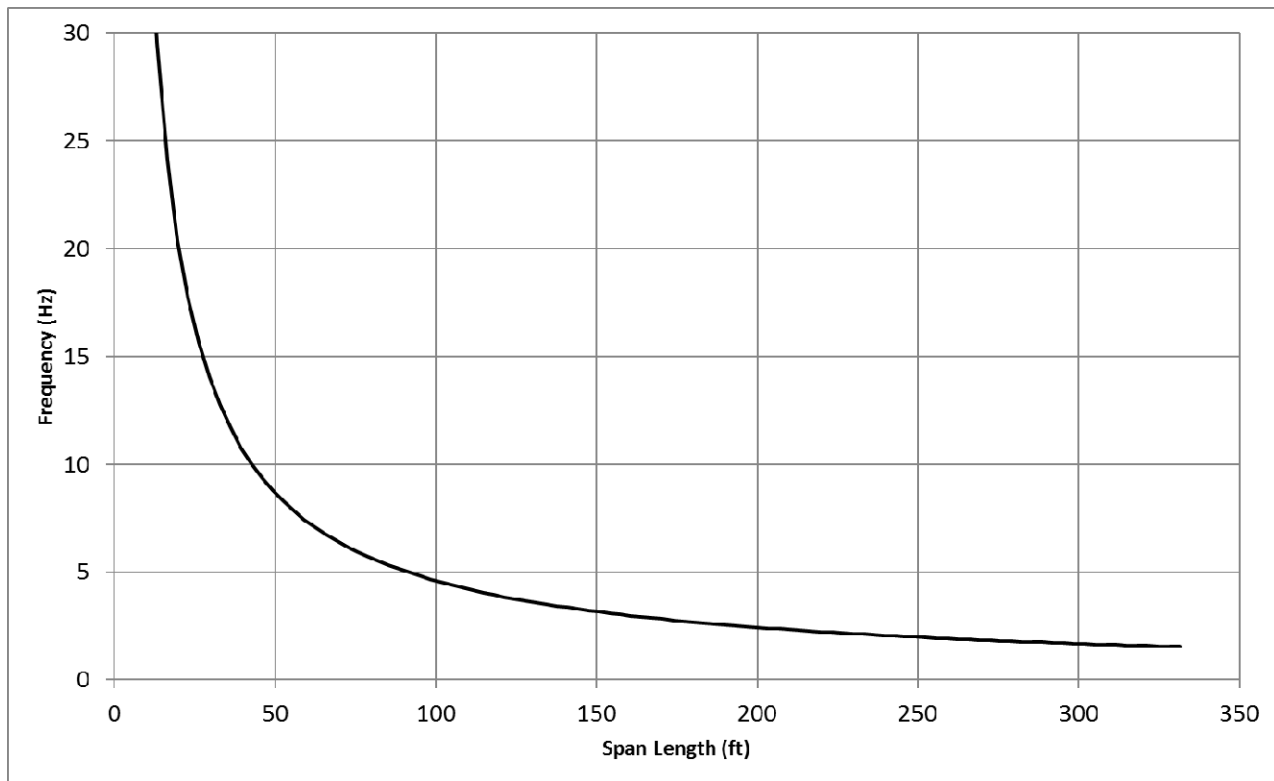
For spans with end diaphragms at abutments (fixed supports at abutments), the following L shall apply:

- Single span, fixed at 1 abutment: consider as 2 continuous spans, with the first span equal to 0.05 times the girder length, and the second span the girder length
- Single span, fixed at both abutments: consider as 3 continuous spans, with the first and the third span equal to 0.05 times the girder length, and the second span the girder length
- Multiple spans, fixed at 1 abutment: consider as multiple spans, with the first span equal to 0.05 times the adjacent girder length, and the interior spans the girder lengths
- Multiple spans, fixed at both abutments: consider as multiple spans, with the first and last span equal to 0.05 times the adjacent girder length, and the interior spans the girder lengths

For single arch, arch-rib, or stiffened girders of bowstrings, L shall be the half span.

Refer to Figure 12-11 for the recommended lower bound vertical frequency threshold.

1 **Figure 12-11: Recommended Lower Bound Threshold of Vertical Frequency**



### 12.6.3.2 Recommended Lower Bound Torsional Frequency of Span

Recommendations for allowable torsional frequency are to proportion structures to favorably resist high-speed train actions.

Torsional frequency analysis shall consider the flexibility of superstructure, bearings, shear keys, columns, and foundations.

For torsional frequency analysis, 2 conditions shall be investigated, consistent with vertical frequency analysis:

- **Condition #1** – a lower bound estimate of stiffness and upper bound estimate of mass
- **Condition #2** – an upper bound estimate of stiffness and lower bound estimate of mass

Modeling requirements for lower and upper bound estimates of stiffness and mass are given in Section 12.6.8.

For Conditions #1 and #2, the first torsional frequency,  $\eta_{\text{torsion}}$ , of the span is recommended to be greater than 1.2 times the corresponding first natural frequency of vertical deflection,  $\eta_{\text{vert}}$ .

### 12.6.3.3 Recommended Lower Bound Transverse Frequency of Span

Recommendations for allowable transverse frequency are to proportion structures to favorably resist high-speed train actions.

For transverse frequency analysis, 2 conditions shall be investigated:

- **Condition #1** – consideration of flexibility of superstructure only, excluding the flexibility of bearings, columns, and foundations, assuming supports at ends of the span are rigid.
- **Condition #2** – consideration of flexibility of superstructure and substructure, including flexibility of bearings, columns, shear keys, and foundations.

For transverse frequency analysis, a lower bound estimate of stiffness and upper bound estimate of mass shall be used, refer to Section 12.6.8.

For Condition #1, the first natural frequency of transverse deflection,  $\eta_{trans}$ , of the span is recommended to be greater than 1.2 Hz.

For Condition #2, no frequency recommendation is provided but shall be recorded for future structural assessment.

### 12.6.4 Track Serviceability Analysis

Track serviceability analysis, using modified Cooper E-50 loading (LLRM), provides limits to allowable structural deformations. These track serviceability limits were developed for structures supporting continuous welded rail without rail expansion joints.

Deformation limits were developed for limit states based on maintenance, passenger comfort, and track safety requirements.

For track serviceability analysis, the flexibility of the superstructure and substructure (i.e., bearings, shear keys, columns, and foundations) shall be considered.

For all track serviceability analysis, in order to avoid underestimating deformations, a lower bound estimate of stiffness and an upper bound estimate of mass shall be used.

Modeling requirements are given in Section 12.6.8.

#### 12.6.4.1 Track Serviceability Load Cases

Track serviceability loads cases shall include the following:

- Group 1a:  $(LLRM + I)_1 + CF_1 + WA$
- Group 1b:  $(LLRM + I)_2 + CF_2 + WA$
- Group 1c:  $(LLRM + I)_m + CF_m + WA$
- Group 2:  $(LLRM + I)_1 + CF_1 + WA + WS + WL_1$

- Group 3:  $(LLRM + I)_1 + CF_1 + OBE$

Where:

$(LLRM + I)_1$  = 1 track of Modified Cooper E-50 (LLRM) plus impact

$(LLRM + I)_2$  = 2 tracks of Modified Cooper E-50 (LLRM) plus impact

$(LLRM + I)_m$  = multiple tracks per Section 12.5.2.1-D of Modified Cooper E-50 (LLRM) plus impact

$I$  = vertical impact factor from LLRR (Section 12.5.2.2)

$CF_1$  = centrifugal force maximum from LLRR and LLV (one track) (Section 12.5.2.3)

$CF_2$  = centrifugal force maximum from LLRR and LLV (2 tracks) (Section 12.5.2.3)

$CF_m$  = centrifugal force maximum from LLRR and LLV (multiple tracks) (Section 12.5.2.3)

$WA$  = water loads (stream flow) (Section 12.5.1.10)

$WS$  &  $WL_1$  = wind on structure and wind on 1 1000' LLRM train (Section 12.5.2.6)

$OBE$  = Operating Basis Earthquake per *Seismic* chapter

Note that Group 1c is used for Section 12.6.4.4 only.

Static analysis and linear superposition of results shall be allowed for Groups 1a, 1b, 1c, and 2.

For determining OBE demands in Group 3, equivalent static analysis, dynamic response spectrum, or time history (linear or non-linear) analysis shall be used as per the *Seismic* chapter and the approved Seismic Design and Analysis Plan. Refer to the *Seismic* chapter for additional OBE modeling requirements.

For track serviceability analysis, non-linear modeling of RSI effects (refer to Section 12.6.8.5) is not required, but may be used. For Group 3, superposition of static (i.e.,  $(LLRM + I)_1 + CF_1$ ) and either static or dynamic OBE shall be allowed.

For Groups 1-3, the effects of  $CF$ ,  $WA$ ,  $WS$ ,  $WL$  loads shall be included only if it is conservative to do so. These loads shall be excluded if found to counteract the deflections associated with  $(LLRM + I)$  loading.

#### 12.6.4.2 Vertical Deflection Limits: Group 1a

Vertical deflection limits for Group 1a shall be used to address maintenance, passenger comfort, and track safety issues.

For Group 1a, the maximum static vertical deck deflection ( $\max \Delta_{1a}$ ), with  $(LLRM + I)_1$  and  $CF_1$  in the most unfavorable position, shall not exceed the limits shown in Table 12-9.

**Table 12-9: Vertical Deflection Limits: Group 1a**

Limit	Span Length (feet)				
	$L \leq 125$	$L=175$	$L=225$	$L=275$	$L \geq 330$
$\max \Delta_{1a}$	$L/3500$	$L/3180$	$L/2870$	$L/2550$	$L/2200$

- 1 Note: Limits apply for both non-ballasted and ballasted track
- 2 For span lengths not explicitly referenced in Table 12-9, use linear interpolation.

#### 12.6.4.3 Vertical Deflection Limits: Group 1b

- 3 Vertical deflection limits for Group 1b shall be used to address maintenance, passenger comfort,
- 4 and track safety issues.
- 5 For Group 1b, the maximum static vertical deck deflection ( $\max \Delta_{1b}$ ), with  $(LLRM + I)_2$  and  $CF_2$
- 6 in the most unfavorable position, shall not exceed the limits shown in Table 12-10.

**Table 12-10: Vertical Deflection Limits: Group 1b**

Limit	Span Length (feet)				
	$L \leq 125$	$L=175$	$L=225$	$L=275$	$L \geq 330$
$\max \Delta_{1b}$	$L/2400$	$L/2090$	$L/1770$	$L/1450$	$L/1100$

- 7 Note: Limits apply for both non-ballasted and ballasted track
- 8 For span lengths not explicitly referenced in Table 12-10, use linear interpolation.

#### 12.6.4.4 Vertical Deflection Limits: Group 1c

- 9 Vertical deflection limits for Group 1c shall be used to provide practical guidance for structures
- 10 containing 3 or more tracks operating at speeds less than 90 mph. This guidance is consistent
- 11 with established European codes.
- 12 For Group 1c, where the structures support 3 or more tracks,  $(LLRM + I)_m$  and  $CF_m$  loading shall
- 13 be applied in a manner consistent with the case of multiple tracks on structures as described per
- 14 Section 12.5.2.1-D.
- 15 The tracks selected for loading shall be those tracks that will produce the most critical design
- 16 condition on the member under consideration.
- 17 For Group 1c, where structures support 3 or more tracks, the maximum static vertical deck
- 18 deflection ( $\max \Delta_{1c}$ ), with loads in the most unfavorable position, shall not exceed  $L/600$  for all
- 19 span lengths. This limit applies for both non-ballasted and ballasted track.
- 20 In the event that structures support 3 or more tracks, and 3 or more trains can be anticipated to
- 21 be on the same structure simultaneously at speeds greater than 90 mph, limits defined for
- 22 Group 1b shall apply. For these structures, representative live load conditions shall be
- 23 developed on a case-by-case basis and defined in the TSIDAP.

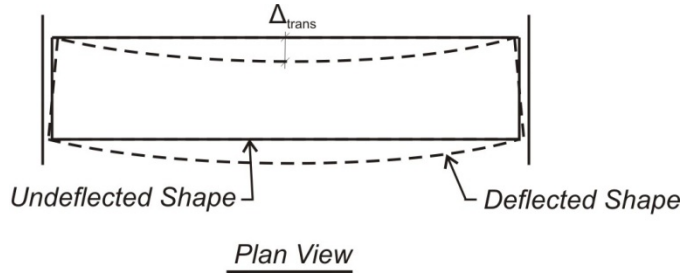


#### 12.6.4.5 Transverse Deflection Limits

Transverse deflection limits shall be used to address maintenance, passenger comfort, and track safety issues.

The maximum transverse deflection within the span ( $\Delta_{trans}$ ), as depicted on Figure 12-12, shall not exceed the limits shown in Table 12-11.

**Figure 12-12: Transverse Span Deformation Limits**



**Table 12-11: Transverse Deflection Limits**

Group	$\Delta_{trans}$ (feet)
1a	$L^2/(864,800)$
1b	$L^2/(447,200)$
2	$L^2/(276,800)$
3	$L^2/(276,800)$

Note: Limits apply for both non-ballasted and ballasted track

#### 12.6.4.6 Rotation about Transverse Axis Limits

Rotation about transverse axis limits shall be used to control rail axial and bending stress, provide traffic safety (i.e., guard against wheel unloading due to abrupt angular changes in track geometry), and provide passenger comfort.

Due to rotation about the transverse axis, imposed longitudinal rail displacement is a linear function of the distance between the rail centroid and top of the bridge bearings. This imposed longitudinal displacement causes axial rail stress.

The maximum total rotation about transverse axis at deck ends ( $\theta_t$ ), depicted on Figure 12-13, shall be defined by the following equations:

$$\theta_t = \theta, \text{ for abutment condition}$$

$$\theta_t = \theta_1 + \theta_2, \text{ between consecutive decks}$$

The maximum relative longitudinal displacement at the level of the rail ( $\delta_t$ ) due to rotation about transverse axis, depicted on Figure 12-13, shall be defined by the following equations:

$$\delta_t = \theta h, \text{ for abutment condition}$$

$$\delta_t = \delta_1 + \delta_2 = \theta_1 h_1 + \theta_2 h_2, \text{ between consecutive decks}$$

Where:

$\theta_t$  (radians) = total rotation about transverse axis, refer to Table 12-12

$\delta_t$  (inches) = total relative displacement at the level of the rail, refer to Table 12-12

$\theta$  (radians) = rotation of the bridge bearing at abutment

$\theta_1$  (radians) = rotation of the first bridge bearing

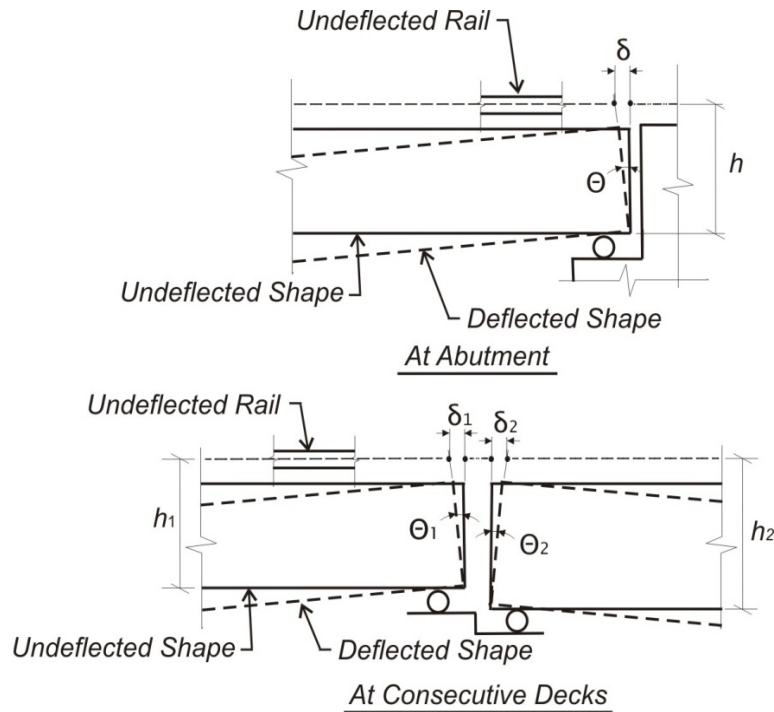
$\theta_2$  (radians) = rotation of the second bridge bearing

$h$  (inches) = the distance between the rail centroid and the top of the bridge bearing at abutment

$h_1$  (inches) = the distance between the rail centroid and the top of the first bridge bearing

$h_2$  (inches) = the distance between the rail centroid and the top of the second bridge bearing

1 **Figure 12-13: Rotation about Transverse Axis at Deck Ends**



- 2
- 3 The total rotation about transverse axis ( $\theta_t$ ) and the corresponding maximum relative
- 4 displacement at the level of the rail ( $\delta_t$ ) shall not exceed the limits shown in Table 12-12.

**Table 12-12: Rotation about Transverse Axis and Relative Displacement at the Level of the Rail Limits**

Group	$\theta_t$ (radians)	$\delta_t$ (inches)	
		Non-ballasted Track	Ballasted Track
1a	0.0012	0.33	0.33
1b	0.0017	0.33	0.33
2	0.0026	0.67	0.67
3	0.0026	0.67	0.67

5

**12.6.4.7 Rotation about Vertical Axis Limits**

6 Rotation about vertical axis limits shall be used to control rail axial and bending stress, provide

7 track safety, and provide passenger comfort by limiting changes in horizontal track geometry at

8 bridge deck ends.

9 Due to rotation about the vertical axis, imposed longitudinal rail displacement is a linear

10 function of the distance between the centerline of span and the outermost rail. This imposed

11 longitudinal displacement causes axial rail stress.

The maximum total rotation about vertical axis at deck ends ( $\theta_v$ ), depicted on Figure 12-14 shall be defined by the following equations:

$$\theta_v = \theta, \text{ for abutment condition}$$

$$\theta_v = \theta_A + \theta_B, \text{ between consecutive decks}$$

The maximum relative longitudinal displacement at the outermost rail ( $\delta_v$ ) due to rotation about vertical axis, depicted on Figure 12-15, shall be defined by the following equations:

$$\delta_v = \theta w, \text{ for abutment condition}$$

$$\delta_v = \delta_A + \delta_B = \theta_A w_A + \theta_B w_B, \text{ between consecutive decks}$$

Where:

$\theta_v$  (radians): total rotation about vertical axis, refer to Table 12-13

$\delta_v$  (inches): total relative displacement at the outermost rail, refer to Table 12-13

$\delta_A$  (inches): relative displacement at the outermost rail, first span

$\delta_B$  (inches): relative displacement at the outermost rail, second span

$\theta$  (radians): rotation of the bridge at abutment

$\theta_A$  (radians): rotation of the first span

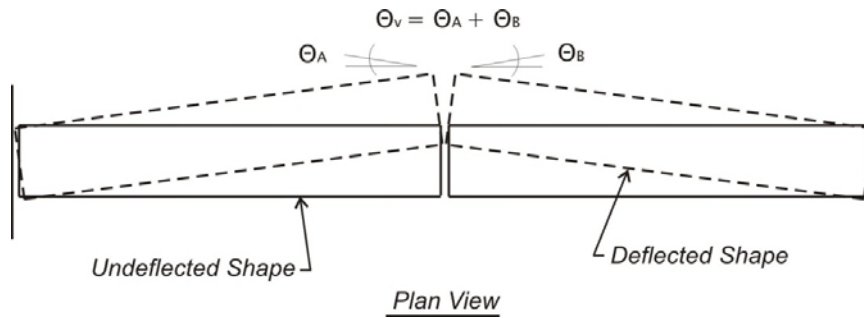
$\theta_B$  (radians): rotation of the second span

$w$  (inches): the distance between the centerline span and outermost rail centroid at abutment

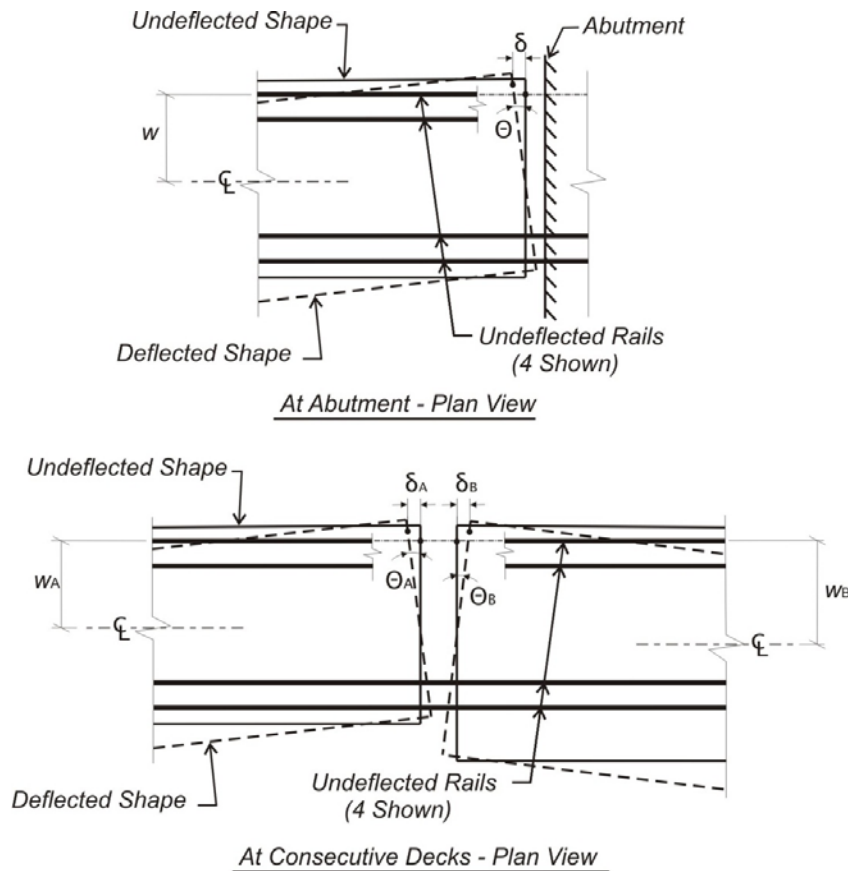
$w_A$  (inches): the distance between the centerline span and outermost rail centroid of first span

$w_B$  (inches): the distance between the centerline span and outermost rail centroid of second span

**Figure 12-14: Rotation about Vertical Axis at Deck Ends – Global View**



**Figure 12-15: Rotation about Vertical Axis at Deck Ends – Local View**



The total rotation about vertical axis ( $\theta_v$ ) and the corresponding maximum relative displacement at the outermost rail ( $\delta_v$ ) shall not exceed the limits shown in Table 12-13.

**Table 12-13: Rotation about Vertical Axis and Relative Displacement at Outermost Rail Limits**

Group	$\theta_v$ (radians)	$\delta_v$ (inches)	
		Non-ballasted Track	Ballasted Track
1a	0.0007	0.33	0.33
1b	0.0010	0.33	0.33
2	0.0021	0.67	0.67
3	0.0021	0.67	0.67

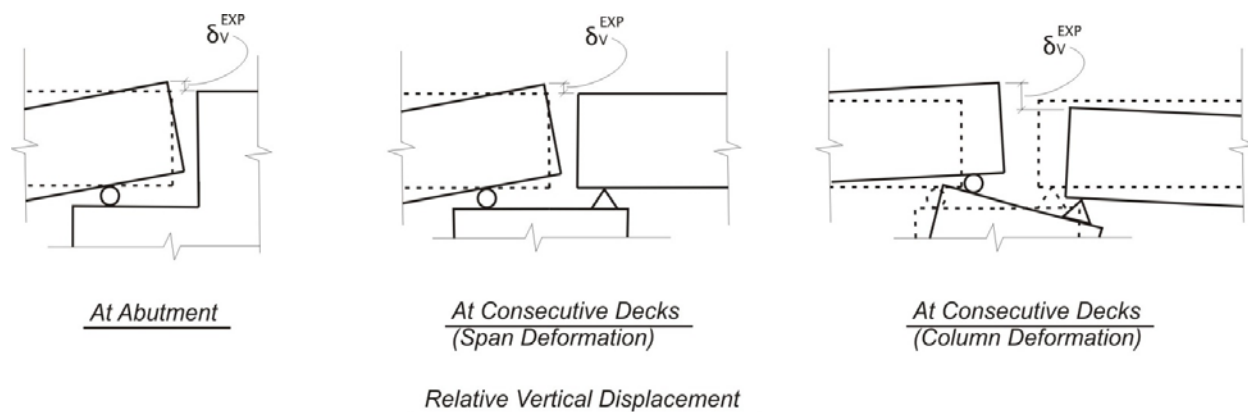
#### 12.6.4.8 Relative Vertical Displacement at Expansion Joints – Track Serviceability

Relative vertical displacements (RVD) at structural expansion joints,  $\delta_v^{EXP}$ , shall be limited in order to ensure track safety due to deck end rotation and vertical bearing deformation. Structural expansion joints between adjacent deck ends, and between deck ends and abutments shall be considered.

The relative vertical displacement at expansion joints ( $\delta_v^{EXP}$ ), depicted on Figure 12-16, shall not exceed the limits shown in Table 12-14.

Refer to Section 12.6.5.3 for additional RVD limits for rail-structure interaction analysis.

#### Figure 12-16: Relative Vertical Displacement at Expansion Joints – Track Serviceability



**Table 12-14: Relative Vertical Displacement at Expansion Joints Limits – Track Serviceability**

Group	$\delta_v^{EXP}$ (inch)
1a	0.25
1b	0.25
2	-
3	-

Note: Limits apply for both non-ballasted and ballasted track

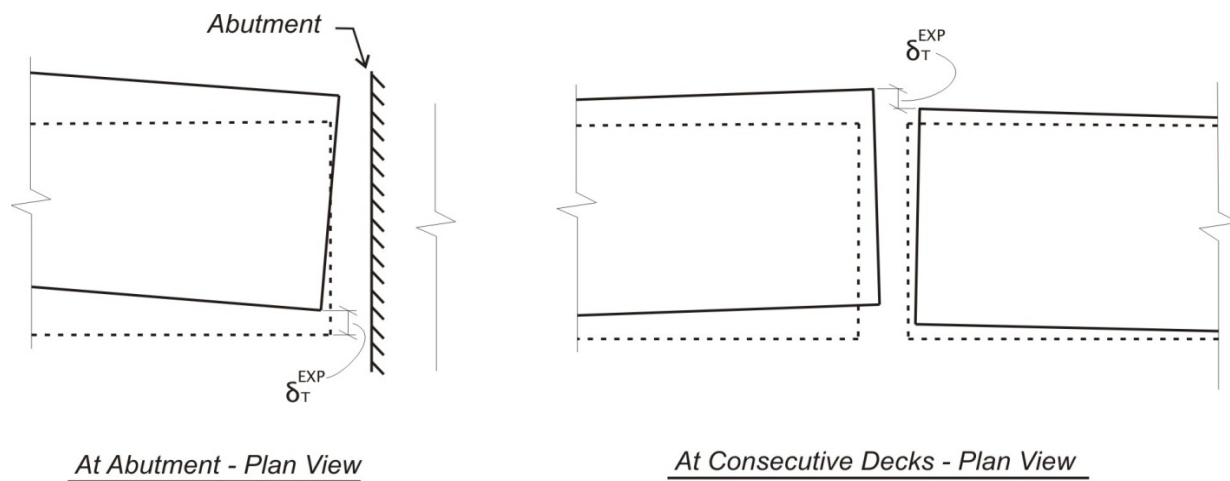
#### 12.6.4.9 Relative Transverse Displacement at Expansion Joints – Track Serviceability

Relative transverse displacements (RTD) at structural expansion joints,  $\delta_T^{EXP}$ , shall be limited in order to ensure track safety subject to shear key and lateral bearing deformation. Structural expansion joints between adjacent deck ends, and between deck ends and abutments shall be considered.

The relative transverse displacement at expansion joints ( $\delta_T^{EXP}$ ), depicted on Figure 12-17, shall not exceed the limits shown in Table 12-15.

Refer to Section 12.6.5.4 for additional RTD limits for rail-structure interaction analysis.

**Figure 12-17: Relative Transverse Displacement at Expansion Joints – Track Serviceability**



**Table 12-15: Relative Transverse Displacement at Expansion Joints Limits – Track Serviceability**

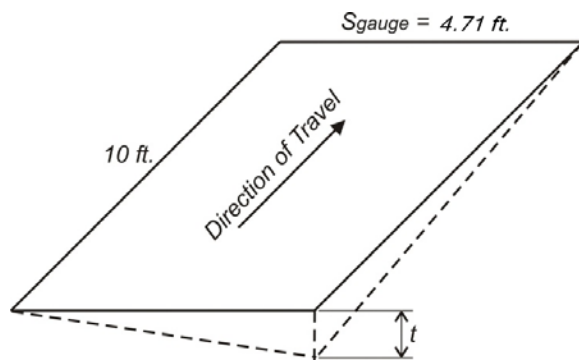
Group	$\delta_T^{EXP}$ (inch)
1a	0.08
1b	0.08
2	-
3	-

Note: Limits apply for both non-ballasted and ballasted track

#### 12.6.4.10 Deck Twist Limits

The deck twist,  $t$ , is defined as the relative vertical deck of a given truck wheel from a plane defined by the remaining truck wheels on a track gauge of 4.71 feet over a truck length of 10 feet, refer to Figure 12-18. Deck twist limits ensure that the wheel contact points of a truck are not too far from a plane.

**Figure 12-18: Deck Twist Diagram**



Maximum deck twist ( $t_{max}$ ) below tracks shall not exceed the limits shown in Table 12-16.

**Table 12-16: Deck Twist Limits**

Group	$t_{max}$ (inches)
1a	0.06
1b	0.06
2	0.17
3	0.17

Note: Limits apply for both non-ballasted and ballasted track

#### 12.6.5 Rail-Structure Interaction Analysis

Rail-structure interaction (RSI) analysis, using modified Cooper E-50 loading (LLRM), shall be used to limit relative longitudinal, vertical, and transverse displacements at structural expansion joints, and limit axial rail stress in order to minimize the probability of rail fracture.



Deformation and rail stress limits were developed considering the accumulation of displacement demands and rail bending stresses under the controlling load combinations.

Details of RSI modeling requirements are given in Section 12.6.8.5.

For RSI analysis, the flexibility of the superstructure and substructure (i.e., bearings, shear keys, columns, and foundations) shall be considered.

For all RSI analysis, in order to avoid underestimating deformations and rail stress, a lower bound estimate of stiffness and an upper bound estimate of mass shall be used.

Limits on expansion joint displacement, fastener performance, and rail stress are provided in Sections 12.6.5.2 through 12.6.5.6. These limits only apply if all assumptions and modeling requirements given in Section 12.6.8.5 are valid. For structures requiring alternative assumptions or modeling techniques, an approved design variance and a special RSI analysis per Section 12.6.8.6 shall be required.

For ballasted track, structural expansion joint relative movements must be limited to prevent not only rail overstress, but also ballast fall-through, deconsolidation, and destabilization. For all TSI-critical structures with ballasted track, each structural expansion joint shall be detailed for ballast retainment in order to prevent ballast fall-through, deconsolidation, or destabilization at the joint. The detailing, which shall be determined by the Contractor, is dependent on the displacement demands of a given joint, the structural configuration, and type of loading being considered.

#### 12.6.5.1 Rail-Structure Interaction Load Cases

Rail-structure interaction (RSI) load cases include the following:

- Group 4:  $(LLRM + I)_2 + LF_2 \pm T_D$
- Group 5:  $(LLRM + I)_1 + LF_1 \pm 0.5T_D + OBE$

Where:

$(LLRM + I)_1$  = single track of Modified Cooper E-50 (LLRM) plus vertical impact effect

$(LLRM + I)_2$  = 2 tracks of Modified Cooper E-50 (LLRM) plus vertical impact effect

$I$  = vertical impact factor from LLRR (Section 12.5.2.2)

$LF_1$  = braking forces (apply braking to 1 track) for LLV loading (Section 12.5.2.4-B)

$LF_2$  = braking and acceleration forces (apply braking to 1 track, acceleration to the other track) for LLV loading (Section 12.5.2.4-B)

$T_D$  = temperature differential of  $\pm 40^\circ\text{F}$  between rails and deck, applied to the superstructure

$OBE$  = Operating Basis Earthquake per the *Seismic* chapter

Groups 4 and 5 are to provide relative longitudinal, vertical, and transverse displacement limits at expansion joints, and design for uplift at direct fixation fasteners. Groups 4 and 5 are also used to limit rail stress, accounting for thermal effects (i.e.,  $\pm T_D$ ).

Modeling of non-linear RSI effects, as given in Section 12.6.8.5, shall be required to give realistic demands. Experience has shown that linear modeling of RSI is overly conservative.

For Group 5, non-linear time-history OBE analysis (i.e., non-linear RSI) shall be used for design.  $(LLRM + I)_1 + LF_1$  may be idealized as a set of stationary load vectors placed upon the structure in the most unfavorable position. Refer to the *Seismic* chapter for additional OBE modeling requirements.

### **12.6.5.2 Relative Longitudinal Displacement at Expansion Joints**

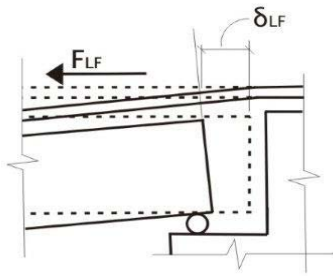
Relative longitudinal displacements (RLD) at structural expansion joints,  $\delta_L^{EXP}$ , shall be limited in order to control rail axial stress. Structural expansion joints between adjacent deck ends, and between deck ends and abutments shall be considered.

RLD at structural expansion joints,  $\delta_L^{EXP}$ , has components due to both structural translation and structural rotation. For structural rotation, RLD is a function of distance from center of structure rotation to rail centroid. Therefore,  $\delta_L^{EXP}$  shall be monitored relative to the original rail centroid location, and consist of structural movement alone.

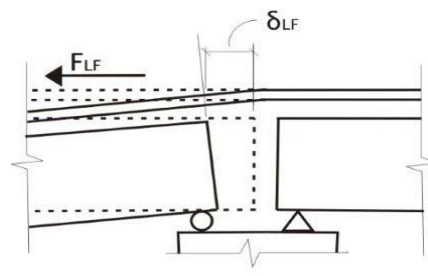
$\delta_L^{EXP}$  consists of separate components:

- $\delta_{LF}$  = component due to acceleration and braking only, refer to Figure 12-19
- $\delta_{LLRM+I}$  = component due to vertical train plus impact loads only, refer to Figure 12-20
- $\delta_{OBE}$  = component due to OBE only (refer to Figure 12-21), comprising:
  - $\delta_{OBE(L)}$  = longitudinal displacement subcomponent due to OBE
  - $\delta_{OBE(V)}$  = rotation about vertical axis subcomponent due to OBE
  - $\delta_{OBE(T)}$  = rotation about transverse axis subcomponent due to OBE
  - $\delta_{OBE} = \delta_{OBE(L)} + \delta_{OBE(V)} + \delta_{OBE(T)}$
- $\delta_{TD}$  = component due to temperature differential ( $\pm T_D$ ) between superstructure and rail

1 **Figure 12-19:  $\delta_{LF}$  Definition**

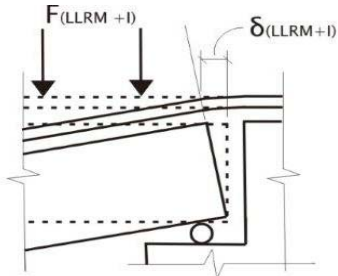


At Abutment

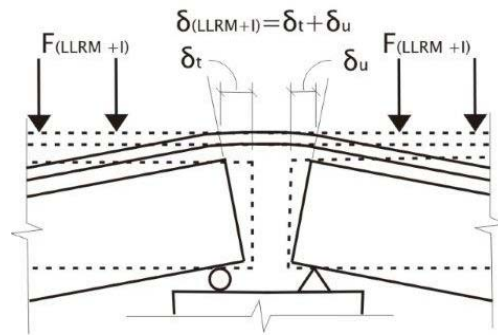


At Consecutive Decks

4 **Figure 12-20:  $\delta_{LLRM+I}$  Definition**

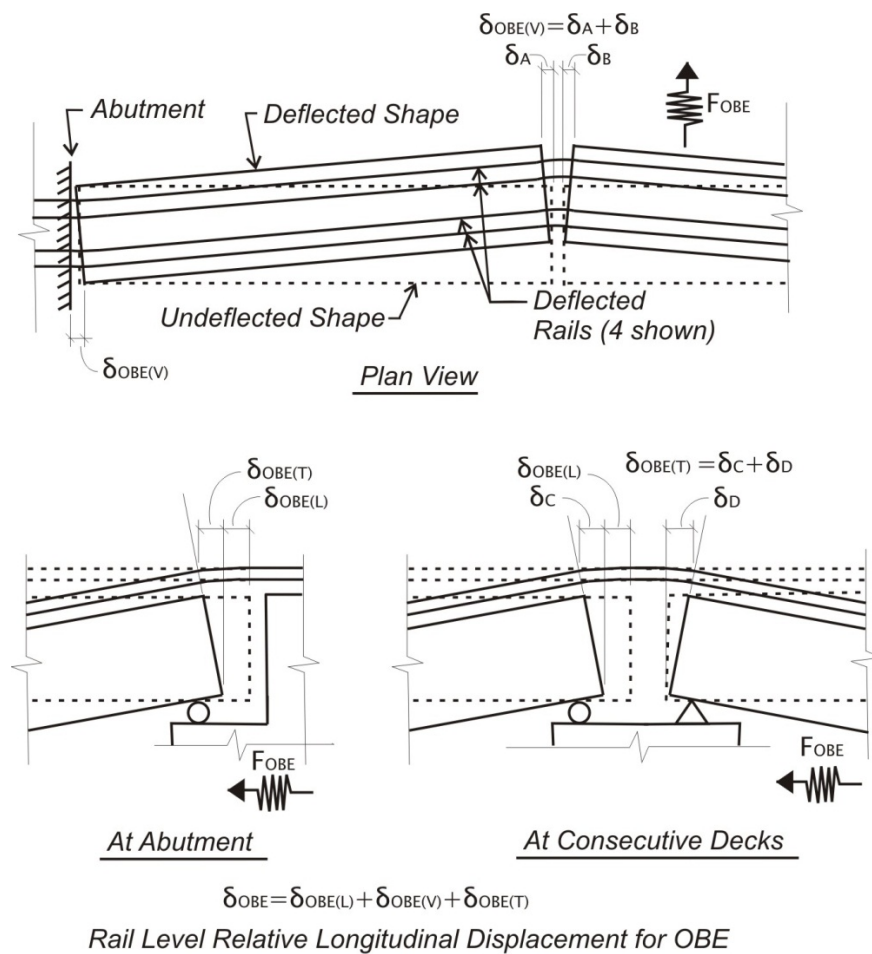


At Abutment



At Consecutive Decks

1 **Figure 12-21:  $\delta_{OBE}$  Definition**

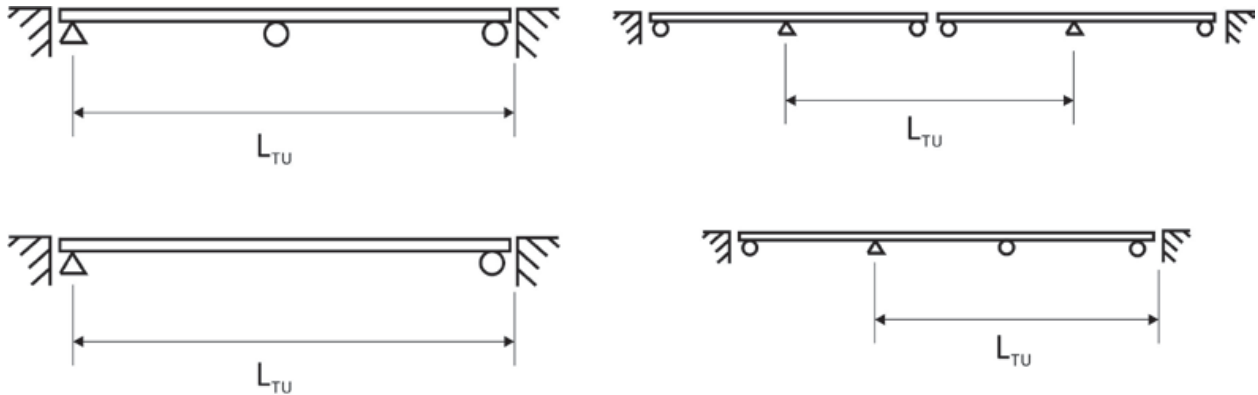


2  
3  
4 The RLD at expansion joints measured relative to the original rail centroid location ( $\delta_{L}^{EXP}$ ) shall  
5 not exceed the limits shown in Table 12-17.

6 Note that in order to prevent having separate load cases for relative displacement and rail stress  
7 design, the expected temperature differential demands are added to the displacement limits.

8 The temperature differential demands are dependent on the structural thermal unit ( $L_{TU}$ ), which  
9 is defined as the point from fixed point of thermal expansion to the next adjacent fixed point of  
10 thermal expansion as depicted on Table 12-22. The maximum  $L_{TU}$  shall not exceed 330 feet  
11 without an approved design variance and special RSI analysis per Section 12.6.8.6.

1 **Figure 12-22: Structural Thermal Unit ( $L_{TU}$ ) Definition**



2

**Table 12-17: Relative Longitudinal Displacement at Expansion Joints Limits**

Group	$\delta_L^{EXP}$ (inch)	
	Non-ballasted Track	Ballasted Track
4	$0.70 + \delta_{TD,Expected}$	$0.50 + \delta_{TD,Expected}$
5	$2.33 + 0.5\delta_{TD,Expected}$	$2.25 + 0.5\delta_{TD,Expected}$

3 Where:

4  $\delta_{TD,Expected}$  = expected RLD measured relative to the original rail centroid location due to  
5  $T_D$  loading per Section 12.6.5.1. For most structures,  $\delta_{TD,Expected}$  can be approximated by:

6 
$$\delta_{TD,Expected} = \alpha(\Delta T)L_{TU}$$

7 Where:

8  $\alpha$  = coefficient of thermal expansion for the superstructure

9  $\Delta T$  = 40°F temperature differential per Section 12.6.5.1. ( $\Delta T$  always positive for  
10 calculation of  $\delta_{TD,Expected}$ )

11  $L_{TU}$  = length of structural thermal unit at a given expansion joint, refer to Figure 12-22.

12 For any structure that  $\delta_{TD,Expected}$  cannot be approximated with the above equation,  $\delta_{TD,Expected}$  shall  
13 be verified by monitoring RSI models subject to  $T_D$  loading per Section 12.6.5.1. When a special  
14 RSI analysis per Section 12.6.8.6 is required, a detailed temperature analysis shall be required to  
15 justify the determination of  $\delta_{TD,Expected}$ .

### 12.6.5.3 Relative Vertical Displacement at Expansion Joints

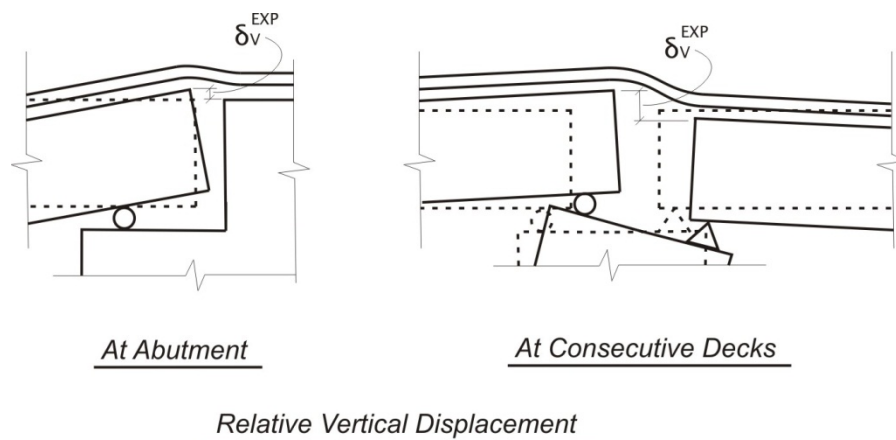
Relative vertical displacement (RVD) at structural expansion joints,  $\delta_v^{EXP}$ , shall be limited in order to control rail bending stress. Structural expansion joints between adjacent deck ends, and between deck ends and abutments shall be considered.

The flexibility of the superstructure and substructure (i.e., bearings, shear keys, columns, and foundations) shall be considered when calculating RVD.

The relative vertical displacement at expansion joints ( $\delta_v^{EXP}$ ), depicted on Figure 12-23 shall not exceed the limits shown in Table 12-18.

Refer to Section 12.6.4.8 for additional RVD limits for track serviceability analysis.

**Figure 12-23: Relative Vertical Displacement at Expansion Joints**



**Table 12-18: Relative Vertical Displacement at Expansion Joints Limits**

Group	$\delta_v^{EXP}$ (inch)	
	Non-ballasted Track	Ballasted Track
4	0.25	0.50
5	0.50	0.75

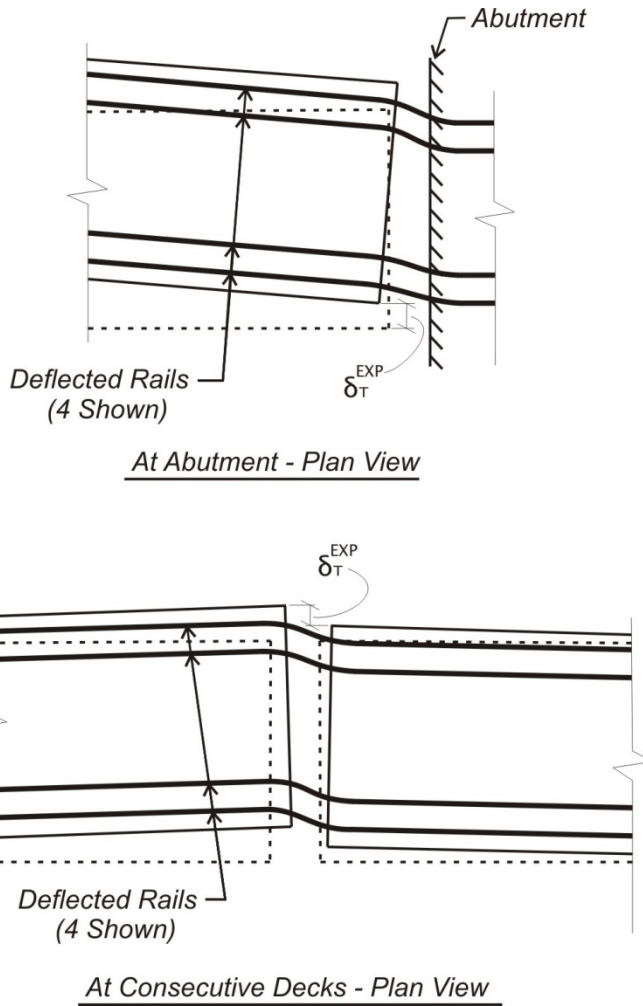
### 12.6.5.4 Relative Transverse Displacement at Expansion Joints

Relative transverse displacement (RTD) at structural expansion joints,  $\delta_T^{EXP}$ , shall be limited in order to control rail bending stress. Structural expansion joints between adjacent deck ends, and between deck ends and abutments shall be considered.

The relative transverse displacement at expansion joints ( $\delta_T^{EXP}$ ), depicted on Figure 12-24, shall not exceed the limits shown in Table 12-19.

1 Refer to Section 12.6.4.9 for additional RTD limits for track serviceability analysis.

2 **Figure 12-24: Relative Transverse Displacement at Expansion Joints**



3

**Table 12-19: Relative Transverse Displacement at Expansion Joints Limits**

Group	$\delta_T^{EXP}$ (inch)	
	Non-ballasted Track	Ballasted Track
4	0.08	0.16
5	0.16	0.24

4

**12.6.5.5 Uplift at Direct Fixation Fasteners for Non-Ballasted Track**

5 For nonballasted track under Groups 4 and 5, the fastener uplift anchorage capacity shall be  
6 designed to the factors of safety shown in Table 12-20.

**Table 12-20: Minimum Factor of Safety for Uplift on Direct Fixation Fasteners**

Group	Minimum Factor of Safety
4	2.0
5	1.33

Note: Limits apply for non-ballasted track only

Special fasteners and/or fastener anchorages may be required adjacent to structural expansion joints (SEJ) due to increased uplift demands. For each SEJ, the contractor shall identify specific fastener and anchorage details that meet the Group 4 and 5 joint demands determined from RSI analysis. Any modifications to typical fastener or anchorage details shall be defined in the TSIDAP with locations identified on the plans. Factors of safety shown in Table 12-20 shall apply.

#### 12.6.5.6 Axial Rail Stress

Axial rail stress limits were developed considering the total allowable rail stress minus rail bending stress due to vertical wheel loads, relative displacements at structural expansion joints, and minus initial axial rail stress due to rail temperature and preheat during installation (refer to the *Trackwork* chapter).

The axial rail stress limits pertain to axial only rail stresses generated by RSI.

Axial rail stress limits ( $\sigma_{\text{rail}}$ ) for rails on TSI-critical structures and adjacent abutment or at-grade regions shall be per Table 12-21.

**Table 12-21: Axial Rail Stress Limits**

Group	Range of $\sigma_{\text{rail}}$	
	Non-ballasted Track	Ballasted Track
4	$-14 \text{ ksi} \leq \sigma_{\text{rail}} \leq +14 \text{ ksi}$	$-12 \text{ ksi} \leq \sigma_{\text{rail}} \leq +14 \text{ ksi}$
5	$-23 \text{ ksi} \leq \sigma_{\text{rail}} \leq +23 \text{ ksi}$	$-21 \text{ ksi} \leq \sigma_{\text{rail}} \leq +23 \text{ ksi}$

Note: Compression = Negative (-), Tension = Positive (+)

#### 12.6.6 Dynamic Structural Analysis

Dynamic structural analysis of high-speed train passage (LLV) is required in order to determine resonancy induced dynamic impact ( $I_{\text{LLV}}$ ) effects, and limit vertical deck accelerations. Maximum dynamic amplification occurs at resonance, when the structure's natural vertical frequency coincides with the frequency of axle loading.

For all dynamic structural analysis of high-speed train passage (LLV) the flexibility of the superstructure and substructure (i.e., bearings, shear keys, columns, and foundations) shall be considered.



To avoid over or underestimating the resonant speeds, 2 conditions must be investigated:

- **Condition #1** – lower bound estimate of stiffness and upper bound estimate of mass
- **Condition #2** – upper bound estimate of stiffness and lower bound estimate of mass

Modeling requirements for lower and upper bound estimates of stiffness and mass are given in Section 12.6.8.

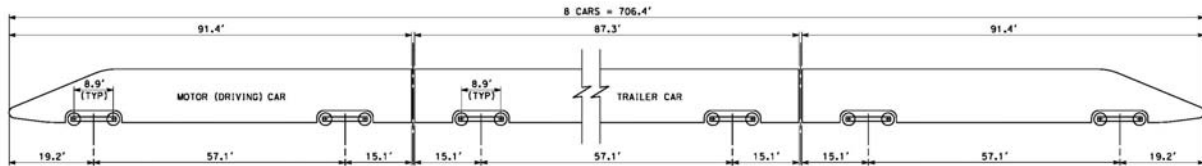
### 12.6.6.1 High-Speed Train Loading (LLV)

Dynamic structural analysis of high-speed train passage shall consider representative trainsets (LLV), idealized as a series of moving vertical loads at specified axle and truck spacings. Modeling of the train suspension system shall not be required for dynamic structural analysis.

Five trainsets, depicted on Figure 12-25 to Figure 12-29 collectively form LLV.

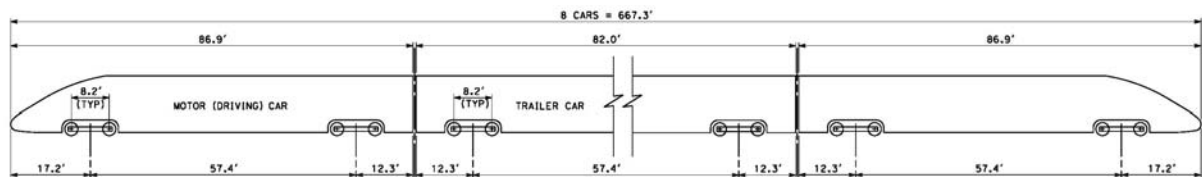
Dynamic structural analysis using all 5 trainsets shall be performed, subject to the suite of speeds given in Section 12.6.6.2.

**Figure 12-25: Type 1**



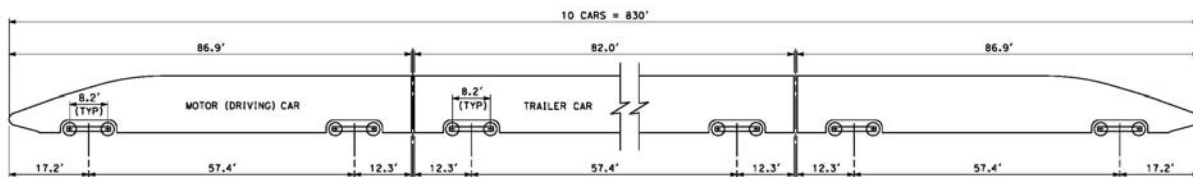
Maximum Axle Load = 18.7 tons      Train Weight (Empty) = 509 tons

**Figure 12-26: Type 2**



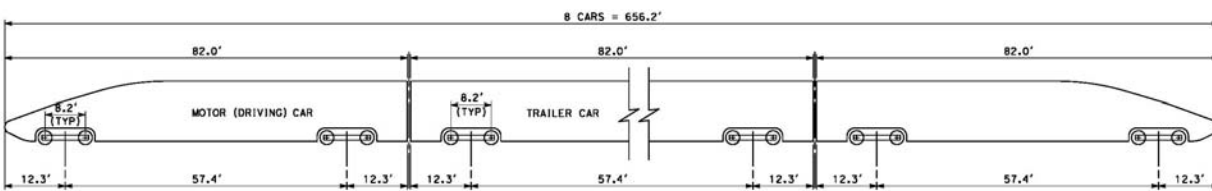
Maximum Axle Load = 16.5 tons      Train Weight (Empty) = 491 tons

**Figure 12-27: Type 3**



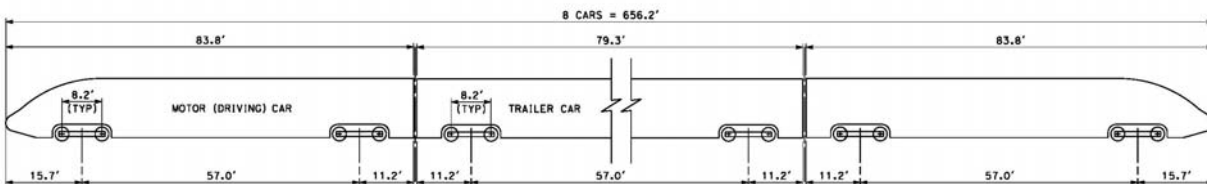
Maximum Axle Load = 12.4 tons      Train Weight (Empty) = 457 tons

**Figure 12-28: Type 4**



Maximum Axle Load = 15.4 tons      Train Weight (Empty) = 444 tons

**Figure 12-29: Type 5**



Maximum Axle Load = 18.7 tons      Train Weight (Empty) = 493 tons

#### 12.6.6.2 Train Speeds

Dynamic structural analysis using all 5 trainsets shall be performed, subject to the following suite of speeds:

- Speeds from 90 mph up to maximum speed of 1.2 times the line design speed (or 250 mph, whichever is less), by increment of 10 mph
- Smaller increments of 5 mph for  $\pm 20$  mph on each side of the first 2 resonant speeds

#### A. Resonant Speeds

For simple spans, resonant speeds may be estimated by:

$$V_i = n_o d / i,$$

Where:

$V_i$  = resonant speeds,

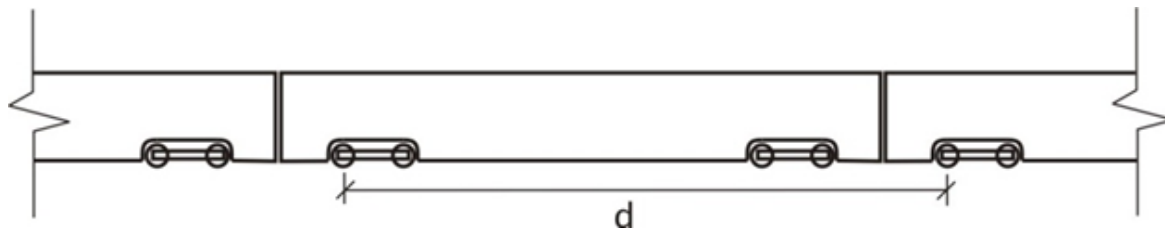
$n_o$  = first natural frequency of vertical deflection

$d$  = characteristic wheel spacing, refer to Figure 12-30

$i$  = resonant mode numbers (e.g., 1, 2, 3, 4, ...)

For structures not consisting of simple spans, resonant speeds shall be determined by the dynamic structural analysis model.

**Figure 12-30: Characteristic Wheel Spacing,  $d$**



#### B. Cancellation Speeds

In addition to resonance, cancellation effects also contribute to the overall dynamic response of elevated structures. For simple spans, cancellation speeds may be estimated by:

$$V_i = \frac{2n_o L}{2i - 1},$$

Where:

$V_i$  = cancellation speeds,

$n_o$  = first natural frequency of vertical deflection

$L$  = simple span length

$i$  = cancellation mode numbers (e.g., 1, 2, 3, 4, ...)

When  $L/d = 1.5$ , an optimal design condition exists for which the first mode of resonance aligns with the second mode of cancellation. In this condition, the primary dynamic residual response generated by repeated axle loads can be suppressed. Due to uncertainties associated with the service life of the structure, it may be unrealistic to design a given structure solely for a single characteristic wheel spacing. Nevertheless, optimal span lengths for potential trainsets shall be considered for design.

For non-simple span structures, the interaction between resonant and cancellation speeds may not be readily apparent and shall be investigated by a more detailed dynamic structural analysis.

### **12.6.6.3 Dynamic Vertical Impact Effects**

For the high-speed trainsets (LLV), the dynamic model shall be used to determine the dynamic impact effect ( $I_{LLV}$ ).

To determine ( $I_{LLV}$ ), the maximum dynamic response value,  $\xi_{dyn}$  shall be found for each structural response for single track loading (LLV) over the range of speeds given in Section 12.6.6.2.

Compared against the corresponding static response value,  $\xi_{stat}$ , the dynamic impact effect is:

$$I_{LLV} = \max \left[ \frac{\xi_{dyn}}{\xi_{stat}} \right]$$

### **12.6.6.4 Vertical Deck Acceleration**

Vertical accelerations of TSI-critical structure decks are limited to avoid unsafe wheel-rail contact, and also to minimize passenger discomfort.

When evaluating vertical deck accelerations, an upper bound estimate of stiffness and lower bound estimate of mass shall be considered.

Vertical acceleration of TSI-critical structure decks shall be found for single track loading (LLV) over the range of train speeds given in Section 12.6.6.2. The vertical deck acceleration shall be monitored at the centerline of the loaded track.

The vertical deck acceleration shall be limited to:

- +/- 16.1 ft/s<sup>2</sup> (0.50g) for non-ballasted track
- +/- 11.3 ft/s<sup>2</sup> (0.35g) for ballasted track

For acceleration limits to be experienced within the train car body, refer to Section 12.6.7.

### **12.6.7 Dynamic Vehicle-Track-Structure Interaction Analysis**

For typical structures, limiting the span deflections, relative displacements between spans, expansion joint widths, rail stress, and deck acceleration provides sufficient guidance for track safety and passenger comfort.

A design variance shall be required for TSI-critical structures exceeding any deformation, rail stress, or acceleration limits within Sections 12.6.4, 12.6.5, and 12.6.6, or TSI-critical structures that depart from current service proven design concepts as determined by the Authority during approval of the Track-Structure Interaction Design and Analysis Plan (TSIDAP) per Section

12.6.1. A dynamic vehicle-track-structure interaction (VTSI) analysis, which considers the interaction between the vehicle, track and structure, may be required as part of conditional approval for the design variance.

When a dynamic VTSI analysis is required, the Contractor shall submit a Vehicle-Track-Structure Interaction Design and Analysis Plan (VTSIDAP) for approval by the Authority. The VTSIDAP shall provide the following detailed information regarding the analysis approach:

- The vehicle models to be used – including mass, stiffness, and damping characteristics of the wheels, trucks, suspension, and body
- The number of trainsets, speeds, and number of cars used for the purpose of analysis
- The approach to be used to generate random track irregularities consistent for the appropriate FRA Track class
- The structural definition, including model boundaries and representation of adjacent at-grade track
- The track properties considered, including rail section, fastener, and ballast properties as applicable
- The method used to couple the dynamic train system with the dynamic structure system, including modeling of wheel-rail contact
- The method used to monitor wheel-rail contact forces and carbody accelerations

Additional information for VTSI analysis may be required, as determined by the Authority.

#### **12.6.7.1 Dynamic Vehicle-Track-Structure Interaction Analysis Requirements**

For dynamic VTSI, both a dynamic structural model and dynamic trainset models shall be used. The interaction of the structure and trainset models shall be considered in either a coupled or iterative method.

Details of structural modeling requirements are given in Section 12.6.8.

Due to uncertainty of trainset selection, multiple trainset models shall be proposed for dynamic VTSI. Each of the dynamic trainset models shall be consistent with characteristic loading of LLV trainsets as defined in Section 12.6.6.1, and consider the mass, stiffness, and damping characteristics of the wheels, trucks, suspension, and body.

It is known that vehicle response is highly sensitive to track irregularities. For dynamic VTSI analysis, random track irregularities shall be considered directly within the VTSI model. Random theoretical irregularities shall be developed for FRA Track Classes using a power spectral density function that may be distributed into the time domain by applying the spectral representation method.

Dynamic VTSI analysis shall consider a series of speeds ranging from a minimum of 90 mph up to maximum speed of 1.2 times the line design speed (or 250 mph, whichever is less). Refer to Section 12.6.6.2 for train speed increment requirements.

Dynamic VTSI analysis shall consider single track (i.e., 1 trainset) loading only.

For the dynamic VTSI analysis, a sufficient number of cars shall be used to produce maximum load effects in the longest span of the structure. In addition, a sufficient number of spans within a long aerial structure shall be considered to initiate any resonance effects in the train suspension.

### 12.6.7.2 Dynamic Track Safety Criteria

Track safety depends primarily upon the contact forces between the rail and the wheel. The ratio of lateral to vertical forces (L/V ratio) is typically used as the primary indicator of derailment. In addition, the magnitude of lateral and vertical forces imparted by the wheel to the rail must be controlled.

During dynamic VTSI analysis, the dynamic track safety limits shown in Table 12-22 shall be satisfied for all trainsets and speeds.

**Table 12-22: Dynamic Track Safety Limits**

Parameter	Dynamic Track Safety Criteria
Maximum Single Wheel L/V Ratio	$(L/V)_{\text{wheel}} \leq 0.80$
Maximum Truck Side L/V Ratio	$(L/V)_{\text{truck side}} \leq 0.6$
Minimum Single Wheel Dynamic Vertical Load	$V_{\text{wheel,dynamic}} \geq 0.15 \cdot V_{\text{wheel,static}}$
Maximum Net Axle Dynamic Lateral Force	$L_{\text{axle,dynamic}} \leq 0.40 \cdot V_{\text{axle,static}} + 5 \text{ kips}$

Where:

$(L/V)_{\text{wheel}}$  = Ratio of lateral forces to vertical forces exerted by a single wheel on the rail

$(L/V)_{\text{truck side}}$  = Ratio of lateral forces to vertical forces exerted by any 1 side of a truck on the rail

$V_{\text{wheel,dynamic}}$  = Dynamic vertical wheel reaction

$V_{\text{wheel,static}}$  = Static vertical wheel load

$L_{\text{axle,dynamic}}$  = Dynamic lateral axle reaction

$V_{\text{axle,static}}$  = Static vertical axle load

### 12.6.7.3 Dynamic Passenger Comfort Criteria

Passenger comfort depends primarily upon the accelerations experienced by passengers within the train car body during travel on and off TSI-critical structures.

- 1 During dynamic VTSI analysis, the lateral acceleration within the car body is limited to +/-  
2 1.6 ft/s<sup>2</sup> (0.05 g) for all trainsets and speeds.
- 3 The vertical acceleration within the car body is limited to +/- 1.45 ft/s<sup>2</sup> (0.045 g) for any trainset  
4 across the required speed range.

### 12.6.8 Modeling Requirements

- 5 The following modeling requirements for static and dynamic analysis of high-speed train TSI-  
6 critical structures shall be used for project-wide consistency.

#### 12.6.8.1 Model Geometry and Boundary Conditions

- 7 The model shall represent the TSI-critical structure span lengths, vertical and horizontal  
8 geometries, column heights, mass and stiffness distribution, bearings, shear keys, column or  
9 abutment supports, and foundation conditions.

- 10 For isolated TSI-critical structures, with no adjacent structures, the model shall represent the  
11 entire structure including abutment support conditions.

- 12 For TSI-critical structures with repetitive simply supported spans, the model shall have a  
13 minimum of 20 spans. Boundary conditions at the ends of the model shall represent the stiffness  
14 of any adjacent spans or frames.

- 15 For TSI-critical structures with repetitive continuous span frames (i.e., each frame consists of  
16 multiple spans with moment transfer between the deck and columns), the model shall have a  
17 minimum of 5 frames. Boundary conditions at the ends of the model shall represent the stiffness  
18 of adjacent spans or frames.

- 19 Soil springs at the foundations shall be developed based on reports required in the *Geotechnical*  
20 chapter.

- 21 For modeling of earthen embankments or cuts at bridge approaches, refer to Section 12.6.8.7.

#### 12.6.8.2 Model Stiffness

- 22 Structural elements shall be represented by the appropriate sectional properties and material  
23 properties.

- 24 For frequency analysis, dynamic structural analysis, and dynamic VTSI analysis, both upper  
25 and lower bound estimates of stiffness shall be considered.

- 26 For track serviceability and RSI analysis, a lower bound estimate of stiffness shall be considered.

- 27 For steel superstructure and steel column members, the following shall apply:

- 28 • Upper bound stiffness: full steel cross sectional properties, and expected material properties  
29 (larger than nominal specified per AASHTO LRFD BDS with California Amendments) shall  
30 be used.

- Lower bound stiffness: reduced steel cross sectional properties considering shear lag effects if necessary, and nominal material properties shall be used.

For reinforced, pre-stressed, and post-tensioned concrete superstructure members, the following shall apply:

- Upper bound stiffness: full gross bending inertia,  $I_g$ , and modulus of elasticity corresponding to expected material properties (1.3x nominal) per CSDC shall be used. Consideration shall be made for composite action of the superstructure with non-ballasted track, and barriers or derailment walls when determining upper bound bending inertias.
- Lower bound stiffness: effective bending inertia,  $I_{eff}$ , per CSDC, and modulus of elasticity corresponding to nominal material properties shall be used.

For concrete column members, the following shall apply:

- Upper bound stiffness: full gross bending inertia,  $I_g$ , and modulus of elasticity corresponding to expected material properties (1.3x nominal) per CSDC shall be used.
- Lower bound stiffness: cracked bending inertia,  $I_{cr}$ , per CSDC, and modulus of elasticity corresponding to nominal material properties shall be used.

As an alternative to using  $I_{cr}$  per CSDC, an effective bending inertia,  $I_{eff}$ , which considers the maximum moment demand,  $M_a$ , and the cracking moment,  $M_{cr}$ , may be used in accordance with AASHTO LRFD BDS with California Amendments. Also, a moment-curvature representation of the column stiffness may be used.

#### **12.6.8.3 Model Mass**

For frequency analysis, dynamic structural analysis, and dynamic VTSI analysis, both upper and lower bound estimates of bridge mass shall be considered.

For track serviceability and RSI analysis, an upper bound estimate of bridge mass shall be considered.

For structural dead load (DC) mass, the material unit weights per Section 12.5 shall be used as the basis for design. For the upper bound mass estimate, unit weights shall be increased by a minimum of 5 percent. For the lower bound mass estimate, unit weights shall be reduced by a minimum of 5 percent.

For superimposed dead load (DW), upper and lower bound mass estimates shall be considered on case by case basis with a minimum of +/-5 percent.

#### **12.6.8.4 Model Damping**

When performing OBE time history analyses for track serviceability and rail-structure interaction analysis, damping shall be used (per the *Seismic* chapter).



- 1 When performing dynamic structural analysis, the peak structural response at resonant speed is  
2 highly dependent upon damping. The damping values in Table 12-23 shall be used.

**Table 12-23: Damping Values for Dynamic Model**

Bridge Type	Percent of Critical Damping
Steel and composite	0.5%
Pre-stressed, post-tensioned concrete	1.0%
Reinforced concrete	1.5%

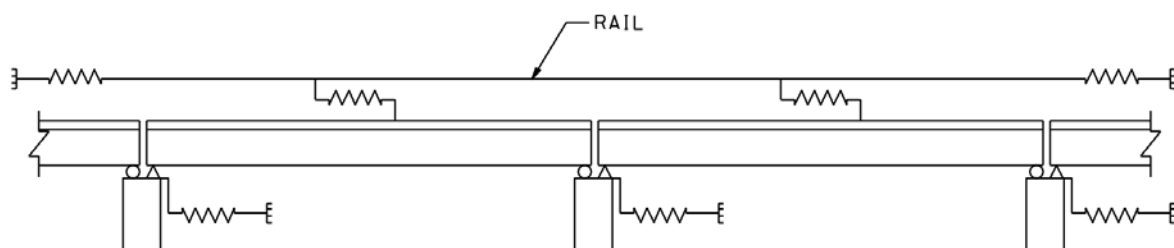
- 3 The damping may be increased for shorter spans (< 65 feet), with supporting evidence to be  
4 provided by the Contractor as part of the TSIDAP per Section 12.6.1.

- 5 When performing dynamic structural analysis using actual LLV, soil damping shall be  
6 considered in accordance with the geotechnical reports described in the *Geotechnical* chapter.

#### 12.6.8.5 Modeling of Rail-Structure Interaction

- 7 Longitudinal actions produce longitudinal forces in continuous rails. These forces are  
8 distributed to the TSI-critical structures in accordance with the relative stiffness of the track and  
9 fasteners, articulation of the structural system, and stiffness of the substructure. Refer to Figure  
10 12-31 for a schematic rail-structure interaction model.

**Figure 12-31: Rail-Structure Interaction Model**



- 13 Rail-structure interaction (RSI) may govern the following:

- 14 • Location and distance between bridge expansion joints
- 15 • Stiffness of the bridge superstructure
- 16 • Stiffness of the supporting columns and foundations

- 17 RSI shall be performed for all structures using either static or dynamic models. In addition, the  
18 model shall, at a minimum, include the axial stiffness of the rails appropriately located upon the  
19 superstructure, and longitudinal bi-linear coupling springs between the track and  
20 superstructure over the length of the model.

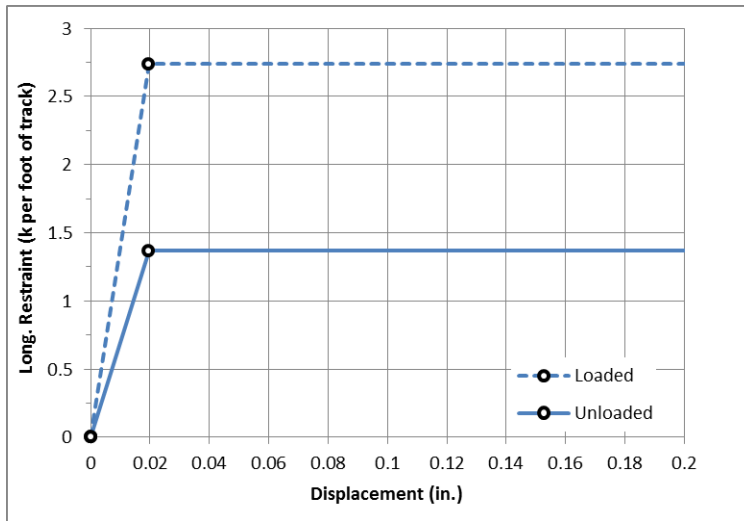
- 1 For purposes of RSI analysis, the continuous welded rail section properties shall be per Table
- 2 12-24.
- 3 The rail section used for analysis shall not be construed as a requirement for track design or
- 4 track construction. Refer to the *Trackwork* chapter for rail section requirements.

**Table 12-24: Rail Section Properties for RSI Analysis**

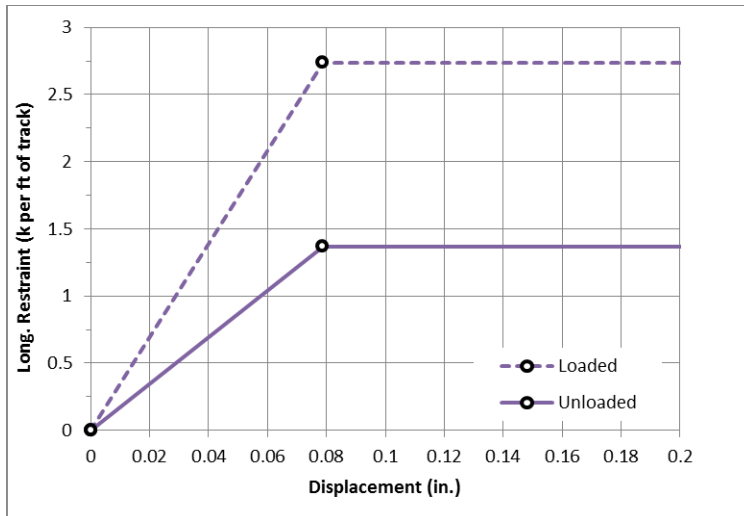
Property	Metric units	US units
Mass per meter:	60.21 kg/m	121.4 lb/yd
Cross-sectional area:	76.70 cm <sup>2</sup>	11.89 in <sup>2</sup>
Moment of inertia x-x axis:	3038.3 cm <sup>4</sup>	73.00 in <sup>4</sup>
Section modulus – Head:	333.6 cm <sup>3</sup>	20.36 in <sup>3</sup>
Section modulus – Base:	375.5 cm <sup>3</sup>	22.91 in <sup>3</sup>
Moment of inertia y-y axis:	512.3 cm <sup>4</sup>	12.31 in <sup>4</sup>
Section modulus y-y axis:	68.3 cm <sup>3</sup>	4.17 in <sup>3</sup>

- 5 The track type (non-ballasted or ballasted) and corresponding fasteners for analysis are to be
- 6 determined as part of the approved TSIDAP per Section 12.6.1.
- 7 Fastener restraint is non-linear, allowing slippage of the rail relative to the track support
- 8 structure. Bi-linear coupling springs shall represent non-ballasted track with direct fixation
- 9 fasteners (refer to Figure 12-32) or ballasted track with concrete ties and elastic fasteners (refer
- 10 to Figure 12-33) between the rails and superstructure on a per track (i.e., 2 rail) basis. The non-
- 11 ballasted relationship represents a pair of fasteners each with 1.54 kip (6.85 kN) unloaded
- 12 longitudinal restraint at 27-inch spacing. The ballasted relationship represents a pair of fasteners
- 13 on a concrete tie each with 1.54 kip (6.85 kN) unloaded longitudinal restraint at 27-inch tie
- 14 spacing. In each case, the longitudinal restraint is 1.37 k (unloaded) per foot of track and 2.7 k
- 15 (loaded) per foot of track. The yield displacement varies from 0.02" (non-ballasted) to 0.08"
- 16 (ballasted).

**Figure 12-32: Non-Ballasted Track with Direct Fixation Fasteners: Bi-linear Coupling Springs**



**Figure 12-33: Ballasted Track with Concrete Ties and Elastic Fasteners: Bi-linear Coupling Springs**



In practice, variations in fastener/tie spacing may be required to accommodate structural expansion joints, deck skew, or other geometric constraints.

Uniform longitudinal restraint shall be verified using the following uniformity criteria:

- Distributed longitudinal restraint calculated for fastener locations over any 10 foot length of track along the structure shall be within +/-20 percent of the assumed uniform bi-linear coupling relation.

For TSI-critical structures that meet the uniformity criteria, but are designed assuming longitudinal restraints not consistent with either Figure 12-32 or Figure 12-33, the structure shall

be considered to have a nonstandard fastener configuration (NSFC). These structures require an approved design variance and special RSI analysis per Section 12.6.8.6.

For TSI-critical structures that do not meet the uniformity criteria, the structure shall be considered to have a non-uniform fastener configuration (NUFC). These structures require an approved design variance and a special RSI analysis per Section 12.6.8.6.

The total number of longitudinal bi-linear coupling springs per each span shall not be less than 10 and the spacing between the springs shall not be more than 10 feet.

For vertical and lateral (i.e., transverse) stiffness of fasteners, defined as per foot of track (pair of rails) the following properties shall be used as applicable:

- Non-ballasted track:

- Vertical compressive stiffness: 4100 k/ft per foot of track
- Vertical tensile (uplift) stiffness: 75 to 4100 k/ft per foot of track
- Lateral Stiffness: 420 k/ft per foot of track

- Ballasted track:

- Vertical compressive stiffness: 2100 k/ft per foot of track
- Vertical tensile (uplift) stiffness: 2.8 k/ft to 2100 k/ft per foot of track
- Lateral Stiffness: 420 k/ft per foot of track

In accordance with above, assumed vertical fastener stiffness (compression and tension) as well as assumed lateral fastener stiffness shall be shown on the plans.

As a means to meet RSI criteria per Section 12.6.5, the Contractor may propose alternative track solutions (e.g., NSFC, NUFC, Rail Expansion Joints) through the design variance approval process. The design variance shall be supplemented with a special RSI analysis per Section 12.6.8.6.

#### **12.6.8.6 Special Rail-Structure Interaction Analysis**

RSI limits in Section 12.6.5 are developed considering typical fastener configurations on typical structures. For those systems that do not meet these assumptions, new limits shall be developed using a refined analysis.

A special RSI analysis shall be required for those structure and track designs requiring a design variance related to Section 12.6.5. Specific design variances requiring special RSI analysis include, but are not limited to: designs requiring nonstandard fastener configurations (NSFC), non-uniform fastener configurations (NUFC), structures with thermal units ( $L_{TU}$ ) greater than 330 feet, and rail expansion joints (REJs).

The Contractor shall identify and document structure types requiring special RSI analysis as part of the Type Selection process described in Section 12.8.1.1. After completion of Type

Selection and upon determination that the selected structure type requires a special RSI analysis, the Contractor shall develop a Rail-Structure Interaction Design and Analysis Plan (RSIDAP) as part of the design variance submittal. The RSIDAP shall formally identify elements requiring special consideration, including but not limited to: refined fastener properties, detailed temperature analysis, refined ballast/non-ballasted properties, and rail expansion joint locations. A detailed proposal of analysis procedures used to verify track performance (including track safety, passenger comfort, track maintenance, and rail stress) shall be submitted as part of the RSIDAP.

Examples of special analysis required may include, but are not limited to: development of new RSI limits, development of new analytical model elements, local rail stress modeling, site-specific temperature analysis, analysis of impacts to track maintenance, etc.

#### 12.6.8.7 Modeling of Rail-Structure Interaction at Model Boundaries

Where an abutment occurs at the ends of TSI-critical structures, the rails and bi-linear coupling springs shall be extended a distance of  $L_{ext}$  from the face of the abutment. At the model boundary (i.e., at  $L_{ext}$  from abutment), a horizontal boundary spring representing the rail/fastener system behavior shall be used. The boundary spring, which represents unloaded track, shall be elastic-perfectly plastic, with an elastic spring constant of  $k$  (in units of k/feet) yielding at  $P_b$  (in units of kips), which represents the maximum capacity of an infinite number of elastic fasteners.

The yielding of the boundary spring at  $P_b$  is a threshold value that shall be checked throughout the RSI analysis. If at any point during the analysis the boundary spring yields at force  $P_b$ ,  $L_{ext}$  should be increased and the analysis should be repeated until elastic boundary spring behavior is verified.

The boundary spring behavior depends on the type of track. Values of  $k$ ,  $P_b$ , and  $L_{ext}$  are shown for non-ballasted and ballasted track types in Table 12-25. Note that the minimum recommended values of  $L_{ext}$  are dependent on the average span length of the TSI-critical structures (denoted  $L_{avg}$ ):

$$L_{avg} = \frac{(L_1 + L_2 + \dots + L_n)}{n} = \text{the average span length}$$

**Table 12-25: Minimum Recommended Track Extension and Boundary Spring Properties**

Non-Ballasted Track (fasteners yield at 0.02 inches) with EN 60 E 1 rail			
Yield Load per foot of non-ballasted track	k (kips/ft)	P <sub>b</sub> (kips)	Min. Recommended L <sub>ext</sub> (feet)
1.37 kips/ft of track [1.54 kips (6.85 kN) fasteners @ 27" o.c.]	23,800	39.7	0.1L <sub>avg</sub> + 325
Ballasted Track (fasteners yield at 0.08 inches) with EN 60 E 1 rail			
Yield Load per foot of ballasted track	k (kips/ft)	P <sub>b</sub> (kips)	Min. Recommended L <sub>ext</sub> (feet)
1.37 kips/ft of track [1.54 kips (6.85 kN) fasteners @ 27" o.c.]	11,900	79.5	0.1L <sub>avg</sub> + 300

In the event that an additional bridge or other elevated structure is located within the L<sub>ext</sub> model boundary distance from the face of an earthen abutment, the additional structure (including the loads and modeling requirements presented in this section) shall also be included in the RSI analysis model.

The assumptions used to develop Table 12-25 were expected to apply to the majority of TSI-critical structures, which are assumed to be in simply-supported configuration with uniform distribution of fasteners. Where a special rail-structure interaction analysis is required per Section 12.6.8.6, additional investigation shall be required to appropriately define the model boundary.

#### 12.6.8.8 Modeling of At-Grade Track for TSI-Critical Structures

Typically, TSI-critical structures, such as bridges, aerial structures or grade separations, interface with at-grade track upon earthen embankments or cuts at abutment regions. Requirements and guidelines for modeling the at-grade track and abutment regions are provided below:

- For RSI Section 12.6.5.1 Groups 4 and 5 load cases, the vertical and lateral stiffness of non-ballasted or ballasted track upon earthen embankments or cuts shall be considered to accurately predict relative displacements at abutment expansion joints, and rail stress at the abutment and at-grade regions.
- The modeling of earthen embankments or cuts is not required for track serviceability (Section 12.6.4) or dynamic structural analysis (Section 12.6.6). However, if earthen embankments or cuts are modeled, the vertical and lateral stiffness of non-ballasted or ballasted track upon earthen embankments or cuts shall be considered.
- For dynamic VTSI analysis (Section 12.6.7) the vertical and lateral stiffness of non-ballasted or ballasted track upon earthen embankments or cuts shall be considered to accurately predict the wheel-rail contact forces and carbody accelerations when the vehicle passes through transition zones located between the elevated structure and at-grade regions.

- For vertical stiffness of at-grade track, consideration of the deformation moduli of second loading of a plate load test shall be made in accordance with the geotechnical reports required in the *Geotechnical* chapter.
- For lateral (i.e., longitudinal and transverse) stiffness of track upon earthen embankments or cuts, consideration of embankment flexibility, non-ballasted track or ballast tie embedment, passive pressure, and friction shall be made in accordance with the geotechnical reports required in the *Geotechnical* chapter.
- OBE ground motions shall be applied concurrently at structural foundations and earthen embankments or cuts to capture the effects between the vibrating structure and the relatively stationary track upon earthen embankment or cut. For design of earthen structures, lag times and/or amplification effects shall be considered for OBE ground motions in accordance with the geotechnical reports required in the *Geotechnical* chapter.

Under certain conditions, the performance of some TSI-critical structures may be highly sensitive to the modeling of at-grade track stiffness properties. Such conditions include, but are not limited to: TSI-critical culverts with limited cover, structures supporting complex transition slabs between at-grade and aerial structures, and short stiff embankments serving as transition between two aerial structures. Such conditions shall be considered on a case-by-case basis by the Contractor subject to approval by the Authority. A detailed modeling approach shall be provided as part of the TSIDAP per Section 12.6.1.

## **12.7 Structural Design of Stations, Surface Facilities, Buildings and Ancillary Structures**

The static design of Primary Type 2 and Secondary stations, surface facilities, buildings and ancillary structures shall conform to the requirements of the CBC, with supplementary provisions herein. For the static design of Primary Type 1 stations, surface facilities, buildings and ancillary structures, refer to Section 12.5.

For the seismic design of Primary Type 2 and Secondary stations, surface facilities, buildings and ancillary structures, refer to the *Seismic* chapter.

For foundation design, refer to the requirements in the *Geotechnical* chapter.

### **12.7.1 Load Requirements for Stations, Surface Facilities, Buildings, and Ancillary Structures**

Primary Type 2 and Secondary stations, surface facilities, buildings and ancillary structures shall conform to the requirements of the CBC, with supplementary provisions herein.

#### **12.7.1.1 Dead Load and Superimposed Dead Load**

Dead load and superimposed dead load shall include but not be limited to the following:

- 1 • Dead weight of structural members and architectural finishes
- 2 • Dead weight of road surface and of backfill above the structures
- 3 • Dead weight of surcharge loads
- 4 • Dead weight of equipment and appurtenances
- 5 Refer to Section 12.5 for the unit weights of materials.

#### **12.7.1.2 Train Load**

- 6 Refer to Section 12.5 for train loading.

#### **12.7.1.3 Roof Load**

- 7 Roof live load and reduction factors shall be in accordance with the CBC.

#### **12.7.1.4 Floor Load**

- 8 Floor live load, including parking structures, shall be in accordance with the CBC with no
- 9 reduction in floor live load.
- 10 Station platforms and concourse areas shall be designed for a floor live load of 100 psf.
- 11 Emergency and maintenance walkways shall be designed for a floor live load of 100 psf.
- 12 Floor live loads on service walkways and sidewalks shall be designed for a live load of 100 psf,
- 13 or a concentrated load of 2,000 pounds applied anywhere on the walkway and distributed over
- 14 a 4 feet by 2 feet area.
- 15 The structural system supporting access doors at street level shall be designed for a floor live
- 16 load of 350 psf.
- 17 Storage area floor live loads shall be 100 psf.
- 18 Areas where cash carts are used shall be designed to accommodate a point live load of
- 19 350 pounds per wheel. Wherever station configuration requires that cash carts cross pedestrian
- 20 bridges, bridges shall be designed to accommodate this live load.
- 21 Operations Control Centers shall be designed for a floor live load of 100 psf.
- 22 Equipment room floors for such uses as signals, communications, power, transformers, battery
- 23 storage and fan rooms shall be designed for a floor live load of 350 psf, and a 2,000 pound
- 24 concentrated load (or the actual equipment weight if known) located to produce the maximum
- 25 load effects in the structural members.
- 26 Pump rooms, service rooms, storage space, and machinery rooms shall be designed for floor
- 27 live load of 250 psf, to be increased if storage or machinery loads so dictate.



1 Stairways shall be designed for a floor live load of 100 psf or a concentrated load of 300 pounds  
2 on the center of stair treads, whichever is critical.

3 Maintenance buildings will require overhead cranes and crane rails or floor mounted hydraulic  
4 jacks to lift individual cars from trains. The car loads are unknown until a vehicle is selected.  
5 The Designer shall coordinate with the Authority to obtain design requirements for crane  
6 design.

#### 12.7.1.5 Vehicular Load

7 Parking areas for automobiles shall be designed to the load as specified in the CBC. Structures  
8 supporting buses shall be designed to carry HL-93 loading in accordance with AASHTO LRFD  
9 BDS with California Amendments.

10 Gratings in areas that are subject to vehicular loading shall be designed to carry HL-93 loading.

#### 12.7.1.6 Miscellaneous Loads

11 Stationary and hinged cover assemblies internal to HST facilities shall be designed for a  
12 minimum uniform live load of 100 psf or a concentrated live load of 1,000 pounds over a 2 feet  
13 by 2 feet area. Deflection at center of span under 100 psf load shall not be more than 1/8 inch.

14 Gratings in sidewalks and in areas protected from vehicular traffic shall be designed for a  
15 uniform live load (LL) of 300 psf.

#### 12.7.1.7 Slipstream Effects from Passing Trains

16 Refer to Section 12.5.2.7 for slipstream effects from passing high-speed trains.

17 Where structural elements can also be subjected to wind load, loading due to the slipstream  
18 effects from passing trains shall be considered to occur in combination with wind load. For  
19 Primary Type 2 and Secondary structures, refer to CBC for load combinations. For Primary  
20 Type 1 structures, refer to Table 12-4 for load combinations.

21 Where trains are enclosed between walls and with a ceiling and deck, the design requirements  
22 for tunnels shall be met (refer to the *Tunnels* chapter) including the following:

- 23 • Minimum cross section area of the through trackway
- 24 • Evacuation
- 25 • Fire/Life Safety
- 26 • Medical Health Criteria

27 In addition, transient air pressure analyses (as in a tunnel ventilation analysis) shall be used to  
28 determine the maximum transient air pressure acting on the walls and ceiling. These pressures  
29 shall be used for design of those elements such as uplift of ceilings or lateral pressure on walls  
30 and doors.

### 12.7.1.8 Collision Loads in Stations

Columns in stations shall be classified into 3 groups, according to the following criteria:

**GROUP A** – This group consists of columns where the clearance measured from the TCL to face column is no less than 16.5 feet. No collision impact forces shall be applied.

**GROUP B** – GROUP B columns are those located in a row of columns that run adjacent and parallel to the HST track and that do not meet the criteria of GROUP A. Columns in the row are classified as GROUP B, with the exception of the first and last columns (see GROUP C below). The column row shall include a column protection wall throughout its length. The performance of column and protection wall to this loading shall be a no collapse requirement.

- The column protection wall shall comprise a lower guide wall together with an upper guide beam integrated to the columns as shown on Figure 12-34. Due to the presence of the column protection wall, the GROUP B columns need not withstand full face collisions, but only grazing impacts by trains that have already derailed. The lower guide wall and the upper guide beam shall be designed to withstand collision impact loads.

- Columns and column protection walls shall be designed for 1 of the following horizontal collision impact loads, whichever produces the most adverse effect:

- Columns shall be designed to resist a 900 kip force parallel with the TCL acting together with a 350 kip force at 90 degrees to the TCL, both 4 feet above low rail level and 225 kip force at 90 degrees to the TCL, 10 feet above TOR.

- Lower guide wall shall be designed to resist a 900 kip force parallel with the TCL acting together with a 350 kip force at 90 degrees to the TCL, both 4 feet above top of low rail.

- Upper guide beam shall be designed to resist a 225 kip force at 90 degrees to the TCL, acting 10 feet above top of low rail.

**GROUP C** – GROUP C consists of the first and last columns in a row that do not belong to Group A or Group B.

- The collision loads for GROUP C columns, as indicated above, are as follows:

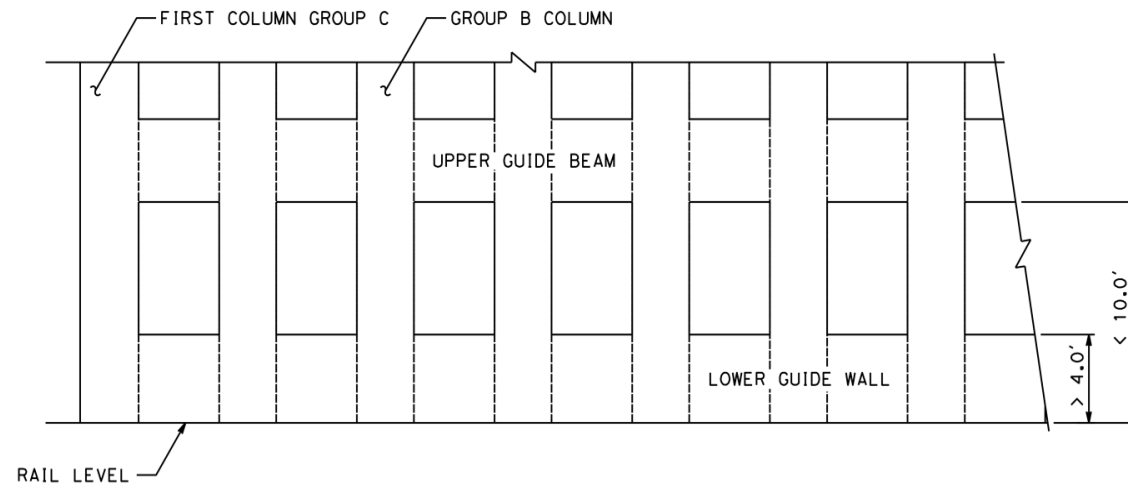
- Columns shall be designed for 1 of the following horizontal collision impact loads, whichever produces the most adverse effect. The performance for this loading is a no collapse requirement. A 2250 kip force parallel with the TCL acting together with an 800 kip force at 90 degrees to the TCL, both acting 4 feet above top of low rail

- A 225 kip force at 90 degrees to the TCL, acting 10 feet above low rail level

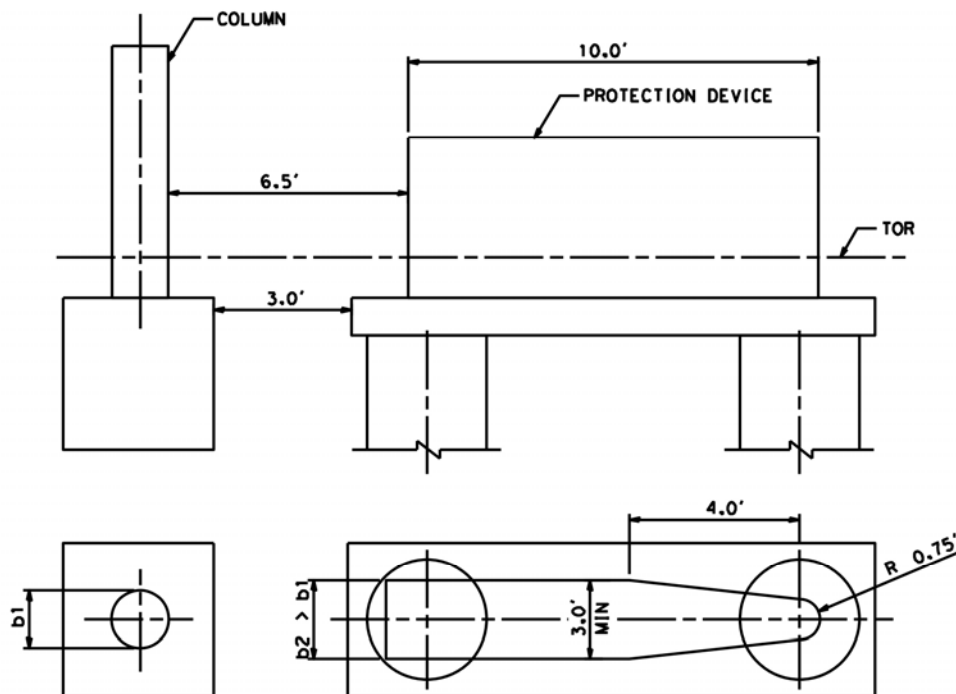
Alternatively, a protection device designed to resist the GROUP C impact loads shall be provided at the open face of the column as shown in Figure 12-35. The column in this figure shall be designed for the GROUP B column impact loads.

Collision loads shall be combined as shown in Table 12-4 as an Extreme Load combination.

**Figure 12-34: Collision Loads for Each Group of Columns**



**Figure 12-35: Protection Device**



#### 12.7.1.9 Collision Loads on Platforms

Platforms shall be designed to withstand a horizontal collision impact load of 225 kips applied at 90 degrees to the TCL of the nearest track located anywhere along the platform.

A 1-foot-wide void shall be provided around columns that are within platform areas to prevent transfer of collision loads to the column.

#### 12.7.1.10 Wind Loads

Wind loads including both windward and leeward sides of buildings and other structures shall be in accordance with the provisions of CBC, with  $I_w = 1.15$ .

#### 12.7.1.11 Effects of Temperature, Shrinkage and Creep

Effects of temperature, shrinkage, and creep shall be considered for structures above ground, as per requirements of the CBC.

#### 12.7.1.12 Frequency and Vibration Limits

Primary Type 1 station structures, Primary Type 2 station structures and Primary Type 2 pedestrian bridges shall be designed to meet the following requirements for pedestrian comfort:

- The comfort criteria shall be defined in terms of acceptable acceleration of any part of the station platform or deck occupied by the public. The extreme acceleration demands of any part of the station platform or deck shall not exceed the following limits:

- +/- 2.3 ft/s<sup>2</sup> for vertical vibrations
- +/- 0.7 ft/s<sup>2</sup> for lateral vibrations
- +/- 1.3 ft/s<sup>2</sup> for crowd condition vertical vibrations

For Primary Type 2 station structures and Primary Type 2 pedestrian bridges, dynamic pedestrian load models specified in Section 12.7.1.13 shall be applied on the station or pedestrian structure to determine the extreme vertical and lateral acceleration demands.

For Primary Type 1 station structures, dynamic pedestrian load models specified in Section 12.7.1.13 shall be applied on the station structure and the high-speed train loading (LLV) specified in Section 12.6.6.1 shall be considered simultaneously on the track structure to determine the extreme vertical and lateral acceleration demands.

- A verification of the comfort criteria need not be performed if the fundamental frequency of the station platform or deck is greater than the following:
  - 5 Hz for vertical vibrations
  - 2.5 Hz for horizontal (lateral) and torsional vibrations. Transverse frequency analysis shall consider the flexibility of superstructure only, excluding the flexibility of bearings, columns, and foundations, assuming the supports at the ends of the span are rigid.

#### 12.7.1.13 Dynamic Pedestrian Load Models for Comfort Criteria

Dynamic pedestrian load models specified herein shall be used to perform the dynamic analysis of Primary Type 1 station structures, Primary Type 2 station structures and Primary Type 2 pedestrian bridges. Damping values specified in Table 12-23 shall be used. Refer to the *Seismic* chapter for provisions of determining Rayleigh damping for time-history analyses.

### A. Vertical Concentrated Load

For verification of vertical vibrations, a vertical concentrated load,  $F_{p1}$  in kips as specified in Equation 12.7.1.13-1, shall be used to perform the dynamic analysis. The vertical concentrated load shall be moved across any horizontal direction on the structure at a constant speed of 5.6 fps to capture the extreme vertical acceleration demands of any part of the station platform or deck. This load shall be applied concurrently with the lateral concentrated load specified in Equation 12.7.1.13-2 and may be distributed over a 7.5 feet by 7.5 feet area. The portion of static load in Equation 12.7.1.13-1 (2.9 kip) does not generally contribute to the dynamic response and may be omitted.

$$F_{p1} = 2.9 + 0.22 \cdot k_v \cdot \sin(2\pi \cdot f_v \cdot t) \quad (\text{Equation 12.7.1.13-1})$$

Where:

$f_v$  = natural vertical frequency of the structure (Hz), that is the closest to the frequency range of 1.583 to 2.10 Hz.

$t$  = elapsed time (second)

$$k_v = \begin{cases} 0.3f_v & \text{for } f_v \leq 1 \text{ Hz} \\ 1.2f_v - 0.9 & \text{for } 1 \text{ Hz} < f_v < 1.583 \text{ Hz} \\ 1.0 & \text{for } 1.583 \text{ Hz} \leq f_v \leq 2.10 \text{ Hz} \\ -f_v + 3.1 & \text{for } 2.10 \text{ Hz} < f_v < 2.80 \text{ Hz} \\ 0.3 & \text{for } f_v \geq 2.80 \text{ Hz} \end{cases}$$

### B. Lateral Concentrated Load

For verification of lateral vibrations, a lateral concentrated load,  $F_{p2}$  in kips as specified in Equation 12.7.1.13-2, shall be used to perform the dynamic analysis. The lateral concentrated load shall be applied normal to the moving direction of vertical concentrated load to capture the extreme lateral acceleration demands of any part of the station platform or deck. This load shall be applied concurrently with the vertical concentrated load specified in Equation 12.7.1.13-1 and may be distributed over a 7.5 feet by 7.5 feet area.

$$F_{p2} = 0.055 \cdot k_h \cdot \sin(2\pi \cdot f_h \cdot t) \quad (\text{Equation 12.7.1.13-2})$$

Where:

$f_h$  = natural lateral frequency of the structure (Hz), that is the closest to the frequency range of 0.792 to 1.05 Hz.

$t$  = elapsed time (second)

$$k_h = \begin{cases} 0.6f_h & \text{for } f_h \leq 0.5 \text{ Hz} \\ 2.4f_h - 0.9 & \text{for } 0.5 \text{ Hz} < f_h < 0.792 \text{ Hz} \\ 1.0 & \text{for } 0.792 \text{ Hz} \leq f_h \leq 1.05 \text{ Hz} \\ -2f_h + 3.1 & \text{for } 1.05 \text{ Hz} < f_h < 1.40 \text{ Hz} \\ 0.3 & \text{for } f_h \geq 1.40 \text{ Hz} \end{cases}$$

### C. Vertical Uniform Distributed Load for Crowd Conditions

For verification of crowd condition vertical vibrations, a vertical uniformly distributed load,  $F_{p3}$  in ksf as specified in Equation 12.7.1.13-3, shall be used to perform the dynamic analysis. This load shall be placed in a manner to produce the extreme vertical acceleration demands of any part of the station platform or deck. The portion of static load in Equation 12.7.1.13-3 (0.025 ksf) does not generally contribute to the dynamic response and may be omitted.

$$F_{p3} = 0.025 + 0.0025 \cdot k_v \cdot \sin(2\pi \cdot f_v \cdot t) \quad (\text{Equation 12.7.1.13-3})$$

Refer to Section 12.7.1.13-A for  $f_v$ ,  $t$  and  $k_v$ .

### **12.7.2 Foundations for Equipment Enclosures**

Refer to the *Traction Power Supply System* chapter, *Automatic Train Control* chapter, and the *Communications* chapter.

For other equipment facilities, follow geotechnical recommendations and the provisions of CBC for design of foundations.

### **12.7.3 Foundations for Utility Equipment**

Foundations for utility equipment shall comply with the requirement of CBC and in addition meet the requirements of the individual utility.

## **12.8 Design Considerations for Primary Type 1 Bridges, Aerial Structures, and Grade Separations**

Unless otherwise specified, design of Primary Type 1 bridges, aerial structures, and grade separations shall be performed in accordance with AASHTO LRFD BDS with California Amendments.

Design of Primary Type 1 bridges, aerial structures, and grade separations shall satisfy criteria that exceed those of highway and conventional rail bridges because of the following:

- Particular effects that are critical to HST include:
  - Frequency of repetition (fatigue of materials)
  - Repetitive load applications (dynamic structural response)
  - Interaction of track and structure
- Riding comfort criteria
- High operating demands (life time of structure)
- Limited hours available for inspection, maintenance and repair

To meet the above mentioned criteria, Primary Type 1 bridges, aerial structures, and grade separations shall be designed to conform to the following characteristics:

- Small deflections and good resilience to dynamic responses to ensure passenger safety and a very high level of comfort
- Low probability of resonance
- Conceptual simplicity and standardization for ease of construction, fast track construction and higher maintenance reliability
- Reduction of environmental noise and vibration impact

## 12.8.1 General Design Requirements

### 12.8.1.1 Type Selection

The structural types selected for design and construction of the Primary Type 1 bridges, aerial structures, and grade separations shall be selected in a type selection process. The applicable type selection process specified in the Caltrans OSFP Information and Procedures Guide *for Planning Studies and Type Selection* shall be followed. The Authority's preferred type of superstructure carrying main line structures is prestressed concrete single-cell box girders. Box girders can be precast, precast segmental, or cast in-place, cast-in-place span by span, incrementally launched, or other similar types of construction. The Contractor shall demonstrate to the satisfaction of the Authority that an alternative is equal to or better in all respects than the Authority's preferred type of superstructure. The use of any alternative shall be subject to the Authority's approval.

For specific locations, multi-cell girders, multi-box girders, through girders or through trusses constructed by piece or incrementally launched may be more appropriate.

The Type Selection shall include seismic considerations, foundation recommendations, aesthetics review, traffic maintenance (both highway and rail), drainage considerations and intrusion protection. The Type Selection Report shall include all of the following that apply to the specific bridge, aerial structure, or grade separation:

- Type Selection Memo (refer to Caltrans OSFP Information and Procedures Guide)
- Hydrology and Hydraulics reports
- Aesthetics Design and Review Report
- Geotechnical Engineering Design Report
- Track-Structure Interaction Design and Analysis Plan (TSIDAP) (Section 12.6.1)
- Seismic Design and Analysis Plan (SDAP), refer to *Seismic* chapter
- Complex and Non-Standard Aerial Structures Load Path Report (Section 12.8.7)

1 If rail expansion joints are considered the variance process shall start at Type Selection.

2 For Primary Type 2 and Secondary structures, Type Selection shall follow the requirements of  
3 the Caltrans OSFP Information and Procedures Guide. The Type Selection shall be coordinated  
4 with the Authority and the Party that owns the structure to determine and identify any  
5 constraints that may control the design that are not identified in these Design Criteria.

#### **12.8.1.2 Clearances**

6 Clearances requirements are specified in the *Trackway Clearances* chapter.

#### **12.8.1.3 Water Crossings**

7 Hydraulic requirements for bridge drainage and requirements for water crossings are specified  
8 in the *Drainage* chapter.

#### **12.8.1.4 Deck Arrangement**

9 The arrangement of deck features shall conform to the requirements presented in the Standard  
10 and Directive Drawings.

#### **12.8.1.5 Material Requirements**

##### **A. Concrete Requirements**

11 The minimum 28-day concrete compressive strength ( $f'c$ ) shall be as follows:

- 12 • For piles, shafts, and footing reinforced concrete cast-in-place structures:  $f'c = 4,000$  psi
- 13 • For above ground reinforced concrete cast-in-place structures:  $f'c = 5,000$  psi
- 14 • For cast-in-place prestressed concrete:  $f'c = 6,000$  psi
- 15 • For precast prestressed members:  $f'c = 6,000$  psi
- 16 • Lightweight concrete is not allowed in Primary Type 1 structures. Lightweight concrete  
17 may be used in secondary concrete such as leveling concrete.

18 For design of cast-in-place piles and shafts, the nominal concrete strength shall not be greater  
19 than  $f'c = 4,000$  psi

##### **B. Reinforcing Steel**

20 Reinforcing steel for concrete reinforcement including spiral reinforcement shall conform to  
21 ASTM A706/706M, Specification for Low-Alloy Steel Deformed and Plain Bars for Concrete  
22 Reinforcement.

23 Plain wire for welded wire fabric shall comply with ASTM A82, Specification for Steel Wire,  
24 Plain, for Concrete Reinforcement.

##### **C. Concrete Cover**

25 Minimum concrete cover shall conform to AASHTO LRFD BDS with California Amendments  
26 Table 5.12.3-1, with the following exceptions:



- 1 • Uncased drilled shafts: 6 inches
- 2 • Cased drilled shafts with temporary casing: 4 inches

D. Prestressing Steel

3 Prestressing steel shall conform to the requirements of ASTM A416/A416M, or ASTM A722.  
4 Prestressing strand or wire shall be low relaxation. Additional requirements follow:

- 5 • Only post-tensioning systems that utilize tendons fully encapsulated with grout within the  
6 anchorages and ducts are allowed.
- 7 • Embedded anchors for bars are permitted.
- 8 • Strand or tendon couplers are not permitted.
- 9 • Select the post-tensioning grout for use by the proper application either repair, horizontal,  
10 or vertical. Grout will be mixed with potable water.
- 11 • Grout
  - 12 - Only pre-packaged grout mixes designed for the specific application are permitted. The  
13 grout shall not contain aluminum or other components that produce hydrogen, carbon  
14 dioxide or oxygen gas.
  - 15 - Chemical testing of a fresh dry sample taken from a bag in each lot of prepackaged grout  
16 shall be performed to determine chloride concentrations in accordance with the  
17 following requirement. Total chloride ions shall be less than 0.08 percent measured by  
18 weight of cementitious material according to ASTM C1152.
- 19 • Anchorages
  - 20 - Ensure that anchorages develop at least 95 percent of the actual ultimate tensile strength  
21 of the prestressing steel when tested in an unbonded state, without exceeding the  
22 anticipated set.
  - 23 - Design anchorages so the average concrete bearing stress is in compliance with  
24 AASHTO LRFD BDS with California Amendments.
  - 25 - Test and provide written certification that anchorages meet or exceed the testing  
26 requirements in the AASHTO LRFD Bridge Construction Specifications.
  - 27 - Equip anchorages with a permanent grout cap vented and bolted to the anchorage.  
28 Provide wedge plates with centering lugs or shoulders to facilitate alignment with the  
29 bearing plate. Cast anchorages with grout outlets suitable for inspection from either the  
30 top or front of the anchorage. The grout outlet shall serve a dual function of grout outlet  
31 and post-grouting inspection access. The geometry of the grout outlets must facilitate  
32 being drilled using a 3/8-inch-diameter straight bit to facilitate endoscope inspection  
33 directly behind the anchor plate. Anchorages may be fabricated to facilitate both  
34 inspection locations or may be 2 separate anchorages of the same type – each providing  
35 singular inspection entry locations.

- Anchorages shall be protected with epoxy grout encapsulation with an elastomeric coating.
- Ducts and Pipes
  - Do not use ducts manufactured from recycled material.
  - Use seamless fabrication methods to manufacture ducts.
  - Ferrous metal ducts shall not be used.
  - Precast segmental bridges with internal tendons shall use segmental duct couplers with 6 degrees of alignment allowance at all segment joints.

E. Structural Steel

Structural steel design for bridge-type structures shall meet the requirements of the AASHTO LRFD BDS with California Amendments.

Structural Steel Shapes shall conform to ASTM A6. Additional properties are as follows:

- Wide flange shapes: ASTM A992
- M-shapes, S-shapes, HP shapes: ASTM A572
- Angles, Channels: ASTM A572
- Rectangular and square hollow sections: ASTM A500 Gr B (46 ksi)
- Round hollow sections: ASTM A500 Gr B (42 ksi)
- Steel pipe: ASTM A53 Gr B (35 ksi)
- Plates, Bars: ASTM A36 (36 ksi)
- Bolts: ASTM A325
- Nuts: ASTM A563
- Washers: ASTM F436
- Rods: ASTM F1554

Welding of built up members and steel fabrications shall comply with AASHTO/AWS D 1.5.

Welding of HSS sections and pipes shall comply with AWS D 1.1.

Miscellaneous steel items shall be hot-dip galvanized after fabrication unless completely embedded in concrete and unless noted otherwise.

**Splice Locations** – If splicing of a structural steel member is permitted, indicate and detail the location of the splice. Such locations shall be at or near a cross section of minimum stress.

## 12.8.2 Design Loads and Effects

Primary Type 1 structure loads and load combinations are specified in Section 12.5. Track-Structure Interaction requirements are specified in Section 12.6. Seismic requirements are specified in the *Seismic* chapter.

## 12.8.3 Foundations

### 12.8.3.1 Shallow Foundation Design

Shallow foundations such as spread footings shall be designed in accordance with AASHTO LRFD BDS with California Amendments. Soil and rock engineering properties shall be based on the results of field investigations as presented in the Geotechnical reports described in the *Geotechnical* chapter. Use of presumptive values shall not be allowed.

### 12.8.3.2 Deep Foundation Design

Design of deep foundations shall be based on project-specific information developed for the location(s) and foundation type planned. Soil and rock engineering properties shall be based on the results of field investigations as presented in the geotechnical reports described in the *Geotechnical* chapter. Use of presumptive values shall not be allowed. Bottom clean out of drilled shafts constructed using the wet method shall be verified.

Where permanent steel casing is used for structural capacity, it shall have a minimum wall thickness of 3/4 inch and be provided with internal shear lugs if composite action is to be relied upon. Additionally, the design basis of the steel section shall be reduced to account for corrosion over the life of the structure based on actual soil and ground water conditions. A site specific corrosion study shall be performed to determine the deduction of the wall thickness due to the corrosive characteristics. A minimum 1/8 inch reduction in wall thickness shall be applied. Steel casing shall not be considered for structural support in extremely aggressive corrosive environments.

For trackway shafts greater than 5 feet in diameter, the drilled shafts shall be designed assuming they are offset at the top of the shaft a minimum of 6 inches. Refer to the Standard Specification on Drilled Concrete Piers and Shafts.

Geotechnical Design of micropiles shall be in accordance with AASHTO LRFD BDS with California Amendments, Article 10.9: Micropiles and FHWA-SA-97-070 (Micropile Design and Construction Guidelines, June 2000).

The upper 5 feet as measured from lowest adjacent grade shall be discounted in any axial and lateral load capacity analyses except where measures are provided to prevent future excavations around the shaft or pile group. When determining the demand forces, the upper 5 feet shall be considered.

#### 12.8.4 Steel Structures

Steel through trusses and through girders may be used for longer spans requiring minimal structure depth and other steel built up sections, beams and girders may be used over railroads or highways.

##### 12.8.4.1 Continuous Steel Structures

For continuous girders and other statically indeterminate structures, the moments, shears, and thrusts produced by external loads shall be determined by elastic analysis. The effects of creep, shrinkage, axial deformation, restraint of attached structural elements, and foundation settlements shall be considered in the design.

##### 12.8.4.2 Fracture Critical Members

Fracture critical members shall be designed in accordance with AASHTO LFRD BDS with California Amendments. A load factor of 1.50 shall apply to live load of the fatigue load combination shown in Table 12-4. Field welding in tension zones in fracture critical members is not permitted.

##### 12.8.4.3 High Performance Coating

Steel bridges shall have a high performance coating system such as polysiloxane, polyaspartic modified urethane, or fluoropolymer which may be applied in the field. Primer shall be inorganic or organic zinc as recommended by the manufacturer of finish coats. Coatings including primers shall comply, at a minimum, with South Coast Air Quality Management District (SCAQMD) Rule 113.

The Contractor shall provide services of an independent coating inspector. The independent coating inspector shall be certified under NACE International's Certified Inspector Program as a Certified Coating Inspector.

##### 12.8.4.4 Orthotropic Steel Decks

Steel orthotropic plate decks shall not be used for Primary Type 1 structures.

##### 12.8.4.5 Bearing Replacement

Reinforced jacking points shall be provided and identified clearly on As-Built drawings.

##### 12.8.4.6 Inspection and Maintenance of Steel Structures

Steel bridge construction details shall reflect that safe inspection and maintenance will occur during non-revenue service. For structures over railroads or highways, access for safe inspection and maintenance to the below deck elements shall be provided. Steel box girders and box beams shall have access hatches to allow maintenance and inspection of the member.

#### 12.8.5 Concrete Structures

Bridge, aerial structure, and grade separation superstructures may be constructed using cast-in-place concrete, precast girders either single span, span by span or segmental, as well as cast-in-

place, segmental balanced cantilever or incrementally launched methods. Concrete through girders shall meet the same requirements as steel through girders.

The CEP-FIP Model Code for Concrete Structures shall be used for determining time dependent effects due to creep, shrinkage and prestressing steel relaxation.

#### **12.8.5.1 Longitudinal Tension Stresses in Prestressed Members**

AASHTO LRFD BDS with California Amendments shall be used for allowable longitudinal tension stresses. Tension stresses are not allowed in pre-compressed tensile zones after all losses have occurred.

#### **12.8.5.2 Additional Requirements for Segmental Trackway Construction**

Shear and torsion design shall conform to AASHTO LRFD BDS with California Amendments, Article 5.8.6.

Principal tensile stresses in webs shall conform to AASHTO LRFD BDS with California Amendments, Article 5.8.5.

Precast segmental concrete construction with dry segment joints shall not be permitted. Joints in precast segmental bridges and aerial structures shall be either cast-in-place closures or match cast epoxied joints.

Hollow columns shall have a solid section with the greatest of the following: minimum 5 feet above finished grade or 12 feet above high water level or a minimum of 1.5 times the maximum column dimension above top of the foundation. Vertical post-tensioning is not allowed in the solid sections. For maintenance access opening, refer to Section 12.8.10. Access openings shall be located outside of potential plastic hinge zones. Internal platforms, ladders and landings shall be provided.

#### **12.8.5.3 Crack Control**

The design of prestressed concrete or reinforced concrete structures shall consider the effect of temporary loads imposed by sequence of construction stages, forming, falsework, and construction equipment, as well as the stresses created by lifting or placing pre-cast members, stress concentration (non-uniform bearing at the ends of pre-cast beams), end block design and detailing, methods of erection, shrinkage, and curing. Structural design, specifications, and detailing of pre-stressed or reinforced concrete members shall meet durability and serviceability requirements with crack widths no greater than allowed by AASHTO LRFD BDS with California Amendments Class 2 exposure condition in construction stages or service. If the concrete member is continuously submerged in water or is in a zone of intermittent wetting and drying, the exposure factor used in AASHTO LRFD BDS with California Amendments Article 5.7.3.4 shall be 0.25 or less.

#### 12.8.5.4 Maintenance and Inspection of Concrete Structures

Inspection and maintenance access openings with steel grating shall be provided into each closed box girder cell. Intrusion by birds and insects to the inside of box girder cell shall be considered. These access openings may be through the girder soffits or in combination with openings between adjacent girder diaphragms. Refer to 12.8.10 for more details.

The minimum headroom inside of typical box girders shall be 6 feet for the span length greater than or equal to 100 feet. For span length less than 100 feet the minimum headroom inside of typical box girders shall be 5 feet.

For the short structures up to total length of 270 feet with multiple short spans, the minimum headroom inside of box girders may be reduced to 4 feet.

In-span hinges and associated expansion joints are not allowed.

#### 12.8.5.5 Continuous Concrete Structures

For continuous girders and other statically indeterminate structures, the moments, shears, and thrusts produced by external loads and prestressing shall be determined by elastic analysis. The effects of creep, shrinkage, axial deformation, restraint of attached structural elements, and foundation settlements shall be considered in the design.

### 12.8.6 General Bridge, Aerial Structure, and Grade Separation Features

#### 12.8.6.1 Bridge Skew

The preferred angle of bridges, aerial structures, and grade separations crossing relative to the TCL is 90 degrees. In cases where a 90 degree crossing cannot be constructed, the skew of the bridge shall be limited so that for each track the deck end is between successive pairs of rail fasteners and the applicable provisions of Section 12.6 are met. The maximum bridge skew from 90 degrees shall not exceed 30 degrees.

#### 12.8.6.2 Embankment Length between Abutments

The length of embankment between abutments shall not be less than 500 feet. The length of embankment between an abutment and a culvert shall not be less than 100 feet. If closer spacing is required, then the embankment shall be specially treated such that a constant gradient of stiffness shall be provided between the 2 adjacent bridges. Refer to the *Geotechnical* chapter for specific requirements for embankment fills and abutment backfill.

#### 12.8.6.3 Trackside Cable Trough Walls

Trackside cable trough walls shall be provided, as described in Section 12.5.2.13-B.

#### 12.8.6.4 Intrusion Protection

Bridges, aerial structures, and grade separations shall be protected from errant highway vehicles as well as from derailed trains as described in the *Rolling Stock and Vehicle Intrusion Protection* chapter and as required in the following:

A. Highway Traffic Intrusion

Primary Type 1 substructures, as required by the *Rolling Stock and Vehicle Intrusion Protection* chapter, shall be protected by an appropriate barrier as specified AASHTO LRFD BDS with California Amendments Article 3.6.5.1 or designed for the force presented in AASHTO LRFD BDS with California Amendments, Article 3.6.5.2.

B. Railroad Intrusion

Primary Type 1 substructures located adjacent to conventional railroad shall be protected as specified in the *Rolling Stock and Vehicle Intrusion Protection* chapter. The design shall follow the requirements of AASHTO LRFD BDS with California Amendments, Article 2.3.3.4. If an independent intrusion barrier is not provided, the substructure shall be designed to resist collision loads (CL) per Section 12.5.2.14, and Table 12-4.

**12.8.6.5 Uplift**

Hold-down devices shall be provided at bearings where the vertical force due to the MCE seismic load opposes and exceeds 50 percent, but is less than 100 percent, of the dead load reaction. The dead load reaction shall include the dead load of structural components and permanent attachments (DC) and the dead load of non-structural attachments (DW). In this case, the net uplift force for the design of the hold-down device shall be taken as ten percent of the dead load reaction.

If the vertical seismic force at bearings results in net uplift, the hold-down device shall be designed to resist the larger of either:

- 120 percent of the difference between the vertical seismic force and the dead load reaction, or
- Ten percent of the dead load reaction.

For multicolumn bents, there shall be no net uplift at any column under Service load combinations. Net uplift is allowed for Strength and Extreme load combinations.

For deep foundations, there shall be no net uplift on piles under Service load combinations. For uplift of piles under Strength and Extreme load combinations, refer to the *Geotechnical* chapter.

For shallow foundations, there shall be no net uplift under any load combinations. For stability of shallow foundations, refer to the *Geotechnical* chapter.

**12.8.6.6 Friction**

Friction shall be considered in the design where applicable.

**12.8.6.7 Sound Barriers**

Both the presence and absence of sound barriers shall be considered in the evaluation of stress, vibration, and deflection.

#### 12.8.6.8 Drainage

Drainage from bridges, aerial structures, and grade separations shall be accomplished by sloping the deck towards the center of the deck, and sloping the girders towards a pier support or abutment. Water shall be collected and conveyed to a drainage pipe cast into the concrete substructure. The pipe shall pass through the pier columns and abutment walls to exit through the foundations to a point of discharge. Column reinforcing in potential plastic hinge zones shall not be interrupted for drain pipes. Refer to the *Drainage* chapter for other requirements.

#### 12.8.6.9 Expansion Joints

Expansion joints shall be provided between girder ends and between girder end and abutment walls to allow superstructure movements and prevent water and other material from falling from the superstructure. Expansion joints are not required to resist highway or rail traffic loads, but shall be protected from ballast if ballast is used. Expansion joints may be part of a bridge drainage system.

The design life of expansion joints is given in the *General* chapter. Expansion joints shall be detailed to allow replacement during the non-revenue service maintenance work window. Refer to Section 12.6 for limits to the length of thermal units, and limits to relative expansion joint displacements.

##### A. Structural Expansion Joints

Structural expansion joints (SEJ) shall be watertight and their design shall achieve the following:

1. Provide free movement space in the bridge longitudinal and transverse directions for the following:

- Service 1 load combination.
- Service 3 load combination plus OBE. Regardless of number of tracks on the structure, consideration of the train effects shall include 1 train vertical live load plus impact, 1 train longitudinal braking force and mass of 1 train, applied at the center of train mass. The train and its associated loads shall be placed and applied to produce maximum movement of the joint gap. The load factors for thermal and settlement loads shall be 0.50.

Expansion joints and connections to the structure shall be capable of resisting loads transmitted through the ballast under these loading conditions.

2. Under a MCE earthquake, local damage of structure expansion joints in the longitudinal and transverse directions is allowed. The Contractor shall consider the effects of structural pounding in their analysis, design and detailing. The Contractor shall verify that damaged joint elements will not induce changes to the structural behavior leading to collapse of the structure. Extreme 3 load combination shown in Table 12-4 shall be used.



3. For ballasted track, if buried type expansion joints are used, no part of the structure expansion joints shall protrude above the top surface of the protection layer for waterproofing membrane.

The Contractor shall adjust the joint gap during installation to accommodate the effects of prestressing including shrinkage and creep, and the difference between the ambient temperature and the design temperature.

Longitudinal and transverse movements developed prior to the installation of expansion joint devices need not be considered. The longitudinal movement for each load effect shall be provided on the as-built drawings to facilitate future replacement.

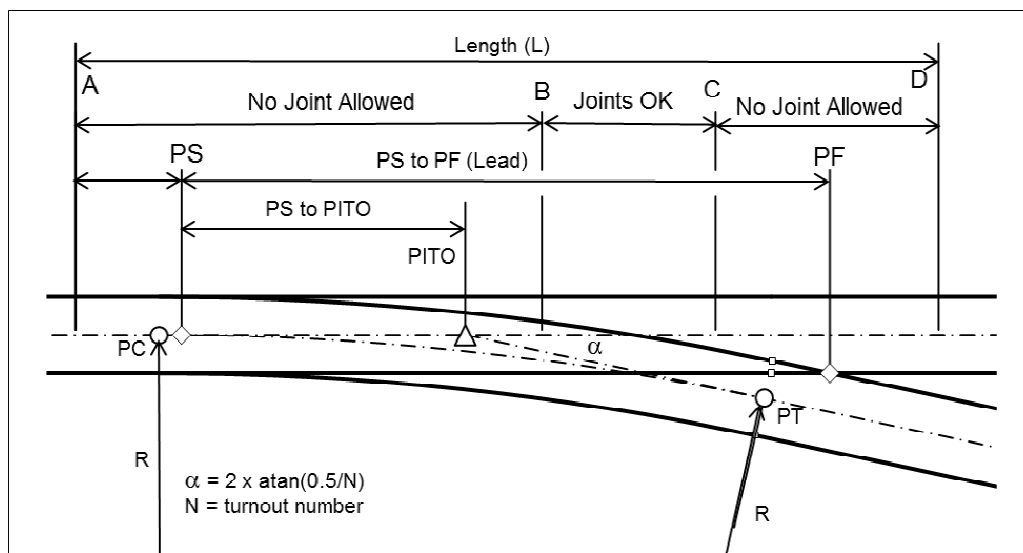
The installation of turnouts and SEJ shall be based on requirements in Section 12.8.6.9-B.

#### B. Structure Expansion Joints in Structures Supporting Special Trackwork

SEJs may be placed within turnout and crossover units as needed to minimize relative movement between structures and track. SEJs shall not be located within areas of special track supporting plates nor within the vicinity of the movable portions of switches and frogs. SEJs under special trackwork units shall be perpendicular or close to perpendicular to the orientation of the track. Potential movement of the structure relative to the track shall be oriented with the alignment of the track.

Permissible and prohibited locations for joints are illustrated on Figure 12-36 and Figure 12-37 and the limiting location dimensions given in Table 12-26 and Table 12-27.

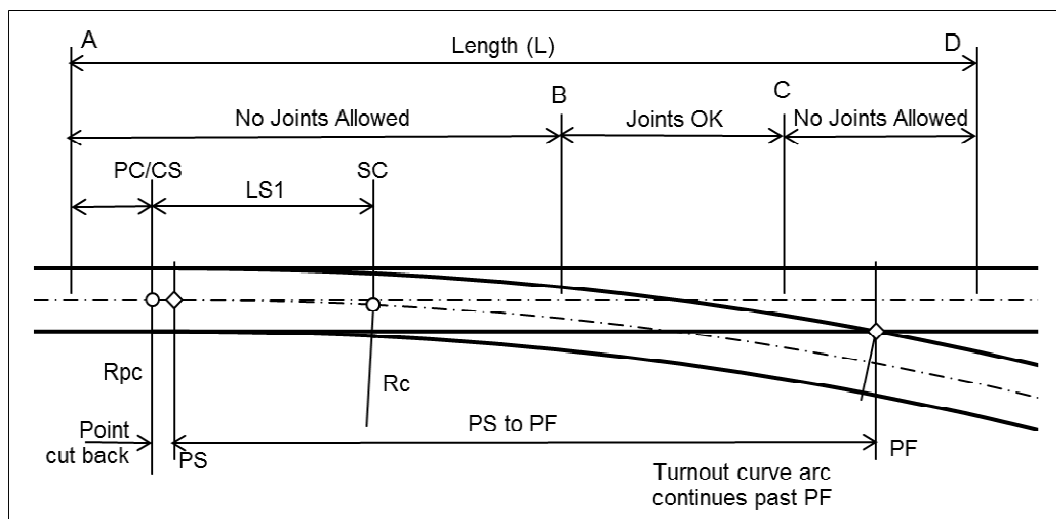
**Figure 12-36: Joint Location Limitations at Low-Speed Turnouts**



**Table 12-26: Joint Location Limitations at Low-Speed Turnouts**

Turnout Properties					Location of points defining limits of joints (feet)				Length of No Joint / Allowed Joint Zones (feet)		
Internal Radius (feet)	PC to PS (feet)	N	Turnout angle $\alpha$	PS to PI (feet)	PS to A	PS to B	PS to C	PS to D	No Jt. A to B	Jt. OK B to C	No Jt. C to D
620.0	1.69	9	6d21m35s	32.75	20.0	40.6	62.2	86.8	60.6	21.6	24.6
950.0	1.93	11	5d12m18s	41.25	20.0	50.3	77.0	107.2	70.3	26.7	30.2
1,750.0	2.58	15	3d49m06s	55.75	25.0	68.3	104.5	145.8	93.3	36.2	41.3
3,250.0	3.25	20	2d51m51s	78.00	25.0	93.1	142.5	198.0	118.1	49.4	55.5
4,650.0	3.87	24	2d23m13s	93.00	25.0	111.3	170.4	237.0	136.3	59.1	66.6

**Figure 12-37: Joint Location Limitations at High-Speed Turnouts**



**Table 12-27: Joint Location Limitations at High-Speed Turnouts**

Turnout Properties					Location of points defining limits of joints (feet)				Length of No Joint / Allowed Joint Zones (feet)		
Design Speed (mph)	Entry Radius R <sub>pc</sub> (feet)	entry spiral length (feet)	body radius R <sub>c</sub> (feet)	Switch point cutback (feet)	PC to A	PC to B	PC to C	PC to D	No Jt. A to B See note	Jt. OK B to C	No Jt. C to D
60	10,000	90.00	5,000	5.53	23.97	134.3	207.3	270.7	157.8	73.0	63.4
80	18,000	120.00	9,000	7.53	22.47	180.0	278.0	363.1	202.5	98.0	85.1
110	34,000	160.00	17,000	9.76	20.24	246.6	387.3	498.0	266.8	140.7	110.7
150	80,000	220.00	32,000	15.07	14.93	348.9	542.1	694.2	363.8	193.2	152.1

Note: For the 110 mph turnout 1 structural joint may be placed up to 42 feet in advance of point B. For the 150 mph turnout 1 structural joint may be placed up to 58 feet in advance of point B.

#### 12.8.6.10 Longitudinal Joints in Structures Supporting Special Trackwork

In zones of special trackwork, where tracks will cross between parallel superstructure elements such as girders, those superstructure elements shall be connected into a continuous deck that can support the tracks as well as a derailed train with loads described in Section 12.5.2.13. The deck shall be cast in place or made continuous with a longitudinal closure strip between parallel decks. The transverse strengthening shall also include rigid diaphragms, post-tensioning, welded steel plates or other such strengthening elements. Railroad box sections post-tensioned together transverse to the track alignment shall be considered to have a continuous deck. This lateral continuity shall extend through the entire length of special trackwork such that:

- No longitudinal expansion joints in zones of low-speed turnout special trackwork from 25 feet before the initial point of switch to 25 feet beyond the final point of switch
- No longitudinal expansion joints in zones of high-speed turnout special trackwork from 30 feet before the initial point of switch to 30 feet beyond the final point of switch

For point of switch (PS) refer to Figure 12-36 and Figure 12-37. This continuity is independent of the structural expansion joints described in Section 12.8.6.9.

#### 12.8.6.11 Bearings

AASHTO LRFD BDS with California Amendments shall be used for design of bearings. Elastomeric bearings, disk bearings, spherical bearings and seismic isolation bearings are allowed. If seismic isolation bearings are used, a design variance shall be submitted following the requirements of the *Seismic* chapter. Longitudinal and lateral restraints for all bearing types shall be arranged to result in a symmetrical deflection shape (in plan) as shown on Figure 12-12 and Figure 12-15.

The design life of bearings shall be as presented in the *General* chapter. Since bearings will be replaced during the life of the structure, an inspection and replacement plan for bearings shall

1 be provided. Inspection and replacement shall be allowed only during the non-revenue service  
2 hours.

#### **12.8.6.12 Rail Stress and Structural Deformation Limits**

3 Refer to Section 12.6 for additional requirements related to track-structure interaction, which  
4 includes rail stress and structural deformation limits.

#### **12.8.6.13 Span Arrangement on Repetitive Span Aerial Structures**

5 Amplification of vibrations and poor ride quality has been observed on high-speed trains  
6 traveling on long aerial structures where the same span is repeated many times. In order to  
7 mitigate this response, long repetitive span aerial structures shall have their typical span length  
8 modified every 20 spans. The modification shall include at least 2 consecutive spans differing in  
9 length by not less than 20 percent from the typical span length.

#### **12.8.6.14 Camber and Deflections for Structures**

10 Steel bridge, aerial structure, and grade separation superstructures shall be cambered to  
11 compensate for the vertical track alignment and the sum of deflections under dead load of  
12 structural components and permanent attachments (DC) and dead load of non-structural and  
13 non-permanent attachments (DW). Camber diagrams shall be provided on the plans. The  
14 sequence of load application to account for construction stages shall be considered for  
15 determining the required camber.

16 For prestressed and reinforced concrete bridge, aerial structure, and grade separation  
17 superstructures, diagrams showing a predicted deck profile at the completion of civil  
18 construction shall be provided on the plans and shall consider the sequence of load application  
19 to account for construction stages. The vertical difference between the vertical track alignment  
20 and the predicted deck profile at the completion of civil construction shall not be less than the  
21 distance from top of low rail to top of deck as defined in the *Track* chapter. The total long-term  
22 predicted camber growth measured after the completion of civil construction shall be less than  
23  $L/5000$  ( $L$ =span length).

24 To ensure rider comfort, the deflection of longitudinal girders under normal live load plus  
25 dynamic load allowance shall be as described in Section 12.6.

#### **12.8.6.15 Structure Deformation and Settlement**

26 The control of deformations through proper structural design is of paramount importance in  
27 obtaining acceptable ride quality for the rail vehicles and passengers. Consider structure  
28 deformations, including foundation settlement, for their effects on structural behavior and on  
29 trackwork. As a minimum, trackway piers and abutments settlement as measured at the top of  
30 concrete of the finished trackway girder deck shall be limited as prescribed in the *Geotechnical*  
31 chapter.

#### 12.8.6.16 Superelevation on Structures

Superelevation of curved tracks shall be accomplished through the trackwork, refer to the *Trackwork* chapter. Girder decks shall maintain a level attitude transverse to the track with deck slope allowed only for drainage.

#### 12.8.6.17 Walkways, Parapets, and Sound Walls

Loads on walkways shall be as described in Section 12.7.1.4. Walkways shall be precast or cast-in-place concrete. The walkway shall be on the cable trough cover and the walkway surface shall consist of a non-skid material. The cable trough cover shall be designed to resist loads described in Section 12.7. Walkways shall follow requirements on the Standard and Directive Drawings and as specified in the *System Safety and Security* chapter.

Parapets shall be provided along edges of bridges, aerial structures, and grade separations. Parapets shall be designed for wind loads, slipstream effects, and other loadings, as described in Section 12.5. Parapets shall be designed to accommodate installation of sound walls.

Parapets and safety railings shall be designed to withstand the forces described in Section 12.7.1.6. In locations where conduit risers are required along the alignment, parapets may be required to support conduits.

Temporary railings may be necessary to provide safety after girders are placed and before parapets are placed. Temporary railings shall meet the same requirements as safety railings. Temporary railings may be needed between construction contracts.

The height of sound walls shall be as defined in the Table 12-1. The structure and connection between parapet and structure deck shall be designed to resist the load combinations as described in Table 12-4 to accommodate installation of sound walls. No longitudinal gaps shall be permitted between the bottom of sound wall and the parapet or deck, nor any vertical gaps between adjacent sound wall panels.

#### 12.8.6.18 Differential Foundation Settlements and Differential Vertical Displacements of Adjacent Substructures

Differential foundation settlements and differential vertical displacements of adjacent substructures alter the profile of HST tracks, which ultimately affect track performance.

Differential foundation settlements shall be calculated as described in the *Geotechnical* chapter.

Differential vertical displacements of adjacent substructures shall be calculated from the Service 1 load combination after completion of construction. Consideration of differential vertical displacements shall include, but is not limited to the following:

- Adjacent substructures founded on different foundation system, such as deep foundation with a shallow foundation adjacent to it. For this instance, the vertical displacement due to live load at the track level over the shallow foundation (smaller stiffness in general) can be greater than vertical displacement at the track level over the deep foundation (larger

1 stiffness in general), which results in differential vertical displacement between adjacent  
2 substructures.

- 3 • Adjacent substructures of different structural system, such as straddle bent with a single  
4 column bent adjacent to it. For this instance, vertical displacement due to live load or long-  
5 term camber growth due to creep and shrinkage of the cross beam elements can be  
6 generated at the track level over the straddle bent location while infinitesimal vertical  
7 displacement can be expected at the track level over the adjacent single column bent  
8 location, which results in differential vertical displacement between adjacent substructures.
- 9 • Adjacent substructures of different column heights of greater than 15 feet. For instance, the  
10 vertical elongation of a taller column due to uniform temperature effects can be greater than  
11 vertical elongation of a shorter column, which results in differential vertical displacement  
12 between adjacent substructures.

13 The combination of differential foundation settlements and differential vertical displacements  
14 of adjacent substructures shall be measured at top of deck, and shall not exceed the allowable  
15 settlement limits described in the *Geotechnical* chapter.

#### 12.8.7 Complex and Non-Standard Structures

16 For the definition of complex and non-standard structures, refer to the *Seismic* chapter. The  
17 following specific requirements shall apply:

- 18 • Straddle and outrigger bents have cap beams that extend beyond the edges of  
19 superstructure toward columns located outside of the superstructure.
- 20 • The load path necessary to accommodate longitudinal actions of the superstructure shall be  
21 defined in a report and submitted with the Type Selection Report to the Authority, refer to  
22 Section 12.8.1.
- 23 • Torsion cracking in the primary load path is not permitted in concrete beam members.  
24 Compatibility torsion is allowed.
- 25 • Torsional rotation of concrete columns is not permitted under seismic actions when high  
26 bending and shear stresses occur.

#### 12.8.8 Emergency Access/Egress Points

27 Emergency access/egress points shall be provided at intervals and in the manner specified in the  
28 *System Safety and Security* chapter. The safety gate shall be designed to resist wind and  
29 slipstream forces as described in Section 12.5 as well as live loads as described in Section 12.7.

#### 12.8.9 OCS and Traction Power Facility Gantry Pole Supports

30 Girder decks shall be designed to accommodate OCS and Traction Power Facility Gantry pole  
31 supports. For loads and load combinations, refer to Section 12.5. Conduit or sleeves for future

conduit shall be provided from external power sources to the OCS and Traction Power Facility Gantry poles.

#### 12.8.10 Maintenance of Primary Type 1 Bridges, Aerial Structures, and Grade Separations

Because of the large number of structures along the alignment, special care shall be taken in the design to reduce maintenance requirements. The following requirements shall apply:

- Reinforced or prestressed concrete structures are preferred over steel structures. As part of Type Selection per Section 12.8.1, the Designer shall justify the use of steel structures for each such structure, demonstrating the benefits of that steel structure.
- Bearings shall be easily accessible for inspection. They shall be adjustable and replaceable at any time during the life of the structure without disrupting train normal operations. Bearing replacement shall be completed within non-revenue service hours. During this period, train speed may be limited at locations where bearings are being replaced. The design documents shall provide a description of the procedures for bearing replacement, including the location of the jacks with safety nuts and a calculation of forces.
- Access arrangements for maintenance and inspection of exterior surfaces or equipment attached thereto shall be provided from the ground or from movable gantries. At intervals not greater than 300 feet and at 1 location in the case of a shorter isolated structure, 2.5 feet x 5 feet access openings with steel grating for inspection and maintenance shall be provided in the bottom slabs close to the expansion joint piers
- Wherever the diaphragm has sufficient depth a minimum opening of 5 feet in height and 6 feet in width shall be provided. If the depth of the diaphragm is limited, an opening of the largest possible size shall be provided, but not smaller than 3 feet in height and 6 feet in width.
- For tall and long structures where access openings in the bottom slabs are not feasible due to distance from the ground, a permanent access stairway, ladder, landing and platform from the ground shall be provided on each pier to allow for inspection and maintenance of bearings and potential column plastic hinge. The permanent stairway, ladder, landing and platform shall be designed to remain elastic for Extreme 3 load combination. A lifting hook capable of lifting 7,500 pounds shall be embedded and provided at locations near bearings and column plastic hinge and safely reachable by the maintenance crew for inspection and maintenance work. All the steel members of the permanent access stair shall be hot dip galvanized. The secure access control shall be provided at the entrance of the permanent access stair.
- If a pier is not accessible from the ground beneath (e.g., such as river crossing bridge), a 2.5 feet x 5 feet access opening with steel grating shall be provided in the bottom flange so that the pier top can be reached from the inside of the box girder. Adjacent to this opening, a work platform shall be provided at the pier top for the maintenance of bearings.

- In the design of bridges, the vertical load of the maintenance gantry on the deck overhang shall be taken into consideration. The maintenance gantry shall be represented by a line load of 30 kips over a length of 13 feet applied to the edge of the cantilever deck.
- A lifting hook capable of lifting 7,500 pounds shall be embedded in the underside of the superstructure top slab above each access opening. Access openings shall be equipped with lockable galvanized steel gratings.
- The box sections shall be vented by drains or screened vents at proper locations such as at the low point of the junction of the web and bottom flange, or at the corners of the blister to drain the water or prevent the build-up of the potential hazardous gas that might endanger inspection personnel. The minimum diameter of the vent holes shall be 2 inches and at placed at intervals not greater than 10 feet.
- Formworks inside the CIP hollow concrete section shall be removed for ease of inspection.

## 12.9 Design Considerations for Primary Type 2 and Secondary Structures

### 12.9.1 Primary Type 2 Structures

Where a highway, roadway, freight, pedestrian or utility structure spans over HST track and has the potential to affect HST service, the structure is classified as a Primary Type 2 structure. For the definition of Primary Type 2 structure, refer to the *Seismic* chapter.

#### 12.9.1.1 New Primary Type 2 Structures

For seismic design of the above new Primary Type 2 structures, refer to the *Seismic* chapter. The static design of the above new Primary Type 2 structures shall meet the requirements of the Third Party that owns the structure and the following requirements:

- Caltrans Office of Special Funded Projects (OSFP) *Information and Procedures Guide for Planning Studies and Type Selection* shall be followed in order to obtain approval from the third party that owns the structure.
- Expansion joints and other features requiring routine maintenance shall be located outside the access controlled HST right-of-way.
- Foundations of abutments, columns, walls, and slopes shall be located outside the Authority's access restricted right-of-way.
- Horizontal and vertical clearances defined in the *Trackway Clearances* chapter shall be met.
- The requirements for intrusion protection barriers and screens defined in the *Rolling Stock and Vehicle Intrusion Protection* and *Overhead Contact System and Traction Power Return System* chapters shall be met. The loading due to the slipstream effects from passing trains shall be considered to occur in combination with wind load for design of the barriers and screens.
- The requirements defined in the *Grounding and Bonding Requirements* chapter shall be met.



- The applicable system interface requirements provided through system interface coordination shall apply.

#### 12.9.1.2 Existing Primary Type 2 Structures

Based on the Rehabilitation Strategy Plan included in the Scope of Work, the Contractor shall repair and retrofit the existing Primary Type 2 structure to meet the requirements of the Third Party that owns the structure and the requirements for a new Primary Type 2 structure, as defined in Section 12.9.1.1.

#### 12.9.2 Design Considerations for Secondary Structures

For the definition of Secondary structures, refer to the *Seismic* chapter.

Secondary structures shall meet the requirements of the third party that owns the structure, and the supplementary provisions specified in the CHSTP requirements.

### 12.10 Design Considerations for Earth Retaining Structures

Earth retaining structures shall be designed to withstand earth pressures and accommodate deformations in accordance with the *Geotechnical* chapter. Structural design of earth retaining structures such as reinforced concrete cantilever retaining walls, trenches, portal walls, and the like, shall follow the design requirements of AASHTO LRFD BDS with California Amendments.

Top of retaining walls, including fill, cut, and trench walls, shall be at least 1 foot above finish grade. Wall heights may be increased as required for flood elevation and intrusion protection requirements. Walls with Access Detering (AD) or Access Restricting (AR) fencing, per the *Civil* chapter, shall be installed on top of the Earth Retaining Structures.

Earth retaining structure design shall provide a reserved clear area for future installation of each OCS pole foundation for no less than 25 feet vertically down from TOR. Reinforcement strips of MSE walls, tie rods, footings, and other supporting or foundation elements as well as conduits and other utilities shall not be allowed within the reserved area. The reserved area shall have a diameter of 5 feet coinciding with the center of each OCS pole foundation. The center of the reserved area shall be set at an equal longitudinal spacing of not more than 30 feet. For the transverse offset distance from the TCL, refer to the Standard and Directive Drawings or requirements provided through the system interface coordination. The reserved area on each side of the rail shall be directly opposite each other and perpendicular to the TCL. The longitudinal spacing from the centerline bearing of the structural abutment to the centerline of the future OCS pole foundation shall be equal to 1/2 of the equal spacing. The loads, load combinations, and limit states shown in Table 12-2 applied at top of low rail elevation shall be applied as part of live load surcharge (LLS) in the design of earth retaining structures.

Earth retaining structure design shall provide a reserved clear area for future installation of each traction power facility gantry pole foundation for no less than 25 feet vertically down from

TOR. Tie rods, footings, and other supporting or foundation elements as well as conduits and other utilities shall not be allowed within the reserved area. For locations and dimensions of the reserved area, refer to the Standard and Directive Drawings or requirements provided through the system interface coordination. The reserved area on each side of the rail shall be directly opposite each other and perpendicular to the TCL. The loads, load combinations, and limit states shown in Table 12-2 applied at top of low rail elevation shall be applied as part of live load surcharge (LLS) in the design of earth retaining structures.

#### **12.10.1 Retaining Walls**

Retaining walls shall be designed in accordance with requirements of the *Geotechnical* chapter. Retaining walls shall be constructed with expansion joints in walls a maximum of 96 feet apart. Weakened plane joints shall be a maximum of 24 feet apart. Design of reinforced concrete retaining walls shall follow requirements in AASHTO LRFD BDS with California Amendments. For design of the MSE wall, refer to section 12.10.2.

#### **12.10.2 MSE Walls**

Mechanically stabilized earth walls shall be designed in accordance with requirements of the *Geotechnical* chapter.

#### **12.10.3 Trenches**

Trenches are below grade structures with a retaining structure on both sides. Often the retaining structures are joined by a common reinforced concrete foundation. This form of trench, combined foundation, is also commonly called a U-section or U-wall. Waterproofing of the bottom of slab, and outside of walls is required if the top of concrete foundation slab is below the water table (refer to Section 12.11.3). For hydrostatic pressure (buoyancy), refer to Section 12.11.2.7.

Wall heights shall be based on flood and intrusion protection. The wall height shall be the greater of either 1 foot above finished grade or 2 feet above 100-year flood elevation where applicable.

Excavation support systems shall not be used as whole or part of the permanent walls.

#### **12.10.4 Earth Retaining Structure Intrusion Protection**

HST earth retaining structures shall be protected from errant highway vehicles and derailed trains as described in the *Rolling Stock and Vehicle Intrusion Protection* chapter, and as required in the following.

##### **12.10.4.1 Highway Traffic Intrusion**

HST earth retaining structures shall be protected by a continuous Caltrans type concrete barrier as specified in the *Rolling Stock and Vehicle Intrusion Protection* chapter. The wall shall be

designed for the force presented in AASHTO LRFD BDS with California Amendments Article 3.6.5.

#### 12.10.4.2 Railroad Intrusion

HST earth retaining structures located adjacent to conventional railroad shall be protected as specified in the *Rolling Stock and Vehicle Intrusion Protection* chapter. Where an independent intrusion protection cannot be constructed due to limited space, the earth retaining structure (located adjacent to the conventional railroad), shall be constructed as described in *Rolling Stock and Vehicle Intrusion Protection* chapter and the wall shall be designed to resist collision loads presented in Section 12.5.2.14.

#### 12.10.5 Struts

Struts may be used to support earth pressures in trenches. The height of struts shall have a minimum 27 feet vertical clearance from TOR and comply with the *Trackway Clearances* chapter.

#### 12.10.6 Trench Drainage

Trenches shall be drained to the low point of sag curves. Sump pumps and an interconnected sump and pump room shall house the pumps. Earth pressures on the sump structure shall be as required in the geotechnical reports described in the *Geotechnical* chapter. Sump structures shall be waterproofed from ground water intrusion (refer to the *Drainage* chapter).

#### 12.10.7 Trench Emergency Access/Egress Points

Emergency access/egress points shall be provided at intervals and in the manner specified in the *System Safety and Security* chapter.

### 12.11 Cut-and-Cover Structures

The criteria set forth in this section govern the static load design of cut-and-cover underground structures with the exception of pile foundations. Cut-and-cover structures include line structures, cross passages, sump pump structures, underground stations, vaults, ventilation structures, and other structures of similar nature. Portal and ventilation requirements and minimum cross sectional tunnel areas shall be as required in the *Tunnels* chapter.

The design of structures within the scope of this section shall be in accordance with the provisions set forth in the CHSTP design criteria and shall also meet the requirements of the AASHTO LRFD BDS with California Amendments, CBC, ACI, AISC and AWS, except where such requirements are in conflict with the CHSTP design criteria.

#### 12.11.1 Structural System

The structural system for cut-and-cover line structures shall be single and/or multi-cell reinforced concrete box structures, with walls and slabs acting one-way in the transverse

1 direction to form a continuous frame. Excavation support systems shall not be used as whole or  
2 part of the permanent walls. Expansion or contraction joints are required at locations of major  
3 change in structural sections such as from line structure to station. Construction joints shall  
4 have continuous reinforcing steel, non-metallic waterstops and sealants.

### 12.11.2 Loads and Forces

5 Components of cut-and-cover structures shall be proportioned to withstand the applicable  
6 loads and forces described in Section 12.5.

7 Cut-and-cover structures shall, at minimum, be designed for the forces described herein.

#### 12.11.2.1 Zone of Influence

8 Zone of Influence is defined as the area above a positive Line of Influence, which is a line from  
9 the critical point of substructure at a slope of 2 horizontal to positive 1 vertical (line sloping  
10 towards ground level) or the area below a negative Line of Influence, which is a line from the  
11 critical point of substructure at a slope of 2 horizontal to negative 1 vertical (line sloping away  
12 from ground level).

#### 12.11.2.2 Future Traffic Loads

13 An area surcharge shall be applied at the ground surface both over and adjacent to  
14 underground structures to simulate potential future construction, railway, vehicular, and  
15 sidewalk loads as specified in Sections 12.11.2.3 through 12.11.2.5.

16 Such construction may result in permanent loads or temporary loads from construction  
17 equipment from the stockpiling of construction materials, or from the deposition of excavated  
18 earth. It is possible that loads such as those from hauling trucks, may be applied inadvertently  
19 to the underground structures due to their innate inconspicuousness.

#### 12.11.2.3 Alternative Traffic Loading

20 For underground structures beneath or adjacent to operating railroads, vertical and lateral  
21 surcharge shall be based on the Cooper E-80, as defined by AREMA.

22 For underground structures adjacent to existing highway bridge overcrossings, vertical and  
23 lateral surcharge shall be based on the operating loads from the Contractor's equipment with a  
24 minimum surcharge loading equivalent to a 100-ton crawler crane.

25 For all other underground structures (i.e., not beneath or adjacent to operating railroads or  
26 highway bridge overcrossings), vertical and lateral surcharge shall be based on the HL-93  
27 loading according to the AASHTO LRFD BDS with California Amendments. The distribution of  
28 the HL-93 loading shall be in accordance with the following:

- 29 • For fill heights  $\leq 2$  feet – the concentrated loads shall be applied directly to the top of the  
30 underground structure.

- For fill heights > 2 feet – the concentrated loads shall be distributed over a square area, the sides of which shall equal 1.75 times the depth of the fill.
- When distribution areas overlap, the total load shall be uniformly distributed over an area defined by the outside limits of the individual areas.

For design of the top slab of underground structures supporting the alternative traffic loading, impact loading (I) shall conform to AASHTO LRFD BDS with California Amendments Article 3.6.2.2.

The fill height shall be measured from the top of ground or pavement to the top of the underground structure.

#### 12.11.2.4 Existing Structures

Existing structures that are to remain in place above underground structures shall either be underpinned in such a manner as to avoid increased load on the underground structure, or the underground structure shall be designed to support the existing structure directly. Third party structures supported directly on Primary Type 1 structures shall obtain a specific approval in writing by the Authority.

Underground structures shall be designed for additional loading from existing adjacent buildings or structures unless the existing structures are permanently underpinned or have existing foundations extending below the zone of influence.

An existing structure shall be considered to be adjacent to an underground structure when the horizontal distance from the building line to the nearest face of the underground structure is less than 2 times the depth of the underground structure invert below the building foundation.

Each adjacent existing structure shall be considered on an individual basis. In the absence of specific data for a given height of building and type of occupancy, applicable foundation loads shall be computed according to the CBC and the additional uniform lateral pressure on that portion of the underground structure sidewall below the elevation of the building foundation shall be provided in the geotechnical reports described in the *Geotechnical* chapter .

#### 12.11.2.5 Requirements for Future Structures

Cut-and-cover structures shall be designed to accommodate future development when in close proximity to the HST right-of-way. Requirements are provided below for the default case. Additional requirements may be specified by the Authority at site specific locations.

##### A. Clearance

Structures over or adjacent to HST underground structures shall be designed and constructed so as not to impose any temporary or permanent adverse effects on underground structures. The minimum clearance between any part of the adjacent structures to the exterior face of underground substructure shall be 7 feet–6 inches. Minimum cover of 8 feet shall apply to the structures over or adjacent to HST underground structures.

B. Surcharge

- 1 Cut-and-cover structures shall be designed with an area surcharge applied at the ground  
2 surface both over and adjacent to the structures. The area surcharge is considered static uniform  
3 load with the following value:

D (feet)	Additional Average Vertical Loading (psf)
D>20	0
5<D<20	800-40D
D<5	600

- 4 Note: D is the vertical distance from the top of the underground structure roof to the ground surface.  
5

C. Shoring

- 6 Shoring is required for excavations in the Zone of Influence, as defined in Section 12.11.2.1.

D. At-Rest Soil Condition

- 7 Refer to the geotechnical reports described in the *Geotechnical* chapter for soil loads and  
8 pressures needed for design.

E. Stress Redistribution

- 9 Refer to the geotechnical reports described in the *Geotechnical* chapter for stress redistribution  
10 caused by temporary shoring of the permanent foundation system.

F. Dewatering

- 11 Dewatering shall be monitored for changes in groundwater level. Recharging will be required if  
12 existing groundwater level is expected to drop more than 2 feet.

G. Piles Predrilled

- 13 Piles shall be predrilled to a minimum of 10 feet below the Line of Influence. Piles shall be  
14 driven in a sequence away from HST structures. No piles shall be allowed between steel-lined  
15 tunnels.

H. Vibration During Pile Driving

- 16 Underground structures shall be monitored for vibration during pile driving operations for  
17 piles within 100 feet of the structures. Tunnels shall also be monitored for movement and  
18 deformation. Requirements for monitoring shall be provided by the geotechnical reports  
19 described in the *Geotechnical* chapter.

I. Future Excavation Adjacent to Cut-and-Cover

- 20 The design of cut-and-cover structures shall consider an unbalanced lateral load condition due  
21 to possible future excavation or scour of 30 percent of total depth on 1 side of the structure.  
22 Under this condition the structures shall remain stable meeting CHSTP requirements.

### 12.11.2.6 Earth Pressure

#### A. Vertical Earth Pressure

Depth of cover shall be measured from the ground surface or roadway crown, or from the street grade, whichever is higher, to the top of underground structure surface. Saturated densities of soils shall be used to determine the vertical earth pressure. Recommended values shall be presented in the geotechnical reports described in the *Geotechnical* chapter.

#### B. Lateral Earth Pressure

For the purpose of these criteria, cut-and-cover box sections are defined as structures with stiff walls, which are restrained at the top so that the amount of deflection required to develop active pressure is not possible. Refer to the *Geotechnical* chapter for earth pressures required for design.

### 12.11.2.7 Hydrostatic Pressure (Buoyancy)

The effects of hydrostatic uplift pressure shall be considered whenever ground water is present. The hydrostatic uplift pressure is a function of the height of water table above the foundation plane, and shall be assumed uniformly distributed across the width of the foundation in proportion to the depth of the base slab below the design ground water table.

Structures shall be checked for both with and without buoyancy to determine the governing design condition. Maximum design flood levels shall be indicated in the Hydrology Report.

### 12.11.2.8 Flotation

For design flood levels and flood zone, refer to the Hydrology Report, if applicable.

Cut-and-cover structures subject to ground water table and/or located within the flood zone shall be checked and provided with adequate resistance to flotation.

No permanent dewatering system shall be assumed for the design of underground cut-and-cover structures.

#### A. Factor of Safety

The structure shall have a minimum factor of safety against flotation at any construction stage of 1.05, excluding any benefit from skin friction from perimeter of the structures.

The structure, when complete, shall have a minimum factor of safety against flotation during the 100-year flood level of 1.10, excluding skin friction resistance from perimeter of the structure.

The use of tiedowns, tension piles or other elements specifically designed to resist uplift forces shall be permitted and included in the flotation calculations. Refer to the *Geotechnical* chapter for other requirements for buoyancy resisting elements.

The dead weight of the structure used in the flotation calculations for the underground structures shall exclude the weight of each of the following:

- 1 • Any building above the structure
- 2 • Any live load internal or external to the structure
- 3 • Any loads that are not effective at the time
- 4 • Two feet of backfill over the roof except when checking against the 100-year and 500-year
- 5 flood levels

#### 12.11.2.9 Miscellaneous Loads

##### A. Walkway Cover Live Loads

- 6 Stationary and hinged cover assemblies shall be designed for the loads on walkways per Section  
7 12.7.1.4. Deflection at center of span under 100 pounds per square foot uniform live load shall  
8 not be more than 1/8 inch. Hinged cover material shall comply with NFPA 130 requirements.

##### B. Live Loads and Equipment Loads for Ventilation Structures

- 9 Refer to Section 12.7 for roof and floor live loads and equipment loads for ventilation structures.

#### 12.11.2.10 Seismic Design of Underground Structures

- 10 Refer to the *Geotechnical* chapter for the demand requirements for seismic design of tunnels and  
11 underground structures. If ductility is required to meet seismic demands in an underground  
12 structure, then the requirements provided in CSDC for lateral confinement reinforcing of  
13 concrete pier walls shall be satisfied.

#### 12.11.2.11 Reinforced Concrete Underground Station Structures

- 14 Underground station structures and their appurtenant structural elements such as entrances  
15 shall be designed in accordance with AASHTO LRFD BDS with California Amendments and  
16 shall conform to the applicable requirements specified in Section 12.7.
- 17 Subsurface exploration shall be carried out to determine the presence and influence of geologic  
18 and environmental conditions that may affect the performance of station structures and  
19 reported by 1 or more geotechnical reports described in the *Geotechnical* chapter.
- 20 • Load combinations and load factors to be used are those shown in Table 12-4. The load  
21 resistant factors to be used are those provided by AASHTO LRFD BDS with Caltrans  
22 Amendments and their referenced AASHTO Tables 3.4.1-2, 3.4.1-3, and 12.5.5-1. In addition,  
23 the effects of EH, EV, ES, LLS, DD, DW, and WA shall be applied simultaneously in all their  
24 maximum and minimum values to produce the envelope of moment, torsion, shear, and  
25 axial force to produce the greatest demands to the structural framing. These load values  
26 shall cover the forces on the station structure at all phases of construction. Refer to AASHTO  
27 LRFD BDS with California Amendments Section 5.14.2.3.2.
  - 28 • Final earth pressures and design assumptions for soil-structure interaction shall be provided  
29 by the geotechnical reports described in the *Geotechnical* chapter.



- Vertical pressure on foundation slabs shall be divided into hydrostatic and earth pressure components. The hydrostatic component shall be distributed across the width of the foundation in proportion to the depth of slab below the design groundwater table.
- Distribution of the earth pressure moment shall be based on specified construction procedures, and elastic and plastic subgrade reaction foundation effects provided by the geotechnical reports described in the *Geotechnical* chapter.
- For design, the horizontal earth pressure distribution diagram for multiple braced flexible walls shall be provided by the geotechnical reports described in the *Geotechnical* chapter. Compression forces shall not be considered in shear design of the top and bottom slab in box sections.

#### 12.11.2.12 Reinforced Concrete for Cut-and-Cover Structures

Concrete for cut-and-cover structures shall be designed to attain the required chemical resistance to the environment, low permeability, water tightness and water absorption as specified in accordance with the Durability Report to meet the design life.

Concrete for cut-and-cover structures shall meet the requirements of the Standard Specifications, and the following minimum requirements:

- Strength – Minimum  $f'c$  shall be 4000 psi at 28 days.
- Proportioning Materials – The maximum water-cement ratio shall be 0.40 with 4.5 percent to 7.5 percent air entrainment.

#### 12.11.2.13 Reinforcing Steel for Cut-and-Cover Structures

Reinforcing steel in structural components shall use U.S. Customary Units, meet the requirements of the Standard Specifications, and meet the following requirements:

- Use reinforcing steel conforming to ASTM designation A 706 Grade 60 ( $F_y=60$  ksi).
- Use uncoated reinforcing steel and welded wire fabric when the concrete surface is not in contact with soil/water (or waterproofing).
- Use epoxy coated reinforcing steel meeting the requirements of the Standard Specifications for all permanent concrete members when the concrete surface is in contact with soil/water (or waterproofing).
- Spacing of main reinforcement shall not exceed 12 inches.

#### 12.11.2.14 Camber

The tunnel roof shall be cambered to mitigate the effect of long-term loads (i.e., dead load plus vertical surcharge). The camber shall be calculated in accordance with the AASHTO LRFD BDS with California Amendments, Article 5.7.3.6. In computing the long-term deflection it shall be no less than the instantaneous deflection multiplied by a factor of 2.

### 12.11.3 Waterproofing of Underground Station Structures

Roofs, walls, and floors slabs of underground stations including auxiliary spaces except as otherwise noted, shall be waterproofed. To ensure adequate inspection and long term performance, no blind side waterproofing shall be used.

Provisions shall be made to collect and drain water potentially seeping through the roof, walls, or floor. The leakage through structural elements shall be limited to a maximum of 0.001 gallon per square foot of structure per day; no dripping or visible leakage from a single location shall be permitted.

The manufacturer and installer of the waterproofing system shall submit a list of a minimum of 5 successful projects of similar design and complexity completed within the past 5 years.

The Designer shall design for any openings or other penetrations through the waterproofing layer and for appropriate protection measures for the waterproofing membrane including the chamfering of corners of the structure, external protection, etc. Components of the waterproofing system shall comply with applicable Volatile Organic Compound (VOC) regulations.

#### 12.11.3.1 Underground Station Structures

##### A. Roofs

Station roofs shall be completely waterproofed. Waterproofing and the boundary condition details at reglets and flashings shall be provided.

##### B. Walls

Exterior station walls shall be completely waterproofed. Mezzanine walls enclosing public areas and entrance walls shall be furred out, and provisions shall be made for collecting and draining seepage through these walls. The depth of the furring shall be governed by the space required for the placing of fare collection and other equipment, and architectural requirements, such as the minimum thickness of the wall finish. The fastening of the finish to the wall shall be such that water can drain off the walls freely and that it will not corrode the fasteners.

##### C. Floor Slabs

For station floor slabs, no special waterproofing provisions shall be made where the water can drain freely into the floor drainage system, and where such a leakage and drainage is not objectionable from a corrosion, operational, or visual standpoint.

Drainage shall be provided at public areas of the station floor slab.

##### D. Base Slabs

Waterproofing shall be applied under station base slab.

##### E. Appendages

Differential vertical movements of the station body and its appendages, such as wings or entrances at shafts, due to ground re-expansion as a result of returning of ground water, may

1 cause cracks at joints and other locations. Special attention shall be given to design detailing to  
2 mitigate this problem. Where such movements cannot be avoided, properly designed  
3 waterproof joints between such appendages and the station body shall be provided.

### 12.11.3.2 Cut-and-Cover Underground Trackway Structures

#### A. Cut-and-Cover Box

4 Exterior membrane waterproofing shall be applied to the outside of the cut-and-cover box as  
5 indicated on the Standard and Directive Drawings. Any seepage through the walls or the floor  
6 shall be carried away by the track drainage.

#### B. Transition Structure

7 For underground structure daylight transition structures, where U-sections or trenches with  
8 exposed sidewalls are used, special attention shall be given to controlling shrinkage cracks in  
9 sidewalls between construction joints.

#### C. Rooms

10 The following rooms or spaces shall be completely waterproofed, including all wall and roof  
11 surfaces in contact with the earth. Floor drains shall be provided. Refer to the *Mechanical*  
12 chapter.

- 13 • Electrical Rooms (includes spaces that house train control facilities, substation facilities,  
14 switchgear, ventilation fans, pumps, and other electrical equipment)
- 15 • Train Control and Auxiliary Equipment Rooms
- 16 • Substation , Switchgear, Fan Rooms, and Similar Equipment Rooms

#### D. Pump Rooms

17 Floor drains shall be provided to prevent the accumulation of seepage as required in the  
18 *Mechanical* chapter.

#### E. Cross-passages and Emergency Exits

19 Cross-passages and emergency exits shall be provided at intervals and in the manner specified  
20 in the *System Safety and Security* chapter

### 12.11.3.3 Waterstops and Sealants

21 Waterstops and sealants shall be used in construction joints in exterior walls, floors, and roofs.

### 12.11.3.4 Waterproofing Materials

22 Bentonite waterproofing shall not be used.

### 12.11.3.5 Water tightness

23 The cut-and-cover structure shall be designed and constructed so that it achieves a functional  
24 waterproofed underground structure for the duration of its design life. The design,

construction, and maintenance of the cut-and-cover structure shall meet the water-tightness criteria stipulated below at substantial completion and acceptance by the Authority:

- Local infiltrations limit 0.002 gallons per square foot of structure per day and no dripping or visible leakage from a single location shall be permitted.
- No drips shall be permitted overhead or where they have the potential to cause damage to equipment, malfunctioning of any electrical power, signaling, lighting, control, communication equipment, or compromise electrical clearances.
- A drainage system shall be provided to accommodate water infiltration as specified herein in accordance with tunnel and portal drainage.
- No water ingress shall cause entry of soil particles into the tunnel.
- No material used in preventing or stemming water ingress shall compromise the fire safety of the works or the durability of the structures in which they are used.
- Embedded electrical boards, electrical conduits, and other similar elements shall be completely waterproofed and watertight.
- The interface between cut-and-cover structure section with bored tunnel and other structures (e.g., building structures, emergency egress structures, etc.), shall be designed and constructed such that the joint between the 2 structures is fully watertight.

#### **12.11.4 Water Holding and Conveyance Structures**

Water conveyance or water holding structures that cross the HST alignment shall be designed to meet ACI 350 Code Requirements for Environmental Structures and Commentary with all Errata, and the seismic criteria in the seismic chapter. The jurisdiction owning or operating the facility may have additional requirements that shall be followed.

#### **12.11.5 Shoring Support Systems**

For specific requirements of soil loadings refer to the *Geotechnical* chapter. The design of shoring support systems shall consider several factors, including but not limited to, the following:

- Soil and groundwater conditions
- Width and depth of excavation
- Configuration of the structure to be constructed within the cut
- Size, foundation type and proximity of adjacent structures
- Utilities crossing the excavation, or adjacent to the excavation
- Requirements for traffic decking across the excavation
- Traffic and construction equipment surcharge adjacent to the excavation
- Settlements of adjacent structures

- Noise restrictions

### 12.11.6 Structural Fire Resistance

Underground structures can be exposed to extreme events such as fires resulting from incidents inside the structure. Underground structure design shall consider the effects of a fire on the concrete supporting elements. The concrete elements should be able to withstand the heat of the specified fire intensity and period of time given in the *Tunnels* chapter without loss of structural integrity. Protection from fire shall be determined by concrete cover on the reinforcing, additional finish, and special treatment of the concrete mixes.

## 12.12 Support and Underpinning of Structures

This section includes design requirements for the support and underpinning of existing structures to remain over or adjacent to new HST facilities that require construction below grade.

The Designer, in coordination with the Authority, shall investigate existing structures that are to remain over, or adjacent to, the construction sites of new HST facilities. The Designer shall prepare the necessary designs for the protection or permanent support and underpinning of such existing structures.

The types of buildings and structures that require support and underpinning include the following:

- Buildings and structures that extend over the HST structures to such an extent that they must be temporarily supported during construction and permanently underpinned.
- Buildings and structures immediately adjacent to the HST structures that will require temporary support during construction.
- Buildings and structures that are affected by groundwater lowering. In certain areas, uncontrolled lowering of the groundwater for HST construction can cause settlements of buildings either adjacent to or at some distance from HST excavations.

The design shall conform to the applicable requirements of the AASHTO LRFD BDS with California Amendments (where highway bridges are involved), AREMA (where railway bridges are involved), CBC (where buildings are involved), ACI, AISC, and AWS, except where such requirements conflict with the criteria.

### 12.12.1 Depth of Support Structures

Underpinning walls or piers that support buildings or other structures and that also form a portion of the excavation support system shall extend to a minimum depth of 2 feet below the bottom elevation of the excavation.

## 12.12.2 Methods

Methods used to protect or underpin buildings or other structures shall be given in the geotechnical reports described in the *Geotechnical* chapter.

### 12.12.2.1 Protection Wall Method of Structure Protection

Under light loading and favorable soil conditions, the supporting system for the excavation is sufficient to protect light structures. Under heavier loading conditions, a reinforced concrete cutoff wall, constructed in slurry-filled trenches or bored pile sections braced with preloaded struts, shall be considered as an alternative to underpinning or as a means to avoid settlement due to dewatering. The protection wall method of structure protection shall be provided in the geotechnical reports described in the *Geotechnical* chapter.

### 12.12.2.2 Stabilization of Soil

Soil stabilization techniques such as compaction grouting shall be considered as alternatives in lieu of underpinning. Refer to the *Geotechnical* chapter for soil stabilization requirements.

### 12.12.2.3 Temporary Bracing Systems

A properly designed temporary bracing system is important for the effectiveness of underpinning and for protection wall support. In addition to the general requirements for support of excavations, which are provided in the specifications, the special requirements for the installation and removal of the temporary bracing systems that relate to the designs of underpinning and protection walls, such as the levels of bracing tiers, the maximum distances of excavation below an installed brace, and the amount of preloading shall be indicated on the design drawings. The detailed design of the temporary bracing system shall be designed by the Contractor. Refer to the *Geotechnical* chapter for earth pressures information.

### 12.12.2.4 Pier, Pile, or Caisson Method of Underpinning

If soil conditions, structure size and proximity to an excavation dictate piers, piles or caissons for underpinning of an existing structure, such piers, piles, or caissons shall extend below a sloping plane, which is defined as follows: The plane passes through a horizontal line that is located 2 feet below the bottom of the excavation, and that is also located within the vertical plane containing the face of that excavation closest to the structure foundation to be underpinned; the plane shall slope upwards and away from the excavation at an inclination, which shall be established by the Designer, on a case-by-case basis. The supports shall be founded on stable soil mass and extended beyond the slope of the soil wedge failure plane. The pier, pile, or caisson method of underpinning shall be provided in the geotechnical reports described in the *Geotechnical* chapter. Refer to the *Geotechnical* chapter for soils information.

### 12.12.2.5 Seismic Design of Temporary Shoring and Underpinning

Seismic loads shall be considered in the design of temporary shoring and underpinning. Soil parameter shall be determined from the Geotechnical reports described in the *Geotechnical* chapter. For seismic demand spectra and performance requirements for temporary construction

- 1 structures, refer to the *Seismic* chapter. Shoring of excavations adjacent to developed facilities  
2 shall be required to maintain at-rest soil condition and monitored for movement.

### 12.13 Areas of Potential Explosion

- 3 Areas of new buildings adjacent to facilities where the public has access or that cannot be  
4 guaranteed as a secure area, such as parking garages and commercial storage and warehousing,  
5 shall be treated as areas of potential explosion.
- 6 NFPA 130, Standard for Fixed Guideway Transit and Passenger Rail Systems, life safety  
7 separation criteria shall be applied that assumes such spaces contain Class-I flammable or Class-  
8 II or Class-III combustible liquids. For structural and other considerations, separation and  
9 isolation for blast shall be treated the same as for seismic, and the more restrictive requirement  
10 shall be applied.

### 12.14 Structure Interface Issues

- 11 Design of structures shall accommodate the interface requirements as specified in the contract  
12 documents and the requirements provided through system interface workshop meetings.

#### 12.14.1 Cable Trough

- 13 A cable trough shall be provided on both sides of the trackway. The cable trough shall be  
14 continuous through the entire system. The top of the cable trough shall be used as a safety  
15 walkway as a non-skid surface is provided.

#### 12.14.2 Grounding and Bonding

- 16 Refer to the *Grounding and Bonding Requirements* chapter for grounding and bonding design of  
17 structures including, but not limited to the following:
- 18 • Bridges, aerial structures, and grade separations
  - 19 • Trenches and retaining walls
  - 20 • Cut-and-cover structures
  - 21 • Buildings and support facilities
  - 22 • New and existing third party structures
  - 23 • Miscellaneous structures (e.g., cable trough, sound wall)

#### 12.14.3 Drainage

- 24 Water shall be drained from the trackway and conveyed to the drainage system. Refer to the  
25 *Drainage* chapter for drainage requirements.

#### 12.14.4 Conduit Risers

At traction power facilities, stand-alone radio sites and train control sites located at structures, provisions to accommodate a minimum cable and conduit weight of 350 pounds per linear foot shall be made for the installation of embedment of conduits, raceways and pull-boxes up the sides of specific columns, girders, and parapets of aerial structures, walls of earth retaining structures, and walls of cut-and-cover structures. For columns, girders and parapets, these provisions shall be located at 3 columns nearest to the site. For walls of earth retaining structures and walls of cut-and-cover structures, these provisions shall be made for a minimum length of 200 feet nearest to the site. Further, details shall be provided for the installations such that no damage results to the structures.

#### 12.14.5 Embedded Conduits

Sleeves embedded in the slabs and webs of the aerial structures, cable troughs and parapet walls to provide routing for future electric cable installation shall be provided as specified in the contract documents and the requirements provided through system interface workshop meetings.

#### 12.14.6 Trackside Equipment

Provisions shall be made to support trackside equipments. This equipment shall be located in line with the OCS poles.

#### 12.14.7 Emergency Access/Egress Stairs

Emergency access/egress points shall be provided at intervals and in the manner specified in the *System Safety and Security* chapter.

#### 12.14.8 Concrete Anchors

These criteria shall apply to anchors for overhead applications or any anchor subjected to sustained tensile loads where failure of the anchor could result in risk to life or limb.

Anchors shall be embedded in confined concrete. Length of embedment in unconfined concrete shall not be considered effective embedment length.

Use of adhesive anchors in overhead applications or in sustained tension is prohibited. Undercut anchors shall be used.

#### 12.14.9 Utilities

For utility requirements affecting HST structures, refer to the *Utilities* chapter.



#### **12.14.10 Field-drilled Anchors for OCS Pole and OCS Support Structures**

- 1 The Contractor shall provide a minimum 6-inch reinforcement spacing for the installations of
- 2 the future field-drilled anchors for OCS poles and OCS support structures either onto the top of
- 3 the trench walls, retained fill walls, or onto the faces of the trench walls, retained fill walls, and
- 4 the cut-and-cover structures.

## Chapter 13

### Tunnels

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	May 2013	Revision 1



## Table of Contents

<b>13</b>	<b>TUNNELS .....</b>	<b>13-1</b>
13.1	Scope .....	13-1
13.2	Regulations, Codes, Standards, and Guidelines .....	13-1
13.3	Basic High-Speed Train Tunnel Configuration and Geometry .....	13-2
13.3.1	Train length .....	13-2
13.3.2	Train Operating Speed .....	13-2
13.3.3	Lining Type .....	13-2
13.3.4	Tunnel Configuration .....	13-2
13.3.5	Clearances .....	13-3
13.3.6	Walkways .....	13-4
13.3.7	Trackwork .....	13-5
13.3.8	Escape Shafts and Escape Tunnels .....	13-5
13.3.9	Crosspassages .....	13-5
13.3.10	Equipment Requirements and Tunnel Niches .....	13-5
13.3.11	Rolling Stock .....	13-6
13.3.12	Aerodynamic Considerations .....	13-6
13.3.13	Minimum Tunnel Cross-Sectional Areas .....	13-7
13.3.14	Bored Tunnel .....	13-8
13.3.15	Mined Tunnel .....	13-8
13.3.16	Transition between Cut-and-Cover and Bored Tunnel .....	13-8
13.3.17	Ventilation Shafts .....	13-8
13.4	Tunnel Portals .....	13-9
13.4.1	Sonic Booms at Tunnel Portals .....	13-9
13.4.2	Portal Layouts .....	13-10
13.4.3	Slope Failure or Landslide .....	13-10
13.4.4	Unsymmetrical Pressure .....	13-10
13.4.5	Insufficient Bearing Capacity of Ground .....	13-11
13.4.6	Rock Fall or Debris Flow .....	13-11
13.4.7	Adjacent Structures .....	13-11
13.5	Tunnel Ventilation Requirements and Configurations .....	13-11
13.5.1	Mechanical Ventilation .....	13-11
13.5.2	Ventilation Configurations .....	13-11
13.5.3	Design Considerations .....	13-12
13.5.4	Train Operation .....	13-12
13.5.5	Maintenance Vehicles .....	13-12
13.5.6	Access Shafts and Tunnels .....	13-12
13.5.7	Crosspassages .....	13-12
13.6	Materials .....	13-13
13.6.1	Concrete .....	13-13
13.6.2	Structural Steel .....	13-13
13.6.3	Durability .....	13-13
13.7	Fire Protection .....	13-13
13.8	Design Loads .....	13-14
13.8.1	Design Load Considerations .....	13-14
13.8.2	Ground Considerations .....	13-15

**Chapter 13 – Tunnels**

13.8.3	Groundwater Considerations .....	13-15
13.8.4	Seismic Load Considerations.....	13-15
13.8.5	Train Derailment Loads .....	13-15
13.8.6	Loading Combinations and Load Factors .....	13-16
13.8.7	Aerodynamic forces.....	13-16
13.8.8	Flotation.....	13-16
13.8.9	Construction Methods .....	13-16
13.8.10	Temporary Structures .....	13-16
13.8.11	Effects from Adjacent Structures.....	13-16
13.9	Structural Analysis .....	13-16
13.9.1	Concrete Design.....	13-16
13.9.2	Steelwork Design .....	13-17
13.9.3	Dynamic Analysis .....	13-17
13.9.4	Seismic Analysis .....	13-17
13.9.5	Fatigue Analysis .....	13-17
13.9.6	Mining in Rock.....	13-18
13.9.7	Tunneling in Soft Ground .....	13-18
13.9.8	Portals and Shafts .....	13-18
13.9.9	Enlargements .....	13-18
13.10	Ground Movement.....	13-18
13.10.1	Purpose of Predicting the Ground Movement.....	13-18
13.10.2	Method of Analysis .....	13-19
13.10.3	Protection of Adjacent Assets .....	13-19
13.11	Measurement and Monitoring .....	13-20
13.11.1	Instrumentation and Monitoring Design .....	13-20
13.12	Waterproofing.....	13-20
13.12.1	Requirements .....	13-20
13.12.2	Waterproofing Measures .....	13-20
13.12.3	Junction between New and Existing Structures .....	13-20
13.13	Drainage.....	13-21
13.13.1	Tunnels and Underground Structures .....	13-21
13.13.2	Portals .....	13-21
13.14	Special Ground Conditions .....	13-21
13.14.1	Design of Tunnels and Underground Structures.....	13-21
13.14.2	Squeezing Ground .....	13-21
13.14.3	High-Pressure Water Inflows .....	13-21
13.14.4	Gases Dissolved in Groundwater.....	13-22
13.14.5	Geothermal Ground .....	13-22
13.14.6	Gassy Ground .....	13-22
13.14.7	Rock Bursts.....	13-22
13.14.8	Fault Zones .....	13-22
13.14.9	Liquefaction .....	13-22
13.14.10	Landslips .....	13-23
13.15	System-wide Interfaces .....	13-23
13.15.1	Catenary Support Provisions .....	13-23
13.15.2	Warning Systems and Location Cases .....	13-23
13.15.3	Embedded Conduit and Cable Trough.....	13-23
13.15.4	Facility Services and Infrastructure .....	13-24
13.15.5	Design Requirements for Cable Troughs.....	13-24
13.15.6	Fencing.....	13-25

13.15.7	Prevention of Unauthorized Access .....	13-25
13.15.8	Grounding and Bonding .....	13-25
13.16	Tunnel Portal Facilities .....	13-26
13.16.1	Noise Mitigation Hood .....	13-26
13.16.2	Portal Ventilation Building .....	13-26
13.16.3	Power Facilities .....	13-26
13.16.4	Access Road .....	13-27
13.16.5	Emergency Vehicle Assembly and Turnaround Area .....	13-27
13.16.6	Rescue Area/Passenger Assembly Area .....	13-27
13.16.7	Fire Hydrants and Water Supply .....	13-27
13.16.8	Area Lighting .....	13-28
13.16.9	Cross Track Emergency Vehicle Access .....	13-28
13.16.10	Paved Emergency Egress from Tunnels .....	13-28
13.16.11	Train Surface Evacuation and Fire Control Zone .....	13-28
13.16.12	Sound Powered Telephone Access, Emergency Intercoms, and Emergency Telephone Facilities .....	13-28
13.16.13	Overhead Contact System Motorized Disconnect Switch .....	13-29
13.16.14	Emergency Command Post .....	13-29
13.16.15	Communications Facilities .....	13-29
13.16.16	Emergency Power Supply .....	13-29
13.16.17	Surface Drainage .....	13-29
13.16.18	Tunnel Seepage and Wash Water .....	13-30
13.16.19	Detention Basin .....	13-30
13.16.20	Parking for Tunnel Maintenance and Traction Power Facility .....	13-30
13.16.21	Helicopter Landing Pad .....	13-30
13.16.22	Landscaping .....	13-30
13.16.23	Aesthetic Treatment of Rock and Concrete Surfaces .....	13-30
13.16.24	Public Utilities .....	13-30
13.16.25	Crossovers .....	13-31
13.17	Portal Infrastructure Requirements for Different Categories of Tunnel .....	13-31
13.17.1	A Shorter Tunnel up to One-Half Mile in Length .....	13-31
13.17.2	Several Short Tunnels Close Together .....	13-32
13.17.3	Shorter Tunnel with Constrained Access .....	13-32
13.17.4	Tunnel Longer than One-Half Mile in Length .....	13-33
13.17.5	Longer Tunnel with Constrained Portal Access .....	13-33

## Tables

Table 13-1:	Requirements for Arrangement of Portal Hood Structure.....	13-9
-------------	--	------

## Figures

Figure 13-1: EUREKA Fire Curve for Trains .....	13-14
---	-------

## Acronyms

ACI	American Concrete Institute
ANSI	American National Standards Institute
ATC	Automatic Train Control
CCTV	Closed-Circuit Television
CIRIA	Construction Industry Research and Information Association
CPTED	Crime Prevention Through Environmental Design
FHWA	Federal Highway Administration
HST	High-Speed Train
M&E	Mechanical and Electrical
NFPA	National Fire Protection Association
OCS	Overhead Contact System
OD	Outer Diameter
TPF	Traction Power Facility





## 13 Tunnels

### 13.1 Scope

This chapter provides design criteria for bored tunnels, mined tunnels, and shafts and includes cross-sectional area requirements, structural design of tunnels and other underground structures, requirements at portals, and aerodynamic requirements. Refer to the *Structures* chapter for the design criteria of cut-and-cover structures. When referred to the *Structures* chapter, replace *cut-and-cover structures* with *bored/mined tunnel*.

This document applies to the design of new structures where High-Speed Trains (HST) are permitted to operate exclusively.

### 13.2 Regulations, Codes, Standards, and Guidelines

Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards. Key resource materials providing guidance useful for tunnel design and construction issues include:

- NFPA 130, Standard for Fixed Guideway Transit and Passenger Rail Systems, 2010 Edition
- California Code of Regulations (CCR), Title 8, Chapter 4, Subchapter 20 – Tunnel Safety Orders
- California Public Utilities Commission (CPUC) General Orders (GOs), Public Utility Codes, Rules of Practice and Procedure, and the Policies and Guidelines
- International Union of Railways (UIC), 779-11R – Determination of Railway Tunnel Cross-Sectional Areas on the Basis of Aerodynamic Considerations
- Federal Highway Administration (FHWA), Technical Manual for Design and Construction of Road Tunnels - Civil Elements, FHWA-NHI-09-010
- American Association of State Highway and Transportation Officials (AASHTO), Manual on Subsurface Investigations, MSI-1
- U.S. Army Corps of Engineers, Engineering and Design, Tunnels and Shafts in Rock, Manual No. 1110-2-2901
- Construction Industry Research and Information Association (CIRIA), Prediction and Effects of Ground Movements Caused by Tunneling in Soft Ground Beneath Urban Areas, CIRIA Funders Report CP/5

- 1 • California Department of Transportation (Caltrans)
  - 2 – Caltrans Highway Design Manual
  - 3 – Caltrans Project Development Procedures Manual
- 4 • American Railway Engineering and Maintenance of Way Association (AREMA) Manual for
- 5 Railway Engineering

### **13.3 Basic High-Speed Train Tunnel Configuration and Geometry**

#### **13.3.1 Train length**

- 6 Tunnels shall be designed for train lengths of 660 feet to 1,312 feet with an additional 1 percent
- 7 trainset length tolerance.

#### **13.3.2 Train Operating Speed**

- 8 Tunnels shall be designed to allow for train operations at speeds of up to 220 miles per hour
- 9 (mph).

#### **13.3.3 Lining Type**

- 10 The type of lining to be implemented for any particular underground structure shall be
- 11 determined by the designer, provided that all performance criteria have been demonstrated to
- 12 have been satisfied during the design process.
- 13 All tunnels and underground structures forming part of the permanent works may have
- 14 structural lining consisting of one or more of the following lining types:
  - 15 • Unreinforced or reinforced concrete cast in-situ lining
  - 16 • Unreinforced or reinforced precast concrete segmental lining
  - 17 • Reinforced sprayed concrete lining

#### **13.3.4 Tunnel Configuration**

- 18 The following tunnel configurations shall be considered:
  - 19 • Single-track twin bored tunnel
  - 20 • Single-track twin mined tunnel
  - 21 • Twin-track single bored tunnel with a separation wall
  - 22 • Twin-track single mined tunnel with a separation wall

1 Twin-track single bored/mined tunnel without a separation wall may be permitted at certain  
2 speeds and tunnel length subject to a design variance approval.

3 Fire and life safety and passenger-evacuation strategy must be considered when establishing  
4 the appropriate tunnel configuration.

### 13.3.5 Clearances

5 Required clearances to the HST trackway and facilities are outlined in the *Trackway Clearances*  
6 chapter. Refer to the *Trackway Clearances* chapter for the static envelope, dynamic envelope, and  
7 fixed equipment envelope.

8 Refer to the Standard and Directive Drawings for OCS and Pantograph envelope. Tunnel cross-  
9 sections shall accommodate intermediate and continuous tunnel facility equipment.

10 Refer to the Standard and Directive Drawings for tunnel configurations and sizes and typical  
11 cross-section drawings.

12 The following continuous equipment may be located in the free tunnel cross-sectional area. The  
13 size of infrastructure is indicative.

- 14 • 3 x 6 inch outer diameter (OD) high voltage electric cables per track
- 15 • 10 x 3 inch OD low voltage electric cables
- 16 • 8-inch OD dry standpipe (fire line)
- 17 • 2-inch OD emergency air pipe
- 18 • 13-inch OD sump pump discharge pipe
- 19 • 1-inch OD leaky feeder cable
- 20 • 1-inch OD earthing tape and corrosion protection
- 21 • 2-inch OD handrail
- 22 • Tunnel air cooling circulatory water system (if required)
- 23 • Communications and signaling equipment
- 24 • Trackside concessionaire radio cables
- 25 • OCS and feeder wires
- 26 • System-wide safety ground conductor
- 27 • Sliding crosspassage doors (only required in double track tunnels with separation wall  
28 between tracks)

29 If the total cross-sectional area of the equipment is undefined, a minimum allowance of 20  
30 square feet shall be provided for assessing the minimum tunnel free cross-sectional area.

The following intermittent equipment may be located in the free tunnel cross-sectional area, and a space allowance shall be necessary outside the fixed equipment envelope. No allowance in the free cross-sectional area is necessary as the equipment is not continuous.

- OCS assembly
- Pulley wheels and mounting hardware for OCS
- Signage
- Tunnel walkway lighting
- Emergency radio antennae
- Fire telephone box
- Blue light station (at crosspassages)
- Rail lubricator tank
- Train control/signal case
- Fire extinguishers
- Fire isolation valves
- Ventilation equipment such as jet fans (if required)
- Low voltage and 25kV disconnect switch and mounting hardware
- Auxiliary transformers
- Wayside power cubicle
- Trackside Automatic Train Control (ATC)/Communications equipment

See Standard and Directive Drawings for typical cross-sections of tunnel with disconnect switch, auxiliary transformer, or wayside power cubicle.

### **13.3.6 Walkways**

Each track shall be paralleled by 1 walkway along the full length of the tunnel. In the case of twin tunnels with a single track in each tunnel, the safety walkways will be located on the side of crosspassages. In the case of twin-track single tunnels with a separation wall, the safety walks shall be located on the side of the track adjacent to the separation wall as specified in the *System Safety and Security* chapter.

The walkway envelope shall be unobstructed to a height of 7 feet—6 inches and 3 feet maximum width as shown in the *Trackway Clearances* chapter.

The edge of walkway concrete shall be located at a lateral distance equal to a minimum of 6 feet-3 inches from the track centerline and 8 inches vertically above top of rail to ensure dynamic clearance of the Rolling Stock. The distance from the walkway envelope to the track

centerline has been set to establish the tunnel diameter consistent with the assumption that personnel shall not be in the tunnel outside the train during revenue operation.

No permanent or temporary obstacles such as pipes, conduits, lights, signs, signals or stored materials are permitted within the stated clearance.

Handrails shall be installed 3 feet-6 inches above walkway providing a route to a safe area. The handrail shall not interfere with the stated clearance envelopes, including the walkway clearance envelope.

### **13.3.7 Trackwork**

A direct fixation (non-ballasted) trackform is preferred in tunnels and is standard practice in new construction for rail tunnels due to its low maintenance requirements. Maintenance is a key consideration in tunnels due to restricted access, limited space for tamping activities, and short possession times. The invert concrete for bored and mined tunnels shall allow sufficient space for the systemwide cable trough, drainage, and high voltage power conduits.

### **13.3.8 Escape Shafts and Escape Tunnels**

Refer to the System Safety and Security chapter for details on the intervals of escape shafts and escape tunnels for various tunnel configurations.

The minimum dimensions of the door opening shall be 6-feet wide by 7-feet-6-inches high. All exits shall be equipped with lighting and signs. Refer to Standard and Directive Drawings and *System Safety and Security* chapter for details.

### **13.3.9 Crosspassages**

Crosspassages shall have horseshoe-shaped or circular cross-sectional profile depending on the construction methodology.

Minimum dimensions of walkway envelope in the crosspassage shall be 6-feet wide by 7-feet-6-inches high. The minimum dimensions of the doors shall be 6-feet wide by 7-feet-6-inches high.

There shall be adequate space in the invert of the crosspassages for drainage and cable trough(s).

See the *System Safety and Security* chapter for further crosspassage requirements and the *Civil* chapter for crosswalk configuration.

### **13.3.10 Equipment Requirements and Tunnel Niches**

Arrangements and locations of the fixed equipments will vary at tunnel enlargements, niches, crosspassages and interfaces with other tunnel and structural sections.

1 Refer to Standard and Directive Drawings showing niche and recess details. Niches and  
2 enlargements present no difficulties in mined tunnels but may present a significant construction  
3 challenge in bored soft ground tunnels where groundwater pressure is present.

4 The effects of discontinuities introduced by the niches to the maximum pressure variation shall  
5 be taken into account during design.

### **13.3.11 Rolling Stock**

6 Rolling stock profiles for candidate high-speed rail Rolling Stock were prepared to enable  
7 calculation of the cross-sectional area of the Rolling Stock. Refer to the Standard and Directive  
8 Drawings in the *Trackway Clearances* chapter. The information was derived from a variety of  
9 sources and requires verification following selection of the high-speed Rolling Stock. The cross-  
10 sectional area of Rolling Stock shall be relied on only for the purpose of calculating the free  
11 tunnel cross-sectional area required to comply with the Technical Specification for  
12 Interoperability (TSI) medical health criteria and aerodynamic drag calculations. When the HST  
13 Rolling Stock is selected, the free tunnel cross-sectional area calculations shall be finalized in  
14 accordance with the Rolling Stock criteria.

### **13.3.12 Aerodynamic Considerations**

15 The design of tunnels and underground structures shall satisfy the Aural Comfort Criteria and  
16 the Medical Health Criteria as required by UIC 779-11R "Determination of Railway Tunnel  
17 Cross Sectional Areas on the Basis of Aerodynamic Considerations."

18 To satisfy the Aural Comfort Criteria, co-efficient of pressure tightness,  $\tau$ , shall be no less than  
19 25 seconds.

20 To satisfy the Medical Health Criteria, the maximum pressure variation in tunnels and  
21 underground structures along any train shall not exceed 1.45 psi during the time taken for the  
22 train to pass through the tunnel at the maximum permitted speed.

23 The maximum pressure variation in tunnels and underground structures for any train intended  
24 to run in the specific tunnel shall not exceed 1.45 psi during the time taken for the Rolling Stock  
25 to pass through the tunnel at a maximum speed of 220 mph.

26 The free cross-sectional area of the tunnel, based on the Blockage Ratio, shall be determined so  
27 as to comply with the maximum pressure variation permitted, taking into account all the types  
28 of traffic planned to run in the tunnel at the maximum speed at which the respective vehicles  
29 are authorized to run through the tunnel.

30 Features may be used on Rolling Stock and infrastructure, including sealing of Rolling Stock,  
31 closure of air conditioning inlets and outlets, tunnel entrance shape, shafts, etc., which reduce  
32 the pressure variation in the tunnel or the Rolling Stock to the allowable pressure variation of  
33 1.45 psi.

The designer shall confirm the maximum allowable pressure variations and acceptable medical limits using acceptable, verifiable and certified data from existing revenue operation, trials, testing, research, Technical Specification for Interoperability, EN 14067: Railway Applications – Aerodynamics, UIC guideline 779-11R, and/or other equivalent or superior national and international codes, guidelines or practice.

The following programs may be used to aid the design process:

- SEALTUN – Used for the determination of a minimum cross-sectional area of railway tunnels for sealed trains based on the Aural Comfort Criteria and Medical Health Criteria.
- TRENDSHAFT – Used to show the influence of various numbers of airshafts on the maximum pressure changes for a large number of operating scenarios.
- DE-SHAFT – Used to calculate the indicative train pressures for tunnels with airshafts.

### 13.3.13 Minimum Tunnel Cross-Sectional Areas

The free cross-sectional area is defined as follows:

$$\text{Free cross-sectional area} = T - (\text{RS} + \text{INV} + \text{FE} + \text{TOL})$$

Where:

T = the cross-sectional area of the tunnel

RS = the cross-sectional area of the Rolling Stock

INV = the cross-sectional area of the invert concrete

FE = the sum of the continuous fixed equipment area

TOL = the construction tolerance of the tunnel

Based on a sealed-train cross-section of 150 square feet and train length of 1,312 feet, and an unsealed-train cross-section of 140 square feet and (tunnel length/train length) ratio of 3.4, the following criteria shall be met:

- For single-track tunnel configuration, including double-track tunnel with separation wall where the operational speed is 220 mph (dedicated high-speed passenger train tunnels), the tunnel free cross-sectional area for each track shall not be less than 630 square feet.
- For twin-track tunnel configuration, including double-track tunnel without separation wall where the operational speed is 220 mph (dedicated high-speed passenger train tunnels), the tunnel free cross-sectional area shall not be less than 1,615 square feet.



#### **13.3.14 Bored Tunnel**

A circular cross-sectional profile shall be adopted for tunnel boring machine or shield driven tunnels. The main running tunnels shall be designed to be supported by precast concrete segmental linings or sprayed concrete lining installed as the excavation progresses. This excavation method shall be applied to underground excavations for which one or more of the following applies:

- Long tunnel drives
- In urban areas where ground settlement must be limited
- Tunneling in soft ground
- Tunneling in water bearing ground.

#### **13.3.15 Mined Tunnel**

A horseshoe-shaped cross-sectional profile shall be adopted for mined tunnels. This excavation method shall be applied to underground excavations for which one or more of the following applies:

- Openings are of short lengths
- Tunneling in conditions typically not deemed suitable for full face machine driven tunnels
- Tunnel cross-section is large
- Overburden is shallow
- Tunnel geometry is complex and includes construction of chambers, crosspassages, bifurcations, crossovers, widening and other transitional structures.

#### **13.3.16 Transition between Cut-and-Cover and Bored Tunnel**

The transition between the cut-and-cover and bored tunnel will introduce discontinuity on the surface of the tunnel. This discontinuity shall be eliminated through design or modeled in a program to confirm that it satisfies the ventilation requirements and the Aural Comfort Criteria and the Medical Health Criteria.

#### **13.3.17 Ventilation Shafts**

The requirement for ventilation shafts is dependent on, but not limited to the following:

- Tunnel configuration
- Size and length of the running tunnels
- Type and frequency of the Rolling Stock
- Fire/Life Safety Strategy

## 13.4 Tunnel Portals

Portal design shall specifically take into account the following:

- Topographical conditions
- Geotechnical conditions
- Aerodynamic conditions (piston effects)
- Method of construction
- Environmental conditions
- Drainage

Wherever possible, tunnel portals shall be designed to minimize permanent cut slopes to limit the extent and scope of disturbance of existing ground conditions, including groundwater regimes and drainage, environment and visual aspects, and maintenance. This shall be achieved by construction of cut-and-cover structures and backfill to the original slope angle at bored or mined tunnel portals wherever the conditions in Section 13.16 allow.

Tunnel portals shall be designed to blend in with the surroundings.

### 13.4.1 Sonic Booms at Tunnel Portals

To mitigate aerodynamic effects, especially sonic boom, portal structures shall be inclined at least 45 degrees from the vertical. Where there are buildings near tunnel portals or there are special environmental requirements, portal hood structures shall be arranged in accordance with the requirements in Table 13-1.

**Table 13-1: Requirements for Arrangement of Portal Hood Structure**

Distance from buildings to tunnel portals	Are there any special environmental requirements for the buildings?	Monitoring point	Peak values of micro-pressure waves
Less than 150 feet	Yes	Building	As per the requirements
	No		$\leq 0.42 \text{ lbs/ft}^2$
More than or equal to 150 feet	Yes	Places 70 feet from tunnel portals	$< 1.05 \text{ lbs/ft}^2$

The type and length of the trains, the length of the tunnels, the effective area of the clearances of the tunnels, the type of the tracks in the tunnels and the topographic conditions and the residential situations near the tunnel portals shall be taken into account in the arrangement of the portal hood structures.

1 The design of the portal hood structures shall comply with the following:

- 2 • The types of the hood structures shall be determined with a practical and aesthetic view and  
3 in accordance with the environmental conditions near the portals. Perforated hood  
4 structures that have the shape similar to that of the cross-sections of the lined tunnels shall  
5 be adopted.
- 6 • In case the cross-sections of the hood structures remain the same, pressure-releasing  
7 apertures shall be arranged on the side and top of the hood structures. The area of the  
8 pressure-releasing holes may be determined in accordance with the actual conditions and  
9 shall be 1/5 to 1/3 of the effective area of the tunnel clearance.
- 10 • Reinforced concrete shall be adopted for the hood structures.
- 11 • In case of tunnel portals where hood structures are to be arranged, retaining walls of the  
12 subgrade, if any, shall be placed beyond the scope of the buffer structures.

#### **13.4.2 Portal Layouts**

13 Portal infrastructure elements are described in Section 13.16.

14 Wherever possible, portal and shaft facilities, including ventilation buildings, shall be located  
15 away from the tunnel or shaft entrances so that if required, portal facilities can form separate  
16 construction contracts from tunnel construction. Portal facilities and tunnels construction shall  
17 then be carried out in parallel to optimize the schedules, to minimize knock on effect of delays,  
18 and to take construction interfaces off the critical path of construction activities.

19 If tunnel portals are crossed over by highways, the highways shall be provided with protection  
20 fences and monitoring devices.

21 If the distance between the portals of two neighboring tunnels is less than 100 feet, the two  
22 tunnels shall be connected by an open tunnel.

#### **13.4.3 Slope Failure or Landslide**

23 Portal design shall minimize the risk of land slip and loosened soil or rock that may result in  
24 material falling onto the tracks at any time, including during a seismic event. See Standard and  
25 Directive Drawings and the *Geotechnical* chapter for prediction, prevention, and design  
26 parameters for landslide.

#### **13.4.4 Unsymmetrical Pressure**

27 Unsymmetrical pressure may act on the tunnel section and large stresses may develop.  
28 Measures shall be taken to balance the earth pressure by using counterweight fill or cut for  
29 slope stabilization.

#### 13.4.5 Insufficient Bearing Capacity of Ground

The ground in the portal zone may consist of unconsolidated deposits or in weathered rock. Design shall ensure that there will be no excessive settlement or deformation at the portal zone due to insufficient bearing capacity.

#### 13.4.6 Rock Fall or Debris Flow

The portal zone shall be located to minimize risk of rock falls and debris flows. When this is not practicable, adequate measures shall be taken to mitigate against rock falls and debris flows.

Rock fall and debris diversion or containment features (traps) utilizing trench excavations and berms shall be constructed to positively assure that no rocks, rock slides, or other debris such as soil or snow slides from mountain slopes above the portal area can reach the tracks or damage equipment or structures. Appropriate simulation software may be used to support determination of trench depths and fence sizes.

Where a portal opens directly from a very steep slope or cliff, such that a simple rock trap is not practicable, then the noise mitigation hood or other hood protection shall be extended far enough from the face of the cliff to protect the tracks and other structures. Protective structures shall be designed to withstand impact loads from all rock falls or slides that can be reasonably anticipated.

Each individual situation shall be evaluated through site-specific risk-assessment analysis.

See Standard and Directive Drawings and the *Geotechnical* chapter for designs to mitigate against rock falls and debris flows.

#### 13.4.7 Adjacent Structures

Impacts to the surrounding area as a result of construction activities (such as noise and dust) shall be considered when locating a portal zone.

### 13.5 Tunnel Ventilation Requirements and Configurations

#### 13.5.1 Mechanical Ventilation

*Mechanical ventilation requirements will be developed based on the equipment and operating characteristics of HST.*

#### 13.5.2 Ventilation Configurations

Mechanical tunnel ventilation shall consist of one or more of the following configurations:

- Jet fans in the tunnel
- Axial fans located at tunnel portals

- 1 • Axial fans and saccardo nozzles located at tunnel portals
- 2 • Axial fans located at ventilation shafts or ventilation tunnels

### **13.5.3 Design Considerations**

3 The mechanical tunnel ventilation configuration shall be defined for each tunnel by  
4 consideration of the following factors:

- 5 • Safety
- 6 • Efficiency
- 7 • Maintenance
- 8 • Redundancy
- 9 • Reliability
- 10 • Fixed equipment and civil infrastructure (including access)
- 11 • Economics

### **13.5.4 Train Operation**

12 For tunnel lengths of less than 6 miles, the following conditions shall be assumed:

- 13 • For twin bored or mined tunnels, one train is in each tunnel.
- 14 • For single mined tunnels with a separation wall (two train ways), one train is in each  
15 trainway.
- 16 • For single bored tunnels without a separation wall, one train is in the tunnel.

### **13.5.5 Maintenance Vehicles**

17 The mechanical tunnel ventilation shall also be designed for the maintenance vehicles operating  
18 in or passing through the tunnel.

### **13.5.6 Access Shafts and Tunnels**

19 If access shafts or tunnels are required by the tunnel ventilation design, these shall also be used,  
20 wherever possible, for mechanical tunnel ventilation.

### **13.5.7 Crosspassages**

21 In order to prevent strong fluctuations of air movement while preventing internal re-circulation  
22 of air within the crosspassage, one side of a crosspassage shall be airtight during normal  
23 operation.

## 13.6 Materials

### 13.6.1 Concrete

- 1 Refer to the *Structures* chapter for reinforced concrete design requirements.

### 13.6.2 Structural Steel

- 2 Refer to the *Structures* chapter for structural steel design requirements.

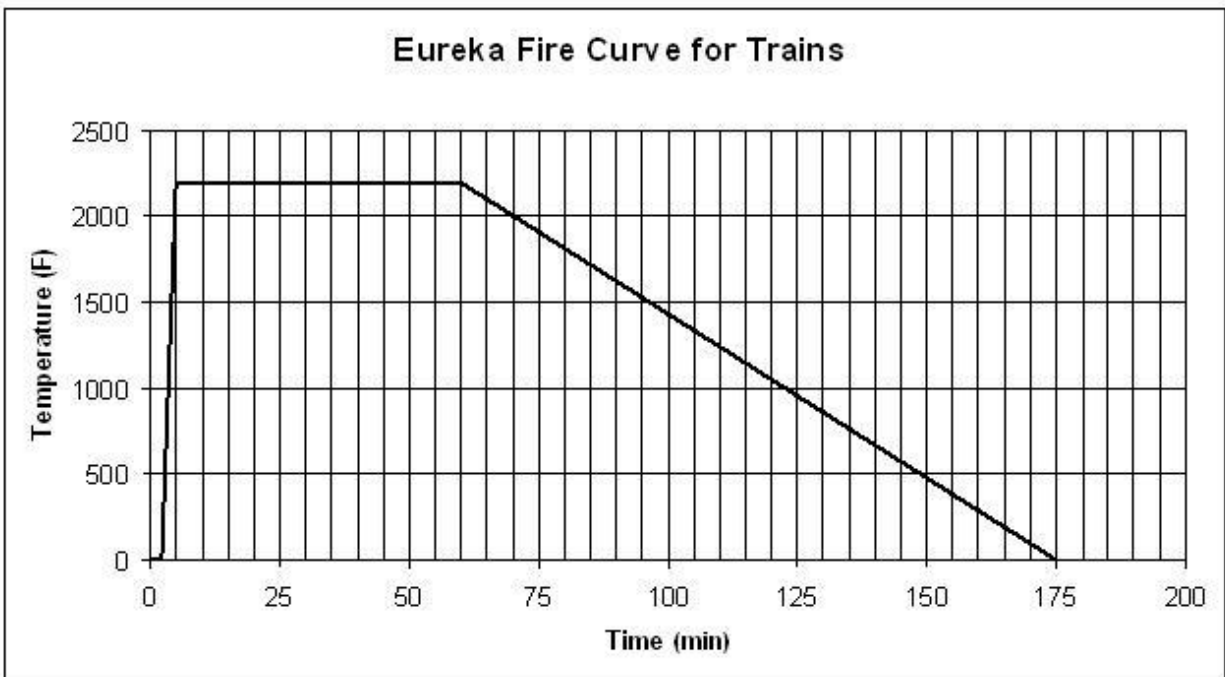
### 13.6.3 Durability

- 3 Refer to the *General* chapter for the design life of structures.
- 4 The assessment shall consider the following factors that influence lining durability, including
- 5 but not limited to the following:
  - 6 • Acid attack
  - 7 • Chlorides and sulfate attack
  - 8 • Alkali silica reaction
  - 9 • Carbonation
  - 10 • Watertightness
  - 11 • Wetting and drying
  - 12 • High ambient temperatures
  - 13 • Stray current
  - 14 • Cast in items and fixings
  - 15 • Coating to reinforcement
  - 16 • Water/cement ratio
  - 17 • Gas and water permeability
  - 18 • Corrosion protection
  - 19 • Use of fiber-reinforced concrete
  - 20 • Contaminated ground

## 13.7 Fire Protection

- 21 The fire performance of the finished tunnel surface shall withstand the temperature of the fire
- 22 for a specified duration of time. The specified “temperature-time curve” is defined by the
- 23 EUREKA Fire Curve for Trains in Figure 13-1:

1 **Figure 13-1: EUREKA Fire Curve for Trains**



2  
3  
4 Tunnels and underground structures shall be designed and constructed with non-flammable  
5 materials in accordance with NFPA 130.

6 Tunnels with precast, high-strength concrete elements shall be protected such that explosive  
7 spalling is prevented.

8 Walking surfaces designated for evacuation of passengers shall be constructed of  
9 noncombustible materials.

10 Remote vertical exit shafts and ventilation structures shall be not less than Type I (332)  
11 noncombustible construction as defined in NFPA 220.

12 Ancillary areas shall be separated from trainway areas within underground trainway sections  
13 by a minimum 2-hour fire resistive construction.

## 13.8 Design Loads

### 13.8.1 Design Load Considerations

14 The design shall be carried out by a consistent method using two-dimensional approximations  
15 or a three-dimensional analysis. However, the designer shall provide examples of where the  
16 method has been used before for tunnel design, and verification of the assumptions made about  
17 the behavior of the ground or the lining.

1 Live loads over the tunnel shall be considered when the depth of overburden is less than the  
2 tunnel width.

3 Refer to the *Structures* chapter for design load combinations and effects to underground  
4 structures.

### 13.8.2 Ground Considerations

5 Requirements for geotechnical design, including requirements for the field and laboratory  
6 investigations to be performed, are described in the *Geotechnical* chapter.

### 13.8.3 Groundwater Considerations

7 Tunnels shall be designed as undrained (i.e., with waterproofing) with the objective of  
8 eliminating impacts to groundwater and surface water resources due to groundwater  
9 drawdown. In the event that groundwater pressures are excessive, the designer may propose  
10 design alternatives to the undrained tunnel criterion. The designer shall demonstrate that their  
11 proposed alternative shall not impact the groundwater or surface water resources due to  
12 groundwater drawdown. Performance of equivalent designs alternative to an undrained tunnel  
13 shall meet criteria for protection of groundwater resources described herein and accepted by  
14 regulatory agencies having jurisdiction.

15 The groundwater data shall be used for the following:

- 16 • Design water management and dewatering programs
- 17 • Evaluate influences of dewatering on existing structures and on project excavations
- 18 • Evaluate impacts on surface and groundwater resources
- 19 • Determine water discharge volumes and treatment requirements (chemical)
- 20 • Calculate uplift pressures on excavations.

21 For further hydrostatic pressure requirements, refer to the *Structures* chapter.

### 13.8.4 Seismic Load Considerations

22 For seismic design criteria for underground structures, refer to the *Seismic* and *Geotechnical*  
23 chapters.

### 13.8.5 Train Derailment Loads

24 For train derailment load requirements for underground structures, refer to the *Structures*  
25 chapter.



### **13.8.6 Loading Combinations and Load Factors**

- 1 Refer to FHWA-NHI-09-010 Road Tunnel Manual and *Structures* chapter for the loading  
2 combination requirements and safety factors.

### **13.8.7 Aerodynamic forces**

- 3 The design and installation of fixed equipment in tunnels shall take into account in combination  
4 with dead and live loads, the aerodynamic effect from passing trains, which produces a  
5 traveling wave of alternating pressure and suction along the tunnels.  
6 Refer to the *Structures* chapter for Slipstream Effects (SS) Loads.

### **13.8.8 Flotation**

- 7 Tunnels and underground structures shall be designed to resist flotation forces. The upper or  
8 lower bound of the groundwater level shall be taken into account in deriving hydrostatic loads,  
9 whichever is more critical for the structure or condition being considered.

### **13.8.9 Construction Methods**

- 10 Segmental linings shall be designed for all handling, stacking, and erection forces with an  
11 allowance for impact or dynamic loading due to handling. Where it is proposed to use  
12 tunneling shields or machines, the lining shall be designed to resist all forces that may be  
13 applied by such equipment.  
14 The distribution of ground pressures shall consider the probable construction method, the  
15 relative rigidity of the lining and the interaction of the lining with the ground. The design of  
16 tunnels in soil shall not allow for any long-term load relief by ground arching over the crown  
17 (i.e., full overburden pressure shall be assumed).

### **13.8.10 Temporary Structures**

- 18 Temporary structures shall be designed taking into account all necessary temporary works and  
19 special construction techniques required as part of the construction of the permanent works.

### **13.8.11 Effects from Adjacent Structures**

- 20 For surcharge loads due to adjacent buildings, refer to the *Structures* chapter.  
21 Additional loads due to the driving of adjacent tunnels shall be considered.

## **13.9 Structural Analysis**

### **13.9.1 Concrete Design**

- 22 Refer to the *Structures* chapter for concrete design requirements.

In addition to dead, live, hydrostatic, and seismic loads in the permanent condition, concrete structures shall be designed for handling loads arising during manufacture and construction.

### 13.9.2 Steelwork Design

Refer to the *Structures* chapter for steelwork design requirements.

In addition to dead, live, hydrostatic, and seismic loads in the permanent condition, steel structures shall be designed for handling loads arising during manufacture and construction.

### 13.9.3 Dynamic Analysis

The scope of the dynamic analysis shall include, but not be limited to, the following:

- Determination of vertical and horizontal modes and amplitudes of vibration in track support elements
- Identification of those modes and amplitudes of vibration that may be excited by the proposed Rolling Stock
- Estimation of the magnitude and frequency of the stress induced in the track support elements due to modes of vibration that may be excited by the proposed Rolling Stock
- Determination from the results of the dynamic analyses of the actual dynamic factors that are appropriate to be used in the design of the track support elements
- Estimation of the vertical and horizontal rail head accelerations

Both longitudinal and transverse structure geometry shall be analyzed and designed when considering dynamic effects.

Refer to the *Structures* chapter for trainset traction and braking forces.

### 13.9.4 Seismic Analysis

Refer to the *Seismic* and *Geotechnical* chapters for seismic analysis procedure.

The additional earthquake forces shall be applied in conjunction with any pre-existing soil loads (active or at-rest as appropriate) and water loads if applicable. When deriving the dynamic component of earthquake load, the bulk unit weight shall be used for soil above and below water level since the pore water will move with the soil particles and experience the same acceleration.

### 13.9.5 Fatigue Analysis

For both concrete and steelwork structures, stress ranges induced by the Rolling Stock shall be combined with any other cyclic effects due to the Rolling Stock or other loading to which the

1 structure may be subject when considering cumulative damage. Refer to the *Structures* chapter  
2 for fatigue analysis procedures.

### **13.9.6 Mining in Rock**

3 In accordance with Chapter 6 of FHWA-NHI-09-010, design of tunnels and other underground  
4 excavations in rock shall consider all potential rock stresses, failure modes, and difficult ground  
5 and any combination of conditions that can adversely impact tunnel construction and  
6 performance.

### **13.9.7 Tunneling in Soft Ground**

7 In accordance with Chapter 7 of FHWA-NHI-09-010, design of tunnels and other underground  
8 excavations in soft ground shall consider excavation method, potential ground movement, type  
9 of ground support, and any combination of conditions that can adversely impact tunnel  
10 construction and performance.

### **13.9.8 Portals and Shafts**

11 All cut slopes and excavations shall be designed for any possible mode of failure to meet  
12 minimum factors of safety that are 1.3 or greater for short-term construction (temporary slopes  
13 or construction slopes) and 1.5 or greater for permanent or final design slopes. These factors of  
14 safety shall apply to both soil and rock slopes for all portal and shaft excavations. The designer  
15 shall evaluate all ground conditions affecting stability of the portal or shaft that may include  
16 overburden excavation, weathered rock and unweathered rock in accordance with FHWA-NHI-  
17 09-010. The modes of failure shall be modeled using two-dimensional geologic cross-sections  
18 and/or three-dimensional modeling for use in conducting slope stability analysis using limit  
19 equilibrium methods outlined in the *Geotechnical* chapter to determine the factors of safety of  
20 each slope and slope support methods.

### **13.9.9 Enlargements**

21 Enlargements shall be designed based on numerical methods. If enlargements are of complex  
22 geometry, three-dimensional analysis shall be performed.

## **13.10 Ground Movement**

### **13.10.1 Purpose of Predicting the Ground Movement**

23 The potential ground movement associated with the construction method shall be determined  
24 for the following purposes:

- 25 • To demonstrate that the environmental effects of the tunneling-induced ground movement  
26 have been considered and taken into account.

- To draw attention to those zones where the implementation of the design is likely to cause ground movement.
- To assess the risks of damage associated with the design, to investigate alternatives, and modify the design as necessary.
- If avoidance is not possible, to design protective measures against the likely damage, which measures could include settlement mitigation such as compensation grouting that could themselves have effects requiring assessment.

### 13.10.2 Method of Analysis

Settlement assessment shall be carried out in the following three phases:

- Phase 1 – The shape of the settlement trough resulting from tunneling activities shall be determined by empirical methods such as those described by CIRIA Funders Report CP5 using parameters for ground loss and trough width, which shall be determined from case histories, and taking into account the method of tunneling.

All structures within the predicted zone of settlement shall be categorized by risk in accordance with the damage risk classification in Burland et al (1974). Any building or structure where the predicted settlement from tunneling is less than 0.4 inch and the predicted ground slope is less than 1/500 (equivalent to damage risk category 1 in Burland et al [1974]) shall not be subject to a further assessment. All other buildings or structures shall be subject to a Phase 2 assessment.

- Phase 2 – Buildings and structures subject to settlement, and falling under risk category 2 or greater (see Burland et al [1974]), shall be individually assessed using a limiting tensile strain approach as described by Burland et al (1974), and Boscardin and Cording (1989). Each structure shall also be subject to an inspection to determine its structural form and condition. Each building or structure shall be categorized into one of the damage categories in Burland et al (1974).
- Phase 3 – All buildings or structures that fall under risk category 3 or greater (see Burland et al [1974]), following the Phase 2 assessment, shall be subject to further analysis and assessment using numerical analysis taking into account the soil–structure interaction. The response of such buildings and structures to tunneling induced settlement shall be assessed and appropriate preventative or protective measures identified.

### 13.10.3 Protection of Adjacent Assets

The design process shall include an assessment of the impact of construction on Third Party assets. In this respect, the designer shall assemble as far as reasonably practicable all available records of foundations and other structures/artificial obstructions which could affect and/or be affected.

## 13.11 Measurement and Monitoring

### 13.11.1 Instrumentation and Monitoring Design

- 1 A system of instrumentation and monitoring of ground movement during construction shall be  
2 implemented appropriate to the ground conditions and the methods proposed for construction.  
3 The design shall include locations of instruments, frequency of monitoring, and how the  
4 monitoring will be used to control the construction progress. The following shall be carried out:
- 5 • Verify the predictive ground movement and the effects that such ground movement will  
6 have on adjacent assets by measurement.
  - 7 • Establish monitoring base readings of the construction activity sufficiently in advance to  
8 ensure underlying and seasonal environmental trends are understood.
  - 9 • If the results of monitoring indicate an issue, implement corresponding contingency plans.

## 13.12 Waterproofing

### 13.12.1 Requirements

- 10 Tunnels and underground structures shall be designed and constructed to be undrained.  
11 Provisions shall be made to facilitate repair or restoration to achieve a dry tunnel in the event  
12 that leakage occurs after construction.
- 13 Refer to the *Structures* chapter for waterproofing requirements.

### 13.12.2 Waterproofing Measures

- 14 The design of water resistance measures shall follow the procedure described in *Structures*  
15 chapter and cater for the effects of long-term ground movements and movements associated  
16 with construction of adjacent structures.
- 17 Precast concrete lining shall consist of a gasket system.
- 18 Secondary linings such as in-situ concrete and sprayed concrete linings shall be protected by an  
19 external water resistant membrane, and construction joints shall be provided with water stops.

### 13.12.3 Junction between New and Existing Structures

- 20 The junction between new and existing waterproofing is a potential source of leakage. The  
21 design shall take into consideration the following.
- 22 • New work shall be taken to a point that will provide a strong waterproof joint (i.e., the  
23 finished structure levels shall be the same to prevent a “well” or “dam” forming).

- The new and existing waterproofing shall be compatible. If the new waterproofing is a repair, then spray-applied membranes are recommended.

## 13.13 Drainage

### 13.13.1 Tunnels and Underground Structures

The internal drainage system shall collect condensation, leakage, spilled water, and other flows. The design shall provide space for the drainage system and the variation in position of the structure due to tolerances. Tunnel drainage sumps shall be provided at the lowest point of each section.

### 13.13.2 Portals

Adequate drainage shall be provided to ensure the stability of the slopes at the portal. Water from this drainage shall be directed away from the tunnel entrance.

Provision shall be made to safeguard against flooding from surface drainage and streams.

## 13.14 Special Ground Conditions

### 13.14.1 Design of Tunnels and Underground Structures

The design of tunnels and underground structures shall be based on the design parameters and groundwater levels presented and developed based on the Geotechnical Reports and new data obtained by the contractor.

The design issues related to special ground conditions are in addition to what is stated in the *Geotechnical* chapter.

### 13.14.2 Squeezing Ground

In the design and construction of the tunnels in squeezing ground, the distribution and characteristics of the ground shall be fully understood by a preliminary survey and testing before construction. Investigations, observations, and measurement shall be carried out during construction to allow appropriate measures to be taken.

The lining shall be shaped as close to circular as possible, and the perimeter of the cross-section shall be closed.

### 13.14.3 High-Pressure Water Inflows

In the design and construction of a tunnel to be excavated through ground where there is a risk of high-pressure water inflows in large quantities, an unexpected gush of water shall be avoided in assimilating results of investigations carried out before and during construction.

1 Where there is a danger of particularly large amounts of water inflow, the following measures  
2 shall be taken that are appropriate for the ground and aquifer:

- 3 • Drainage of water or lowering of the groundwater level
- 4 • Chemical injection, cement grouting or freezing of the ground to improve its properties,  
5 reduce permeability, and cut off the water

#### **13.14.4 Gases Dissolved in Groundwater**

6 In the design of tunnels through ground with a risk of gases dissolved in groundwater, a  
7 comprehensive investigation of the topography, geology, and groundwater shall be carried out  
8 during design.

#### **13.14.5 Geothermal Ground**

9 In the design of tunnels through ground with high levels of geothermal energy, a  
10 comprehensive investigation of the topography, geology, and groundwater shall be carried out  
11 during design.

#### **13.14.6 Gassy Ground**

12 Gassy ground conditions shall be classified according to the definitions in CCR Title 8. The  
13 CCR-mandated actions for each tunnel classification shall be applied to design and  
14 construction. Poisonous and explosive gasses shall be anticipated and evaluated in all areas of  
15 petroleum-bearing geologic materials, especially within or near known active or abandoned oil  
16 fields.

#### **13.14.7 Rock Bursts**

17 For the design of tunnels and underground structures located where there is a high risk of rock  
18 bursts, an investigation shall be carried out in advance so that any necessary precautionary  
19 measures can be taken.

#### **13.14.8 Fault Zones**

20 Refer to the *Seismic* chapter for seismic requirements.

#### **13.14.9 Liquefaction**

21 In areas where geotechnical investigation for the tunnels (or that of the planned or joint  
22 development construction) suggests a potential for the seismic consolidation or liquefaction of  
23 the soil, a special structural and geotechnical analysis of both designs shall be undertaken. Refer  
24 to the *Geotechnical* chapter.

#### 13.14.10 Landslips

The designer shall identify and design for all existing and potential landslips that may impact the tunnel alignment, portal location, and design. In particular, allowances shall be made for mitigation measures such as real-time remote ground-movement monitoring, drainage, additional clearances, and repair of lining.

### 13.15 System-wide Interfaces

#### 13.15.1 Catenary Support Provisions

Refer to the *Overhead Contact System and Traction Power Return System* chapters.

#### 13.15.2 Warning Systems and Location Cases

Design of monitoring systems or detectors is required for the following hazards:

- Strong winds (*General* chapter)
- Snow and freezing conditions (*General* chapter)
- Heavy rain and floods (Track Level) (*General* chapter)
- Falling rocks (*Geotechnical* chapter)
- Landslide (*Geotechnical* chapter)
- Fault movement (*Geotechnical* chapter)
- Hazardous gas (*Geotechnical* chapter)

The designer shall advise on appropriate locations and design principles for these devices, depending on the details of the design. Refer to the *Supervisory Control and Data Acquisition Subsystems* chapter.

Devices—including items such as phones and other communication, seismographs, hot box detectors, dragging detectors, track crossing devices, emergency train stopping buttons, protection switches, intrusion detection, broken-rail detection, and other devices—shall be accommodated in the design. The designer shall allow for pre-embedded items, such as conduits, ducts, bolts, outlet boxes, as required for these devices.

Allowance shall be made in the design for foundations and fixation hardware for warning detector location cases.

#### 13.15.3 Embedded Conduit and Cable Trough

Provision of embedments for installation of conduits and cable troughs shall be coordinated with the cableway design. Conduits shall generally be located across and below the trackway at



locations of switch machines, at most equipment rooms for signals and controls, and at tunnel portals. See *Structures* chapter for the design of embedded conduit and cable trough.

#### **13.15.4 Facility Services and Infrastructure**

An allowance shall be made in the design for the following wayside mechanical and electrical (M&E) systems:

- Lighting in tunnels and in tunnel access and egress locations (*Facility Power and Lighting Systems* chapter)
- Power receptacles in tunnels (*Facility Power and Lighting Systems* chapter)
- Emergency exit signs in tunnels (*Facility Power and Lighting Systems* chapter)
- General signage (*Civil* chapter)
- Telephones (*Communications* chapter)
- Ventilation systems in trainway tunnels, exit tunnels, and shafts
- Intruder alarm systems at exit tunnel doors
- Intrusion alarm system at portals
- CCTV at portals (*System Safety and Security* chapter)
- Wayside devices to detect hazards on Rolling Stock (*Rolling Stock-Core Systems Interfaces* chapter)
- Gas monitors (*Supervisory Control and Data Acquisition Subsystems* chapter)
- Seismic monitors (*Supervisory Control and Data Acquisition Subsystems* chapter)
- Hoisting equipment in exit shafts (*Mechanical* chapter)
- Fire services provisions in tunnels (*Fire Protection* chapter)
- Power supply for all wayside M&E services (*Facility Power and Lighting Systems* chapter)
- Earthing/grounding and bonding requirements for all wayside M&E services (For cross-bonding requirements, refer to the *Grounding and Bonding Requirements* chapter)
- Communications (*Communications* chapter)
- Supervisory Control and Data Acquisition system (*Supervisory Control and Data Acquisition Subsystems* chapter)

#### **13.15.5 Design Requirements for Cable Troughs**

A cable trough is required along each track of the HST mainline.

The covers of the cable trough will form part of the walkway surface in tunnels. The cable trough covers shall be designed in accordance with the loads specified in the *Structures* chapter

and *Civil* chapter. As the cable troughs will also act as part of the walkway, they shall provide an even, stable walking surface without creating a tripping hazard and have a non-slip surface.

Cable troughs shall have a spur at niche locations. These spurs shall have the same cross-sectional detail as the cable trough along each track of the HST mainline.

Cable troughs shall extend to crosspassages, shafts, and escape tunnels. These extensions shall have the same cross-sectional detail as the cable trough along each track of the HST mainline.

Cable troughs shall be drained. Typical details are shown on the Standard and Directive Drawings.

Where the cable trough transitions from outside of the tracks to the inside, the cable trough shall cross under the tracks perpendicularly with adequate space to accommodate minimum cable bend radii.

#### 13.15.6 Fencing

AR Fences are required around tunnel portals and any other location where needed to prevent access by persons and large animals onto the tracks. The locations of the fences are shown on the Standard and Directive Drawings. Refer to the *Civil* chapter for details. AR fence shall conform to the Standard and Directive Drawings. Refer to *System Safety and Security* chapter for portal fencing requirements.

Wide gates providing access for emergency vehicles shall be installed at egress locations. Single or double gates shall be located where required for maintenance access. For each individual stretch of fencing, at least 1 gate on each side of the tracks is required. Gates shall require locks with a key for opening from the outside, but no key from the inside.

Fences shall be bonded and grounded in accordance with the *Grounding and Bonding Requirements* chapter and a continuity strap shall be carried below all gates to avoid the hazard of electric shock due to touch potential.

#### 13.15.7 Prevention of Unauthorized Access

Design criteria for doors, gates and locking mechanisms required for the prevention of unauthorized access to emergency exits, rooms containing fixed equipment, corridors, stairwells and other controlled areas in tunnels, are described in the *Civil, Communications, and System Safety and Security* chapters.

#### 13.15.8 Grounding and Bonding

Refer to the *Grounding and Bonding* chapter for requirements specific to underground structures.

## 13.16 Tunnel Portal Facilities

All infrastructure elements listed in Section 13.16 shall be considered and evaluated for potential inclusion at each tunnel portal in the HST system. A site-specific hazard analysis shall be conducted for each tunnel portal to determine the appropriate fire and life safety infrastructure to be provided at that tunnel portal. Consideration shall be given to tunnel length, proximity to other tunnels, surrounding topography, and access/egress routes.

Not all items are necessary at every portal. Following assessment, decisions shall be made on which items shall be located at each portal. Guidance on whether a particular element of infrastructure shall be included at a particular portal is provided in Section 13.17.

### 13.16.1 Noise Mitigation Hood

A noise mitigation hood is a smooth tapered hood (trumpet) designed to mitigate aerodynamic noise (sonic boom) effects. This hood may be up to 150 feet long.

The face of the hood-structure (if provided) shall be inclined at least 45 degrees from the vertical for noise mitigation purposes.

It is possible that tunnels more than 2 miles in length may need portal-hood structures at least 65 feet long designed for pressure-relief. This is in addition to the tapered section of the portal. Under these circumstances, the free cross-sectional area at the open end of the hood shall be at least 1.5 times the free-area cross-sectional area of the tunnel. Two pressure-relief openings (each of 108 square feet minimum area) are needed into each bore within the length of the pressure-relief structure. These openings vent into the adjacent bore or via ducts directly to the open air.

### 13.16.2 Portal Ventilation Building

Tunnel ventilation buildings shall be located at one or both tunnel portals. Portal ventilation buildings shall require direct access to the tunnels and shall be located immediately over or adjacent to the tunnel portals. Refer to the *System Safety and Security* chapter.

If practicable, buildings shall be designed to blend into the natural surroundings. Landscaped backfill and embankments with appropriate vegetation shall be employed. Refer to the *Stations* and *Structures* chapters.

### 13.16.3 Power Facilities

#### 13.16.3.1 Traction Power

For Traction Power Facility (TPF) requirements, refer to the *Traction Power Supply System* chapter.

TPFs shall be located away from the tunnels to the extent practical. The minimum distance of the phase breaks of substations and switching stations from the tunnel portal shall be as specified in the *Traction Power Supply System* chapter.

#### 13.16.3.2 Fixed Facility Power

High or low voltage power is required to operate fixed tunnel and tunnel portal infrastructure. For preliminary design and to assure that reasonable, maximum-conceivable space requirements are identified, fixed facility power shall be located in a shared-use building facility constructed above or adjacent to the tracks at the tunnel portal. This building may also include tunnel maintenance and other facilities. Heavy equipment such as transformers and generators may be housed separately at ground level, with working access to the site access road to better accommodate installation and future replacement. See the *Facility Power and Lighting Systems* chapter for fixed facility power requirements.

#### 13.16.4 Access Road

Road access to portals shall be designed for use by emergency responders, for evacuating passengers and for maintenance staff access. Refer to the *Civil* chapter.

If a TPF is located in proximity to a tunnel portal where road access has been established for other purposes, the TPF may make use of the same access road.

#### 13.16.5 Emergency Vehicle Assembly and Turnaround Area

Assembly and turn-around areas for emergency vehicles shall be provided adjacent to the tunnel portal, at the end of the access road within the fenced portal area. Refer to the *Civil* chapter.

#### 13.16.6 Rescue Area/Passenger Assembly Area

A rescue area/passenger assembly area shall be provided as close as practicable to the tunnel portal. Refer to the Standard and Directive Drawings.

#### 13.16.7 Fire Hydrants and Water Supply

A water supply for tunnel fire-fighting purposes shall be provided to points near the emergency vehicle assembly areas. Refer to the *System Safety and Security* chapter

Hydrants connected to adequate public water-supply systems or alternative permitted water supply of 60,000 gallons (minimum) shall be provided adjacent to the emergency vehicle assembly areas.

The water supply system shall be capable of providing a minimum of 500 gallons per minute at a residual pressure of 100 psi, for a period of at least 2 hours.

### **13.16.8 Area Lighting**

- 1 Lighting systems shall be provided in order to maintain sufficient illumination levels at the  
2 ground surface of the portal site area during train evacuation. Refer to the *Facility Power and*  
3 *Lighting Systems* chapter.

### **13.16.9 Cross Track Emergency Vehicle Access**

- 4 If required, paved access roads shall be provided across the tracks for emergency vehicle use,  
5 which shall connect the emergency vehicle assembly area to each tunnel railway for emergency  
6 vehicle access to the tunnels. Refer to the *Trackwork* and *Civil* chapters. Automatic crossing  
7 warning devices are not required at these crossings as they are for emergency use only when  
8 revenue service has been halted.
- 9 Access areas for Hi-Rail emergency vehicles shall be provided.

### **13.16.10 Paved Emergency Egress from Tunnels**

- 10 A well-lit paved walkway outside the tunnel portal shall connect the tunnel safety walkways to  
11 the passenger assembly/rescue area. Refer to the *Facility Power and Lighting System* and *Civil*  
12 chapters.
- 13 The emergency egress walkway at the portal facility shall have a minimum width of 10 feet.

### **13.16.11 Train Surface Evacuation and Fire Control Zone**

- 14 A train surface evacuation and fire control zone shall be located outside a tunnel portal where a  
15 train exiting a tunnel under emergency conditions can stop to allow passengers to safely  
16 disembark, and may allow emergency responders to reach the train for fire-fighting purposes or  
17 to deal with other emergency situations.

### **13.16.12 Sound Powered Telephone Access, Emergency Intercoms, and Emergency Telephone Facilities**

- 18 Sound Powered Telephone Access and Emergency Intercoms shall be provided within  
19 immediate proximity to the portal area emergency vehicle assembly and the passenger  
20 assembly/rescue areas. Refer to the *Communications* chapter.
- 21 Sound Powered Telephone Access at the portal shall be connected to Emergency Intercoms  
22 located within the tunnel and to the Operations Control Center.
- 23 Emergency Telephones shall be located adjacent to the motorized disconnect switches. Refer to  
24 the *Communications* chapter.

#### 13.16.13 Overhead Contact System Motorized Disconnect Switch

Motor operated disconnect switches, equipped for local and remote operation, shall be provided for each OCS in close proximity to the tunnel portal for use by approved California High-Speed Train personnel. Motor operated disconnect switches shall be required at 3-mile intervals for tunnel length of more than 3 miles. Switch indication panels shall be provided at or adjacent to each switching location, indicating for the benefit of emergency response personnel the status of each switch and whether the OCS is energized/de-energized/grounded. A wayside power control cubicle shall be provided to support the disconnect switches, 10 feet by 8 feet in plan area. Refer to the *Overhead Contact System and Traction Power Return System* chapter and *Traction Power Supply System* chapter.

#### 13.16.14 Emergency Command Post

An Emergency Command Post shall be located as close as practicable to the tunnel portal, emergency vehicle assembly area, and rescue area/passenger assembly areas. The Emergency Command Post location shall be well-lit using site area lighting. It shall contain an emergency telephone, OCS motorized disconnect switch remote control panel, Sound Powered Telephones, Tunnel Fire Alarm System Access, portal lighting controls and sufficient elements of a public address system to adequately support emergency responders.

A Ventilation Control Panel, which allows fire department personnel to ascertain the status and to operate the tunnel emergency ventilation system, shall also be installed at the Emergency Command Post, particularly if a portal ventilation building is not located at that portal.

#### 13.16.15 Communications Facilities

All necessary facilities to support emergency and non-emergency communications transmissions between the portal area and HST operations (which might include radio, telephone, and fiber-optic cable capability) shall be provided. A Communications Tower may be required to assure reliable transmission. Refer to the Communications chapter.

#### 13.16.16 Emergency Power Supply

Refer to the *Facility Power and Lighting Systems* chapter for emergency power supply requirements.

#### 13.16.17 Surface Drainage

Surface run-off from disturbed site-areas shall be routed to a detention basin to prevent contamination of groundwater. Refer to the *Drainage* chapter.

#### **13.16.18 Tunnel Seepage and Wash Water**

- 1 Tunnel seepage water, wash water from any tunnel cleaning, and water from a fire-fighting  
2 incident, system test, or a pipe leak shall be routed through a detention basin or directly to a  
3 local sanitary sewer if available. Refer to the *Drainage* chapter.

#### **13.16.19 Detention Basin**

- 4 A detention basin of adequate size shall be constructed to handle run-off quantities calculated  
5 for the individual portal locations. Refer to the *Drainage* chapter for detention basin  
6 requirements.

#### **13.16.20 Parking for Tunnel Maintenance and Traction Power Facility**

- 7 Separate parking away from the emergency parking areas shall be provided for maintenance  
8 staff for tunnels and other site facilities. Refer to the *Civil* chapter.

#### **13.16.21 Helicopter Landing Pad**

- 9 In emergencies, medical/rescue aircraft or security helicopters may land on an emergency  
10 vehicle assembly area, another site parking area, or on the access road.  
11 If a particularly remote portal area is identified where emergency access is required, but where  
12 road access is considered impractical, then a helicopter landing area shall be established.

#### **13.16.22 Landscaping**

- 13 Landscaping shall be designed with CPTED principles in mind as described in the *System Safety*  
14 *and Security* chapter. Landscaping with backfilling, grading and vegetation shall be employed to  
15 help blend structures into the surrounding natural condition. Refer to the *Stations* and *Support*  
16 *Facilities* chapters.  
17 Access shall be provided to allow maintenance of landscape vegetation.

#### **13.16.23 Aesthetic Treatment of Rock and Concrete Surfaces**

- 18 Freshly exposed rock surfaces in portal excavations planned to be left permanently exposed  
19 shall be treated to appear naturally formed and weathered.  
20 In-situ concrete or sprayed concrete surfaces shall be colored and sculpted to blend into the  
21 surrounding landscape.

#### **13.16.24 Public Utilities**

- 22 Public utilities for servicing the portal site areas shall include water, electricity, telephones, and  
23 sewers as described in the *Utilities* chapter.

### 13.16.25 Crossovers

The proximity of crossover tracks to the different tunnel portals shall be assessed to ensure that single-track working can be maintained effectively through the tunnels during emergencies and when one tunnel railway is closed for major maintenance or repair.

## 13.17 Portal Infrastructure Requirements for Different Categories of Tunnel

Not every item of infrastructure described in Section 13.16 may be required at every HST tunnel portal. The following sections provide general guidance in determining which of these elements of portal infrastructure may or may not be required at a particular tunnel portal, or under certain circumstances, where some listed items of “portal” infrastructure may need to be located some distance from a portal.

The principal factors that influence the decision are the following:

- Site-specific hazard analysis for safety and security considerations
- Length of tunnel
- Proximity of one tunnel to another
- Accessibility to portal locations
- Environmental impacts at a portal location

For additional site-specific requirements, refer to the *System Safety and Security* chapter.

### 13.17.1 A Shorter Tunnel up to One-Half Mile in Length

All infrastructure listed in Section 13.16 shall be considered to be located at one of the two portals of a tunnel.

The following items shall be considered for both portals of a tunnel:

- Noise mitigation hood
- Intrusion protection fence
- Rock fall and debris containment
- Surface drainage
- Tunnel seepage and wash water (if downgrade from the other portal that was preferred for the principal infrastructure installations)
- Detention basin (if downgrade from the other portal)



- 1 • Area lighting (assumes temporary passenger assembly might be needed on the tracks
- 2 outside the portal prior to relocating through the non-incident tunnel to the Rescue Area at
- 3 the other portal)
- 4 • Landscaping
- 5 • Aesthetic treatment of rock and concrete surfaces
- 6 • Emergency telephone facilities
- 7 • OCS-motorized disconnect switches and indication panels

### **13.17.2 Several Short Tunnels Close Together**

8 For a group of short tunnels close together, emergency access and egress and other portal  
9 infrastructure facilities (as listed in Section 13.16) that are provided for 1 portal of 1 tunnel shall  
10 serve the emergency requirements of several tunnels provided that:

- 11 • Such emergency facilities are spaced no further apart than 1/2 mile; and
- 12 • Emergency access and egress to required standards can be provided from the location
- 13 where the emergency facilities have been located, along paved track-slab extending through
- 14 and between the several tunnels, or along an adjacent roadway.

15 Other tunnel portals in the group of short tunnels shall consider the particular infrastructure  
16 items separately listed in Section 13.17.1.

### **13.17.3 Shorter Tunnel with Constrained Access**

17 A shorter tunnel up to 1/2 mile in length with physical conditions at both portals that prevent  
18 construction of required permanent emergency access and egress infrastructure elements  
19 (Section 13.16), or where exceptional environmental conditions exist that render this  
20 construction unacceptable, shall require additional elements and considerations.

21 To serve the tunnel under these circumstances, all permanent emergency infrastructure  
22 elements listed in Section 13.16 shall be provided to each portal, or to a location along the track  
23 within 1/2 mile of each portal, provided that emergency access and egress to required standards  
24 can be provided to the tunnel from the location where emergency facilities have been located,  
25 along paved track-slab or along an adjacent roadway

26 At each portal, the following elements are required to be considered at the immediate location  
27 of the portal:

- 28 • Noise mitigation hood
- 29 • Intrusion protection fence
- 30 • Rock fall and debris containment
- 31 • Surface drainage

- 1 • Area lighting
- 2 • Landscaping
- 3 • Aesthetic treatment of rock and concrete surfaces
- 4 • Emergency telephone facilities
- 5 • Sound Powered Telephone
- 6 • Emergency Intercom
- 7 • OCS-motorized disconnect switches and indication panel

#### 13.17.4 Tunnel Longer than One-Half Mile in Length

8 Infrastructure listed in Section 13.16 shall be considered for both portals, including the need for  
9 portal ventilation buildings at both portals. Tunnel seepage and wash water and detention  
10 basin requirements normally need to be considered only at the portal at lower elevation.

#### 13.17.5 Longer Tunnel with Constrained Portal Access

11 Where a tunnel more than 1/2 mile in length has physical conditions at one or both of the  
12 portals that prevent construction of required permanent emergency access and egress  
13 infrastructure elements (Section 13.16), or where exceptional environmental conditions exist  
14 that render this construction unacceptable, the following shall be considered:

- 15 • All permanent emergency access and egress infrastructure elements (Section 13.16) shall be  
16 provided at an unaffected portal.
- 17 • All permanent emergency infrastructure elements (Section 13.16) shall be provided to each  
18 portal or to a location on the track within 1/2 mile of portals, provided that emergency  
19 access and egress to required standards can be provided to the tunnel along paved track-  
20 slab or along an adjacent roadway from the location where the emergency facilities have  
21 been located. Alternatively, emergency access and egress shall be provided via a mid-line  
22 tunnel rescue shaft or rescue tunnel located within 1/2 mile of affected portals.
- 23 • The infrastructure elements that are listed in Section 13.17.3 as requiring consideration at  
24 both portals of a shorter tunnel with constrained access similarly require consideration at  
25 both portals of a longer tunnel with constrained access.

26 A design variance is required if access to both tunnel portals cannot be provided.



## Chapter 14

### Stations

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	May 2013	Revision 1



## Table of Contents

<b>14</b>	<b>STATIONS .....</b>	<b>14-1</b>
14.1	Scope .....	14-1
14.2	Regulations, Codes, Standards, and Guidelines .....	14-1
14.3	Stations.....	14-2
14.3.1	Design Principles.....	14-2
14.3.2	Platforms .....	14-13
14.3.3	Concourse and Mezzanine .....	14-18
14.3.4	Passenger Circulation .....	14-26
14.3.5	Evacuation from Public Areas .....	14-34
14.3.6	Non-Public/Station Support Areas .....	14-37
14.3.7	Interior Environment.....	14-48
14.3.8	System-wide Furniture, Fixtures, and Equipment .....	14-52
14.3.9	Station Information .....	14-56
14.3.10	Station Finish Materials .....	14-58
14.3.11	Security Provisions.....	14-63
14.4	Station Site Design .....	14-65
14.4.1	Site Design Objectives .....	14-65
14.4.2	Access Hierarchy .....	14-65
14.4.3	Vehicle Access and Circulation .....	14-78
14.4.4	Roadways.....	14-79
14.4.5	Site Infrastructure .....	14-79
14.5	Landscaping .....	14-82
14.5.1	Scope .....	14-82
14.5.2	General Landscaping Principles .....	14-82
14.5.3	Facility Types .....	14-83
14.5.4	Plant Materials.....	14-85
14.5.5	Irrigation .....	14-87
14.5.6	Hardscape .....	14-87

## Tables

Table 14-1:	Peak Period Boardings.....	14-12
Table 14-2:	Platform Obstructions.....	14-14
Table 14-3:	Concourse Circulation and Waiting Area .....	14-22
Table 14-4:	Ticket Sales Facilities.....	14-24
Table 14-5:	Fare Gates .....	14-24
Table 14-6:	Passenger Amenity Space Allocation .....	14-26
Table 14-7:	Queuing Distance Requirements.....	14-27
Table 14-8:	Passageway Width Calculation .....	14-28
Table 14-9:	Passageway Width Minimums.....	14-28
Table 14-10:	Escalator Criteria.....	14-30
Table 14-11:	Stair Width Minimums .....	14-31
Table 14-12:	Capacities of Station Evacuation Elements .....	14-36
Table 14-13:	Station Walking Speeds .....	14-36
Table 14-14:	Miscellaneous Rooms .....	14-44
Table 14-15:	Train Crew Support Spaces .....	14-45
Table 14-16:	Rolling Stock Maintenance Spaces .....	14-45
Table 14-17:	Commissary Spaces .....	14-46
Table 14-18:	Train Control and Communications Spaces.....	14-48
Table 14-19:	Station Noise Levels.....	14-51
Table 14-20:	Station Seating.....	14-53
Table 14-21:	Acceptable Floor Finishes .....	14-60
Table 14-22:	Acceptable Ceiling and Overhead Architectural Treatment.....	14-62
Table 14-23:	Security Devices in Stations .....	14-64

## Figures

Figure 14-1:	Bus Bays.....	14-70
Figure 14-2:	Sawtooth Bus Bays .....	14-71

## Acronyms

ADA	Americans with Disabilities Act
ADAAG	Americans with Disabilities Act Accessibility Guidelines
ANSI	American National Standards Institute
CBC	California Building Code
CCTV	Closed-Circuit Television
CHST	<i>Not Used; use HST for California High-Speed Train</i>
CHSTP	California High-Speed Train Project
HST	High-Speed Train
HV	High Voltage
HVAC	Heating, Ventilation, and Air Conditioning
LEED	Leadership in Energy and Environmental Design
LOS	Level of Service
LV	Low Voltage
NFPA	National Fire Protection Association
OCC	Operations Control Center
OCS	Overhead Contact System
POL	Platform Occupant Load
ppm	people per minute
PSB	Passenger Service Booth
SCR	Station Control Room
TCF	Terminal Control Facility
TVM	Ticket Vending Machine
UPS	Uninterruptible Power Supply





## 14 Stations

### 14.1 Scope

1 This chapter establishes the architectural design criteria for the California High-Speed Train  
2 (HST) stations, site design, and landscaping.

### 14.2 Regulations, Codes, Standards, and Guidelines

3 Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.  
4 This chapter presents design standards and guidelines specifically for the construction and  
5 operation of the high-speed railway based on international best practices and applicable state  
6 and federal requirements.

7 Since the stations will be located in multiple municipal jurisdictions, other transportation  
8 owner/operators' rights-of-way, and/or unincorporated jurisdictions, the CHSTP standards and  
9 guidelines may differ from local jurisdictions' codes and standards. Although state agency  
10 projects are not subject to local city or county codes, the HST System is to provide for  
11 connection and integration with other passenger rail and transit services as well as the  
12 surrounding station area. In the case of such connections, consideration of local codes and other  
13 transportation owner/operators' guidance is appropriate.

14 Applicable codes, rules, standards, and guidelines include but are not limited to the following:

- 15 • Code of Federal Regulations (CFR) Title 28, Part 36 – Nondiscrimination on the Basis of  
16 Disability by Public Accommodations and in Commercial Facilities
- 17 • California Building Code (CBC), Title 24 of California Code of Regulations (CCR)
- 18 • Americans with Disabilities Act (ADA)
  - 19 – ADA Standards for Accessible Design
  - 20 – Guidance on the ADA Standards for Accessible Design
  - 21 – ADA Accessibility Guidelines (ADAAG)
- 22 • National Fire Protection Association (NFPA)
  - 23 – NFPA 101 – NFPA's Life Safety Code
  - 24 – NFPA 130 – Standard for Fixed Guideway Transit and Passenger Rail Systems

- American National Standards Institute (ANSI) 117.1 – standard for accessible design for persons with disabilities
- California Division of the State Architect California Access Compliance Reference Manual

## 14.3 Stations

The purpose of this section is to set forth design guidelines and standards for passenger stations that promote safe, efficient, and high-quality operations for HST service. This section presents information relating to space requirements, building amenities, station performance, circulation, connection, and safety of the patrons and employees of the HST System.

### 14.3.1 Design Principles

There will be a high degree of variability between stations due to different locations, ridership demands, intermodal connections, trip purposes, and local land use. This section identifies minimum design criteria to be considered at HST stations.

#### 14.3.1.1 General

The following principles apply to the design of all stations:

- **Safety** – Safety of station patrons, train passengers, and operating personnel shall be the first priority in station design.
- **Design Life** – Designing for the expected useful life of passenger stations is a fundamental design principle and is addressed in the *General* chapter.
- **Station Size** – Stations shall provide adequate space for all essential station functions including platforms, public circulation, passenger services, station operation offices, Core Systems and plant rooms, and special provisions at Terminal Stations.
- **Clarity** – Stations shall be organized clearly and simply. Passenger circulation routes shall be clearly identifiable and unobstructed.
- **Context** – Each station shall be responsive to its unique physical environment and context.
- **Structural Design** – Structural elements of stations shall conform to the requirements described in the *Structures* chapter.
- **Finish Materials** – Stations shall utilize finish materials that are durable, energy efficient, and easy to maintain.
- **Shared Use** – At special stations serving both high-speed and conventional rail services, shared use does not infer shared functions. Where multiple rail systems will be sharing an enclosure, functional, operational, and support facilities for HST shall be dedicated and shall not be shared with other operators.

- **Future Modifications** – Station design shall consider a “not to preclude” approach and provide sufficient flexibility to accommodate reasonable future updates to the programmatic requirements.

#### 14.3.1.2 Architectural Design Principles

##### A. Introduction

While the majority of this chapter sets forth planning guidelines and technical requirements for HST stations, this section is intended to convey the more subjective and esoteric qualities and aspirations of station architecture. The architectural design philosophy and principles are provided to assist designers in developing station design solutions that achieve the objectives of the California High-Speed Rail Authority (Authority).

##### B. California State Excellence Standards

The CHSTP supports the broad goals set forth by the California Department of General Services (DGS) “Excellence in Public Buildings” (EIPB) program, which promotes “high-performing public buildings and a positive architectural legacy that reflects the State’s commitment to excellence.” (<http://www.documents.dgs.ca.gov/dsa/pubs/eipb.pdf>) This statement communicates the State’s expectations in the building-delivery process. The program seeks to produce high-performing public buildings through “Excellence Goals,” establishing objectives that promote design excellence, sustainability, enduring value, and public benefit.

The Authority supports the State’s following “Excellence Goals” as a basic foundation by which the design of HST stations shall be measured:

- **Architectural Excellence** – Attract outstanding architects who are committed to design excellence, best practices in energy and environmental sustainability, and the other goals of EIPB.
- **Sustainability** – In accordance with Executive Order S-20-04, Leadership in Energy and Environmental Design (LEED) certification of Silver or higher shall be provided.
- **Integrating Art into Public Buildings and Spaces** – Expand the public experience with art while adding to the building’s identity and enhancing the human experience.
- **Cost Effectiveness** – Use performance standards, life-cycle costing, and integrated design to deliver value above the initial financial investments.
- **Universal Design** – Enhance accessibility for all.
- **Safety and Security** – Safe and secure from natural or man-made disasters.
- **Make a Positive Contribution to the Local Community** – Buildings sited and designed to enhance the local built environment. This goal includes the following:
  - Involving community participation
  - Strengthening and revitalizing California’s cities and communities
  - Enhancing the livability of the community

- Supporting economic renewal
- Encouraging multiple uses of public spaces
- Promoting use of public transportation
- Supporting sound growth patterns
- Providing convenient access for customers and employees
- Reducing traffic congestion
- Promoting improved air quality
- **Preservation of Buildings of Historic Value** – Preservation of historic buildings retains the art of architecture that has contributed to the community for decades.

#### C. System Design Philosophy

**Objectives** – The central objective for design of the HST System is to provide a safe, convenient, fast, and efficient means of travel to the users. A secondary fundamental design principle is to present a strong, positive image of the facilities to the citizens of California through excellent architectural design.

**Image** – Given the State’s aforementioned design excellence principles as overarching guidance, the architectural image of stations and facilities will be of particular importance. As perhaps the nation’s first high-speed rail system, the CHSTP will establish a standard of quality for many similar systems to follow. The utmost attention shall be paid to producing architecture commensurate with the overall project goals.

The stations will symbolize a new mode of travel and transportation for this nation, not unlike the great urban railway stations that have become the heart of communities throughout the world. Convenient interchange with other modes of transportation will be the catalyst in creation of successful transportation centers and development of communities in which they are to be built.

**Character** – The HST System will present unique challenges and architectural opportunities. High-speed rail has existed for decades in other parts of the world, but the system will present a new kind of silhouette upon California’s architectural landscape; uncommonly long station buildings and frequent movement of trains will introduce an unfamiliar yet exciting visual experience. While each station will be expected to integrate the functional and safety requirements within the constraints of budget and schedule, each will be expected to convey a strong individual civic character, relating clearly and strongly to its context through careful architectural design.

#### D. Exterior Architectural Principles

**Uniqueness** – Because each community to be served by an HST station is unique, architectural design of each station will be unique. The specific functional needs of each station shall be wrapped with an architectural skin that is site-specific in scale, massing, volume, and material. Although certain internal and functional elements will be common to all stations, the high-

speed rail station image presented to each community will be a carefully considered response to the context of each station site.

**Design Factors** – Specific purpose, context, presence, and image shall be defined individually for each station. Each station shall respond to unique, site-specific design factors, including location, alignment, existing and future neighborhood architectural and historic context, anticipated ridership, climatic variations, vehicular and pedestrian station access, multimodal transfer, protection from the elements, passenger orientation, and familiarity, wayfinding, constructability and sustainability.

All of these unique, site-specific architectural and urban design considerations shall be synthesized into an architectural solution, and be responsive to the unique site and the strict operational and programmatic requirements.

**Community Links** – As a new focal point of the community, the planning and siting of the HST station shall convey an awareness of its prominent presence. It should not seek to dominate its neighborhood but rather to be sensitive to its community and context. Connections, entrances, and station access points shall respond to neighboring residential, commercial, or other uses. Edges of the station area should blend smoothly with the surrounding urban fabric and be responsive to each community's long-range urban development plans.

**Massing** – Vertical location of platforms, Concourses, and entrances will be a significant factor in the massing of each station; at-grade stations with an adjacent at-grade Concourse will convey a significantly different presence within an urban context than will an elevated station raised 60 or more feet above the street. This kind of inherent programmatic difference will be a driving influence in the presence of a station and in the architectural solution.

**Style** – Stylistic trends in early 21<sup>st</sup> century architecture will not remain constant; therefore, while each station shall reflect current and forward-thinking innovation, it shall avoid a design approach that may appear dated decades later. Station context and community concerns shall be reflected in station design. Where stations are to be located in established urban environments, the goal will be to transform and enhance that environment and community while protecting the neighborhood's fabric. Where station sites are to be located in sites without a strong existing character or context, the goal will be to establish a strong architectural foundation for future community growth.

**Image** – Railway stations are a unique building type with a unique purpose. The image of a station shall therefore convey the function of the building through its architectural design. A railway station should look like a railway station. As gateways to the HST System and to major metropolitan centers, stations will convey an arresting image. HST stations will symbolize and celebrate a new means of travel in America.

**Context** – Care shall be taken to be sensitive to the station context, recognizing that an architectural solution appropriate for a densely populated, established urban center may not be appropriate for a smaller suburban community. In many cases, subtlety and understatement

1 may be more appropriate than exhilaration and innovation. California has rich, diverse regional  
2 characteristics, which may be valid sources of local identity for stations. Sensitive architectural  
3 interpretation of these regional variations can contribute to an HST System representative of all  
4 regions that the system will serve.

5 **Finish Materials** – Exterior materials selected for stations shall be for the specific site context  
6 while satisfying system-wide design concerns, including safety, useful life, durability, low  
7 maintenance, replacement, and potential vandalism. Materials shall respond to the context of  
8 the site and convey stability, warmth, brightness, and quality.

9 **Lighting** – The presence of an HST station at night is no less important than during daylight  
10 hours. Prudent and dynamic use of lighting will be a significant design element in a station's  
11 nighttime presence. Station interior lighting shall be designed to create an inviting station  
12 presence when viewed from the exterior, while ensuring energy efficiency and satisfying  
13 sustainable design goals. Station sites, including parking, approaches, landscaping, signage, and  
14 entrances shall be appropriately lighted for user safety as well as for architectural effect.

#### E. Interior Station Design Principles

15 **Station Volume** – The greatness of railway stations in times past has often been the result of  
16 soaring station volumes. Similarly, upon entering an HST station, the patron will experience a  
17 sense of exhilaration, spaciousness, openness, clarity, and system identity. Entrances and  
18 Concourse spaces shall be generously and appropriately sized according to the specific  
19 functional needs of each station. At the same time, a striking architectural image shall be  
20 established through the judicious use of vertical space and interior volume. Ceiling heights may  
21 be modulated wherever the opportunity exists. Low ceilings may be used sparingly as an  
22 intimate contrast to the vertical volumes.

23 **Openness** – Station planning and design shall be founded upon uncomplicated and open  
24 concepts to facilitate free movement of passengers and staff. Adherence to principles of  
25 passenger circulation described in the following sections, particularly with respect to clear,  
26 unobstructed, well-defined, well-lighted routes for public circulation are essential to planning  
27 of public areas and passenger satisfaction.

28 **Interior Lighting** – Liberal use of natural lighting is a guiding principle for station interiors.  
29 Natural light creates drama within interior spaces as it changes constantly throughout the day.  
30 Openings that allow light into the station during the day contribute to a more dynamic station  
31 presence within the community at night. Artificial interior lighting shall be used judiciously to  
32 illuminate the functions requiring task and safety lighting and differentiating between differing  
33 station functions. Light sources and placement shall be attentive to the energy standards  
34 established by the State of California.

#### F. Sustainability and LEED Principles

35 Stations shall be, at minimum, LEED™ Silver Certified as described elsewhere in this Design  
36 Criteria.

### 14.3.1.3 Functional Design Principles

#### A. General

- 1 Specific functional requirements of station spaces are described in this chapter.
- 2 There is no single “footprint” that can be applied at each station site. Each station will be a
- 3 response to essential functional relationships described in this criteria and adapted to the
- 4 unique station site.

#### B. Consistency and Variability

- 5 Elements of station plans can be categorized as functionally consistent or functionally variable.

6 **Consistency** – System-wide, consistent use of design components in stations can result in  
7 reduced capital, operations, and maintenance costs through reduced design and construction  
8 variation, economies of scale, and simplification of operation and maintenance procedures.  
9 Functionally consistent elements include but are not limited to the following:

- 10 • Platform width and length
- 11 • Platform floor finish surface and edge material
- 12 • Station Information Office
- 13 • Ticket Sales Office location and identity
- 14 • Non-public area staff and plant room organization
- 15 • Fare collection equipment
- 16 • Escalators and elevators
- 17 • Passenger Information Systems, including signage and graphics
- 18 • Fire hose and extinguisher cabinets
- 19 • Door hardware

20 **Variability** – Varying station elements in addition to varying the architectural design  
21 approaches described in Section 14.3.1.2 Architectural Design Principles, responds to unique  
22 needs of individual stations. Functionally variable elements include the following:

- 23 • Concourse configuration
- 24 • Finish materials (in accordance with the CHSTP’s acceptable palette)
- 25 • Interior seating
- 26 • Artwork
- 27 • Site layout, landscaping and site furnishings



C. Functional Types

Station functional type will significantly influence station planning and design. Stations are classified as intermediate or terminal types.

**Intermediate Stations** – Stations not functioning as terminals are considered Intermediate Stations. Passengers will typically wait on the platform prior to the train's arrival due to relatively short train dwell times. Signage shall inform passengers where to wait in order to quickly board the proper car when the train arrives.

**Terminal Stations** – Trains will occupy Terminal Station platforms for longer periods of time than will be the case at Intermediate Stations. Terminal stations shall require additional ancillary facilities to facilitate servicing trains prior to turning back. Terminal station platform activities may include provisioning onboard food service, light interior cleaning of the train, trash removal, train crew changeover, and light mechanical inspection. Refer to Section 14.3.6 for specific space requirements.

**Key Intermediate Stations (with Turnback Service)** – Key Intermediate Stations may have more extensive access to connecting feeder services or a higher level of ridership than regular Intermediate Stations, but less than that of Terminal Stations. In addition, during the early stages of the system phase-in, some trains will originate and terminate at these stations in order to optimize utilization of the train fleet. These stations may require some of the elements usually found at a Terminal Station, such as space for train crews to wait in between trains, and may have a higher level of customer services and amenities than regular Intermediate Stations.

**14.3.1.4 Station Planning**

Each station shall be planned and sized to accommodate projected ridership for both normal and emergency conditions. Spaces shall be sized and located for optimal functionality and efficiency, in accordance with the requirements described in this chapter.

Primary station zones include but are not limited to entrances, Free Area, passenger amenities, fare collection, Paid Area, platforms, passenger service areas, station operation offices, ancillary spaces, building Core Systems/plant spaces, and train servicing facilities.

A. General Planning Goals

Station planning goals include the following:

- Safety and security of station occupants
- Avoidance of passenger congestion and compliance with level of service objectives
- Adequate capacity for passenger surges or disruptions in train service
- Adequate emergency evacuation capacity
- Simple and clear passenger circulation within station and surrounding areas
- Unobstructed lines of sight and well-lit spaces

- Accessibility for all passengers
- Efficient and convenient interchange with other modes of transportation

#### B. Station Size

**General** – Every station comprises public and non-public functions and shall be planned individually to accommodate the functional needs for the specific location. Basic station components include the following:

- Platforms (size is a function of train length, vertical circulation and passenger circulation)
- Public Concourse (size is a function of ridership)
- Support Space (size is a function of staffing levels)
- Core Systems and Plant Rooms (size is a function of Core Systems equipment and plant rooms)

**Platforms** – Platforms are the key components for public access to trains. Platform size is a function of train length and width of escalators, stairs, and passenger circulation and will generally be unaffected by ridership. Size shall be sufficient for safe circulation of passengers on platforms during normal and emergency conditions.

**Public Concourse** – Public station areas within the Concourse and mezzanine intended for passenger circulation shall accommodate forecast ridership in the Full Build (2035) or projected Phase I, whichever is higher under estimated peak period and emergency conditions. Refer to the CHSTP Station Area Parking Guidance document for ridership forecasts. Sizing of the public Concourse and related facilities shall consider the appropriate peak period within the daily peak as specified and detailed in Section 14.3.1.5. Public spaces include Concourse paid and Free Areas, passageways, and vertical circulation elements.

**Non-public Areas** – Non-public station areas comprise the majority of Concourse space and are generally unaffected by forecast ridership; space required for these uses is determined by operational and technical needs. Non-public areas include passenger services, operations, Core Systems rooms, plant rooms, and terminal facilities where they occur.

**Terminals** – Terminal stations shall be planned in accordance with the goals, facilities, and functional needs of Intermediate Stations while also providing space to accommodate terminal functions. Specific terminal space requirements are described in Section 14.3.6.5 and throughout this document. Additional platforms may be required to accommodate terminal train operation needs. The special operational conditions of terminals will generally result in a larger station than the intermediate type.

**Station Sites** – Site conditions will influence station planning and size. Each individual site will influence placement and number of exterior entrances and organization of interior spaces. Placement of intermodal transportation connections will depend on availability of adjacent site

1 area and optimum placement in relation to entrances. Where site conditions and passenger use  
2 warrant entrances on two sides of the trackway, dual entrances may be considered.

**C. Passenger Movement**

3 Passenger movement through primary public activity zones is as follows:

- 4 • Passengers enter through a station entrance and into the Concourse where information,  
5 ticketing, waiting, and basic services are easily identified and located.
- 6 • Departing passengers may either proceed to a waiting area or patronize station amenities  
7 within the Concourse.
- 8 • Ticketed passengers may proceed through Fare Gates into the Paid Area-waiting zone.
- 9 • Before the train's departure time, passengers will be instructed by public address (PA) and  
10 dynamic signage to proceed to the appropriate platform to await boarding the train.
- 11 • When the train arrives, waiting passengers board after the arriving passengers alight.
- 12 • The sequence is reversed for arriving passengers.

**D. Level of Service**

13 The primary performance measure used to determine adequacy of pedestrian circulation spaces  
14 within the station shall be Level of Service (LOS), as defined by Fruin,<sup>1</sup> which qualitatively and  
15 quantitatively describes the permissible degree of pedestrian congestion, based on density and  
16 walking speed at key passenger circulation locations within the station. Stations shall be  
17 planned according to LOS B for the peak design year unless noted otherwise.

**E. Station Vertical Configuration**

18 Stations are identified according to vertical platform location, as follows:

19 **Elevated Stations** – Platforms are supported on a superstructure above grade or passing over  
20 surface features.

- 21 • **At-grade Station** – Platforms are located at ground level.
- 22 • **Underground Stations** – Platforms are located below grade.

**F. Station Vertical Organization**

23 The two primary station levels are Platform and Concourse. These areas are typically situated  
24 on different levels but may be at the same level.

25 **Platform** – Provides waiting and circulation space adjacent to the trainway.

26 **Concourse** – Situated between platforms and the entrances/exits and provides space for  
27 waiting, fare collection, passenger circulation, station amenities, support and plant space.

---

<sup>1</sup> *Pedestrian Planning and Design*. John J Fruin, Ph.D. 1987.

1 **Mezzanine** – In some cases, may be planned as an intermediate level between the Concourse  
2 and Platform levels.

G. Station Platform Configuration

3 Station platforms shall be configured using either a Side or a Center Platform, depending on  
4 numerous interdisciplinary design and operational considerations. Most Intermediate Stations  
5 will be configured with Side Platforms.

6 **Side Platform** – A single track is positioned adjacent to each platform. Platform vertical  
7 circulation, amenities, and staff spaces may be duplicated at each platform. Side platforms are  
8 preferable where both through tracks and stopping tracks serve the station.

9 **Center Platform** – Tracks run alongside both sides of a single platform. Passengers utilize the  
10 same vertical circulation elements to access either northbound or southbound trains.

H. Emergency Access

11 In accordance with NFPA 130, local emergency authorities shall respond to a station emergency  
12 according to a pre-approved emergency plan. Emergencies may include a fire within the station  
13 public or non-public areas, a train collision or derailment, loss of station or traction power,  
14 necessity to evacuate passengers, disabled or stalled train at a platform, natural disaster,  
15 presence of hazardous materials, passenger need for first aid, earthquake, or other emergency.

16 In the event of an emergency, the local first-responder shall be summoned and shall require  
17 access into all areas in and around the station. At least one entrance shall be designated as the  
18 emergency entrance. Station design shall incorporate provisions as required by the State fire  
19 marshal and local fire jurisdiction to allow firefighter access to any and all portions of the  
20 station (i.e., dedicated firefighters' entrances, stairs or other). Station plans shall demonstrate  
21 acceptable strategies for emergency access as well as emergency evacuation.

**14.3.1.5 Passenger Accommodation**

A. Public Areas

22 Public station areas intended for passenger circulation shall accommodate forecast ridership for  
23 the Full Build (2035) or projected Phase I, whichever is higher under estimated peak period and  
24 emergency conditions. Refer to the CHSTP Station Area Parking Guidance document for  
25 ridership forecasts.

26 Peak-period ridership is determined using the ridership parameters in the following sections.

B. Boardings

27 Boardings are indicated using a subscript "B" ( $P_B$ ).

28 **Daily Boardings** – Design shall be based on the peak day boardings (as provided in a separate  
29 document).

**Peak Hour Boardings ( $P_{60B}$ )** – Ridership peaking factors to convert peak daily boardings to peak-hour boardings are provided in Technical Memo 4.2 Phase 1 Service Plan.

**Peak 30-minute Boardings ( $P_{30B}$ )** – One-half of all the peak-hour boardings, multiplied by a system surge factor of 1.2.

**Peak 15-minute Boardings ( $P_{15B}$ )** – One-quarter of the peak-hour boardings, multiplied by a system surge factor of 1.3.

**Peak minute Boardings ( $P_{1B}$ )** – Peak-hour boardings divided by 60 and multiplied by a system surge factor of 1.5.

**Table 14-1: Peak Period Boardings**

Symbol	Description	Formula
$P_{60B}$	Peak Hour Boardings	Peak day boardings x Peaking Factor
$P_{30B}$	Peak 30-minute Boardings	$(P_{60B} \div 2) \times 1.2$
$P_{15B}$	Peak 15-minute Boardings	$(P_{60B} \div 4) \times 1.3$
$P_{1B}$	Peak Minute Boardings	$(P_{60B} \div 60) \times 1.5$

**C. Alightings**

Alightings are indicated using a subscript “A” ( $P_A$ ).

Peak alightings ( $P_{60A}$ ,  $P_{30A}$ ,  $P_{15A}$ ,  $P_{1A}$ ) – Assumed to be equal to peak boardings.

**D. Other Station Users**

It is expected that some passengers will be dropped off or picked up at the station. The number of people who drop-off or meet HST passengers is estimated to be one-tenth of the total peak boardings and alightings. These users shall be added to boardings and alightings where appropriate to provide adequate space they may occupy. Total station occupancy also includes station staff, which varies based on operating conditions and station type.

**E. Maximum Train Load**

Certain station facility requirements will be based on estimated maximum passenger occupants in a trainset. High-speed train sets shall accommodate between 900 and 1,000 seated passengers; therefore, the maximum number of alighting passengers shall be 1,000 per train.

**F. Delayed Trains**

Station facilities shall be able to accommodate the additional passengers that will accumulate within the station when a train is cancelled or seriously delayed.

### 14.3.2 Platforms

- 1 The primary function of station platforms is passenger boarding and alighting.

#### 14.3.2.1 Platform Geometry

##### A. Length

- 2 The usable length of a platform along which a HST will stop shall be:

- 3 • *Desirable* – 1,410 feet  
4 • *Minimum* – 1,370 feet

- 5 In order to allow for potential future platform extension, potential for adding tangent track  
6 beyond the platform shall be considered.

##### B. Platform Width

- 7 Platform width shall meet the requirements of NFPA 130 and ADA. The platform width shall be  
8 sufficient to allow accessibility and circulation for the maximum number of passengers based  
9 on projected ridership for the station.

- 10 • Center platform:  
11 – *Minimum* – 30.0 feet  
12 – *Exceptional* – 25.0 feet  
13 • Side platform:  
14 – *Minimum* – 20.0 feet  
15 – *Exceptional* – 18.0 feet

- 16 **Module** – Platform width shall be a multiple of the 12-inch floor module in order to minimize  
17 cutting of floor-tile finish materials where they occur.

- 18 **Tapering** – Where platform end tapering is provided, the taper should be 10 feet long and  
19 gradually increase the distance from the platform edge to centerline of track by 0.5 foot. Taper  
20 shall be located outside the areas of trainset doors used by the public. Taper shall be located  
21 beyond the minimum station platform length.

##### C. Platform Cross Slope and Drainage

- 22 Platforms shall slope away from the platform edge to provide for separation of platform  
23 drainage from track drainage as well as for safety. Drains along the toe of platform slope shall  
24 be provided.

- 25 • *Minimum* – 1.0%  
26 • *Maximum/Exceptional* – 2.1%

#### D. Platform Longitudinal Slope

Platform longitudinal slope shall follow the longitudinal profile of the rail. It is desirable for station tracks to be level. Where track slope at platforms is unavoidable, slope shall not exceed 0.25 percent.

#### E. Platform Curvature

Platforms shall be on a tangent alignment along the usable length. If a horizontal curve is unavoidable, the curve shall be the largest possible radius and shall require a design variance. Spirals and superelevation within platform limits shall be avoided.

#### F. Platform Height Above Rail

Station platforms shall be designed to provide level boarding. Although rolling stock has not yet been selected, the floor height of trainsets under consideration ranges from 45.5 inches to 51.2 inches above the top of rail. When the rolling stock is selected, the platform height above top of rail shall be set to the same as the nominal car floor height at the doors of the vehicle.

The vertical dimension between the train door threshold and the platform edge is as follows:

- Desirable – 0 inches
- Maximum –  $\pm 5/8$  inches

#### G. Platform Edge to Track Centerline

The horizontal dimension between the centerline of track and the platform edge shall be one-half the width of vehicle + 2.75 inches. This allows for a plus or minus construction tolerance of 0.25 inch in regard to the 3-inch maximum gap between platform edge and train door threshold allowed by ADA, measured when the vehicle is at rest.

Refer to the *Trackway Clearances* chapter for the nominal distance between track centerline and platform edge.

#### H. Platform Obstructions

Platform obstructions lower than 8 feet–0 inches above floor level such as columns, walls, stairs, sign posts, shall be set back from the platform edge to allow for safe and efficient platform circulation. Minimum setbacks from platform edge are indicated in Table 14-2.

**Table 14-2: Platform Obstructions**

Element Length	Minimum Dimension from Platform Edge*	Exceptional Dimension from Platform Edge*
Short Obstruction (less than 3'-4")	7'-0"	6'-2"
Long Obstruction (more than 3'-4" <sup>1</sup> )	9'-0"	8'-0"

<sup>1</sup> Two small point obstructions separated by less than 8 feet–0 inches shall be considered a long obstruction.

\* Dimensions are measured parallel to tracks.

Sightlines for train operators, station operators and patrons waiting on the platform shall not be hindered by canopies, signage, or other potential aerial obstructions.

**Columns** – Columns running the length of the platform shall be limited in size and in quantity in order to avoid the effect of a wall. Columns shall be located to avoid obstructing train doors.

**Overhead Contact System Poles** – Poles supporting the Overhead Contact System (OCS) shall not be located on station platforms.

**I. Under-platform Refuge Area**

A refuge space under the platform edge shall be provided at track level, a minimum of 2 feet-6 inches high and 2 feet-6 inches deep along the entire length of the platform. The refuge space shall be open at platform ends to allow egress. High access panels (2 feet-6 inches wide and 2 feet-6 inches) shall be provided at maximum 300 feet on center along the length of the refuge area.

**J. Clearance to Metal Objects**

Metal objects on platforms shall be a minimum distance from the platform edge of 8 feet. Any metallic object closer than 8 feet from the platform edge shall be grounded to the platform counterpoise.

**14.3.2.2 Platform Planning**

**A. Passenger Capacity**

**Width** – Platform width shall accommodate the predicted volumes of passengers boarding and alighting from trains during normal and irregular operation. Width shall be either the minimum width indicated in the previous section or the width required to accommodate the Platform Occupant Load, whichever is larger.

**Occupant Load** – Adequate platform capacity shall be confirmed by first calculating the maximum Platform Occupant Load (POL). The POL is the maximum number of passengers who will gather on the platform during the interval immediately following the departure of one train and the arrival of the next during the peak hour.

**Irregular Operation** – During normal operation, passengers will alight from a train in accordance with estimated peak-hour flow. However, there may be causes (mechanical or operational) for all passengers traveling in a fully loaded train to disembark at any station. These passengers may be required to detrain onto a platform already occupied by peak-hour passengers waiting to board.

At Intermediate Stations, maximum POL during irregular operation therefore is calculated as one fully loaded train per platform edge plus the boarding load in peak or non-peak direction.

At Terminal Stations, maximum POL during irregular operation is calculated as one full train plus boarding load in peak direction.



1 **Emergency Evacuation** – The irregular operation scenario is the basis for emergency evacuation  
2 of the POL.

3 **Minimum Platform Area** – The platform shall be sized to accommodate the POL, allowing  
4 25 square feet per boarding passenger for passengers to circulate and wait during the peak  
5 period (pedestrian walkway LOS B). Upon train arrival, the addition of alighting passengers on  
6 the platform may result in a temporary LOS C for a short duration. Minimum platform area  
7 shall be exclusive of the 2-foot-0-inch safety edge strip, and platform obstructions such as  
8 escalators, stairs, columns, or other fixed elements.

B. Platform Edge

9 **Floor Finish** – Platform edge design and materials shall be standardized throughout the HST  
10 System. A standard 24-inch-wide clear zone of tactile floor tile shall be provided in accordance  
11 with state and federal regulations along the entire length of each platform edge. Adjacent to this  
12 edge strip and adjacent to selected vehicle doors a continuous 12-inch-wide zone of tiles shall be  
13 provided in a contrasting color. Platform edge provisions shall be confirmed with current  
14 accessibility requirements.

15 **Lighting** – Continuous overhead lighting shall be provided along the platform edge.

C. Platform Ends

16 Platform ends shall be delineated by railings and security gates at elevated and at-grade  
17 platforms, or end walls and gates at underground stations. Platform end gates shall be alarmed  
18 and appropriate signage applied to discourage unauthorized entry to the trackway or  
19 walkways. Stairs shall be provided as needed between platform and walkways.

D. Protection Screens

20 Provision shall be made for the installation of protection screens between through-tracks and  
21 station platform tracks. Screen height shall be approximately 6 feet-0 inches above platform  
22 finish floor. Screens shall provide acoustical baffles as well as space for potential advertising  
23 panels. Transparent sections to permit cross-platform views are desirable. OCS poles shall be  
24 integrated into the screen configuration where appropriate.

E. Platform Ancillary Spaces

25 Ancillary spaces for Core Systems, operations, storage, and maintenance functions shall be  
26 necessary on, adjacent to, or beyond public-platform areas. These spaces shall be located where  
27 circulation flow and sight lines shall not be obstructed. Terminal station platforms shall be  
28 provided with operations and maintenance facilities including a Platform Agent Booth (where  
29 operationally appropriate), and cleaning and storage rooms at selected stations. Underground  
30 and Terminal Stations may require additional special platform ancillary spaces.

31 Under-platform space (excluding the refuge area) may be used to run cables and other  
32 mechanical and electrical systems. Sections shall be divided with appropriate fire separations.

F. Weather Protection

1 **Canopies** – Canopies shall be provided along the entire length of the platforms and extend  
2 transversely from the outer wall of each platform to at least 12 inches beyond the edge of  
3 platform. Coordinate canopies with the OCS.

4 **Wind Screens** – Wind screens may be used if local wind conditions justify passenger protection.  
5 Wind screens shall be glazed to ensure visibility and detailed with consideration for regular  
6 maintenance needs.

7 Clear-glazed enclosures with seating are an acceptable alternative to continuous platform  
8 screening; however, windscreen enclosures shall not obstruct views or circulation.

G. Vertical Circulation

9 Wherever possible, access to platforms from the Concourse or Mezzanine will be arranged to  
10 encourage distribution and collection of passengers along the entire platform length.

H. Concealed Spaces

11 Corners, recessed areas, and other potential hiding places shall be minimized.

I. Platform Amenities

12 Refer to Section 14.3.8 for furnishings, fixtures and equipment to be provided on platforms.

J. Grounding and Bonding

13 For grounding and bonding requirements on platforms, refer to the *Grounding and Bonding*  
14 *Requirements* chapter.

**14.3.2.3 Special Platform Conditions**

A. Platforms in Shared Use Corridors

15 At stations where HSTs and conventional passenger trains are expected to stop, platform design  
16 shall accommodate both high-speed and regional rail platform requirements. Specific  
17 compatibility issues related to platform length, height, clearances and door location markers  
18 shall be considered. Dedicated platforms which serve each type of service are preferred.

B. Platforms Adjacent to Through Tracks

19 The speed of trains on tracks adjacent to station platforms shall not exceed 125 mph. At stations  
20 accommodating through-trains operating on tracks adjacent to the platform, platforms shall be  
21 marked to indicate locations for passengers to wait. Passengers shall wait 5 feet-0 inches  
22 (minimum) from the platform edge. Where this zone is marked, 3.0 feet shall be added to the  
23 setback from platform edge to large obstructions so that minimum setback is 12 feet-0 inches.

C. Train Stopping Position

24 It is assumed that both single (200 m) and coupled (400 m) trainsets shall be operated from  
25 commencement of system operation. Platform finishes, equipment and signage shall therefore  
26 reflect the use of both shorter and longer train lengths.

### 14.3.3 Concourse and Mezzanine

The Concourse is the interior space between station entrances and platforms, including the free and Paid Areas. The Concourse is the gateway to the rail service and provides passengers with information, ticketing, and waiting areas for passengers and “meeters and greeters.”

Where height is available, a mezzanine level may provide an intermediate space where some of these passenger and staff facilities are located. This section addresses Concourse public areas. Non-public Concourse areas are addressed elsewhere in this chapter.

#### 14.3.3.1 Concourse Planning Standards

**Openness** – The Concourse shall be open, spacious and have a high level of visibility to optimize passenger orientation and staff surveillance.

**Obstructions** – Passenger facilities such as public toilets, ticketing windows, information kiosks, and all associated queues shall be located clear of primary paths of pedestrian movement. Primary pedestrian circulation areas shall be free of columns and other obstructions.

**Ceiling Height** – Suspended Concourse ceilings shall be no less than 10 feet above the finished floor; a minimum of 16 feet is preferred. Clearance beneath signs or other obstacles shall be no less than 8 feet-0 inches. A ceiling utility zone no less than 3 feet-0 inches clear height shall be provided between suspended ceilings and the structure above.

**Concourse Slope and Drainage** – Floor drains in Concourses shall be provided. However, Concourse floors need not slope to drains except where weather protection cannot prevent wind-blown rain from entering the station.

#### 14.3.3.2 Entrances

**General** – Entrances provide a gateway between the station building, the station site, and the surrounding community. As such, they shall be distinctively designed, clearly identifiable from either the station interior or exterior, and easily accessible. Entrances may be freestanding or integrated into surrounding developments provided they are clearly identifiable as station entrances. For stations with more than one major entrance, one entrance shall be designated as the main entrance and be provided with all required passenger facilities. Other entrances may be provided with minor amenities and ticketing provisions. A free passageway shall connect secondary entrances to the primary entrance.

**Location** – Locate entrances wherever passengers may need to connect with other modes of transportation, e.g., adjacent to taxi and bus connections, kiss and ride, passenger parking, etc.

**Number** – Stations shall have at least two entrances. Entrances shall also function as emergency exits that meet exit requirements. Entrances may be supplemented with emergency exit-only doors as required to provide the required exiting capacity.

**Size** – The minimum width of each entrance shall be 10 feet-0 inches. At least one entrance shall have a width of 15 feet-0 inches. Floors directly adjacent to entrances shall be level for at least 10 feet-0 inches inside and outside of the entrance.

**Doors** – Automated sliding or swinging doors, 3 feet-0 inches minimum width per leaf, shall be provided. A minimum clear space of 2 feet-0 inches shall be provided to each side of a door or group of doors. Each entrance shall have a minimum width of one-half of the required egress width for the station.

**Other Considerations** – Entrances shall be ADA and universally accessible. Exterior entrances shall provide wind, rain, and, if appropriate, flood protection. Vestibules will be considered where station temperatures will be dramatically different from exterior temperatures. Every entrance shall be provided with a rolling shutter or security gate to allow for station closure.

#### 14.3.3.3 Free Area

**General** – The Concourse Free Area provides space for patrons to circulate freely without a ticket and where space is provided for station entrances, passenger circulation, public restrooms, passenger information, and ticketing. Intermediate stations will preferably provide a single Free Area in order to consolidate passenger amenities and staff functions within a single managed space. It shall be located on the ground level adjacent to the entrances to facilitate passenger orientation upon entering the station. The space shall immediately convey the dynamic image and identity of an HST station.

**Multiple Free Areas** – At Terminal Stations and special Intermediate Stations where station access conditions necessitate separation of entrances and multiple Free Areas, free pedestrian walkways shall connect the Concourse Free Areas. At stations with intermodal transfers, the locations of other modes may make this infeasible and duplication of passenger ticketing and information services may be necessary.

**Non-HST Patron Circulation** – Depending upon the station site configuration, the station may be used for circulation through the site or neighborhood. Where appropriate, the station Free Area will allow for circulation through the station and across station area tracks. This will enhance the importance of the free public space and integration of the station into station area development. Where pedestrian circulation is provided through the station, provisions shall be made for secure closure of the station after operating hours.

##### A. Free Area Circulation

**General** – The Concourse Free Area contains circulation space for passengers traveling between station entrances and the Paid Area. Primary routes of passenger movement within the Free Area shall connect all primary entrances with one another without obstruction. Ticket sales, passenger information, public toilets, and waiting areas shall be located adjacent to primary circulation paths and shall be clearly visible to passengers. Passenger information within the Free Area shall include system signage, passenger information system displays (including a prominent timetable screen displaying train arrivals and departures), and a large-scale clock.

1 **Location** – Concourse circulation will typically be located on the ground level adjacent to  
2 entrances for clarity of function for passengers entering the station.

3 **Configuration** – Wherever practical, turn-backs and U-turns shall be avoided since passengers  
4 circulate between entrances, ticketing facilities, and the Fare Collection Line. Cross flows shall  
5 be avoided between boarding and alighting passengers. Configuration shall allow for passenger  
6 circulation between HST and other rail or transit systems.

7 **Sizing** – Adequate width shall be provided for unobstructed horizontal circulation between  
8 entrances and the Concourse Paid Area. Circulation width is based on peak-hour passenger  
9 boardings and alightings. Net Free Area circulation width ( $C_f$ ), exclusive of any obstructions, is  
10 calculated as shown in Table 14-3.

11 This circulation width shall be exclusive of other required spaces such as waiting areas,  
12 information kiosks, or queues for ticket purchases. Where the primary circulation route  
13 bifurcates towards multiple exits, the minimum width may be proportionate to the estimated  
14 number of entrance users.

#### B. Free Waiting Areas

15 **General** – Free waiting areas provide places for passengers and the general public to wait prior  
16 to entering the Paid Area or leaving the station.

17 **Location** – Waiting areas shall be located so they are easily accessible but do not impede the  
18 principal circulation paths between entrances and the Paid Area. As some of this space is  
19 dedicated for “meeting and greeting” of passengers, waiting areas shall be located adjacent to  
20 primary circulation paths. Spaces shall be organized so that those waiting do not impede flows  
21 for others going to and from the platforms. There may be single or multiple waiting areas  
22 within the Free Area. Queues for ticketing facilities may not encroach into waiting areas.

23 **Amenities** – Seating and provisions for visual display and audio announcement of train arrival  
24 and departure information shall be provided in waiting areas. Waiting areas shall include  
25 power outlets for laptops, Wi-Fi, and television monitors.

26 **Sizing** – Total area for waiting within the Free Area ( $W_f$ ) shall be sized as shown in Table 14-3.

#### C. Ticketing and Station Information

27 **General** – Ticketing and station information provisions are located within the Concourse Free  
28 Area and shall be directly visible to passengers entering the station.

29 **Ticketing** – Space shall be provided within the Free Area for a Ticket Sales Office and ticket  
30 vending machines (TVMS). Queuing areas for ticketing functions shall not encroach into other  
31 required Free Area spaces. Space requirements and standards are discussed in Section 14.3.6.1.  
32 Fare collection is addressed in Section 14.3.3.5.

33 **Station Information** – A Passenger Information Counter shall be provided within the Free Area  
34 as described elsewhere in this chapter. This counter and its queue shall be located adjacent to

the primary circulation routes and shall not obstruct primary passenger circulation. Other unstaffed information kiosks providing written information, maps, train schedules, etc., shall be located within waiting areas and near entrances.

#### 14.3.3.4 Paid Area

**Defined** – The Concourse Paid Area is located between the Fare Collection Line and the platforms. It includes Concourse circulation space, waiting areas, paid public restrooms (at Terminal Stations only), mezzanine (where occurs) and vertical circulation elements leading to platforms. Vertical circulation to platforms shall be clearly visible from the Fare Collection Line, permitting passengers to access any platform. Platforms are located within the Paid Area but are addressed separately in Section 14.3.2.

**Access** – Access into the Paid Area requires a paid fare and possession of a valid ticket.

**Demarcation** – The demarcation between Free Area and Paid Area is located at the Fare Collection Line, which employs mechanical Fare Gates and a Passenger Service Booth (PSB).

**Security Provisions** – Space for security equipment and staff shall be provided as defined in Section 14.3.11.

**Passenger Information** – Extensive directional signage and passenger information shall be displayed throughout the Paid Area.

##### A. Paid Area Circulation

**General** – Direct movement between the Fare Collection Line and platforms shall be facilitated through clear sight lines and logical configuration. Access to the platform may require vertical circulation including stairs, escalators, and elevators (vertical circulation is described in Section 14.3.4.3). Waiting areas shall be located within the Concourse Paid Area and shall not impede major circulation routes. Avoid turn-backs wherever practical, providing direct routes between the fare line and vertical circulation.

**Sizing** – Adequate width shall be provided for unobstructed horizontal circulation between the Fare Collection Line and vertical circulation leading to platforms. Circulation width is based on peak-hour passenger boardings and alightings. Net Paid Area circulation width ( $C_p$ ), exclusive of any obstructions or other required spaces such as waiting areas, is calculated as shown in Table 14-3.

##### B. Paid Waiting Areas

**General** – Paid waiting areas provide a place for passengers to wait prior to entering a platform. Short-term seating, information screens, and waste receptacles are located in this area.

**Location** – Waiting areas shall be located so they are easily accessible but do not impede the principal circulation paths between the Fare Collection Line and vertical circulation leading to platforms. Spaces shall be organized so that those waiting do not impede circulation of others going to and from the platforms. There may be single or multiple waiting areas within the

Concourse Paid Area. Additional waiting areas may be provided on platforms but shall not impede platform circulation.

**Amenities** – Seating and provisions for visual display and audio announcement of train arrival and departure information shall be provided in waiting areas. Waiting areas shall include power outlets for laptops, Wi-Fi, and television monitors. Public wireless services shall be coordinated with the station communication system. Views between Concourse and platforms shall be provided wherever possible.

**Sizing** – Total area for waiting within the Paid Area ( $W_p$ ) shall be sized as shown in Table 14-3.

**Table 14-3: Concourse Circulation and Waiting Area**

Symbol	Description	Formula <sup>1,2</sup>
$C_f$	Net Free Area circulation width	$(P_{15B} + P_{15A}) \div (15 \times 10 \text{ people/ft/min})$
$C_p$	Net Paid Area circulation width	$(P_{15B} + P_{15A}) \div (15 \times 10 \text{ people/ft/min})$
$W_f$	Net waiting area in Free Area	$[(P_{15B} \times 1.1) + (P_{15A} \times 0.1)] \times 14 \text{ square feet}$
$W_p$	Net waiting area in Paid Area	$P_{15B} \times 14 \text{ square feet}$

<sup>(1)</sup>  $P_{15B}$  = Peak 15 minute boardings;  $P_{15A}$  = Peak 15 minute alightings;

<sup>(2)</sup> at LOS B

### C. Premium Club Lounge

**General** – Full build-out Terminal Stations shall incorporate into the Concourse Paid Area a Premium Club Lounge for business travelers and “frequent rider” customers. The facility shall provide premium amenities, including computers, printers, internet, conference room, ticketing, light snacks and beverages, passenger services, and concierge services. Dedicated restrooms may be provided for customers.

**Location** – The Premium Club Lounge shall be located within the Concourse Paid Area, close to the entrance to the platforms, and will preferably provide natural light. This facility may be operated on a subscription basis as a for-profit enterprise, an amenity included with certain classes of tickets, and/or with frequent traveler status as a prerequisite.

**Designated Stations** – Terminal Stations, including San Francisco Transbay, Sacramento, Los Angeles, Anaheim and San Diego.

**Size** – 600 square feet, minimum

### 14.3.3.5 Fare Collection

#### A. Elements of Fare Collection

Stations shall be designed to allow for the use of Fare Gates. Provisions for fare collection in the station shall include the following:

- Ticket Sales Office
- Ticket vending machines
- Fare gates

It is assumed that 80 percent of all peak-hour passengers will purchase tickets at the station. The remaining 20 percent of peak-hour passengers will purchase tickets via other means prior to arriving at the station. Detailed information regarding fare collection policies and required equipment are addressed in a separate operations document.

#### **B. Ticket Sales Office**

**General** – It is assumed that 15 percent of all peak-hour passengers will purchase tickets at the Ticket Sales Office from station staff in lieu of the TVMs. Other ticketing transactions may also be conducted, including refunds, ticket adjustments, or retrieval of reserved tickets.

**Location** – One secure Ticket Sales Office shall be provided at the primary Concourse Free Area between the public and non-public areas and shall be readily visible from station entrances. In the event that a station provides more than one Concourse Free Area, a second Ticket Sales Office may be required only if warranted by substantial patronage and acceptable within HST operations policies. Access shall be via a secure corridor, through a security door with viewing panel.

**Window Quantity** – Ticket windows shall be calculated as shown in Table 14-4, rounded up to the nearest whole number. A spare window may be included to accommodate ticket staff shift changes. At least one ticket window shall be universally accessible to all passengers.

**Ticket Office Size** – Each window shall be minimum 5 feet-0 inches wide. A minimum of 75 square feet of ticket office area shall be provided for each window. Refer to Section 14.3.4 for queuing distance at windows.

**Related Provisions** – Ticket sales administrative offices and other ticketing-related offices shall be located adjacent to the Ticket Sales Office within the non-public area.

#### **C. Ticket Vending Machines**

**General** – It is assumed that 65 percent of peak-hour passengers will obtain their tickets at TVMs within the stations, either by purchasing tickets or printing pre-purchased tickets.

**Location** – TVMs shall be located in the Concourse Free Area near the Ticket Sales Office and adjacent to the main circulation routes. The machines and corresponding queuing space (summarized in Section 14.3.4) shall not encroach into the passenger circulation space. TVMs shall be located against a wall, in view of entrances. For stations with multiple entrances, it may be appropriate to locate TVMs at secondary entrances in order to ensure logical passenger flow. TVMs shall be grouped into clusters. Depending on the size of the station and the number of TVMs, multiple clusters may be appropriate.



**TVM Quantity** – TVMs shall be provided to meet peak passenger demand per Table 14-4, rounded up to the nearest whole number. One additional TVM shall be provided for out-of-service conditions.

**TVM Size** – For space planning purposes, TVMs can be assumed to be 3 feet-4 inches wide, 3 feet-0 inches deep and 6 feet-0 inches high, and spaced at 4 feet-0 inches on center. TVMs shall fit into the standard station module. Space shall be allowed for addition of a minimum of 10 percent more TVMs if passenger demand requires additional machines.

**Table 14-4: Ticket Sales Facilities**

Element	Station Quantity	Station Minimum
Ticket Windows	$P_{60B} \div 600$	2
Ticket Vending Machines	$P_{60B} \div 280$	3

#### D. Fare Gates

**General** – Fare Gates shall demarcate the separation between Free and Paid Areas. Standard gates shall accommodate most passengers while oversized gates shall be available for people with luggage and persons with special accessibility needs. A PSB (Section 14.3.8.2) shall be located adjacent to the main array of gates to assist passengers with operation of Fare Gates.

**Size** – Standard width Fare Gates are approximately 2 feet-0 inches clear opening and spaced 3 feet-0 inches on center. Oversized gates are 3 feet-0 inches clear opening and spaced 4 feet-0 inches on center. Provisions shall be made for underfloor power and data conduits.

**Quantity** – The number of Fare Gates provided for arriving and departing passengers is based on peak ridership and 50 people per minute (ppm) capacity for fully open gates per Table 14-5.

**Table 14-5: Fare Gates**

Station Type	Travel Direction	Quantity Formula <sup>1,2</sup>
Terminal	Arriving Gates	(Train Capacity / headway in minutes) / 50 ppm Fare Gate capacity
Terminal	Departing Gates	$(P_{15B} / 15) / 50$ ppm Fare Gate capacity
Intermediate	Arriving Gates	$(1.125 \times P_{1A}) / 50$ ppm Fare Gate capacity
Intermediate	Departing Gates	$P_{1B} / 50$ ppm Fare Gate capacity

<sup>1</sup>  $P_{15B}$  = Peak 15 minute boardings;  $P_{15A}$  = Peak 15 minute alightings

<sup>2</sup>  $P_{1B}$  = Peak minute boardings;  $P_{1A}$  = Peak minute alightings

**Additional Gates** – The number of gates shall be rounded to the next highest integer. One additional gate shall be provided if the number of required gates is less than 10. Two additional

gates shall be provided if the number of required gates is equal to or greater than 10. Where practical, expansion capacity for additional future Fare Gates may be provided.

**Oversized Gates** – At each Fare Gate array, at least one Fare Gate shall be oversized. More may be necessary for stations serving more passengers with luggage or as required by ADA.

**Emergency Gates** – At least 2 emergency and/or service gates shall be provided along the free/paid line to be used by staff and in case of emergency. More gates may be warranted for emergency evacuation.

**Queuing** – Refer to Section 14.3.4 for queuing distances required at Fare Gates.

**Operation** – Under emergency situations, Fare Gates shall remain in the open position.

**Provision for Security Measures** – An allowance for potential security facilities shall be provided. This space shall be located on the Free Area side of each gate array. The size of this space shall be 20 feet deep by the total width of the Fare Gates.

#### 14.3.3.6 Public Restrooms

**Locations** – Intermediate Stations shall provide public restrooms only within the Concourse Free Area.

Terminal Stations shall provide public men's and women's restrooms within both the Concourse Free Area and the Concourse Paid Area. Additionally, two unisex restrooms shall be provided within the Paid Area for mobility and special needs passengers.

Public restrooms shall be located adjacent to main circulation routes. Restroom entries shall be clearly visible from circulation routes.

**Size** – Restroom facilities shall be based on projected occupant loads. The minimum occupant load for the facility will be based on 15-minute peak station patrons (P<sub>15</sub>) and applicable code requirements.

**Access** – Paid area restroom entrances shall be screened vestibules in lieu of doors. Free area restroom entrances shall be provided with lockable doors. Access to Free Area restrooms will be controlled by station staff.

**Provisions** – Toilet stalls shall be oversized (minimum 3 feet-0 inches wide x 6 feet-0 inches deep) to accommodate passengers with luggage. Partitions shall be ceiling-mounted. Surfaces and fixtures shall be vandal resistant. Washable surfaces and floor drains are required. Accessories shall include hands-free air dryers, soap dispensers, paper towel dispensers, toilet paper dispensers, framed mirrors, and foldable baby changing tables in both men's and women's restrooms.

A janitor's room shall be provided adjacent to every set of public restrooms.

### 14.3.3.7 Passenger Amenity (Commercial) Spaces

**Size** – It is likely that passenger amenity spaces will be provided at stations for commercial passenger services. In order to not preclude addition or inclusion of these spaces, a standard assumption for passenger amenity space can be used as shown in Table 14-6.

**Table 14-6: Passenger Amenity Space Allocation**

2035 Daily Boardings	Passenger Amenity Space
Less than 5,000	3,000 square feet
5,000-10,000	6,000 square feet
More than 15,000	9,000 square feet

**Location** – Where passenger amenity space is provided in the Free or Paid Area, it shall be located close to major passenger circulation routes or waiting areas but shall not impede passenger flow.

**Services** – Routes to supply passenger amenity spaces shall be separated from passenger circulation routes.

### 14.3.4 Passenger Circulation

#### 14.3.4.1 Circulation Planning

Effective station planning provides passengers with clear circulation patterns, as consistent as possible from station to station, so that passengers are able to safely and comfortably circulate throughout the HST System. Station circulation spaces include platforms, mezzanines, Concourses, bridges, pedestrian walkways, elevators, escalators, stairs, ramps, and emergency routes. Access and circulation shall be simple, obvious, and comfortable, recognizing many passengers will be unfamiliar with traveling on a high-speed rail system.

##### A. Basis for Sizing

Generally, public spaces provided for passenger circulation shall be sized to accommodate peak-period ridership for Fruin LOS B during normal station operations. Where space is constrained by physical conditions that cannot be mitigated cost-effectively, facilities may be designed for a peak LOS C as approved by the Authority.

Stations shall provide evacuation capacity in accordance with NFPA 130 and as described in Section 14.3.5.

##### B. Circulation Principles

Circulation patterns shall consider the following principles:

- **Right-hand Rule** – Observe right hand flow for pedestrian circulation.
- **Cross Flows** – Avoid passenger cross flows wherever possible.
- **Dead Ends** – Avoid dead end conditions wherever possible.

- **Obstructions** – Columns, queues, kiosks, equipment, etc., shall not encroach into circulation routes.
- **Directional Travel** – Circulation routes and station layouts shall minimize changes in direction.
- **Visual Orientation** – Circulation routes and station layouts shall facilitate passenger orientation by means of placing each sequential stage of circulation (ticket gates, escalator, platform, etc.) in clear view of the current stage.
- **Decision Points** – Where there is a need to make a directional decision, the necessity for passengers to make multiple decisions at a single location shall be avoided.
- **Safety** – Concealed corners or recesses, which may be used as hiding places, shall be avoided. Passengers’ perceptions of safety are a fundamental system requirement.

#### C. Queuing Space

Space for passengers to queue at circulation elements shall be provided (per Table 14-7). This space shall not overlap or encroach into primary circulation routes. Because escalators will be reversible, unobstructed queuing space shall be allocated at the top and bottom of every escalator.

**Table 14-7: Queuing Distance Requirements**

Element	Dimension (min.)
Escalator (top and bottom from working points)	15 feet 0 inches
Stair (top and bottom from first tread)	15 feet 0 inches
Elevator Entrance	8 feet 0 inches
Ticket Gates, both free and paid sides	20 feet 0 inches
Ticket Sales Windows	20 feet 0 inches
Ticket Vending Machines	8 feet 0 inches
PSB	10 feet 0 inches

#### 14.3.4.2 Horizontal Circulation

Adequate horizontal circulation width is essential to achieve the general circulation planning principles listed in the preceding section. Horizontal circulation width provisions are based upon accommodating the flow of a given number of passengers during the peak minute, occupying a given width, and walking at a given average speed.

##### A. Passageways

**Calculated Width** – Minimum net width of any passageway shall be calculated on the basis of the sum of the peak 15-minute boarding and alighting passenger flows.

Passageway width may be calculated based upon LOS B horizontal circulation unit capacity (10 people per foot per minute) during peak-minute surges as shown in Table 14-8.

**Table 14-8: Passageway Width Calculation**

Symbol	Description	Formula <sup>1, 2, 3</sup>
$P_u$	Unconstrained Passageway width	$(P_{15B} + P_{15A}) \div (15 \times 10 \text{ p/ft/m})$
$P_c$	Constrained Passageway width	$P_u + B$

(1)  $P_{15B}$  = Peak 15 minute boardings;  $P_{15A}$  = Peak 15 minute alightings  
 (2) at LOS B,  
 (3)  $B$  = Buffer Zone – 1'-6 inches for space adjacent to each wall, barrier or railing, 2 feet-0 inches at platform edge except in cases when the train remains at the platform edge such as during emergency train evacuation

**Minimum Width** – The greater of the calculated passageway width and minimum passageway widths shall be provided (as shown in Table 14-9).

**Table 14-9: Passageway Width Minimums**

Element	Dimension
Public areas	16 feet 0 inches preferred minimum
Public areas with unidirectional movement	6 feet 0 inches absolute minimum
Public areas with bidirectional movement	8 feet 0 inches absolute minimum

**Equivalent Capacity** – Passageways shall have a capacity at least equal to the capacity of any stairs and escalators that feed it. Passageways leading directly to/from platforms shall be sized to accommodate potential 50 percent surge loadings of boarding and/or alighting passengers.

**Constraints** – Horizontal circulation may be constrained by walls, barriers, or platform edges as in a passageway, pedestrian bridge, platform or corridor, or may be unconstrained as in an open Concourse or mezzanine. Where passageways are constrained, extra width shall be added to allow a “buffer zone” (indicated in Table 14-8), which is a distance from which people tend to stay away from vertical surfaces when walking.

#### **B. Pedestrian Bridges and Tunnels**

Minimum width of pedestrian bridges and underground tunnels shall be calculated in the same way as described in the previous Passageways section. Natural light shall be introduced into bridges, where practical.

#### **C. Height**

Minimum horizontal circulation ceiling height shall be 10 feet-0 inches above the finished floor. Suspended signs, fixtures, or fittings shall have a vertical clearance of at least 8 feet-0 inches above the finished floor or as required to conform to accessibility standards.

D. Floor Slope

- 1 Floors of horizontal circulation elements shall not exceed a 5 percent slope.

E. Moving Pedestrian Walkways

- 2 Where continuous horizontal Concourse or passageway circulation distances exceed 600 feet,  
3 the use of moving pedestrian walkways may be considered.

F. Non-public Corridors

- 4 At non-public spaces, corridor width connecting service and plant rooms shall be no less than  
5 3 feet-8 inches clear width or as determined by building code. Corridor width shall consider  
6 access needs for installing or replacing equipment.
- 7 Wherever possible, non-public circulation routes for station staff, supplies, and refuse collection  
8 shall be physically separated from public circulation routes.

**14.3.4.3 Vertical Circulation**

A. Planning Principles

- 9 **Primary Mode** – Escalators shall be provided as the primary mode of vertical circulation to  
10 accommodate normal peak period boarding and alighting passenger flows within station public  
11 areas. Elevators may be considered as the primary mode of station vertical circulation at low-  
12 ridership stations at the discretion of the Authority.
- 13 **Secondary Mode** – Stairs shall be provided in addition to escalators as required for emergency  
14 evacuation. Refer to Section 14.3.5 for means of escape during station evacuation.
- 15 **Escalator Alternative** – At least 1 stair shall be provided between levels for passengers who  
16 prefer not to use escalators.
- 17 **Right-hand Flow** – Stairs shall generally be located to the right of a single escalator for a  
18 passenger preparing to descend. When stairs are adjacent to two escalators, the stairs shall be  
19 located between the escalators. If stairs are wider than 15 feet, the stair may be divided and  
20 moved to the outside of the escalators.
- 21 **Orientation** – Stairs and escalators shall be oriented to direct passengers towards their  
22 destination and to avoid U-turns.
- 23 **Future Stair Replacement** – Stairs shall be designed so that they may be replaced by escalators  
24 if demand requires in the future, if feasible. This includes the stair support structure and space  
25 allocations for escalator operational elements.
- 26 **Vertical Distance** – Where vertical circulation is provided in excess of that required for peak  
27 flows, an escalator for upward flow shall be provided wherever the vertical travel distance  
28 exceeds 10 feet-0 inches. An escalator for downward flow shall be provided wherever the  
29 vertical travel distance exceeds 25 feet-0 inches.

B. Escalators

**Quantity** – Escalator quantities vary by station and ridership. At least 1 extra escalator shall be provided between Concourse and Platform levels as a backup during escalator maintenance.

**Installation** – Escalators shall be installed at a 30-degree slope between lower and upper working points. Refer to Standard and Directive drawings for additional dimensional criteria. Lifting hooks shall be provided above escalators for initial installation and future replacement.

**Specifications** – Per Table 14-10

**Table 14-10: Escalator Criteria**

Escalator Elements	Planning Criteria
Type	Heavy-duty reversible
Operation	Operated either from station control room or locally by key; motion detectors top and bottom for energy conservation
Tread Width	40 inches
Flat Steps	5 flat steps at top landings and 4 flat steps at bottom landings
Speed	Dual-speed, 90 feet per minute and 120 feet per minute
Capacity	70 people per minute
Opposing Escalators	80 feet-0 inches minimum working point to working point
Clear Headroom	10 feet-0 inches absolute minimum above nosing

**Emergency Operation** – In emergency situations, escalators running in the opposite direction from the exiting flow can be stopped remotely and used as stairs for exiting. Escalators with an emergency stop button shall be provided with accompanying signage.

**Weather Protection** – Escalators shall be protected from direct rainfall by a roof or canopy.

**Queuing Distance** – Refer to Section 14.3.4 for escalator queuing requirements.

**Side Clearances** – Refer to Standard and Directive Drawings for clearances between structural members and floor slabs to allow for escalator truss and finish materials.

**Machine Spaces** – Each escalator shall be provided with an upper machine space and a lower machine room (or pit) housing motors and other escalator equipment. Machine spaces shall be adequate for machine maintenance and accessible from removable floor cover plates. When located in ceilings above public spaces, machine space within the suspended ceiling shall be concealed. Drainage of escalator machine rooms shall be provided for maintenance and cleaning.

### C. Public Stairs

**Quantity** – Stairs in addition to escalators shall be provided where required to accommodate NFPA 130 emergency egress conditions. Refer to Section 14.3.5 for emergency evacuation criteria.

**Width** – Stairs required for emergency evacuation shall provide exiting capacity as required to supplement the escalator emergency capacity. Minimum public stair width shall be as required by NFPA 130 or in Table 14-11 (whichever is greater).

**Table 14-11: Stair Width Minimums**

Element	Minimum Width
Stairs next to escalators	6 feet-0 inches
Platform stairs	6 feet-0 inches
Stair-only entrances	8 feet-0 inches
Emergency stairs	6 feet-0 inches

**Alignment** – Align lower stair working point with lower escalator working point. Stairs shall slope at 30 degrees to align with escalators.

**Queuing Distance** – Refer to Section 14.3.4 for stair and escalator queuing requirements.

**Emergency Stairs** – Emergency stairs shall be provided where escalators and stairs provided for normal operation are insufficient for emergency evacuation of the station. Emergency stairs shall be easily accessible from public areas but shall not be used during normal operation. No ancillary rooms may be accessed from the emergency stairs.

**Headroom** – Minimum headroom above public stairs shall be at least 10 feet.

**Materials** – Stair treads and nosings shall be constructed of slip-resistant, robust, non-combustible materials, suitable for and proven to be utilized in high-traffic transportation areas. The leading edge of nosings shall be eased to 1/4-inch to 1/2-inch radius. Risers shall be raked back a minimum 1 inch from edge to back of the tread below.

Where stairs are exposed to weather, provide a floor drain at the bottom of the stair.

**Weather Protection** – Stairs may be covered depending on level of use and expected level of inclement weather for station region. Where exterior stairs are adjacent to covered escalators, extend covering over stairs in a single roof structure.

### D. Ramps

Ramps may be utilized where there are small changes in elevation or for wheelchair access. Ramp width follows horizontal circulation requirements and shall comply with building code



1 and accessibility requirements. Ramps may be appropriate vertical circulation instead of a small  
2 number of stairs.

#### E. Passenger Elevators

3 **General** – Elevators shall be provided and designed for all patrons, including patrons carrying  
4 large luggage, strollers, bicycles, stretchers, for passengers in wheelchairs and for the movement  
5 of supplies. Wherever access to the platform requires a change of level, an elevator shall be  
6 required.

7 **Quantity** – At least one elevator shall be provided to connect each station level. Space and  
8 structural accommodation shall be provided for the future addition of a redundant passenger  
9 elevator between station levels. Two or more elevators between levels may be required initially  
10 at Terminal Stations in order to accommodate passenger flow requirements and passengers  
11 with luggage.

12 **Location** – Wherever practicable, elevators between the Concourse and platforms shall be  
13 located near the center of the platform. Elevator doors shall face the platform end and not the  
14 platform edges.

15 **Size** – Elevators shall be 4,000-pound passenger-type with center-opening doors. Elevators shall  
16 be sized to move standard station maintenance equipment between levels.

17 **Speed** – Rated travel speed shall be either 125 feet per minute (fpm) or 150 fpm with a  
18 maximum wait time of 30 seconds.

19 **Queuing** – Refer to Section 14.3.4 for elevator queuing requirements.

20 **Materials and Finishes** – Elevators shall be glazed on three sides with clear glass to increase a  
21 real and perceived sense of safety. Doors shall be provided with glazed vision panels. Hoistway  
22 structure shall be heavy steel framing. Solid wall and ceiling panels shall be Type 304 brushed  
23 stainless steel except in marine environments where Type 316 is required. Floors shall be non-  
24 slip, durable material proven to be appropriate in a heavy-use transit environment.

25 **Machine Rooms** – Elevator machine rooms, where required, shall be located at the lowest level,  
26 as close as possible to the elevator.

27 **Weather Protection** – Elevator entrances shall be covered to protect passengers and facilities  
28 from inclement weather.

#### F. Service Elevators

29 **General** – Service elevators shall be provided at Terminal Stations only for movement of goods,  
30 refuse and equipment between station levels for train inspecting, cleaning, restocking and  
31 repairs. Passenger elevators shall not be used as service elevators.

32 **Number** – There shall be one service elevator serving each platform at Terminal Stations.  
33 Service elevators shall be provided at designated Intermediate Stations only where it is

1 anticipated there will be frequent movement of goods or equipment between the non-public  
2 areas and platforms. A service corridor shall be provided either beneath or above track level  
3 connected to the non-public station areas.

4 **Capacity** – Minimum 3,500-pound capacity service elevators shall be provided.

#### 14.3.4.4 Accessibility

##### A. Goals

5 **General** – Stations shall be designed and constructed to be readily accessible to and usable by  
6 individuals with disabilities. Stations shall fully comply with relevant requirements of Title 24  
7 of the 2010 CBC, with the 2010 version of the federal ADA Standards for Accessible Design, and  
8 the current version of the ADAAG.

9 As revisions to the reference standards are issued, they shall become immediately applicable to  
10 the design of future stations and related station facilities.

11 **Universal Design** – The principle of Universal Design shall be applied to the design of all  
12 stations and related passenger facilities. In addition to passengers with mobility, hearing, and  
13 sight impairments, stations shall be accessible to passengers with mental impairments, elderly  
14 passengers with or without assistants, families with infants and children, and young  
15 unaccompanied passengers. Station and related passenger facilities shall be designed for easy  
16 access by first-time patrons, non-English speaking passengers, and foreign visitors.

17 **Scope** – Station elements used to deliver Universal Design include, but are not limited to, easily  
18 recognizable accessible paths, convenient elevators, escalators, and stairs, and clearly visible  
19 and audible passenger information systems. Accessible paths shall be uninterrupted from  
20 station property boundaries through station entrances, through Concourse, mezzanine, and to  
21 the train platforms. Accessible paths shall be designed to equally serve boarding, alighting, and  
22 transferring passengers.

##### B. Miscellaneous Provisions

23 **Accessible Routes** – Every public entrance shall be fully accessible. Elevators shall be provided  
24 between station levels. Ramps may be used between small changes in elevation. All public  
25 station facilities, including station entries, Concourses and platforms, ticket gates, ticket  
26 windows and machines, elevators, restrooms, and public amenity spaces shall be fully  
27 accessible. Directional signage shall clearly indicate accessible routes.

28 **Site Access** – Where public or private property development with public paths connects  
29 directly to stations, station plazas, or related station facilities, interface entrances shall be  
30 provided that are fully compliant with aforementioned accessibility standards. Furthermore, if  
31 such interface with the Authority's property is not the primary development entrance, a fully  
32 accessible path of travel shall be provided from the main entrance, through the development, to  
33 the point of interface with the station property.

1 **Non-public Areas** – Accessible provisions for non-public station areas and station staff areas  
2 are specified in relevant sub-sections of this chapter.

C. Emergency Egress for the Disabled

3 Evacuation of disabled passengers and station patrons will vary considerably with individual  
4 station configurations. Basic requirements for the egress of disabled patrons, passengers and  
5 staff are included in the 2010 CBC as well as CHSTP's Operations and Maintenance documents.

**14.3.5 Evacuation from Public Areas**

**14.3.5.1 Basic Goal**

6 A clear and consistent methodology for passengers, staff, and other station patrons to quickly  
7 and safely evacuate stations in the event of an emergency is essential. This methodology shall  
8 be consistent with operational policies and as established in NFPA 130 and related safety  
9 standards.

10 This section addresses means of evacuation from the public areas of stations. Evacuation from  
11 station non-public areas is addressed in Section 14.3.6. Firefighter access into stations during  
12 emergencies is addressed in Section 14.3.1.

**14.3.5.2 Causes for Platform Evacuation**

A. Fire

13 Due to the use of non-combustible building materials throughout stations and a high level of  
14 fire prevention and detection, station fires are unlikely. Nonetheless, station fires as well as train  
15 fires shall be considered as potential emergencies necessitating rapid and complete station  
16 evacuation.

B. Evacuation Scenario: Side Platform Station

17 At Side Platform stations, the evacuation scenario assumes that a fully occupied train enters the  
18 station during the peak hour. The platform is already occupied by passengers accumulated  
19 during the 15 minutes since the previous train departed and are now waiting to board the  
20 arriving train. Arriving passengers shall disembark onto the platform. This combination of  
21 peak-hour boarding and alighting passengers shall be rapidly evacuated from the station.

C. Evacuation Scenario: Center Platform Station

22 At Intermediate Center Platform Stations, the evacuation scenario assumes that two fully  
23 occupied trains arrive simultaneously during the peak period and, together with boarding  
24 passengers, shall be simultaneously evacuated.

25 Terminal Center Platform Stations shall take into account the uni-directional scheduling of  
26 arriving trains and the limitations of trackwork to consider whether a second fully loaded train,  
27 arriving concurrently at a single Center Platform is physically possible. If not, evacuation of a  
28 single fully loaded train with boarding passengers is sufficient.

### 14.3.5.3 Evacuation Goal

#### A. Principle

- 1 In accordance with NFPA 130 and CBC Sections 419 and 433, sufficient routes shall be available  
2 to evacuate the POL from the affected station platform in 4 minutes or less and from the most  
3 remote point on the platform to a point of safety in 6 minutes or less.

#### B. Multiple Escape Routes

- 4 Evacuation routes shall be planned so that a passenger confronted by a fire can turn away and  
5 make a safe escape. To achieve this goal the maximum travel distance to an escape route such as  
6 an escalator, stair, passageway, or entrance shall not exceed 325 feet.

### 14.3.5.4 Evacuation Components

- 7 The following factors shall be considered in emergency evacuation planning.

#### A. Peak Period

- 8 The emergency occurs during the peak 15 minutes of the peak hour at the ultimate peak design  
9 year.

#### B. Platform Occupant Load

- 10 Maximum POL is based on the evacuation scenarios described above. Platforms shall provide a  
11 minimum 10 square feet per occupant during emergency conditions or the minimum dimension  
12 shown in Section 14.3.2.1, whichever is greater.

#### C. Platform Egress

- 13 An egress point can be the first riser of a stair or escalator, a horizontal exit with an appropriate  
14 door or gate that exits to grade (including an exterior refuge zone from a Center Platform), or a  
15 fire-rated door (alongside platforms or at the ends of Center Platforms if a refuge zone is  
16 provided at the end of the platform).

#### D. Vertical Circulation

- 17 Only stairs and escalators may be used for evacuation between station levels; elevators are not  
18 considered for evacuation. Escalators normally moving in the direction opposite to evacuation  
19 shall be remotely stopped and used as stairs. One escalator is assumed to be out of service. For  
20 vertical circulation capacities, refer to Table 14-12.

#### E. Gates

- 21 All ticket gates, oversized gates and emergency gates are available and can be opened by station  
22 staff for free, full-width (non-turnstile) passage in the exit direction. For gate capacities, refer to  
23 Table 14-12.

**Table 14-12: Capacities of Station Evacuation Elements**

Element	Minimum Width
Escalators (fixed 40-inch tread width)	1.41 people per inch per minute x 40 inches = 56 ppm
Stairs	1.41 people per inch per minute
Fare Gates	50 people per minute
Oversized and Emergency Gates	2.08 people per inch per minute

**F. Walking Speeds**

For calculation of walking speed assumed for station evacuation, refer to Table 14-13.

**Table 14-13: Station Walking Speeds**

Element	Minimum Width
Horizontal evacuation on platforms	124 feet per minute
Horizontal evacuation on Concourse	200 feet per minute
Vertical evacuation (up direction)	48 feet per minute
Vertical evacuation (down direction)	60 feet per minute

**G. Place of Ultimate Safety**

**Elevated Stations** – Where platforms are open to the elements and the Concourse is below, the place of ultimate safety may be the Concourse level.

**At-Grade Stations** – Where platforms are located at grade level with the Concourse above or adjacent to the platform or tracks, the point of ultimate safety shall be a point outside the station enclosure.

**Underground Stations** – The place of ultimate safety shall be an uncovered public thoroughfare at ground level outside the station.

**14.3.5.5 Determining Means of Escape from Stations**

**A. Normal Operation**

The station components described in Sections 14.3.3 and 14.3.4 for normal operation shall comprise the baseline station plan. Normal operation layout shall be tested against the 6-minute evacuation standard. If the normal exiting routes are inadequate for emergency evacuation, additional capacity for evacuating the POL shall be provided.

**B. General Methodology**

Confirm the capacity of escalators and stairs to evacuate the platform in less than 4 minutes as follows:

$$\text{POL} \div \text{Platform Egress Capacity} < 4 \text{ minutes}$$

- Confirm the capacity of all gates to evacuate the POL freely within 6 minutes or less.
  - Confirm the capacity of all evacuation elements to evacuate the platform from its most remote point to a point of safety in 6 minutes or less. Evacuation time considers the following factors:
    - Walking time on the platform
    - Walking time between platform and Concourse
    - Walking time on the Concourse to a point of safety
    - Walking time between Concourse and grade level (where applicable)
- Where multiple entrance/exits are planned, the longest walking distance shall be used to calculate Concourse walking time.

#### C. Excessive Evacuation Time

If evacuation time exceeds 6 minutes, capacity shall be increased or evacuation distances shortened. Adding dedicated emergency exit stairs is preferable to adding escalators or public stairs, considering they are more economical and would be needed only in the case of emergency. Adding emergency gates is preferable to adding Fare Gates, considering they are more economical and would be needed only in the case of emergency.

### 14.3.6 Non-Public/Station Support Areas

This section pertains to non-public areas of the station required for the operation and maintenance of the station and the system. These include passenger service areas, station and system operation offices and other ancillary spaces including maintenance and building services.

**Size** – Refer to Room Data Sheets for minimum dimensional requirements for non-public areas. Room sizes and numbers may vary between stations depending on the station's location, type and function. Where room areas and/or heights are not indicated, a reasonable amount of space anticipated for each specific station shall be provided.

#### 14.3.6.1 Passenger Service Areas

Passenger service spaces require public interface with station patrons. As such, they shall be adjacent to station public areas, preferably at ground level. These spaces shall be consistent throughout the system in order to provide continuity and familiarity to passengers and station staff.

##### A. Lost and Found

**Function** – Space is used to store and manage passenger's lost items as well as to collect and return these items to and from passengers.

**Location** – Concourse Free Area adjacent to the Station Manager's office.

1 **Provisions** – At Intermediate Stations, minimum area of 80 square feet; at Terminal Stations,  
2 minimum area of 120 square feet. A door opening into the Concourse Free Area as well as an  
3 interior door connecting with the Station Manager's office shall be provided. Storage shelving  
4 and an internal passenger service counter shall be provided.

**B. Police Office**

5 **Function** – Selected stations shall provide an office for police responsible for station  
6 surveillance, passenger interface and railroad security.

7 **Location** – Concourse Free Area, near a station entrance, between public and non-public areas,  
8 adjacent to the Security office.

9 **Provisions** – Minimum area 160 square feet. A passenger service and observation counter shall  
10 be provided. Some stations may require holding cells and/or canine support facilities.

**14.3.6.2 Standard Station Operation Offices**

11 These back-of-house areas are required at every station but require no public interface. Rooms  
12 and spaces shall be arranged according to function and have separate non-public accesses.  
13 Access control shall be appropriate to each function. It is preferable for these offices to be  
14 located at Concourse ground level unless otherwise indicated. The following is a non-  
15 exhaustive list of these spaces.

**A. Station Manager's Office**

16 **Function** – This serves as the office for the Station Manager.

17 **Location** – Adjacent to other non-public administrative offices.

18 **Provisions** – Minimum area 270 square feet.

**B. Station Control Room**

19 **Purpose** – At Intermediate Stations, the Station Control Room (SCR) is where passenger  
20 circulation, ticketing, fare control, security, and building maintenance operations are monitored  
21 and controlled. The SCR may also function as an incident-response command center. This will  
22 be the place where first responders would coordinate activities with station personnel in the  
23 event of an emergency or security incident.

24 Local train-control capability within the immediate vicinity of the station shall also be available  
25 at this location. In the event of a large-scale train-control system failure, it shall be possible to  
26 move trains on a station-to-station basis by a station train dispatcher working under the  
27 direction of the train dispatcher in the central control facility until system control for that  
28 territory can be restored. This train control component of the SCR would not be staffed under  
29 normal conditions.

1 **Location** – Designers need to be familiar with the operating plan and shall coordinate their  
2 efforts with the Core Systems Designer to determine the appropriate location and configuration  
3 of any such facilities at Intermediate Stations.

4 **Provisions** – Minimum area of 1,100 square feet. Access flooring for underfloor cable routing  
5 shall be provided. Layout for the SCR shall be in accordance with a system-wide standard.

C. Station Computer Room

6 **Function** – The Station Computer Room houses the servers that are needed in order to operate  
7 the ticketing and station operation systems and for communication with the Operations Control  
8 Center (OCC) and/or Regional Control Center (RCC).

9 **Location** – Adjacent to the SCR and/or Operation Maintenance Office.

10 **Provisions** – Minimum area 500 square feet. Controlled heat and humidity and a link to the  
11 Uninterruptible Power Supply (UPS).

D. Ticket Administration Office

12 **Function** – Used for non-public administrative ticketing functions.

13 **Location** – Adjacent to the Ticket Sales Office.

14 **Provisions** – Minimum area 160 square feet. Cash handling and ticket storage shall be provided  
15 in an adjacent secure room.

E. Cash Handling and Ticket Storage Room

16 **Function** – Used for processing cash received from ticket sales and for storage of blank tickets.

17 **Location** – Adjacent to the Ticket Administration Office. A secure route shall be provided from  
18 this area to a place where money and tickets can be transferred to money transport vehicles.

19 **Provisions** – Minimum area 260 square feet including a partitioned 60 square feet for ticket  
20 storage. A safe shall be provided in this room for temporary storage of tickets and cash.

F. Security Office

21 **Function** – This office provides a control center for station security.

22 **Location** – The Security Office shall be located adjacent to the Station Administration Office and  
23 the Police Office.

24 **Provisions** – Minimum area 160 square feet. The office shall have video screens to monitor the  
25 station area Closed-Circuit Television (CCTV).

G. Facility Maintenance Office

26 **Function** – Building services administration and maintenance.

27 **Provisions** – Minimum area 330 square feet.



#### H. Staff Break Room

1 **Function** – For station staff on break.

2 **Location** – Grouped with other staff functions.

3 **Provisions** – Minimum area 200 square feet or as required to provide 25 square feet per staff  
4 within a typical shift. Basic kitchenette facilities and staff lockers shall be provided for storage of  
5 personal items.

#### 14.3.6.3 Special Station Operation Offices

6 These back-of-house areas are required only at designated stations and require no public  
7 interface. It is preferable for these offices to be located at Concourse level unless otherwise  
8 indicated. The following is a non-exhaustive list of these spaces.

##### A. Terminal Control Facility

9 **Function** – Terminal Control Facilities (TCFs) are proposed to have immediate supervisory  
10 oversight over train and passenger operations within each specific terminal. TCF personnel  
11 shall control local operation of the Public Address/Customer Information Sign system, and  
12 directly manage terminal rail and facility operations on a local level. Dispatching of trains shall  
13 be controlled by main line dispatchers at the OCC or RCC who shall interface closely with TCF  
14 personnel. Train dispatching capability shall be available at the TCF if necessary in emergency  
15 circumstances. The Fire Command Center and control of the Fire Alarm System shall be located  
16 in the TCF. The Terminal Manager and Assistant Terminal Managers shall monitor operations  
17 from the TCF.

18 **Location** – TCFs shall be required at Terminal Stations and select Key Intermediate Stations  
19 acting initially as Terminal Stations.

20 **Provisions** – The TCF minimum area shall be 1,500 square feet at Terminal Stations, and may be  
21 less at Key Intermediate Stations. The room shall have access flooring for underfloor cable  
22 routing to support communications and train monitoring equipment. Basic kitchenette facilities  
23 and staff lockers shall be provided for storage of personal items for immediate occupants.

##### B. Incident Room

24 **Function** – Incident Rooms, located adjacent to but separate from the TCF, shall be equipped  
25 with communications and train-monitoring equipment to assist senior CHSTP personnel in the  
26 strategic management of significant operating incidents, without interfering with the tactical  
27 activities taking place in the TCF proper during such an incident. In situations where access to  
28 the TCF is not affected by the event, emergency responders and other support personnel may  
29 choose to join railroad managers in the Incident Room to assist in the management of an event.

30 **Location** – Incident Rooms shall be located at terminals and Key Intermediate Stations where  
31 there is a TCF, and shall be located immediately adjacent to the TCF.

**Provisions** – An Incident Room's minimum area shall be 750 square feet at Terminal Stations, and may be less at Key Intermediate Stations.

**C. Building Management Office**

**Function** – Terminals and select Key Intermediate Stations shall have a Building Management Office that will monitor or control building systems at the facility by means of a Building Management System. It is expected that included in the items to be monitored or controlled shall be the following:

- Elevators
- Escalators
- Station power and electrical systems
- Heating, ventilation, and air conditioning (HVAC)
- Emergency ventilation
- Storage of station/systems plans and inspection records
- Other building maintenance and supervisory control and data acquisition (SCADA) systems

**Location** – The Building Management Office shall be located near the Operations Maintenance Office and TCF at terminals and select Key Intermediate Stations.

**Provisions** – The Building Management Office shall be a minimum area of 1,200 square feet at Terminal Stations, and may be less at Key Intermediate Stations. At Intermediate Stations, functions shall be monitored from the SCR.

**D. Incident Command Post**

**Function** – An Incident Command Post (ICP) is defined by NFPA 130 as the location at the scene of an emergency where the Incident Commander is located and where command, coordination, control, and communications are centralized. An underground facility, such as a major rail terminal, is required to have an ICP of fixed location at street level that provides all the systems and resources necessary to support the command and control functions needed by emergency responders.

Major underground terminals, such as the Transbay Transit Center in San Francisco, shall have fixed-location ICPs associated with them.

**Location** – The Incident Command Post shall be located at street level separate from but in general proximity to station.

**Provisions** – The Incident Command Post shall be a minimum area of 1,500 square feet at select Terminal Stations.

**E. Training and Meeting Room**

**Function** – Staff meetings, staff training, and emergency command.

1 **Location** – Training and meeting rooms shall be required at designated stations only, adjacent  
2 to the Station Administration Office.

3 **Provisions** – Training and meeting rooms shall be a minimum area of 200 square feet.

F. Operation Maintenance Office

4 **Function** – Administration work and parts/equipment storage for the system operations and  
5 engineering staff.

6 **Location** – An Operation Maintenance Office shall be required at Terminal Stations and other  
7 designated stations, accessible to the loading dock.

8 **Provisions** – An Operation Maintenance Office shall be a minimum area of 1,100 square feet.

G. Staff Locker Rooms

9 **Function** – Personal storage and separate showering for male and female staff during shift.

10 **Location** – Staff locker rooms shall be required at Terminal Stations and Key Intermediate  
11 Stations with turnback service only and grouped with other staff functions.

12 **Provisions** – Staff locker rooms shall be sized as required for the estimated staff numbers  
13 during a typical shift. Individual lockable lockers and benches shall be provided for changing  
14 clothes. Shower rooms, separate from the locker area, shall be provided. Size and quantity to be  
15 determined by CBC.

H. Staff Restrooms

16 **Function** – Men's and women's staff restrooms shall be provided (in addition to public  
17 restrooms) in accordance with CBC.

18 **Location** – Staff restroom shall be required at Terminal Stations only, and shall be located in the  
19 Station Operations Office area and adjacent to the Staff Locker Room and Staff Break Room.  
20 Staff will use public restrooms at Intermediate Stations.

21 **Provisions** – The staff restroom size and fixture quantity shall be determined by CBC.

I. Transportation Agency Offices

22 **Function** – Local or regional transportation agency office functions.

23 **Location** – Transportation agency office shall be located at selected stations where  
24 transportation agencies request offices within station.

25 **Provisions** – Transportation agency office size shall be requested by local agencies.

**14.3.6.4 Maintenance Support Spaces**

26 Other station ancillary spaces for maintenance or storage may include the following.

A. Refuse Storage Room

**Function** – Space to store an appropriate volume of recycling and waste produced within the station during a maximum 3-day period between pickups. Trash compacting equipment shall be provided as appropriate for the anticipated quantity of refuse.

**Location** – Ground level where collection trucks can collect, away from public areas and outside of the station where feasible.

**Provisions** – Minimum area of 150 square feet or as required to accommodate current recycling requirements with flexibility to accommodate future changes.

Terminal Stations shall require additional area to accommodate waste generated over 3 days on trains, within station, and by commercial establishments within the station. A hose bibb and floor drain shall be provided.

B. Cleaning Supply Rooms

**Function** – Storage space for cleaning supplies.

**Location** – Adjacent to the Concourse Free Area, the Concourse Paid Area, and the platform. Additional cleaning supply rooms shall also be located near each set of toilet facilities.

**Provisions** – Minimum area 80 square feet. A janitor's sink, mop, and broom racks shall be provided.

C. Station Storage Rooms

**Function** – General storage.

**Location** – Adjacent to the Concourse Free and Paid Areas, and on the platform.

**Provisions** – Concourse minimum area 150 square feet. Platform minimum area 100 square feet.

D. Landscape Maintenance Room

**Function** – Storage of landscaping tools and supplies.

**Location** – Opening directly to station exterior.

**Provisions** – Minimum area 100 square feet.

E. Miscellaneous Rooms

Function and minimum areas in the Concourse non-public area:

**Table 14-14: Miscellaneous Rooms**

Space Name	Minimum Area Required	Special Provisions
Storage and Battery Charging	200 square feet	For electric carts
Staff Equipment Storage	60 square feet	
Consumables Storage	60 square feet	
Advertising Storage	60 square feet	
Materials Storage	60 square feet	
Small Materials Storage	60 square feet	

**F. Loading Zone**

A station loading zone and service entrance shall be sized as appropriate for each Intermediate Station to accommodate minor station-related deliveries, ticket revenue handling, trash collection, police and security-related access, and deliveries to retail concessions within the station. Loading zones and related functions shall be separated from the main station entrances and circulation patterns to prevent disruption of pedestrian and vehicular flows.

**G. Loading Dock**

At Terminal Stations, a loading dock for receiving and delivery of onboard services supplies shall be provided. Size shall be as appropriate to accommodate major station-related deliveries, ticket revenue handling, trash collection, police and security-related access, and deliveries to retail concessions. Dock height shall be as appropriate for medium-sized delivery trucks.

The loading dock and related functions shall be separated from the main station entrances and circulation patterns to prevent disruption of pedestrian and vehicular flows.

**14.3.6.5 Terminal Station Operations Facilities**

At Terminal Stations or other stations as designated in the station operations plan, space for terminal administration, support of train crews, and for rolling stock maintenance prior to turning back shall be provided.

**A. Terminal Administration Offices**

**Function** – Terminal administrative tasks are performed in this space. Some staff may have dedicated workspaces while others may share.

**Location** – Adjacent to the Station Manager’s office.

**Provisions** – Minimum area 100 square feet per assigned staff.

**B. Train Crew Support**

Spaces required for train crews at Terminal Stations and Key Intermediate Stations with turnback service, which may include shift changes and spaces for crews to wait while trains are being turned back, include the following:

**Table 14-15: Train Crew Support Spaces**

Space Name	Minimum Area Required	Special Provisions
Shift supervisor Office	400 square feet	Space for four shift supervisors
Administrative Support Office	100 square feet	Space for one support staff
Train Crew Restrooms	200 square feet	Includes male and female
Train Crew Lounge	750 square feet	Locate adjacent to restrooms.
Ready Room	300 square feet	
Train Crew Lockers/Showers	500 square feet	65% male, 35% female, locate adjacent to restrooms

1

**C. Rolling Stock Maintenance**

- 2 Spaces required for minor maintenance and inspection of the trains and platform include the  
3 following:

**Table 14-16: Rolling Stock Maintenance Spaces**

Space Name	Minimum Area Req'd.	Special Provisions
Gang Foreman Office	100 square feet	
Car Inspector Office	120 square feet	Space for two car inspectors
Cleaners' Room	120 square feet	One for each platform, each to accommodate two cleaners
Maintenance Locker Room	200 square feet	Includes showers and restrooms
Refuse Rooms	75 square feet	Three rooms on platform level
Maintenance Equipment Storage Lockers	160 square feet each	Three rooms located at platform level, distributed along the length of the platform under escalators or stairs for tool and miscellaneous storage. Water and power shall be provided.
Cleaning machine storage	660 square feet	Includes charging facility
General Storage Lockers	400 square feet total	Distribute along the length of the platform, under escalators or stairs where practicable, for restocking of trains.

4

**D. Commissary Requirements**

- 5 Spaces required at Terminal Stations to provide for commissary service on the HSTs include the  
6 following:

**Table 14-17: Commissary Spaces**

Space Name	Minimum Area Required	Special Provisions
Commissary Office	120 square feet	
Food Storeroom	800 square feet	Accessible from loading dock
Commissary Locker Room	350 square feet	Lockers, showers, restroom

Unless the Terminal Station includes a separate service platform, commissary spaces require direct access to station platforms. The preferred locations are above or below the ends of platforms and adjacent to, or in close proximity to, the station service elevators

#### **E. Service Access**

**General** – Terminal Stations require service access between platforms, the back-of-house areas of the station, and the station loading zone. Ideally, these access routes will not cross public circulation routes within the station. These corridors will be used by station staff and maintenance personnel and will not be accessible to the public.

**Service Elevators** – At Terminal Stations, as described in Section 14.3.4, dedicated service elevators shall be provided to each platform, connecting with a service corridor that passes above or below the platforms and providing direct non-public access to the station’s back-of-house facilities. It will be desirable to standardize the location of these service elevators at HST stations (i.e., at the north end or south end of the platforms), to facilitate train provisioning and servicing. This will require coordination with the trainset design and the developers of the overall operating plan.

**Service Corridors** – Certain station-related service corridors (such as the corridor linking the Cash Handling and Ticket Storage Room with the loading zone) shall be kept separate from service corridors serving the commercial zones of the station. Concrete floors shall be hardened wherever cash carts will be moved.

#### **F. Turnback Stations**

Designated Key Intermediate Stations may be operated as turnback stations. These stations shall be provided with crew waiting and changing spaces.

#### **14.3.6.6 Building Service and Plant Rooms**

Potential systems to consider include, but are not limited to environmental control, electrical, fire protection, and plumbing and drainage. Station design shall accommodate the building systems and meet the requirements of applicable codes. Additionally, facilities necessary to meet LEED Silver standards and corresponding efficiency and energy use standards shall be included, as determined by the Designer.

1 Access and hoisting provisions shall be provided for installation and future replacement of  
2 station equipment. Clear ceiling height in equipment rooms shall be 16 feet-0 inches minimum  
3 to permit equipment placement and overhead utility routing.

A. Environmental Control

4 Spaces for the environmental control system may include the following:

- 5 • Condensing Unit Room
- 6 • Air Handling Unit Room
- 7 • HVAC Room
- 8 • Tunnel Ventilation Fan Room (underground stations only)

B. Electrical System

9 **Substation** – An external Facility Power Substation is required on the station site for the  
10 purpose of providing normal, backup, and emergency standby power to the station. The  
11 substation shall be located within a 10,000 square feet fenced area that contains high voltage  
12 (HV) switchgear, HV transformers, emergency generator, and a fuel storage tank. The generator  
13 and adjacent fuel tank shall be a minimum of 25 feet-0 inches from any station structure and  
14 separated from adjacent properties. Service vehicle shall provide access.

15 **Station** – Spaces for the facility power system within the station may additionally include the  
16 following:

- 17 • UPS Room – 900 square feet. Two rooms required, one at each end of the station for low  
18 voltage (LV) distribution, transforming, and emergency power
- 19 • Battery Room – 200 square feet. Two rooms required, one room at each end of the station for  
20 LV batteries.

C. Fire Protection

21 Spaces for the fire protection system shall include the following, sized by designers as required:

- 22 • Fire Pump Room
- 23 • Fire Water Tank Room
- 24 • Fire Control Center

D. Plumbing and Drainage

25 Spaces for the plumbing and drainage systems shall include the following and shall be sized by  
26 designers as required:

- 27 • Sump Pump Room
- 28 • Ejector Room



### 14.3.6.7 Core Systems Spaces

- 1 Facilities required for Core Systems are addressed in other chapters of this Design Criteria.  
2 Refer to the *Communications* chapter for detailed design criteria for communications rooms, low  
3 voltage manholes at platform ends and other core and station connectivity interfaces. Station  
4 design shall accommodate the Core Systems and operational requirements, including the  
5 following spaces for train control and communications:

**Table 14-18: Train Control and Communications Spaces**

Space Name	Minimum Area Required	Special Provisions
Train Control and Communications (TCC) Room	1,915 square feet	For train control and communications equipment
Entrance Facility Room	240 square feet	For entry of service cabling into the building. May be co-located with the TCC Room
3rd Party Telecom Room	120 square feet	
Communications Closets	130 square feet each	Locate close to center of each 10,000 square feet of station floor area

6

### 14.3.6.8 Room Data Sheets

- 7 Refer to Appendix 14-A for detailed design data relevant to each of the preceding station  
8 spaces.

## 14.3.7 Interior Environment

### 14.3.7.1 Lighting

- 9 Lighting of passenger stations and surrounding site areas shall be consistent in quality and  
10 quantity throughout the system.

- 11 Station lighting shall promote safety by properly illuminating exit routes and potential hazards.  
12 Varying lighting and illumination levels shall be used to differentiate station functions,  
13 including circulation, entry, fare collection, waiting areas and platforms. Illumination shall  
14 provide uniform distribution and minimize glare. Direct lighting for Concourse and platform  
15 areas shall be used. Indirect lighting is not acceptable for illumination of floor areas.

- 16 Minimum average illumination under normal operation is shown in the *Facility Power and*  
17 *Lighting Systems* chapter.

### 14.3.7.2 Electrical

#### A. System Requirements

- 18 Electrical systems include power supplies (HV, LV, and emergency), normal and emergency  
19 lighting, and grounding and lightning protection. Facilities to support electrical operation of the

station may include standby generators, switchboards, uninterruptible power systems, on-site power generation such as solar power generating facilities, and electrical distribution facilities.

#### **B. Emergency Provisions**

When normal and backup power sources are interrupted, a standby emergency supply generator shall provide station power to select electrical loads connected to the 480 V ac generator bus. Standby emergency supply generator shall provide power for electrical loads such as fire protection, emergency lights, emergency signage, telecommunications systems, elevators (typically one in operation to get to the incident floor), ventilation, station control, UPS system, and LV dc battery supply systems (that provide control power to HV switchgear, etc.). The emergency standby generator shall have fuel capacity to operate for at least 30 hours. In addition to an emergency standby generator, uninterruptable power supply equipment shall be provided for the following electrical loads:

- Emergency lighting with a minimum battery capacity of 90 minutes
- Communication and train control electrical loads with a minimum battery capacity of 4 hours

#### **C. Electromagnetic Compatibility**

All electrical and electronic equipment in the station shall follow CHSTP Electromagnetic Compatibility Program Plan (EMCP) criteria for cable, grounding, equipment design, facility power, motors and controllers, equipment room locations, equipment emission and immunity limits, Fire Command Center-type accepted radio equipment, and human exposure to electric and magnetic fields.

Equipment covered by EMCP criteria includes the following:

- HST Core Systems' equipment, including for traction power, communications, and train control
- Station equipment, including fare collection, security and public safety communications, public communications including public address and telephones, operations information, passenger information, environmental control, fire detection and protection, lighting, auxiliary equipment
- Shared communication and control equipment with other rail operators

#### **14.3.7.3 Plumbing and Drainage**

**General** – Plumbing and drainage systems include domestic water supply, stormwater drainage, sewer and wastewater drainage, and fire protection water supply. Detailed plumbing and drainage requirements in stations are described in a separate chapter of this manual.

**Piping** – Where domestic water piping or drainage piping enters a room through an outside wall, the pipe shall be concealed from public view. Valves shall be concealed in valve boxes not exposed to public view.

**Drains** – Wherever floor drains are required, floors shall be sloped a minimum of 1/8 inch per foot. Drain locations shall be coordinated with architectural floor finish patterns to minimize cutting of materials.

#### 14.3.7.4 Fire Detection and Protection

**General** – Fire protection criteria are described in the *Fire Protection* chapter.

**Automatic Fire Sprinklers** – Placement of automatic fire sprinkler heads shall be coordinated with other mechanical and electrical equipment such as air supply and return, lighting fixtures, public address speakers, etc. Pipes in public area ceilings shall be concealed.

**Fire Suppression System** – An appropriate fire suppression system shall be provided as required by code for specific room functions such as train control and communications rooms.

**Fire Hose Cabinets (FHC)** – In Concourse public areas, recessed fire hose cabinets shall be provided and integrated into the standard wall finish module. FHCs on platforms may be floor mounted and conform to a system-wide design. Hinged doors, clearly labeled “FIRE HOSE CABINET” with locks keyed alike shall be provided.

**Fire Extinguishers** – Fire extinguishers, type and spacing as required by code, shall be recessed into walls in public areas or surface mounted in non-public areas.

#### 14.3.7.5 Noise Control and Acoustical Design

**Purpose** – Noise generated by the train, patrons, external sources, and building systems shall be controlled and reduced through station design. Stations shall be designed to allow for normal conversation, ensure that public announcements can be clearly understood, and allow for communication during emergency conditions.

**Mitigation** – Appropriate mitigations shall be considered inside the station and adjacent to the station. This may include sound absorption materials installed in under-platform areas, ceilings and walls of platform and Concourse areas. Acoustical treatment in stations shall meet acoustical performance criteria as well as architectural, safety and maintenance criteria, as shown in Table 14-19. Performance criteria include – low flammability, low smoke development, non-toxicity, low volatile organic compounds, low maintenance, and vandal-resistance, resistance to water, rotting, and odor.

**Noise Barriers** – Where through-trains pass through stations, noise barriers shall be required between through and stopping tracks. Perimeter noise barriers shall also be required to minimize train noise impacts on surrounding buildings and communities.

**Reverberation Time** – For public station areas, reverberation time shall be a maximum of 1.5 seconds at 500 Hertz (Hz). Maximum station noise levels are shown in Table 14-19.

**Table 14-19: Station Noise Levels**

Location	Maximum Noise Level Permitted
Platform (stopping trains entering and leaving station)	80 dBA
Platform (trains stopped at station)	70-75 dBA
Platform (train passing station)	85-90 dBA
Concourse (near platform with train passing station)	75-80 dBA
Public Station Areas (with mechanical, HVAC or other station equipment operating)	55 dBA
Public Station Areas (tunnel or station ventilation operating in emergency status)	70-80 dBA
Offices and Small Meeting Rooms	40-45 dBA
Control Rooms	35-40 dBA
Computer and Equipment Rooms	55 dBA
Building Plant Rooms (where this cannot be met, warning signs shall require staff to wear hearing protection)	85 dBA

**Notes:**

1. Noise levels measured by a slow response time constant averaged over 1 second.
2. Center of platform shall be used as the reference measurement location.
3. Noise levels due to train-generated noises are design objectives only.

**14.3.7.6 Vibration Minimization**

Ground- and structurally-borne vibration generated by the train and equipment shall be addressed through station, track, and structure design. Passenger and staff comfort shall be maintained and station elements designed to withstand vibration. Vibration from station equipment shall be mitigated to acceptable levels. The vibration impacts to surrounding areas shall be minimized.

Equipment that generates vibration shall be isolated from other station elements to reduce vibration-induced fatigue of other components and equipment. Mechanical and electrical equipment shall be set on isolation pads.

**14.3.7.7 Environmental Control Systems**

**Locations** – HVAC requirements will vary based on station type, station area weather, and other factors. LOS will also vary within the building based on specific room requirements. HVAC shall be provided in Concourse areas but not on platforms except for underground stations.

**Temperature Control** – Temperatures for enclosed public station areas, with the exception of the station operations areas and other unoccupied areas, shall be kept between 65°F and 75°F with a maximum of 65 percent relative humidity. The SCR, ancillary areas, computer rooms, and communication equipment rooms shall be kept cooler with a maximum relative humidity of 60 percent. Main substations, standby generators, the central air conditioning plant, and Air Handling Unit rooms shall be kept below 100°F.

**Ventilation** – Ventilation shall be provided in the Free, Paid and Operations Areas. Refer to the *Mechanical* chapter of this manual.

**Redundancy** – Redundancy is not required for environmental control systems for public areas. Back-up mechanical HVAC systems shall be provided for areas and equipment rooms that are essential to HST operations, including the SCR, Building Maintenance Office, System Equipment rooms, and other supporting facilities.

**Controls** – Temperature and humidity shall be controlled from the SCR.

**Space Requirements** – HVAC space requirements may include chiller room, air handling unit room, control room, and ventilation room among other spaces. Refer to Section 14.3.6 and Room Data Sheets for additional information.

#### 14.3.7.8 Flood Protection

**Site** – Stations sites shall be analyzed for proximity to a flood plain or other potential sources of water infiltration. Openings into stations shall be protected to a minimum of 4 feet-0 inches above the 100-year design flood level.

**Entrances** – Station entrances shall be protected, as appropriate, by steps, a sloped plaza apron, or ramping up to a landing to include minimum flood protection of 2 feet-0 inches above surrounding grade level as anticipated for the maximum design-year flood level.

**Waterproofing** – Portions of station facilities identified below the water table shall be appropriately waterproofed to prevent infiltration of ground water for the design life of the facility.

#### 14.3.8 System-wide Furniture, Fixtures, and Equipment

In addition to equipment described elsewhere in this design criteria, the platforms and Concourse, both Free and Paid Area, shall be provided with the following system-wide furniture, fixtures, and equipment. System-wide furniture, fixtures, and equipment do not add square footage to the station footprint. These elements shall occupy portions of the spaces described in Sections 14.3.2 and 14.3.2.3.

Where required, access for control/data and power conduits shall be provided.

##### 14.3.8.1 Furniture

###### A. Seating and Benches

**Location** – Concourse Free Area, Concourse Paid Area, platforms.

Within Concourse waiting areas, seating and benches shall be organized into one or more seating groups without obstructing passenger circulation.

On platforms, seating shall be oriented parallel to the track within the central zone between vertical circulation elements or columns and protected from the weather. Seating shall be

- 1 located adjacent to each car. Platform signage to identify where each numbered car will stop  
2 shall be provided.
- 3 **Quantity** – The minimum number of seats to be provided is listed in Table 14-20.

**Table 14-20: Station Seating**

Symbol	Description	Formula <sup>1</sup>
S <sub>f</sub>	Seating at Concourse Free Waiting Area	$[(P_{15B} \times 1.1) + (P_{15A} \times 0.1)] \times 0.25$
S <sub>p</sub>	Seating at Concourse Paid Waiting Area	$P_{15B} \times 0.25$
S	Seating on Platforms	8 seats per car (except where seating might obstruct passenger circulation)

- 4 <sup>1</sup> P<sub>15B</sub> = Peak 15-minute boardings; P<sub>15A</sub> = Peak 15-minute alightings  
5

- 6 **Description** – Seating shall be designed with armrests to discourage lying down. Materials shall  
7 be corrosion resistant where exposed to exterior elements. Interior seating shall be cushioned  
8 and upholstered with a washable, heavy-duty vinyl. Seating shall be secured to the floor with  
9 vandal-resistant fasteners.

**B. Waste and Recycling Receptacles**

- 10 **Location** – Concourse Free Area, Concourse Paid Area, exterior station approaches.
- 11 Receptacles shall be located adjacent to entrances, ticketing facilities and waiting areas.  
12 Locations shall not obstruct passenger circulation. Receptacles shall be protected from direct  
13 weather exposure.
- 14 **Quantity** – As appropriate for each particular station layout.
- 15 **Description** – Triple units for waste, cans and bottles, and paper recycling shall be provided.  
16 Materials shall be non-corrosive.

**C. Passenger Information Counter**

- 17 **Location** – Terminal Stations in the Concourse Free Area, close to the main entrance.
- 18 **Quantity** – One per Terminal Station.
- 19 **Description** – The Passenger Information Counter shall be a system-wide standard design. Each  
20 counter shall be approximately 100 square feet. A freestanding counter shall be staffed with at  
21 least 1 information agent to provide passengers and station visitors with general information  
22 regarding train schedules and use of the high-speed rail system. These counters can also  
23 provide information to departing passengers about the local city and modes of connecting  
24 transportation available outside the station.

At Intermediate Stations, these passenger services shall be provided at the PSB as described elsewhere in this section.

#### **14.3.8.2 Fixtures**

##### **A. Station Assistance Telephone**

**Location** – Concourse Free Area, Paid Area, and platforms

Station Assistance Telephones, sometimes called “White Courtesy Phones,” offer passengers another means to obtain information or assistance.

**Quantity** – One Station Assistance Telephone shall be placed in each Paid and Free Waiting Area and located at a maximum 300-foot walking distance along the length of each platform.

**Description** – Refer to Section 14.3.9 - Station Information for additional information.

##### **B. Drinking Fountains**

**Location** – Concourse Free Area, Concourse Paid Area.

**Quantity** – One drinking fountain shall be located adjacent to each waiting area, and removed from the main circulation paths.

**Description** – Dual height stainless steel, wall-mounted drinking fountains, in accordance with applicable codes.

##### **C. Public Telephones**

**Location** – Concourse Free Area.

Public telephones shall be located in groups of at least 2 phones close to station entrances, adjacent to the Concourse Free Area but outside of primary circulation spaces.

**Quantity** –  $P_{15B} \times 0.01$

**Description** – Coin-operated, wall-mounted or free-standing phones shall be provided by the local telephone utility service. Telephones shall comply with ADA accessibility requirements. When wall-recessed, public telephones shall be integrated into the station finish module as described in Section 14.3.10.2.

##### **D. Closed-Circuit Television**

**Location** – Platforms, other locations as indicated in Room Data Sheets.

**Quantity** – As required on platforms to monitor the public area of each platform, with particular emphasis on platform edges, stair and escalator landings, elevator entrances and platform ends.

**Description** – Ceiling mounted. Refer to the *Communications* chapter.

E. Clock

- 1 **Locations** – Concourse Paid Area, adjacent to the primary circulation route to platforms.
- 2 **Quantity** – One per station.
- 3 **Description** – Analog clock with oversized face, minimum 36-inch diameter. Wall mounted,
- 4 ceiling hung or freestanding floor mounted, minimum 10 feet above finish floor.

F. Map and Information Panel

- 5 **Location** – Concourse Free Area.
- 6 **Quantity** – One vicinity map/information panel close to each station entrance/exit, either
- 7 integrated into the wall finish module or a freestanding kiosk.
- 8 **Description** – Information shall also include racks for printed train timetables and other
- 9 pertinent passenger information. Information panels and kiosks shall be a system-wide design
- 10 and provide information about HST service, including train schedules, fares and policies.

**14.3.8.3 Equipment**

A. Passenger Service Center

- 11 **Location** – Between Concourse Free Area and Paid Area within the Fare Collection Line.
- 12 **Quantity** – One at each Fare Gate array between Free and Paid Areas.
- 13 **Description** – The Passenger Service Center provides information and assistance to station
- 14 passengers as they enter or leave the Paid Area. The Passenger Service Center shall be a system-
- 15 wide standard freestanding unit, approximately 100 square feet in area. Passenger inquiry
- 16 windows shall be provided on the Concourse Free and Paid Areas of the Passenger Service
- 17 Center. Queuing space as shown in Table 14-7 shall be provided. Agent access to the Passenger
- 18 Service Center shall be via a main door on the paid side of the booth. Finish materials shall be
- 19 stainless steel up to approximately 4 feet-0 inches height and fully glazed above.
- 20 **Equipment** – The Passenger Service Center shall be fully enclosed, lighted, and ventilated.
- 21 Counter work space for agent's computer terminal shall be provided. The public address and
- 22 in-station dynamic signage systems shall be controlled from this booth as well as the station
- 23 control room. A communications link between the Passenger Service Center, the SCR or TCF,
- 24 and the OCC shall be provided to facilitate timely and accurate dissemination of train
- 25 information to the public.

B. Platform Agent Booth

- 26 **Location** – Platform Agent Booths shall be considered at Terminal Station platforms only, and
- 27 placed at the Center Platform to allow clear visual surveillance of the entire platform length.



1 **Quantity** – One per Terminal Station platform as appropriate. Where Terminal Stations are  
2 provided with multiple platforms and Operations requires a platform agent, each platform shall  
3 be provided with one Platform Agent Booth.

4 **Description** – This booth is used to monitor train Terminal Station arrivals and departures and  
5 platform passenger circulation. At Intermediate Stations, the monitoring of passengers entering  
6 and exiting trains will be done by the train crew. The Platform Agent Booth shall be a system-  
7 wide standard freestanding unit, approximately 100 square feet in area. Each Platform Agent  
8 Booth shall be staffed with an agent and therefore shall be provided with a work counter,  
9 computer terminal, and communications equipment.

10 Finish materials shall be stainless steel up to passenger window height and fully glazed above.  
11 The Platform Agent Booth shall be fully enclosed, lighted and air conditioned.

12 **Equipment** – Public address capabilities and control of the in-station dynamic signage shall be  
13 integrated into this booth. This facility requires a communications link with the TCF and the  
14 Central Control Facility to facilitate timely and accurate dissemination of train information to  
15 the public.

#### C. Automated Teller Machines (ATM)

16 **Location** – Concourse Free Area

17 **Quantity** – Space for one ATM shall be provided adjacent to each ticket sales office.

18 **Description** – As provided for the convenience of passengers buying tickets. ATMs shall be  
19 freestanding and separated from passenger circulation flows or integrated into the station wall  
20 finish module.

### 14.3.9 Station Information

#### 14.3.9.1 General Principles

21 The key to passenger wayfinding is logical and consistent station planning, conforming to the  
22 passenger circulation principles. Information systems shall be provided as additional support to  
23 assist passengers in finding their way to, within, and from HST stations and related facilities.  
24 Information systems shall reflect the system's commitment to Universal Design and passenger  
25 friendliness. The key features of station information are described in this section.

#### 14.3.9.2 Signage and Graphics

26 **Purpose** – Signage and graphics shall be provided throughout stations as the primary visual  
27 component for passenger wayfinding. Properly designed signage and graphics shall enable  
28 passengers to find their way from origin to destination without asking station staff for  
29 directions.

30 **Consistency** – Signage and graphics are keys to system branding and in defining corporate  
31 identity. Signage and graphics shall identify system elements at stations, sites, and approaches.

Content, size, style, symbols, color, composition, and font shall be consistent. Standard pictograms shall convey information for passengers unable to read the corresponding text.

**Placement** – Signage shall be logically placed to optimize visibility.

**Content** – Other transportation systems and modes shall be integrated into the signage plan. Signage and graphics shall meet the requirements of ADA, CPUC, NFPA 130, and CFR Title 29, Part 1910.

**Lighting** – Where necessary for clarity and visibility whether day or night, signage shall be illuminated either internally or externally, as appropriate.

**Sign Types** – Station signage shall include the following:

- Directional signage – the key to wayfinding. Directional signage shall be placed at every potential decision point for passengers either entering or exiting the station.
- Informational signage
- Regulatory signage – exiting and other code-related signage
- Instructional signage – explains how passengers can use the HST System
- Warning and safety signage
- Maps of the HST System and other local intermodal transit systems, the neighborhood and vicinity
- Train Schedules
- Arrival and departure information boards – provided along major circulation routes, in the ticket sales area, at the ticket barrier, in the Free and Paid Areas and on the platforms. Train information monitors shall be provided in waiting areas and on the platforms.
- Room name signage – Room, area, and facility names shall be provided for these facilities including door signs to rooms.
- Station Name Identification – System and station identification signage shall be provided on the building exterior, at entrances, at the passenger information center, at ticket sales areas and on platforms.

**Signage and Graphics Manual** – Refer to the CHSTP Signage and Graphics Manual for additional detailed information.

#### **14.3.9.3 Public Address (PA) System**

A public address system shall be provided as an auditory means for station staff to announce train arrivals, delays, security concerns and other significant passenger information. Speakers shall be installed throughout the station, including exterior approaches. Speakers shall be mounted high enough to not be readily visible but low enough to ensure voice clarity. Announcements may be made by station staff from the SCR, TCF, Passenger Service Center or

1 Platform Agent Booth (if provided). Refer to the *Communications* chapter of this design manual  
2 for additional information.

#### 14.3.9.4 Information Monitors

3 Video monitors shall be located throughout the station public areas to provide visual  
4 information, announcements and messages regarding train schedules, delays, etc., in  
5 conjunction with the public address system information. Visual information may also include  
6 weather reports, commercial advertising or other. Monitors shall be wall or ceiling mounted,  
7 and located at Concourse Free and Paid Areas and platforms. Refer to the *Communications*  
8 chapter for additional information

#### 14.3.9.5 Customer Service Agents

9 Personnel staffing the Customer Service Center, the Platform Agent Booth (if provided), or  
10 other roving staff shall offer face-to-face information to passengers when signage, public  
11 address systems, and monitors are not sufficient to address a passenger's questions or concern.

#### 14.3.9.6 Advertising

12 Advertising provides system revenue as well as visual interest to passengers awaiting trains. A  
13 system-wide policy shall be developed to locate advertising in a consistent fashion where it  
14 shall not conflict with other station information. Sizes shall be standardized throughout the  
15 system.

#### 14.3.10 Station Finish Materials

##### 14.3.10.1 General Considerations

16 Station finish materials will significantly influence the public perception of the HST System and  
17 its quality. Because passengers, staff, and visitors will have direct contact with station finishes,  
18 proper selection is essential. Although finishes may vary between stations, the finishes shall  
19 conform to consistent standards of quality and aesthetics.

20 The following factors shall be considered when selecting station finish materials:

- 21 • **Safety** – Safety is the highest priority of the CHSTP. Interior station finish materials shall  
22 meet the fire resistive criteria of NFPA 101. Station construction materials shall also conform  
23 to the requirements of the 2010 CBC. Particular attention shall be given to the seismic design  
24 criteria for the attachment of finish materials. Finishes shall be selected to minimize  
25 potential hazard due to combustion or smoke generation.
- 26 • **Durability** – Finish materials shall be highly resistant to wear and accidental or malicious  
27 damage. The difficulties associated with replacing materials in a public transportation  
28 facility dictate the use of high quality and durable finishes that are easily maintained and  
29 that maintain their inherent qualities over time. Materials shall maintain quality for at least  
30 30 years before replacement is necessary. Colors shall not change or fade.

- 1 • **Maintenance** – In addition to initial cost, life-cycle maintenance costs shall be considered in  
2 selection of materials and finishes. Finish materials shall be selected for ease of cleaning and  
3 inherent resistance to staining or soiling. Maintenance shall be carried out without the use of  
4 special equipment. Finish materials shall be selected and detailed with ease of repair or  
5 replacement.
- 6 • **Resistance to Vandalism** – Materials within reach of the public shall be graffiti resistant and  
7 cleanable without special techniques.
- 8 • **Appearance** – Timeless quality shall be conveyed by public area finish materials. Color and  
9 texture shall be harmonized throughout the building exterior and interior.
- 10 • **Color** – Floor material colors shall be integral. Surface-applied color treatments are not  
11 permitted. Color shall consider minimum contrast requirements for tactile paths and  
12 platform-edge materials. Colors shall be predominantly light to medium tones in order to  
13 maintain high and naturally efficient reflectance and illumination levels while concealing  
14 soil.
- 15 • **Texture** – Floor finish materials shall have minimum surface irregularities for ease of  
16 cleaning and to minimize collection of dust and dirt. Where slip resistance is essential,  
17 medium-rough texture may be acceptable.
- 18 • **Corrosion Resistance** – Metal finish materials in public areas shall be stainless steel or  
19 aluminum in non-public areas miscellaneous metals shall be galvanized and painted.
- 20 • **Review** – A materials color board shall be submitted to the Authority for review prior to  
21 specification.

#### 14.3.10.2 Station Design Module

22 A station planning design module of 4 feet-0 inches shall be applied to station public areas for  
23 the purposes of economizing and standardizing material dimensions, integration, and  
24 replacement of system-wide elements, and minimization of custom cutting. Structural elements  
25 shall be spaced on consistent multiples of this module wherever possible. Finish materials may  
26 be subdivided into smaller modular units such as 2 feet-0 inches or 12 inches. This basic detail  
27 module shall apply to vertical and horizontal surfaces.

28 To the fullest extent practicable, joints and pattern lines in floor, walls, and ceilings shall be  
29 coordinated with building movement joints. Special attention is required for the detailing of  
30 multiple movement joints, which the long station structures will require. Finish material  
31 selections shall allow detailing of movement joints with minimal or no custom fabrications of  
32 joint covers, plates, etc.

#### 14.3.10.3 Floors

33 **Durability** – Floors shall be subject to the highest degree of wear due to foot traffic, luggage,  
34 carts, etc. Durability is therefore a critical factor in material selection.

- Slip Resistance** – Floor materials shall be highly slip-resistant in public areas, including exterior entrance approaches, Concourses, stairs and landings, platforms, etc.
- Tactile Routes** – Inset tactile paths and platform edges for visually impaired passengers shall be considered in laying out floor-finish patterns.
- Joints** – Size of floor finishes shall consider minimizing the number and size of joints between units.
- Construction** – Most platform- and Concourse-area floor finishes shall be installed in setting beds over lightweight concrete fill slabs. The fill slabs shall provide for the installation of conduits and other mechanical, electrical and communications systems. Civil-structural dimensions shall allow adequate depth and width for embedded conduits and systems.
- Access Panels** – Floor panels for under-floor system access shall be coordinated with the finish floor patterns and modular dimensions.
- Maintenance** – In addition to previous maintenance requirements, floor finishes shall not be affected by the use of chemical cleaning materials.

**Table 14-21: Acceptable Floor Finishes**

Location	Finishes
Platforms	Smooth concrete (sealed); stone, tile, concrete
Concourse and Mezzanines	Smooth concrete (sealed); stone, tile, terrazzo, marble, recycled composite tile
Pedestrian walkways, bridges, and tunnels	Smooth concrete, stone, tile
Public service areas	Smooth concrete, stone, tile, sheet linoleum and related products, composition tile
Commercial spaces	Provided by tenants, in accordance with HST criteria and building code
Toilet Rooms	Smooth concrete (sealed), stone, tile, pavers, terrazzo, recycled composite tile
Non-public station staff areas	Smooth concrete (sealed or stained), tile
Equipment rooms	Smooth concrete (dust free) with hardener as appropriate, sheet linoleum and related products, composition tile
Storage rooms	Smooth concrete

#### 14.3.10.4 Walls and Columns

- Public Access** – Wall surfaces within reach of the public, from the floor up to 8 feet-0 inches height, shall be hard, dense, non-porous, non-staining, and acid and alkali resistant for long life and low maintenance.

**Transparency** – Transparent walls and railings are desirable to bring daylight into stations, enable views, and facilitate passenger orientation within the station.

**Elevators** – In accordance with the elevator section of this chapter, interior elevators shall be predominantly glazed in accordance with building and elevator codes.

**Installation** – Where prefabricated finish panels are used, consideration of storage and installation of matching materials is essential. Simple access to concealed attachments shall be considered in the design of prefabricated panel finishes.

Acceptable interior wall and column finishes:

- Natural stone
- Precast concrete, smooth finish
- Exposed structural concrete (with stain-inhibiting finishes)
- Smooth cement plaster
- Ceramic tile, porcelain tile, recycled composite tile
- Laminated and tempered glass, transparent or opaque; glass-like materials
- Stainless steel
- Prefinished metal panels
- Gypsum board at non-public, low maintenance spaces

#### 14.3.10.5 Ceilings

**Integration** – The preferred design approach integrates structural and architectural elements and minimizes the need for suspended ceiling solutions.

**Coordination** – Station ceiling design shall coordinate the station architecture with station support services, including mechanical, electrical, lighting, communications, station information, etc. Suspension of these elements shall be coordinated.

**Seismic** – Ceiling suspension systems shall meet or exceed seismic design criteria.

**Module** – Where suspended ceilings are used, the system shall be coordinated with the major station modules.

**Access** – Hinged or other fastening systems shall be included to allow access to concealed systems with a minimum of tools and equipment; rapid removal and replacement, cleaning, opening and closing shall be allowed.

**Table 14-22: Acceptable Ceiling and Overhead Architectural Treatment**

Platforms	Exposed Canopy Structure
Concourse and Mezzanines	Exposed structure with integrated daylighting; suspended metal panels, insulated perforated metal panels, cementitious sprayed acoustic materials
Pedestrian walkways, bridges, and tunnels	Exposed structure, with integrated daylighting where possible
Public station services areas	Exposed where appropriate or suspended modular systems with integrated lighting and HVAC systems
Commercial spaces	Provided by tenants, in accordance with HST criteria and building codes
Toilet Rooms	Suspended, impact and moisture resistant gypsum board with epoxy paint finish
Non-public station staff areas	Suspended gypsum board, painted, suspended acoustic tile systems
Equipment rooms	Exposed structure where appropriate; dust proof finish
Storage rooms	Exposed structure

#### 14.3.10.6 Railings

Guardrails and barriers in station public areas may include clear laminated tempered glass with stainless steel supports and top rails, or stainless steel picket type. Refer to Standard and Directive Drawings. Railings in non-public areas may be galvanized and painted steel.

#### 14.3.10.7 Station Exterior

**General** – This section provides guidance for the selection of exterior materials for station buildings, including platform areas and pedestrian bridges, walkways and tunnels. General considerations described previously apply to exterior as well as interior finish materials.

**Roofs** – Stations are likely to be prominent structures in most locations. Individual station designs may have roof surfaces that are visible from ground level or may use a single material that serves as both roof and wall. In these cases, metal roofing systems may be appropriate. In urban areas, roofs may be visible from existing or future high-rise development. For highly visible roofs, a singular material, such as metal or a membrane system, is preferred.

In accordance with the Authority's policy for sustainable design, roof materials with high petroleum content or proprietary systems are the least preferred types.

The long, linear platform canopy structures shall be assemblies of prefinished materials requiring little or no maintenance over the course of their design life.

All station roofs shall be provided with roof drains. Gutters, rain leaders, and grade-level connections into the station roof and walls shall be integrated into roofs and walls.

**Exterior Walls** – Acceptable exterior wall materials include concrete and precast concrete panels, natural stone, porcelain enamel steel, stainless steel prefinished steel, and aluminum panel systems.

Glass is a desirable material for station exterior walls to provide daylight and promote a sense of openness and security. Energy considerations require that appropriate shading be included with east-, south-, and west-facing glass walls. Convenient glass-cleaning strategies shall also be considered.

Where the track level is below grade, exposed concrete and masonry—including stone, tile, and composite concrete materials—are appropriate.

Where platforms are elevated, large areas of platform enclosures may be architectural fence materials or metal and glass weather screens. Common wall areas shall be transparent to allow views, daylight, and security between station interiors and platforms.

### **14.3.11 Security Provisions**

#### **14.3.11.1 Objectives**

The primary goal of station security is to protect the station passengers and staff, the facilities, and the train systems.

To achieve this goal, in addition to operational provisions, prudent passive station planning techniques shall be employed, including open, unobstructed lines of sight, avoidance of potential hiding places, and use of CCTV where direct surveillance is not possible.

#### **14.3.11.2 Security Facilities**

Primary station security facilities include a Security Office and a Police Office. Secondary station security provisions include the SCR, TCF, the Station Manager's Office and the Platform Agent Booth at designated Terminal Stations. Refer to Section 14.3.6 for facility requirements.

Space shall be allowed between the free and Paid Areas where active security measures may be installed. Refer to Section 14.3.3.5 for space requirements.

#### **14.3.11.3 Security Devices and Alarms**

Non-public and building system spaces shall be secured with a system-wide electronic locking system and intrusion alarms. Station entrance doors and gates shall be equipped with audible alarms connected to the Security Office and the SCR. Design criteria for doors, gates, and locking mechanisms required for the prevention of unauthorized access to emergency exits, rooms with fixed equipment, corridors, stairwells and other controlled areas may be found in the Operations and Maintenance Plans. Security devices shall be provided at each station area as shown in Table 14-23.



#### 14.3.11.4 Station Closure

- 1 The station shall be able to be fully closed and secured during the daily schedule and in case of
- 2 emergency. Security gates shall be used to close the station during non-revenue hours. Security
- 3 gates can be key-controlled from both the inside and the outside of the station. Each station
- 4 entrance shall have at least one main door for use by staff to exit when the station is closed and
- 5 the security gate is closed.

**Table 14-23: Security Devices in Stations**

Station Area	Security Devices and Provisions				
	CCTV	Sensing Device	Emergency Button	Intercom	Increased Lighting
<b>Area open to the public during system operating hours</b>					
Station Entrance	X	X			X
Free Area	X			X	
Ticket Sales Area	X			X	X
Ticket Gates	X			X	X
Paid Area	X			X	
Public Stairs	X				X
Escalators	X				X
Elevators	X			X	X
Platform	X		X	X	X (at edge)
Public Restrooms	X (at entrance)		X		X (at entrance)
<b>Area open to staff at all times and to the public during emergency or special circumstances</b>					
Station Operation Office Entrances	X	X			
Emergency Stairs	X	X		X	
<b>Controlled Access for staff only</b>					
Cash Handling/Ticket Storage	X	X			
SCR	X	X			
Station Computer Room		X			
Building Service/Core System Room Entrances		X			
End of Platform		X			

## 14.4 Station Site Design

### 14.4.1 Site Design Objectives

Station sites shall incorporate Authority's facilities as described in this chapter while achieving the following fundamental site design objectives:

- Optimize multimodal access between the station and other potential transportation modes.
- Facilitate simple, direct, and safe movement of people and vehicles to, from, and within the site. Site design shall contribute to passenger convenience by:
  - Facilitating clear, safe and direct intermodal connectivity;
  - Clearly identifying public station facilities from public rights-of-way; and
  - Minimizing conflicts between modes.
- Promote connectivity to existing and planned pedestrian, bicycle, transit and street networks.
- Orient station facilities to the surrounding community in a pedestrian-friendly, accessible and sustainable manner.
- Coordinate site facilities and site design with local jurisdictions and regional transit providers to ensure a site layout that supports the CHSTP's objectives, as well as local government and regional transit provider objectives and requirements.
- Ensure emergency access to stations, including statutory fire lanes and vehicle access.
- Ensure system-wide consistency in site design.

### 14.4.2 Access Hierarchy

#### 14.4.2.1 Planning Principles

A hierarchy of access modes to and from HST stations will help to ensure safe and efficient service to HST patrons and to resolve potential conflicts between modes. Station sites shall include the following:

- Promote efficiency of person trip access to HST stations
- Ensure multimodal balance of station area access
- Ensure safe multimodal access

The following modes of access are listed from highest to lowest priority.

- Rail-to-rail transfers (HST, Amtrak and/or commuter)
- Pedestrians (walk trips to and from the station)
- Transit, including bus, van, carpool, taxi etc.

- 1 • Bicycles
- 2 • Drop-Off and Pick Up
- 3 • Automobile Parking

#### **14.4.2.2 Rail**

4 Where HST station sites connect with other rail services, passenger transfer between HSTs and  
5 other rail services shall be as seamless and convenient as possible. Walking distances and the  
6 number of required vertical level changes shall be minimized.

7 Integration of HST and other rail services at a single station facility is preferable for passenger  
8 convenience and orientation. Where rail stations are separated by necessity, they shall be  
9 located as close as possible to each other. Visual connections to connecting system platforms  
10 shall be provided where possible. Connecting pedestrian walkways shall be designed to be  
11 obvious and intuitive.

#### **14.4.2.3 Pedestrians**

##### **A. Pedestrian Principles**

12 Pedestrian access shall be afforded the highest priority of access. Guidelines for pedestrian  
13 movement are as follows:

- 14 • Pedestrian routes shall be simple, comfortable, direct, well lit, visually unobstructed and  
15 along or visible from public streets. Pedestrians will seek the shortest route and the site plan  
16 shall anticipate this behavior.
- 17 • Pedestrian routes shall be contiguous and separated from motorized vehicles wherever  
18 practicable. Pedestrian conflicts with other travel modes are to be avoided.
- 19 • Pedestrian routes shall connect the station entrances, transit services, passenger drop-off,  
20 parking, adjacent rights-of-way and other station-area facilities.
- 21 • Pedestrian routes shall connect the station area and station entrance to key intersections and  
22 pedestrian routes adjacent to the site in order to provide ease of pedestrian access to and  
23 from off-site destinations.
- 24 • Pedestrian access and circulation shall address actual and perceived potential security  
25 concerns.
- 26 • Pedestrian routes shall avoid unnecessary turns or dead-ends, routes through parking lots,  
27 and isolated or hidden segments.
- 28 • All elements shall conform to accessibility guidelines. Accessible routes shall connect all  
29 public site facilities and destinations. Site furniture and stairs shall be located outside of this  
30 route.
- 31 • Pedestrian flow shall be designed for right-hand flows where practical.

- Existing and planned pedestrian and bicycle routes shall be considered within a minimum 1/2 mile of the station when designing routes on the station site.

#### **B. Pedestrian Walkways and Sidewalks**

Pedestrian walkways and sidewalks shall provide for the majority of pedestrian circulation. Guidelines are as follows:

- Steps or abrupt changes in level shall be minimized.
- Layout of pedestrian walkways shall provide maximum visibility of and by oncoming vehicular traffic. Routing pedestrian walkways adjacent to columns or walls that reduce pedestrian visibility to vehicle operators shall be avoided.
- Where pedestrian-vehicle interfaces are unavoidable, crosswalks and pathways shall be marked and clearly visible to motorists. Finish, contrasting color, or other elements of design to differentiate pedestrian paths and crossings and increase patron safety and security shall be used.
- Sidewalks are required next to stations or other structures when vehicle circulation or parking is adjacent to the building.
- Provision of canopies for weather protection is encouraged along pedestrian transfer walkways or other exterior high pedestrian traffic areas.

#### **C. Pedestrian Bridges and Underpasses**

Where street-level pedestrian walkways cannot be provided, elevated pedestrian bridges may be necessary. Bridges shall be provided with protective screens and continuous kick plates to prevent objects from being dropped or kicked from the bridge.

Where underpasses or tunnels are required, design shall prioritize patron security including shortest practical length, ample width, bright lighting, and security monitoring.

Where grade changes are greater than allowable ramp slopes, provide stairs, escalators, or elevators in accordance with Section 14.3.4.

#### **14.4.2.4 Intermodal Transfer**

At the station site, facilities are required to allow access and boarding and alighting of other modes of transit. Non-HST passenger volumes shall be estimated and used to determine the appropriate sizes and configuration of pedestrian circulation facilities within the station site.

Maximum walking distance to the station entrance from a transit boarding or alighting area shall be 500 feet. Where possible, transit facilities shall be visible from the station, arterial streets and nearby activity areas. At-grade pedestrian crossings of vehicle lanes shall be minimized.

Facilities for transit vehicles—including access, circulation, boarding, and alighting areas—shall be separated from other traffic where practicable.

1 Waiting areas at transit loading areas—sized according to peak period demand at a pedestrian  
2 LOS B or better—shall be provided.

3 Loading Zones for Buses and Taxis shall be as follows:

- 4 • The required bus (or taxi) design capacity for a station shall be determined based on the  
5 individual requirements for each station.
- 6 • Loading zones for buses and taxis shall be located to provide the most direct and safest  
7 intermodal transfer.

A. Bus

8 **Planning** – Bus facilities shall provide for safe and accessible bus boarding and alighting while  
9 providing an efficient operating space for bus routes. Bus facilities are assumed to remain active  
10 and busy throughout the day, and vacant, underutilized space during off-peak periods shall be  
11 avoided. Where feasible, the site shall be designed to accommodate future bus service growth.

12 **Bus Waiting** – Weather protection or shelter the length of the bus platform shall be provided  
13 with a canopy that extends to cover bus doorways. Boarding zones shall be provided with  
14 lighting, seating, and service information. Light poles, bollards, fire hydrants, and other site  
15 furniture shall be located at least 4 feet from the curb edge.

16 **Station Site Access** – Bus facilities shall be located on the site with easy access from major bus  
17 routes to reduce out-of-direction travel. Where possible, buses shall be segregated from other  
18 traffic entering the facility. When exclusive right-of-way is not available, controlled access may  
19 facilitate free movement of transit vehicles in and out of the station. Controlled access ranges  
20 from a protected left-turn lane out of the station to exclusive bus right-of-way entry and  
21 circulation within the station site.

22 **Bus Facility Geometry** – Bus bays may be designed as sawtooth bus bays or tangent bus bays.  
23 Minimum dimensions for each are dictated by local codes. Sawtooth bays are generally  
24 preferable because they require less curb space than tangent bays. In no situation shall bus bay  
25 geometry require backing (as in angled or diagonal bays). Bus lanes shall have a minimum  
26 width of 12 feet (or as dictated by local codes) and shall be configured to allow buses to pass  
27 each other. Bus facilities shall be planned for one-way operations with right-hand drop-off/pick-  
28 up directly onto bus waiting areas and corresponding pedestrian paths.

29 **Standard Layout for Various Types of Bus Loading Zones** – See bus bay dimensions on  
30 Figures 14-1 and 14-2. For additional design criteria refer to bus operator agency standards.

- 31 • **Recessed Bus Bays** – Where the volume of passenger cars or buses on roads used jointly by  
32 cars and buses warrants, the bus loading zone shall be recessed from the through traffic  
33 lane.

- Recessed bus bays shall be designed parallel to and close enough to the curb so that passengers may enter and leave any door by an easy step to the curb. Upon leaving, the merging lane will enable the bus an easy re-entry into the through traffic lane.

- The loading zone shall have a 10-foot-wide lane, and the total length for a two-bus loading area shall be 120 feet long with a 40-foot tapered section at each end. For each additional bus required, an additional 80 feet of length shall be added at curbside.

• **Parallel-to-Curb Bus Bays** – Parallel to curb base shall have 10-foot wide lanes and a length of 80 feet.

• **Sawtooth Bus Bays** – Sawtooth bus bays will reduce the length of loading zone, but will increase the width of roadway. The critical movement in this layout is the operation of moving a bus out and around a parked bus at the loading zone.

- The minimum roadway width is determined as follows:

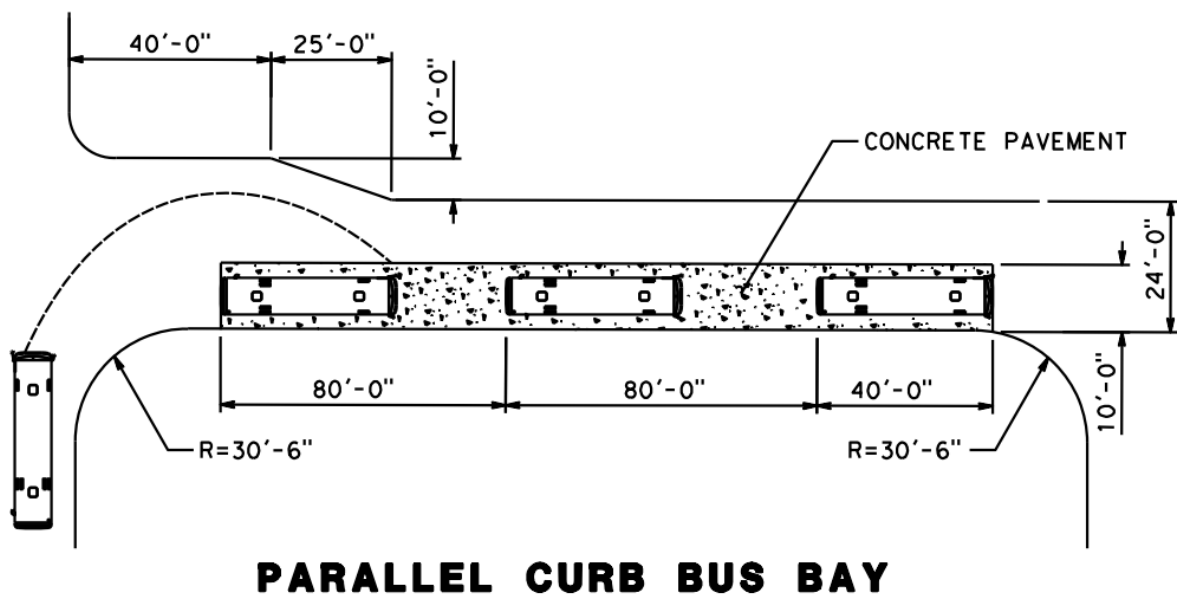
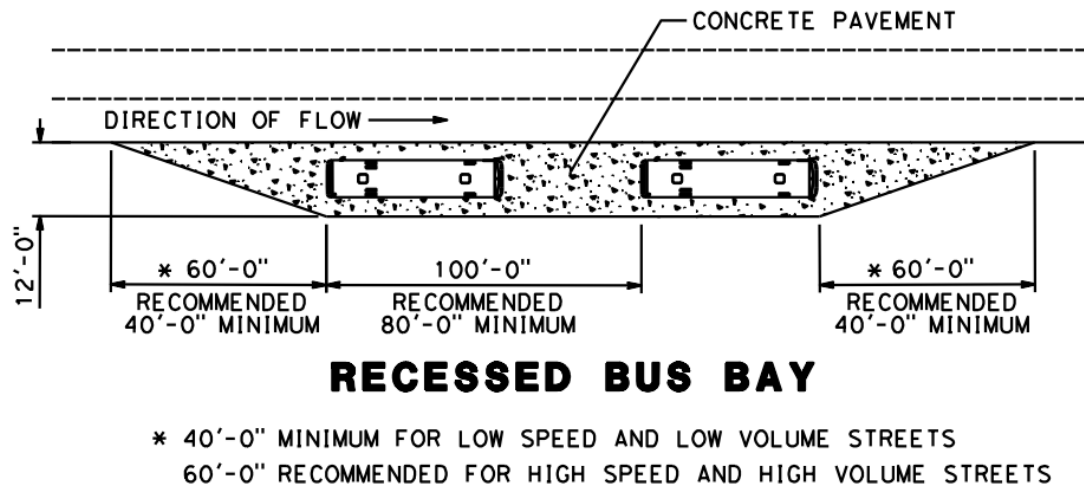
23 feet–8 inches	Clearance path of bus	} = 29 feet–0 inches	Nominal roadway width
2 feet–0 inches	Additional clearance		
3 feet–4 inches	High point		

- The nominal roadway width is the average of the high and low points of the sawtooth and allows a direct comparison with the parallel-to-curb bus bays.

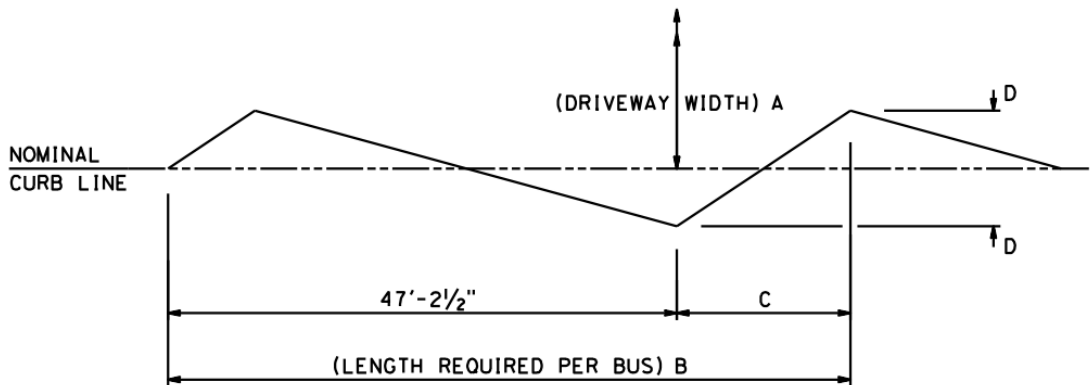
**Layovers** – For routes that terminate at the station, the operator will need to “layover” either on station property or on the street adjacent to the station. If layovers occur at the station, layover parking shall be located to reduce recirculation needed to re-enter service and corresponding congestion. At least 1 bus bay or other bus parking space shall be provided for each route that will layover. Multiple bays are required if the scheduled headway is less than or equal to the layover duration.

## Chapter 14 – Stations

## 1 Figure 14-1: Bus Bays

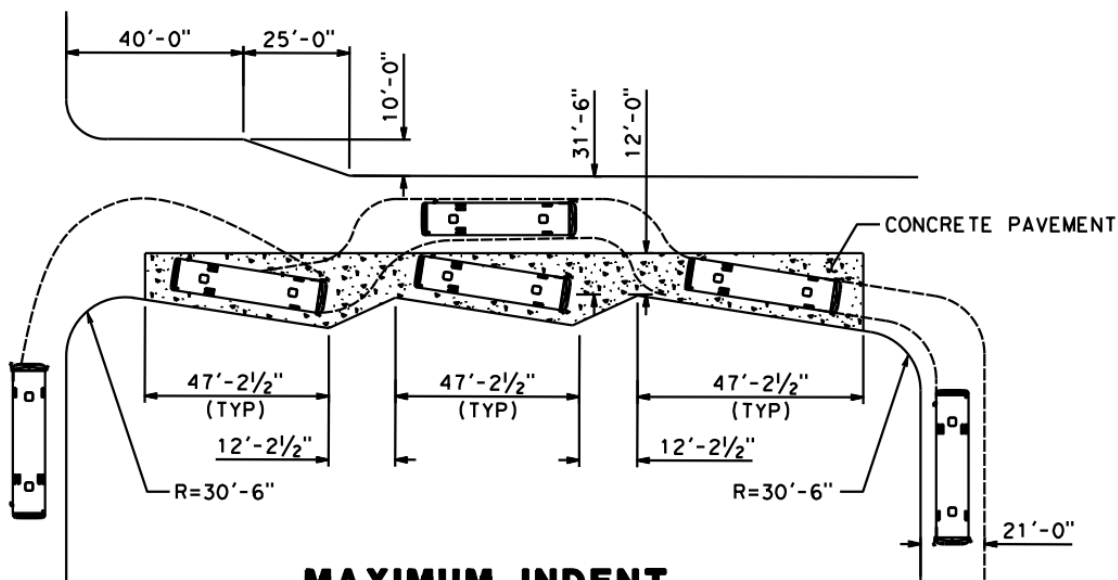


## 1 Figure 14-2: Sawtooth Bus Bays



A	24'	25'	26'	27'	28'	29'	31'-6"
B	80'	76'	72'	68'	64'	60'	59'-5"
C	35'	31'	27'	23'	19'	15'	12'-1/2"
D	0'	0'-8"	1'-4"	2'-0"	2'-8"	3'-4"	8'-6"

## BUS BAY DETAILS



## MAXIMUM INDENT

### B. Intercity Bus

Facilities for intercity buses shall be provided on a station-by-station basis, depending on existing facilities in the surrounding community and estimated potential future demand in that community for intercity bus service.



1 Design standards and guidelines shall follow intracity bus guidelines.

C. Driver Relief Station/Restrooms

2 Where requested by bus companies, driver's restroom facilities shall be provided on site for use  
3 by specific transit agencies. Determination of this need may be made based on the level of  
4 transit service provided at the station and local transit operational plans by the Authority with  
5 the input of local transit agencies. These facilities shall not be open to the general public.

**14.4.2.5 Bicycles**

6 **HST Travel** – Bicycles shall be allowed on trains and therefore access to and from station  
7 entrances shall accommodate bicycles. Bicyclists will be permitted to pass through accessible  
8 Fare Gates.

9 **Circulation Routes** – Where practicable, bicycle circulation shall be segregated from vehicle and  
10 pedestrian flows through the provision of bicycle-only paths direct to the station. In most cases,  
11 however, bicycles will need to make the most effective use of roadways and curb cuts. If bicycle  
12 routes are shared with roadways, bicycle lanes shall be designated.

13 **Storage** – Bicycle racks and lockers shall be provided at station sites. The amount of bicycle  
14 storage will vary based on demand, which depends on surrounding land uses, terrain, bicycle  
15 facilities and other factors. Bicycle parking shall be located as close to the station entrance as  
16 practicable given site constraints and other design guidelines. Where practicable, bicycle  
17 parking shall be within sight of station staff and general station pedestrian traffic for  
18 surveillance. From the bicycle parking area, it shall be easy to access the station entrance and  
19 the surrounding bike network and street system. The area shall be covered, well-lit, secure, and  
20 highly visible.

21 **Sharing** – In addition to bicycle storage racks and lockers, space shall be provided for bicycle-  
22 sharing pick-up and return facilities at appropriate stations.

**14.4.2.6 Pick-Up and Drop-Off Zone**

A. Planning Principles

23 Passenger pick-up and drop-off zones will be used by multiple modes, including private  
24 automobile kiss-and-ride, taxi, paratransit, private shuttle buses, and rental cars.

25 Planning shall encourage vehicle turnover, reduce conflicts, and facilitate traffic flow. This zone  
26 shall be close to the station entrance. Convenient recirculation of vehicles within the station site  
27 shall be provided where feasible in order to reduce congestion on the road network  
28 surrounding the site.

29 Pedestrian routes between the station entrance and vehicle drop-off area shall be direct with no  
30 vehicle lanes to cross. Walking distance from this area to the station entrance shall be less than  
31 600 feet. Lane widths shall allow vehicles to pass those who are stopped. This zone shall be  
32 separated from kiss-and-ride facilities and located to avoid interference with transit operations.

Access roads for kiss-and-ride facilities may be shared provided operation of either facility is not interrupted. However, pick-up/drop-off traffic shall not be routed through parking facilities. Right-hand drop-off, pick-up, and recirculation shall be provided without leaving the station site in order to reduce congestion on surrounding streets.

Depending on the demand for pick-up and drop-off facilities, some station sites may segregate the two uses by having a pick-up area and a drop-off area. If the curb length required to service demand exceeds station frontage by 200 percent, then separate arriving and departing areas with a loading island in front of building will be necessary.

At stations with high demand for pick-up and drop-off, it may be appropriate to provide a waiting area for automobiles outside of the pick-up and drop-off zone so that automobiles are not waiting at the curb for the train to arrive (and therefore blocking other patrons' access) or circulating, which results in increased congestion.

**Passenger Drop-off and Loading Zones for Disabled** shall be as follows:

- Shall be a minimum of 20 feet in length of vehicle pull-up space
- Minimum 20 feet by 5 feet access aisle adjacent and parallel to the vehicle pull-up space
- Access aisle shall connect directly to an accessible route
- Minimum 114 feet vertical clearance at accessible drop-off and loading zones and along at least one vehicle access route to the zone
- The zone shall be signed "PASSENGER LOADING ZONE ONLY" and include the International Symbol of Accessibility in white on a dark blue background (CA Title 24 1131B.1)

**B. Private Shuttle / Van**

Shuttle van access may be located in the pick-up and drop-off area. Shuttle vans include private paratransit, parking shuttles, hotel shuttles, and other services. Separation from other modes depends on demand.

**C. Kiss and Ride**

Passenger kiss-and-ride areas are set aside for private automobile drivers to pick-up and drop-off HST customers. Design of these areas shall consider the following:

**Capacity** – Designer shall determine required design capacity based on the individual requirements of each station.

- Kiss-and-Ride facilities shall be located off-street, as near to the main station entrance as practical, and shall be physically separate from long-term parking areas. Loading is preferred on the right-hand side of the car. The location shall, if possible, be such that the driver may view the station entrance to see exiting passenger.

- 1 • An accessible parking area for persons waiting to pick up persons with disabilities shall be  
2 provided as required by installing appropriate pavement markings and signs.
- 3 • Kiss-and-Ride parking spaces shall be delineated by signs or curb markings as being limited  
4 to short-term use.
- 5 • Kiss-and-Ride parking stalls shall be 18 feet long and 9 feet–6 inches wide and preferably at  
6 a 60-degree angle.
- 7 • Stalls and aisles for passenger drop-off areas shall be larger than those in long-term parking  
8 areas due to the frequent turnover.
- 9 • Preferred parking arrangements for passenger drop-off areas are as follows (in order of  
10 preference):
  - 11 – Parallel to curb
  - 12 – 45 degrees to the drive aisle
  - 13 – 60 degrees to the drive aisle
  - 14 – 90 degrees to the drive aisle
- 15 • Pedestrian crosswalks shall leave a minimum of 20-foot zones on either side of parallel  
16 parking stalls, or greater if directed by local codes.
- 17 • If drop-off areas are provided in a park-and-ride lot, placement shall avoid conflicts with  
18 entering and exiting traffic.

#### D. Rental Car

19 Rental car drop-off or pick-up service may be integrated into the kiss-and-ride area. Scale of  
20 facilities varies by demand and Authority's policy.

#### E. Taxi

21 The number of passengers arriving by taxi will determine the operational characteristics of the  
22 taxi areas. Once taxi activity passes a threshold level of operations, taxis begin to interrupt other  
23 pick-up/drop-off area operations. Further increases in taxi activities may require segregation of  
24 taxi pick-up and drop-off areas. Design of taxi facilities shall follow the following guidelines  
25 based on magnitude of taxi activity:

- 26 • Low – Taxi operations mix with shuttle vans and kiss-and-ride.
- 27 • Medium – Taxi operations are segregated from other modes of arrival. Taxi operations may  
28 be mixed with other modes if pick-up and drop-off areas are segregated.
- 29 • High – Taxi operations are segregated from other modes of arrival; further segregation of  
30 taxi pick-up and drop-off areas

31 Stalls or pick-up/drop-off areas shall be marked according to whether the stalls are "Taxi Only"  
32 or shared with other pick-up/drop-off modes. Space shall be provided for taxi queuing or  
33 corralling prior to pick-up of arriving HST patrons. The queuing space size shall correspond to

projected waiting taxi demand, and it shall be located near to, but not at the taxi pick-up area in order to provide quick service but not interfere with operations at the station entrance. Where analysis indicates significant potential demand for taxi service and passenger queuing at the taxi pick-up location, a staffed taxi dispatcher booth or desk may be required.

Taxi zones shall have a minimum lane width of 8 feet. Parking spaces for taxis shall be 25 feet long and shall be no closer than 20 feet to a crosswalk.

#### 14.4.2.7 Parking

##### A. Planning Principles

Parking may be designated as short term, all day, or long term. Local zoning regulations shall be met or exceeded. The following standards ensure adequate access and performance of parking facilities:

- Parking areas located outside of station sites shall be connected to station entrances by pedestrian walkways, bridges, or underpass structures by pedestrian walkways conforming to the requirements of this section.
- On-site parking shall be provided within easy walking distance to the station entrance. The maximum walking distance for any parking space from the station entrance shall be 1,500 feet or a 7-minute walk.
- For stations where adequate on-site parking cannot be provided adjacent to the station or within a short walking distance, off-site facilities may be developed, which shall be served by parking shuttle services.
- Connections between parking areas and station entrances shall be direct and obvious. Parking shall be designed so that those leaving their cars are “fed” onto primary pedestrian routes.
- Parking facilities shall be located as close as possible to the streets serving the site.
- Parking shall be at 90-degree angles with two-way aisles and no dead ends.
- Pedestrian movements within parking areas will normally occur along drive aisles. Where possible, drive aisles shall be oriented towards the station to facilitate pedestrian flow and to minimize the need to walk between parked cars.
- Striped pedestrian walkways may also be required where pedestrians flow perpendicular to aisles.
- Speed bumps may be considered to reduce vehicle speeds for pedestrian safety.
- Configure parking to allow station access for emergency vehicles including fire equipment and ambulances.

For additional parking criteria, including dimensional layout for station site area parking, refer to the *Civil* chapter.

**B. Multi-level Parking Structures**

1 Parking structures are not anticipated to be included inside station area plans. However,  
2 parking structures provided for the use of patrons shall conform to HST parking requirements.

3 Parking structures shall conform to the following standards:

- 4 • Ground levels shall contain entrances and exits, accessible parking, reservoir areas and  
5 internal ramps, and locations for obtaining parking tickets on entry and toll booths on exit,  
6 as well as parking areas. Kiss-and-Ride areas shall be located outside the parking structure  
7 unless this is not practicable. Upper levels and/or underground levels shall contain only  
8 ramps and as many parking stalls as possible.
- 9 • Where possible, the street level of parking structures shall incorporate active uses to  
10 increase natural surveillance and a sense of pedestrian activity.
- 11 • Traffic circulation within parking structures shall be designed to minimize vehicular travel  
12 distances and number of turns.
- 13 • Uniform long-span structures shall be provided to minimize columns within parking bays  
14 and maximize parking efficiency. Columns shall not encroach on the clear dimensions noted  
15 for cars, unless a small-car stall may be created. Where short-span design is unavoidable,  
16 columns shall be located not closer than every third stall.
- 17 • Parking stalls perpendicular (90 degrees) to aisles are preferred. Internal ramps shall be  
18 designed for two-way travel. Parking stalls shall not be located on curved internal ramps.
- 19 • External speed ramps may be used where appropriate. External ramps shall be designed for  
20 one-way travel and shall merge directly into or diverge directly from roadways. Where  
21 practicable, grade separation of pedestrians and vehicles shall be provided where external  
22 ramps cross pedestrian walkways. Parking stalls shall not be located on speed ramps.
- 23 • Ramp grades shall be kept as low as practicable, and excluding areas of transition, shall not  
24 exceed 5 percent on ramps with parking or 8 percent on ramps without parking.
- 25 • Traveled ways, other than parking aisles and ramps, shall be minimum 24 feet wide for two-  
26 way travel and 16 feet wide for one-way travel or as directed by local ordinances.
- 27 • Parking structure siting shall not create conflicts between pedestrian circulation to stations  
28 and the surrounding community. Driveways that serve parking shall avoid crossing main  
29 pedestrian routes in the station area.
- 30 • Parking structures shall be sized and located to encourage shared use of parking.
- 31 • Parking structures and station entrances shall be connected with continuous, accessible,  
32 covered pedestrian walkways where practicable.

**C. Motorcycle Parking**

33 Motorcycle/scooter parking stalls shall be provided as part of station parking. Local codes shall  
34 be considered in facility design. Motorcycle stalls may be preferably located in leftover space

that would otherwise not be used. Stall sizes shall be consistent with Standard and Directive Drawings. Motorcycle parking shall not be allowed at station entrances, in bicycle parking area, on sidewalks, or other pedestrian walkways.

**D. Carpool/Vanpool Parking**

Where appropriate, reserved parking stalls shall be provided for people who arrive at the station in vanpools and carpools. Initial planning and environmental assessment for the facility shall identify potential need for these stalls. If implemented, the stalls shall be located closer to the station entrance than general parking. Local requirements shall be considered in design.

**E. Car-Sharing/Station Car Parking**

Where appropriate, reserved parking stalls for car-sharing vehicles (i.e., ZipCar, CityCarShare, etc.) vehicles shall be provided. Initial planning and environmental assessment for the facility shall identify potential need for these stalls. If implemented, the stalls shall be located closer to the station entrance than general parking. Local requirements shall be considered in design.

**F. Accessible Parking Requirements**

In accordance with requirements of ADA and state building code, accessible parking shall be provided at facilities where parking is provided. ADA parking shall be located closest to the station entrance with a direct, accessible path leading to the station entrance. For additional accessible parking criteria, refer to the *Civil* chapter.

**G. Staff Parking**

Parking stalls shall be provided, which are consistent with staff functional needs, yet do not pre-empt convenient passenger parking. Staff parking shall be located and identified so that station patrons do not use employee spaces.

**H. Security Parking**

Parking stalls shall be provided for the use of HST security or police personnel. Spaces shall be located close to the station entrance. Security parking shall be identified so that station patrons do not use these spaces.

**I. Passenger Pick-Up (“Cell Phone”) Parking**

Where appropriate, parking shall be provided for people picking up HST passengers while they wait for trains to arrive. Passenger pick-up parking does not need to be located adjacent to stations but pick-up and drop-off zones shall be easily accessible from this parking area. Amenities, such as real-time train arrival information, shall be considered.

**J. Electric Vehicle Charging Stations**

Where appropriate, electric vehicle charging stations shall be provided in station parking facilities. Electrical requirements for electric vehicle charging stations are provided in the *Facility Power and Lighting Systems* chapter.

## **K. Parking Management System**

Because parking will be provided at market rates, control at access and egress points is necessary. Parking facilities shall be designed to allow for queuing for pay-on-entry or pay-on-exit systems. The number of entrance and exit lanes will vary by demand, but at least 2 lanes in each direction are required in the case of maintenance or a stalled vehicle at 1 gate. Traffic loop sensors, traffic gates, and antennas shall be provided in order to implement gate cashiering, “pay on foot,” Smart Card, conventional multi-space meter, or other revenue collection systems.

### **14.4.3 Vehicle Access and Circulation**

#### **14.4.3.1 Planning Principles**

Vehicle access to the station site shall not interfere with access modes of higher priority. The location and design of vehicle entrances and exits shall take into account the following factors:

- Primary station site vehicular access shall be located along higher capacity streets (such as arterial streets), to provide direct connections to local destinations and to protect adjacent neighborhoods from excessive vehicle congestion.
- Access via high-speed arterials shall be avoided.
- Curb cuts shall be minimized.
- Access roadways shall be planned with sufficient traffic storage capacity to meet expected patronage at peak times and to prevent traffic backing up onto public streets.
- The number of access roads shall be limited to reduce confusion and increase efficiency. Intersections shall allow traffic to flow at LOS D or better (Highway Capacity Manual).
- Access points shall be located to reduce crossing movements for inbound traffic (on the right side of the roadway) where possible.
- Exclusive turn lanes shall be provided only where necessary to maintain acceptable traffic operations.

#### **14.4.3.2 Service and Maintenance Vehicle Access**

All station sites shall have a loading zone and parking spaces with special access routes separate from other traffic provided for delivery trucks, service trucks, refuse trucks and other maintenance vehicles. Designated access route for cash-handling vehicles to cash-handling facilities shall be considered. Loading and unloading zones for delivery trucks shall be located to avoid interruption of station operations. An access route shall be considered for installation or future replacement of station equipment and facilities.

#### **14.4.3.3 Emergency Access**

Access for emergency response by fire department and paramedic equipment/personnel shall be provided, consistent with local codes. Fire lanes shall be clearly marked on the pavement. Refer to Section 14.3.1.4 for additional emergency access information.

#### 14.4.4 Roadways

Standards and guidelines for roadways are as follows:

- Street and intersection dimensions shall be planned to facilitate pedestrian and vehicular movement.
- Roadways shall circulate counter-clockwise and be configured to allow for recirculation within the site, allowing passengers to drop off a passenger and then park or retrieve a car from the parking area and then pick up an arriving passenger at the station curb.
- One-way traffic operation shall be provided if adequate right-of-way is available.
- In order to control speeds, roadways shall be no wider than necessary for “design” travel speeds and emergency vehicle access and egress.
- On-street parking shall be considered in order to slow traffic and buffer pedestrians.
- Roadways intended to provide access to bus zones, park-and-ride stalls, and passenger drop-off areas shall be designed in accordance with Section 14.4.2 and local transit provider design guidelines.
- Provisions for passing a stalled vehicle shall be made along roadways exiting from public streets.
- Where there are main sidewalks and crosswalks, there shall be no wide-turning radii, driveways, garage entrances or dedicated turning lanes that require pedestrian refuge islands.

For additional roadway criteria refer to *Civil* chapter.

#### 14.4.5 Site Infrastructure

##### 14.4.5.1 Site Utilities

- Sitework design at stations shall include investigation of existing utilities for relocation or removal as required.
- Locations of major tree installations shall be identified in time to coordinate with necessary underground utilities.
- Utility design criteria are included in the *Utilities* chapter.

##### 14.4.5.2 Site Grading and Drainage

- Paved and planter surfaces shall conform to the existing prevailing site topography and slope towards site drains.
- Site surfaces shall be sloped away from buildings and sheet flowing onto paved passenger circulation routes leading to station entries shall be avoided.



- 1 • Sites shall be sloped in a way that minimizes the potential for water infiltration at points of
- 2 pedestrian access to stations.
- 3 • Rain leaders from roofs shall be connected to underground drainage conduits. Splash blocks
- 4 and surface outlets on paved pedestrian areas shall not be permitted.
- 5 • Site drainage criteria are included in the *Drainage* chapter.
- 6 • Grading criteria are included in the *Geotechnical* chapter.

#### **14.4.5.3 Site Lighting**

7 **Purpose** – Site areas shall be well lighted. The purposes of site lighting include providing for

8 passenger safety, to aid with wayfinding and to ensure clarity and visibility throughout the site.

9 **Source** – Site lighting sources shall be coordinated with adjacent street lighting to provide the

10 same or similar quality of light. Site pedestrian walkways, sidewalks, parking areas, bus

11 shelters, etc. shall use metal halide or conforming high pressure sodium sources unless noted

12 otherwise.

13 **Poles** – Mount site lighting on medium to low aluminum poles on round concrete bases

14 adjacent to pedestrian walkways or in planters within parking areas.

15 **Light Spill** – Care shall be taken to prevent site lighting from spilling onto adjacent properties.

16 Light shields may be used to direct site lighting towards the intended areas.

17 **Plantings** – Care shall be taken to prevent plantings from interfering with essential site lighting

18 and signage.

19 **Lighted Signage** – Directional, informational and regulatory signage shall be clearly

20 illuminated to permit visibility throughout the site.

21 **Exterior Building Lighting** – Coordinate with station lighting as indicated in Section 14.3.7.

22 Facility lighting criteria is described in detail in the *Facility Power and Lighting Systems* chapter.

#### **14.4.5.4 Site Signage**

23 Site signage information may be found in Section 14.3.9 - Information Systems.

#### **14.4.5.5 Site Security**

24 Station site security infrastructure includes the following:

- 25 • CCTV cameras
- 26 • Emergency intercoms
- 27 • Customer assistance intercoms
- 28 • Station area lighting
- 29 • Access control system

- 1 • Intrusion detection system
- 2 • Signs indicating emergency procedures
- 3 • Fire truck access lanes and signs

#### 14.4.5.6 Finish Materials

4 Refer to Section 14.5 - Landscaping for site hardscape, paving and retaining wall materials.

#### 14.4.5.7 Architectural Fencing and Gates

- 5 • Architectural fences and gates in public areas of the station site are described in this section.
- 6 Utilitarian chain-link fencing shall not be used in public station plaza areas.
- 7 • Architectural fences and gates for individual stations shall be compatible with station
- 8 architecture.
- 9 • Preferred materials for fencing materials are stainless steel, painted galvanized steel, or
- 10 powder coated steel.

#### 14.4.5.8 Site Furnishings

11 In addition to site criteria mentioned previously, the following site furnishings shall be  
12 provided as appropriate, including but not limited to the following:

- 13 • Benches and individual outdoor seating
- 14 • Bus and taxi queue shelters – Where passengers connect to other transportation modes at
- 15 stations, shelters shall provide comfortable waiting areas, with appropriate shade and
- 16 weather protection and wayfinding provisions. Additional information may be found in
- 17 Section 14.4.4 Intermodal Transfers.
- 18 • Security phones, alarms and related accessories
- 19 • Flagpoles
- 20 • Waste receptacles
- 21 • Bollards (as needed to prevent unauthorized vehicle entry) – These shall be removable and
- 22 if necessary for emergency or maintenance vehicle access, securable with a padlock.
- 23 • Public phones

24 Site furnishings shall be designed with respect to design life, durability, weather resistance,  
25 maintainability, vandalism resistance as well as attractive appearance and functionality for  
26 patrons.

#### 14.4.5.9 Station Plaza

27 An entry plaza shall be planned to demarcate a gateway to the station and a place for  
28 pedestrians to congregate. Size and design will vary by site.

#### 14.4.5.10 Communication and Electrical Buildings

If additional communication or electrical buildings are required on the station site away from the station, the buildings shall be co-located with service vehicle parking. Also, ancillary buildings shall be located in close proximity to HST tracks and away from general vehicle circulation. As a security measure, communications and electrical buildings shall be located away from passenger areas and not be identified with signage.

### 14.5 Landscaping

#### 14.5.1 Scope

Landscaping criteria describe planting, hardscape, and irrigation for individual HST station sites, support facilities, line sections, trackways, portals, and wayside structures and repair of existing landscaping.

**Erosion Control** – Erosion control at stations, other facilities, and along the alignment shall follow standards in the Caltrans Highway Design Manual and Erosion Control Technical Guide.

**Planting** – Criteria is included for selection of new plant materials, paving, and site furniture as well as replacement and maintenance of existing plant materials.

**Irrigation** – Criteria are included for modification of existing irrigation systems and for the design of efficient irrigation systems for new planting.

#### 14.5.2 General Landscaping Principles

**Goals** – Facilities will be situated within a wide diversity of microclimates, temperatures, humidity, soil conditions, air quality, and visual contexts. Therefore, the facility landscaping shall be designed and selected to be appropriate for each individual site in lieu of a system-wide landscaping approach.

Consistent system-wide landscaping goals include the following:

- The materials and mass of structures shall be softened through addition of plantings, water features, artwork, or other passenger-friendly site amenities that respond to the individual site microclimates and physical site contexts.
- The passenger and visitor experience, while entering or departing the station, shall be enhanced.
- Passenger circulation routes outside stations shall be defined.
- Shade shall be created and excessive heat gain shall be reduced in front of station entries; windbreaks and shelters shall be provided from precipitation where appropriate.

- Existing station area urban design concepts, site-specific historic landmarks, and natural features and views shall be related.

**Site Circulation** – Landscape features shall contribute to the clarity of the passenger circulation path from the site perimeter to the station entries. Landscape features shall not obstruct direct pedestrian routes to and from the station entrances.

**Entry Plaza** – Stations shall provide at least one entry plaza at or near grade level, where the major opportunities for tree size planters will be located. The need for shading structure in the station plaza area may rely on a mix of semi-mature trees and fabricated structures to moderate the large amounts of hard surfaces at elevated station plazas.

**Safety** – Trees, planters, and site furnishings shall not obstruct lines of sight for passengers and station staff nor create potential hiding areas.

**Maintenance at Stations** – New plant materials shall be selected for compatibility with local microclimates, resistance to pests, moderate or low irrigation requirements, and integration with any existing planting to remain. Planting areas shall be designed to minimize maintenance costs and potential for theft or vandalism.

Maintenance of landscaping requires control or prevention of the growth of plant materials inside the facility perimeter. Plant materials that may be used for screening of the facilities shall avoid migration past the facility perimeter.

**Utility Coordination** – New planting areas shall be coordinated with existing and planned station area underground and overhead utilities. Locations of deep tree wells with underground utilities shall be coordinated.

**Grounding Coordination** – Landscaping design shall be coordinated with ground grid locations to avoid conflicts between tree roots and grounding, in accordance with the *Grounding and Bonding Requirements* chapter.

### 14.5.3 Facility Types

#### 14.5.3.1 Elevated Stations

**Upper Levels** – Opportunities for introduction of plant materials on upper levels at elevated stations will be limited by programmatic, operational, maintenance, and structural conditions.

**Planters** – Where above-grade raised or recessed planters are contemplated, structural loads, waterproofing, irrigation, drainage, and routine maintenance access shall be addressed.

#### 14.5.3.2 At-Grade Stations

**Plantings** – At-grade stations offer opportunities to bring landscape planting areas close to the station. The use of trees native to the station site will aid in connecting the station to the

1 surrounding urban context. If there are other street tree species used near the station plaza area,  
2 they may be appropriate to use for neighborhood continuity.

### 14.5.3.3 Rights-of-Way – Trackways

3 **General Policy** – Ornamental landscape planting along trackway shall not be permitted within  
4 the Authority’s right-of-way except with approval by the Authority. Only drought-resistant  
5 ground cover requiring minimal maintenance shall be approved along trackways. Hardscape  
6 features such as retaining walls and terraces may be used, in addition to plant materials, to  
7 maintain slopes along trackways.

8 Special trackway planting may be approved in urban areas, where train speeds will be low, and  
9 along tracks approaching stations. Native and locally proven plant materials are encouraged.  
10 Trackway planting at stations shall be coordinated with the overall station area landscape  
11 design concept.

12 Special trackway landscape is permitted only in areas with roadway vehicle access; however,  
13 landscape maintenance is not permitted in areas used by maintenance-of-way track vehicles.

14 Slope stabilization, erosion, and dust control during construction are addressed by Caltrans  
15 Standards.

16 **Setbacks** – Minimum distance between track centerline and any landscape and tree planting is  
17 addressed in the *Trackway Clearances* chapter.

18 **Maintenance** – Refer to the Operations and Maintenance Plan for additional information about  
19 maintenance of landscaping within the Authority’s right-of-way. Maintenance and irrigation  
20 requirements shall not interfere with train operations.

### 14.5.3.4 Tunnel Portals

21 **Objective** – Landscaping around tunnel portal sites includes mitigation of trees and other  
22 natural features that will be permanently altered by portal construction. Solutions that carefully  
23 contrast natural features with made-made portal features are more successful and enduring and  
24 require less maintenance.

25 **Plantings** – Plant material selection shall be limited to surrounding existing species.

26 **Tunnel Facilities** – Larger portal area facilities such as power substations, communications  
27 equipment rooms, ventilation equipment rooms and maintenance storage rooms may be  
28 partially recessed into portal structures with planted roof areas using native vegetation. Access  
29 roads, maintenance storage yards, vehicle parking areas, and emergency response staging areas  
30 may create graded or paved flat areas in previously sloped terrain. Cuts and fills shall be  
31 treated to prevent erosion and planted for compatibility with surrounding topography and  
32 existing vegetation.

### 14.5.3.5 Support Facilities

**Scope** – See the *Support Facilities* chapter for a detailed description.

**Urban Context** – Support facilities located in urban areas may require dense landscape screening and architectural walls. Use of selected trees and shrubs for screening may be appropriate.

**Wayside Facilities** – Landscaping at wayside facilities along the Authority’s right-of-way shall be low maintenance. Perimeters around equipment shall be surrounded with concrete, precast paving blocks, dense gravel, ballast rock, etc.

**Screen Walls** – Where wayside facilities occur in urban areas, partial screen walls of masonry or ornamental metal fabrications may be appropriate.

### 14.5.4 Plant Materials

#### 14.5.4.1 New Plant Materials

**Planting Areas** – General planting requirements include the following:

- Select attractive planting that enhances the building design and site area.
- Select drought-resistant and locally-native planting within the site area.
- Limit planted slopes to 2:1
- Minimize site maintenance through the use of drought-resistant plantings.
- Avoid the need for extensive irrigation systems or maintenance such as lawns or plant materials that shed large amounts of leaves or fruits.

**Drought Resistance** – In accordance with Authority’s policies for sustainable design goals, 75 percent of new plant materials shall be drought resistant, requiring very low irrigation. Where practicable, native plant materials requiring minimal irrigation shall be selected as a regional solution to station area planning and landscaping.

**Clear Zones** – Plantings within public utility corridors that parallel, cross, or intersect the Authority’s facilities shall be coordinated. Additional criteria for protection and maintenance of the respective underground, surface, or overhead infrastructure shall be coordinated with vegetation that is proposed for stations and facilities.

#### A. Plant Size

Plantings shall be installed in accordance with the following minimum sizes:

- |                    |                 |
|--------------------|-----------------|
| • Ground Cover:    | 1 gallon        |
| • Shrubs:          | 1 and 5 gallons |
| • Perimeter Trees: | 15 gallons      |

- 1 • Circulation Trees: 15 gallons
- 2 • Pedestrian Walkway and Plaza Trees: 24-inch box
- 3 • Parking Area Trees: 24-inch box

#### B. Tree Selection

4 The major tree types to be used at stations and other facilities include the following:

- 5 • **Parking Areas** – Large deciduous trees. Paved parking areas shall be softened with tree  
6 planters at intervals of approximately one tree per ten parking spaces.
- 7 • **Pedestrian Walkways and Station Plazas** – Medium-size flowering deciduous trees.  
8 Pedestrian area trees may also be combined with raised seating planters of stone or concrete.  
9 Topsoil in planters shall be planted with ground covering.
- 10 • **Perimeter Trees** – Extensions of existing street tree planting patterns of the surrounding  
11 station area site. The interface of perimeter trees with the station area site plan shall be  
12 coordinated with local urban design standards or guidelines.

#### C. Green Roofs

13 Use of green roof planting may be considered for stations.

##### 14.5.4.2 Existing Plant Materials

14 **Site Survey** – Station and other facility sites with significant existing planting that will be  
15 impacted by construction shall be surveyed by a landscape architect registered in the State of  
16 California. Trees and shrubs shall be documented for protection in place, relocation, removal,  
17 temporary removal, or replacement.

18 **Existing Trees** – Where local communities have significant, established urban features such as  
19 uniform street tree planting, consideration shall be given to extending the theme into the station  
20 area site plan.

21 **Sustainability** – Landscape design for stations shall conform to the Authority's sustainability  
22 goals. If existing local street tree and landscape area planting requires significant irrigation,  
23 points shall be identified on the site perimeter where appropriate transitions may be made  
24 between the existing high water use plantings and the sustainable plantings.

25 Exceptions can be made where significant existing trees or landscaped park may have  
26 important local historic value. Where such features may become a part of the new station area  
27 site plan, the responsibility for ongoing maintenance shall be clearly defined.

28 Where sites require temporary clearing and restoration of wider project rights-of-way, and if the  
29 project requires replacement of existing plant materials, designers shall take the opportunity to  
30 introduce more sustainable plant materials into the landscape restoration with the permission of  
31 the local jurisdiction.

### 14.5.5 Irrigation

1 **Efficiency** – Landscape irrigation shall be planned to provide efficient water use. Plantings that  
2 are not drought tolerant shall be irrigated separately from the remaining plantings.

3 **Budget** – Station or support facility landscape plans shall begin with clear limits on quantities of  
4 available irrigation water on an annual basis. Plant material selection shall be limited to species  
5 that can thrive on the available irrigation budget.

6 **Collection** – Stormwater systems shall be designed to capture rainfall into street gardens and  
7 bio-swales. Wherever possible, non-potable water suitable for landscape irrigation shall be  
8 used.

9 **Grey Water** – If local municipalities have established gray water infrastructure in place at  
10 station sites, wastewater may be treated on site to local sanitary district specifications and  
11 discharged into the local gray water system.

### 14.5.6 Hardscape

#### 14.5.6.1 Paving

12 This section addresses general landscape criteria for paved areas for vehicles, bikes, pedestrians,  
13 service vehicles, and buses in both public and non-public areas. Further design criteria site work  
14 paving is included in *Civil* chapter.

15 Paved pedestrian areas shall conform to requirements in the 2010 CBC and ADAAG.

16 Roadways shall be separated from pedestrian walkways by 6-inch-high curbs, where curbs are  
17 appropriate considering drainage.

18 Pedestrian area paving materials shall be concrete, precast, or stone pavers. Asphalt paving in  
19 pedestrian areas is permitted only as a temporary repair.

20 Appearance, life cycle cost, and recycling shall be considered when selecting paving materials.  
21 In order to support the CHSTP's sustainability goals, locally produced materials shall be used  
22 wherever practicable.

23 **Tree Grates.** Pedestrian area trees shall be set in tree grates and pruned to allow approximately  
24 8 feet of clearance below horizontal branching. Grates shall be a minimum of 4 feet x 4 feet with  
25 a hole pattern in conformance with ADA standards.

#### 14.5.6.2 Pedestrian Walkways

26 Pedestrian walkway paving materials shall contrast in appearance from vehicle paving  
27 materials.



- 1 Pedestrian walkways shall be constructed of materials approved for pedestrian area paving.
- 2 Pedestrian walkways shall conform to relevant accessibility design requirements of the 2010
- 3 CBC and other local requirements.
- 4 Manholes, grates, drain covers, underground utility access covers, and related site structures
- 5 shall not be located in pedestrian walkways unless absolutely necessary.
- 6 Sloped grades, less than 5 percent are preferred to ramps along pedestrian walkways.
- 7 For additional pedestrian walkway standards within the Station Area Site Design, refer to
- 8 Section 14.4.3.2.

#### **14.5.6.3 Landscape Retaining Walls**

- 9 Landscaping retaining walls higher than 4 feet-0 inches are described in the *Structures* chapter.
- 10 Retaining walls for paving and landscape elevation changes shall be stepped or terraced.
- 11 Materials for retaining walls shall be poured-in-place, reinforced concrete, precast concrete,
- 12 reinforced unit masonry, or modular cut stone.
- 13 Retaining walls shall have appropriate internal drainage to prevent excessive collection of
- 14 moisture. Outlets shall not drain onto paved pedestrian walkways.

## Chapter 15

### Support Facilities

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	May 2013	Revision 1



## Table of Contents

<b>15</b>	<b>SUPPORT FACILITIES</b>	<b>15-1</b>
15.1	Scope	15-1
15.2	Regulations, Codes, Standards, and Guidelines	15-1
15.3	Support Facilities	15-2
15.3.1	Design Principles	15-2
15.3.2	Heavy Maintenance Facility	15-3
15.3.3	Operations Control Center	15-4
15.3.4	Headquarters Building	15-6
15.3.5	Terminal Storage and Maintenance Facilities	15-7
15.3.6	Regional Control Centers	15-8
15.3.7	Line Facilities	15-9
15.4	Support Facilities Site Design	15-10
15.4.1	Site Design Objectives	15-10
15.4.2	Parking	15-10
15.4.3	Vehicle Access and Circulation	15-11
15.4.4	Roadways	15-11
15.4.5	Site Infrastructure	15-11
15.4.6	Landscaping	15-13

## Tables

Table 15-1:	Heavy Maintenance Facility Space Requirements	15-3
Table 15-2:	Operations Control Center Space Requirements	15-4
Table 15-3:	Headquarters Building Space Requirements ( <i>to be further developed</i> )	15-6
Table 15-4:	Terminal Storage and Maintenance Facilities Space Requirements	15-7
Table 15-5:	Regional Control Center Space Requirements	15-8

## Acronyms

ADA	Americans with Disabilities Act
CHST	<i>Not Used; use HST for California High-Speed Train</i>
CHSTP	California High-Speed Train Project
HMF	Heavy Maintenance Facility
HST	High-Speed Train
HVAC	Heating, Ventilation, and Air Conditioning
LOS	Level of Service
MOI	Maintenance of Infrastructure
OCC	Operations Control Center
OCS	Overhead Contact System
RCC	Regional Control Center
TSMF	Terminal Storage and Maintenance Facilities

## 15 Support Facilities

### 15.1 Scope

1 This chapter provides architectural and planning design criteria for California High-Speed  
2 Train (HST) support facilities.

### 15.2 Regulations, Codes, Standards, and Guidelines

3 Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.  
4 This chapter presents design standards and guidelines specifically for the construction and  
5 operation of HST support facilities based on international best practices and applicable state  
6 and federal requirements.

7 Since support facilities will be located in multiple municipal jurisdictions, other transportation  
8 owner/operators' rights-of-way, and/or unincorporated jurisdictions, the CHSTP standards and  
9 guidelines may differ from local jurisdictions' codes and standards.

10 Applicable codes, rules, standards, and guidelines include but are not limited to the following:

- 11 • Code of Federal Regulations (CFR) Title 28, Part 36 – Nondiscrimination on the Basis of  
12 Disability by Public Accommodations and in Commercial Facilities
- 13 • California Building Code (CBC), Title 24 of California Code of Regulations (CCR)
- 14 • Americans with Disabilities Act (ADA)
  - 15 – ADA Standards for Accessible Design
  - 16 – Guidance on the ADA Standards for Accessible Design
  - 17 – ADA Accessibility Guidelines (ADAAG)
- 18 • National Fire Protection Association (NFPA)
  - 19 – NFPA 101 – NFPA's Life Safety Code
  - 20 – NFPA 130 – Standard for Fixed Guideway Transit and Passenger Rail Systems
- 21 • American National Standards Institute (ANSI) 117.1 – standard for accessible design for  
22 persons with disabilities
- 23 • California Division of the State Architect California Access Compliance Reference Manual

## 15.3 Support Facilities

This section describes architectural requirements of the non-station buildings and related facilities to accommodate all activities necessary for the maintenance, operation, and administration of the HST System.

Support Facilities include the following:

- Operations Control Center
- Regional Control Centers (2)
- Terminal Storage and Maintenance Facilities (4)
- Heavy Maintenance Facility
- Administrative Headquarters Building
- Maintenance-of-Infrastructure (MOI) and Rolling Stock Maintenance (RSM) Facilities (7)
- Portal Facilities
- Mid-tunnel Ventilation Structures
- Emergency Access Shafts
- Line Facilities including the following:
  - Traction Power, Switching, and Paralleling Stations
  - Train Control / Communications and Control Wayside Structures and Cabinets
  - Trackside Systems Equipment

### 15.3.1 Design Principles

- **Design Quality** – Support Facilities shall be designed to exhibit the Project’s commitment to excellent design for all types of facilities, exhibiting architectural and functional solutions appropriate to the context in which they are built.
- **Life Cycle** – Materials, assemblies, construction techniques, products, and systems shall be selected for durability, appearance and cost-effectiveness with a design life of 50 years. While many components of the facilities may be replaced in shorter periods of time, all design decisions shall maintain this long-range view.
- **Structural Design** – Structural design elements of Support Facilities shall conform to the requirements described in the *Structures* chapter.
- **Grounding and Bonding** – Service siding platform design shall be coordinated with the requirements specified in the *Grounding and Bonding Requirements* chapter.

### 15.3.2 Heavy Maintenance Facility

**Purpose** – High-speed train sets shall be assembled, tested, and commissioned at the Heavy Maintenance Facility (HMF). During revenue service, periodic major maintenance shall be performed at the HMF, as well as unplanned maintenance and repairs. The HMF shall also provide overnight storage of a portion of the train fleet. Along with the Operations Control Center (OCC), the daily functioning of the HST System may be co-located at the HMF.

**Building Functions** – Refer to Table 15-1 and the Operations and Maintenance Plan for detailed descriptions of functional requirements for the HMF.

The major shop buildings at the HMF shall accommodate 2 full length single-train sets of approximately 700 feet each. They shall accommodate light and heavy industrial processes to completely maintain and repair all components of the high-speed vehicles. The vehicle servicing layout, daylight illumination, and natural ventilation functions of these buildings will influence the architectural design.

**Table 15-1: Heavy Maintenance Facility Space Requirements**

Function	Minimum Area Required
Enclosed Inspection Tracks	X s.f.
Exterior Train Washing	X s.f.
Automated Wheel Inspection	X s.f.
Wheel truing/re-profiling	X s.f.
Heavy duty interior cleaning platform(s)	X s.f.
Toilet Servicing	X s.f.
Inspection "pit" tracks	X s.f.
Traction Power Inspection and Maintenance	X s.f.
Pantograph Inspection and Maintenance	X s.f.
Sanding System Replenishment	X s.f.
Detailed Bogie Inspection and Maintenance	X s.f.
Train Exterior Workshop	X s.f.
Electric Components Inspection and Maintenance	X s.f.
Machining Tool Facility	X s.f.
Parts receiving, storage, and distribution	X s.f.
Administrative, training, and staff supports areas	X s.f.
Operations Control Center	X s.f.

**Access** – HMF roads shall provide efficient circulation and parking for administrative, maintenance, receiving, and operations staff vehicles and private cars. HMF roads shall also allow access to emergency vehicles and controlled access for delivery of maintenance parts and



supplies. The site plan shall also include an appropriate public approach to the administration component of the facility for visitors.

**Sustainability** – In accordance with the Project’s sustainability goals and commitment to 100 percent renewable energy sources, photovoltaic, wind, and other alternative energy harvesting infrastructure shall be included in the design of the HMF.

**HMF Gate House** – The HMF main entry shall be controlled by security personnel in a gatehouse with facilities and equipment to monitor pedestrians and vehicle access on a 24-hour schedule. Personnel at the gatehouse shall monitor arriving and departing vehicles, including staff private vehicles, materials suppliers, visitors, and emergency vehicles.

**Exterior** – As the first building that all visitors will encounter on the site, the gatehouse shall be an architecturally attractive facility, coordinated with the concept and expressions of the main HMF shop buildings. Weather protection shall be provided over the driver section of entering vehicles and vehicle barriers and curbs to protect the building and staff. Security lighting shall be provided to monitor gate and site access.

**Interior** – The gatehouse shall incorporate systems to monitor and remotely operate all perimeter gates at the HMF site. The gatehouse shall include a space for guard operations as well as a compact kitchen, lockers, and a uni-sex toilet room, heating, ventilation, and air conditioning (HVAC) system and interior lighting.

### 15.3.3 Operations Control Center

**Purpose** – The OCC provides a secure location for Operations staff to supervise and control the HST System 24 hours per day, year-round. Refer to Table 15-2 and the Operations and Maintenance Plan for a complete description of the components and functions of the OCC.

**Table 15-2: Operations Control Center Space Requirements**

Function	Minimum Area Required
Control Center Operating Theater	X s.f.
Server Room	X s.f.
Incident Room	X s.f.
Conference Room	X s.f.
Manager Office	X s.f.
Staff Break Room/Kitchenette	X s.f.
Private Staff Offices (8)	X s.f. each
Staff Lockers and Restrooms	X s.f.
Administrative, training, and staff support areas	X s.f.
Equipment Rooms	X s.f.
Reception/Waiting Room	X s.f.

**Location** – The OCC may be a freestanding building or co-located with the HMF. Considering the significant functional differences between the OCC and the HMF Administrative offices, these functions may be located separately from the shop buildings, in a freestanding building. If freestanding, it shall be on a secure location within the HMF site. The OCC shall be expandable to house the full build-out operations.

There is no requirement for direct connection between the OCC and the HMF facilities or staff.

**Functions** – Major activities within the OCC include the following:

- Train dispatching and control
- Supervisory Control and Data Acquisition (SCADA)
- Infrastructure control, monitoring and maintenance
- Monitoring of fire and life safety systems
- Communications and passenger information
- Fleet Management
- Manpower Management
- Emergency response and management
- System Security
- Traction Power Control and Operation

**Building Features** – OCC facility requirements include the following:

- Secure, controlled entry
- Internal security points
- Emergency backup power
- HVAC systems
- Controllable daylight
- Dedicated emergency response workstations.

**Shared Facilities** – If the OCC is co-located with the HMF, the OCC operation may share some staffing and operational functions with the administrative requirements of the HMF. Spaces such as entry lobbies, security check points, break rooms, training rooms, and lockers may be shared.

The equipment located in a Server Room shall be located directly adjacent to the Control Center Operating Theater. The Control and Server Rooms shall have a depressed floor structure with computer flooring.

#### **15.3.4 Headquarters Building**

**Purpose** – The headquarters building shall be the official administrative center of the HST System.

**Phasing** – During the initial Phase 1 planning, design, and construction, the administrative offices may be located in temporary space. As staffing needs grow and administrative program becomes more fully defined, the Authority may require a dedicated Administration Building.

**Location** – In order to support the Authority’s stated policies for station development to “encourage high-density development in and around the HST stations” the Administration Building shall be located with the same goal.

**Design Goals** – The headquarters building will be a high-profile facility and shall exemplify the Authority’s high goals for design excellence. In accordance with design guidelines for stations, the Authority supports the California Division of State Architect, Department of General Services “Excellence in Public Buildings” program, which promotes “high-performing public buildings and a positive architectural legacy that reflects the State’s commitment to excellence.”

**Sustainability** – In addition to high quality design, cost effectiveness, universal accessibility, and other requirements, the headquarters building shall achieve Leadership in Energy and Environmental Design Silver certification.

**Function** – The design of the headquarters building will require integration of modern administrative offices, conference and meeting spaces, public relations functions and related support spaces, within the context of a large historic rail yard context.

Refer to Table 15-3 for a preliminary space-planning program for the Headquarters Building.

**Table 15-3: Headquarters Building Space Requirements (*to be further developed*)**

<b>Function</b>	<b>Minimum Area Required</b>
Main Lobby	X s.f.
Executive Offices (4)	X s.f.
Conference Rooms (2)	X s.f.
Conference Rooms, Small (2)	X s.f.
Administrative Offices	X s.f.
Training Rooms	X s.f.
Staff Offices	X s.f.
Others <i>TBD</i>	X s.f.

### 15.3.5 Terminal Storage and Maintenance Facilities

**Purpose** – Routine servicing, inspection, and light-to-intermediate maintenance of the trains shall occur at the Terminal Storage and Maintenance Facilities (TSMF), which provide overnight, scheduled maintenance, inspection, and storage and morning start-up of revenue services. Refer to Table 15-4 and the Operations and Maintenance Plan for detailed planning information.

**Table 15-4: Terminal Storage and Maintenance Facilities Space Requirements**

Function	Minimum Area Required
Entrance/Assembly Shop	X s.f.
Enclosed Inspection Tracks	X s.f.
Exterior Train Washing	X s.f.
Automated Wheel Inspection	X s.f.
Wheel truing/re-profiling	X s.f.
Heavy duty interior cleaning platform(s)	X s.f.
Toilet Servicing	X s.f.
Inspection “pit” tracks	X s.f.
Traction Power Inspection and Maintenance	X s.f.
Sanding System Replenishment	X s.f.
Parts receiving, storage, and distribution	X s.f.
Administrative, training, and staff support areas	X s.f.

**Function** – The primary functions to be accommodated within the TSMF include the following:

- Overnight storage of trainsets
- Periodic inspection (San Francisco and Los Angeles)
- Quick replacement of running gear, underbody, and pantograph parts
- Train washing and Interior cleaning
- Commissary provisioning

**Type of Building** – The maintenance facilities associated with the TSMFs can be considered smaller versions of the HMFs. In contrast to the HMFs, most of these TSMFs will be located within urban areas. Some TSMFs will be highly visible and in close proximity to industrial, commercial and residential land uses. Although the TSMF buildings will house different maintenance functions, the scale, massing, and proportions shall follow the same design principles as those stated for the HMF.

**Gate House** – The TSMF main entry shall be controlled by security personnel in a gate house with facilities and equipment to monitor pedestrians and vehicle access similar to the HMF gate house.

### 15.3.6 Regional Control Centers

**Purpose** – On the Caltrain and Los Angeles to San Diego shared-use corridors, multiple carriers currently maintain complex joint operations that, independent of HST, are expected to become progressively more so. As a result, the CHSTP Command & Control system is proposed to be designed to include a Northern California Regional Control Center (RCC) and Southern California RCC that will focus exclusively on regional operations on these busy corridors on a 24-hour/7-days-a-week basis. Refer to Table 15-5 and the Operations and Maintenance Plan for a complete description of the components and functions of the RCCs.

**Table 15-5: Regional Control Center Space Requirements**

Function	Minimum Area Required
Control Center Operating Theater	X s.f.
Server Room	X s.f.
Incident Room	X s.f.
Conference Room	X s.f.
Manager Office	X s.f.
Staff Break Room/Kitchenette	X s.f.
Private Staff Offices (8)	X s.f. each
Staff Lockers and Restrooms	X s.f.
Administrative, training, and staff support areas	X s.f.
Equipment Rooms	X s.f.
Reception/Waiting Room	X s.f.

**Location** – RCCs are proposed to be located at the Level 3 TSMF situated near San Francisco and Los Angeles. The RCCs may be freestanding buildings or co-located with the TSMF. The RCCs shall be expandable to house the full build-out operations.

There is no requirement for direct connection between the RCCs and the TSMF facilities or staff.

**Functions** – Major activities within the RCCs include the following:

- Train dispatching and control
- SCADA
- Infrastructure control, monitoring and maintenance
- Monitoring of fire and life safety systems

- Communications and passenger information
- Fleet Management
- Manpower Management
- Emergency response and management
- System Security

**Building Features** – RCC facility requirements include the following:

- Secure, controlled entry
- Internal security points
- Emergency backup power
- HVAC systems
- Controllable daylight
- Dedicated emergency response workstations.

**Shared Facilities** – If the RCC is co-located with the TSMF, the RCC operation may share some staffing and operational functions with the administrative requirements of the TSMF. Spaces such as entry lobbies, security check points, break rooms, training rooms, and lockers may be shared.

The equipment located in a Server Room shall be located directly adjacent to the Control Center Operation Room. The Control and Server Rooms shall have depressed floor structure with computer flooring.

### 15.3.7 Line Facilities

**Type** – Several types of facilities are required along the length of the HST system at various intervals.

- Traction Power Facilities (see the *Traction Power Supply System* chapter)
- Maintenance of Infrastructure Facilities
- Wayside Facilities

**Maintenance of Infrastructure Facilities** – MOI facilities are required at regular intervals (distance to be determined) along the Authority's right-of-way. These facilities shall maintain the track, Right-of-Way, Overhead Contact System (OCS) systems, and related wayside equipment during the nighttime period of non-operation, using non-OCS powered vehicles. Size – 24-26 acres with two buildings approximately 30,000 square feet each.

1 Where space and site conditions permit, MOI facilities may be co-located with the HMF or 4  
2 TSMF.

3 Wayside Facilities includes various equipment cabinets, OCS equipment, cable and conduit  
4 access boxes, radio transmission facilities, and related communication and control equipment  
5 installed along the Authority's right-of-way.

6 MOI design shall be coordinated with the requirements specified in the *Grounding and Bonding*  
7 *Requirements* chapter.

## **15.4 Support Facilities Site Design**

### **15.4.1 Site Design Objectives**

8 Support Facility sites shall achieve the following fundamental site design objectives:

- 9 • Facilitate simple, direct and safe movement of people and vehicles to, from, and within the  
10 site.
- 11 • Ensure emergency access to support facilities including statutory fire lanes and vehicle  
12 access.
- 13 • Ensure system-wide consistency in site design.

### **15.4.2 Parking**

14 Parking may be designated as short term (visitor) or long term. Local zoning regulations shall  
15 be met or exceeded. The following standards ensure adequate access and performance of  
16 parking facilities:

- 17 • On-site parking shall be provided within easy walking distance to the support facility  
18 entrance.
- 19 • Parking shall not necessitate crossing of maintenance tracks to reach the facility entrance.
- 20 • For support facilities where adequate on-site parking cannot be provided adjacent to the  
21 facility or within a short walking distance, off-site facilities may be developed that are  
22 served by parking shuttle services.
- 23 • Parking facilities shall be located as close as possible to the streets serving the site.
- 24 • Parking shall be configured to allow station access for emergency vehicles, including fire  
25 equipment and ambulances.

26 For additional parking criteria, including dimensional layout for station site area parking, refer  
27 to the *Civil* chapter.

### 15.4.3 Vehicle Access and Circulation

#### 15.4.3.1 Planning Principles

The location and design of vehicle entrances and exits shall take into account the following factors:

- Primary site vehicular access shall be located along higher capacity streets (such as arterial streets), providing direct connections to local destinations and protecting adjacent neighborhoods from excessive vehicle congestion.
- Access via high-speed arterials shall be avoided.
- Access roadways shall be planned with sufficient traffic storage capacity to meet expected flow at worker shift times and to prevent traffic backing up onto public streets.
- The number of access roads shall be limited to reduce confusion and increase efficiency. Intersections shall allow traffic to flow at Level of Service (LOS) D or better (Highway Capacity Manual).
- Access points shall be located to reduce crossing movements for inbound traffic (on the right-side of the roadway) where possible.
- Exclusive turn lanes shall be provided only where necessary to maintain acceptable traffic operations.

#### 15.4.3.2 Service and Maintenance Vehicle Access

All support facility sites shall provide a loading zone and parking spaces with special access routes separate from other traffic provided for delivery trucks, service trucks, refuse trucks, and other maintenance vehicles. Loading and unloading zones for delivery trucks shall be located to avoid interruption of facility operations.

#### 15.4.3.3 Emergency Access

Access for emergency response by fire department and paramedic equipment/personnel shall be provided consistent with local codes. Fire lanes shall be clearly marked on the pavement.

### 15.4.4 Roadways

Roadways providing access to parking, bus zones, and staff drop-off areas shall be designed in accordance with the *Civil* chapter and local transit provider design guidelines.

### 15.4.5 Site Infrastructure

#### 15.4.5.1 Site Utilities

- Sitework design at support facilities shall include investigation of existing utilities for relocation or removal as required.
- Locations of major tree installations shall be identified in time to coordinate with necessary underground utilities.



- 1 • Utility design criteria are included in the *Utilities* chapter.

#### 15.4.5.2 Site Grading and Drainage

- 2 • Paved and planter surfaces shall conform to the existing prevailing site topography and  
3 slope towards site drains.
- 4 • Site surfaces shall be sloped away from buildings and shall avoid sheet flowing onto paved  
5 pedestrian circulation routes.
- 6 • Rain water leaders from roofs shall be connected to underground drainage conduits. Splash  
7 blocks and surface outlets on paved pedestrian areas are not permitted.
- 8 • Site drainage criteria are included in the *Drainage* chapter.
- 9 • Grading criteria are included in the *Geotechnical* chapter.

#### 15.4.5.3 Site Lighting

10 **Purpose** – Site areas shall be well-lighted. The purposes of site lighting include providing for  
11 worker and visitor safety, to aid with wayfinding, and to ensure clarity and visibility  
12 throughout the site.

13 **Source** – Site lighting sources shall be coordinated with adjacent street lighting to provide the  
14 same or similar quality of light. Site pedestrian walkways, sidewalks, parking areas, bus  
15 shelters, etc. shall use metal halide or conforming high-pressure sodium sources unless noted  
16 otherwise.

17 **Poles** – Site lighting shall be mounted on medium to low aluminum poles on round concrete  
18 bases adjacent to pedestrian walkways or in planters within parking areas.

19 **Light Spill** – Care shall be taken to prevent site lighting from spilling onto adjacent properties.  
20 Light shields may be used to direct site lighting towards the intended areas.

21 **Plantings** – Care shall be taken to prevent plantings from interfering with essential site lighting  
22 and signage.

23 **Lighted Signage** – Directional, informational, and regulatory signage shall be clearly  
24 illuminated to permit visibility throughout the site.

25 **Exterior Building Lighting** – Station lighting shall be coordinated as indicated in the *Stations*  
26 chapter.

27 Facility lighting criteria is described in detail in the *Facility Power and Lighting Systems* chapter.

#### 15.4.5.4 Site Signage

28 Site signage shall conform to standardized project signage and graphics established for the  
29 CHSTP. System signage information is described in the *Stations* chapter.

#### 15.4.5.5 Site Security

- 1 Station site security infrastructure includes the following:
- 2 • Closed Circuit Television (CCTV) cameras
  - 3 • Emergency intercoms
  - 4 • Station area lighting
  - 5 • Access control system
  - 6 • Intrusion detection system
  - 7 • Signs indicating emergency procedures
  - 8 • Fire truck access lanes and signs

#### 15.4.6 Landscaping

- 9 **Scope** – See the *Stations* chapter for a detailed description of landscaping requirements.
- 10 **Urban Context** – Support facilities located in urban areas may require dense landscape  
11 screening and architectural walls. Use of selected trees and shrubs for screening may be  
12 appropriate.
- 13 **Wayside Facilities** – Landscaping at wayside facilities along the Authority’s right-of-way shall  
14 be low maintenance. Perimeters around equipment shall be surrounded with concrete, precast  
15 paving blocks, dense gravel, ballast rock, etc.
- 16 **Screen Walls** – Where wayside facilities occur in urban areas, partial screen walls of masonry or  
17 ornamental metal fabrications may be appropriate.



## Chapter 16

### Mechanical

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	May 2013	Revision 1



## Table of Contents

16	MECHANICAL (TBD).....	16-1
----	-----------------------	------

### Tables

No table of figures entries found.

### Figures

No table of figures entries found.

## Acronyms


## **16 Mechanical (TBD)**

- 1 TBD. This topic will be included in a future revision of the Design Criteria.





## Chapter 17

### Facility Power and Lighting Systems

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
2	Feb 2014	Revision 2



## Table of Contents

<b>17</b>	<b>FACILITY POWER AND LIGHTING SYSTEMS .....</b>	<b>17-1</b>
17.1	Scope .....	17-1
17.2	Regulations, Codes, Standards, and Guidelines .....	17-2
17.3	Description of the Facility Power System .....	17-4
17.4	Classifications of Electrical Loads .....	17-6
17.5	General Requirements .....	17-7
17.5.1	Safety Design .....	17-7
17.5.2	Uniformity .....	17-8
17.5.3	Reliability, Availability, Maintainability and Safety .....	17-9
17.5.4	Electrical Room/Yard Layout and Security Considerations .....	17-9
17.5.5	Housekeeping Pads .....	17-11
17.5.6	Operating Instructions .....	17-11
17.5.7	FPS Designs in Rooms Housing Sensitive Electronic Equipment .....	17-11
17.5.8	Enclosure Ratings .....	17-12
17.5.9	Underwriters Laboratories Rating .....	17-12
17.5.10	Future Equipment .....	17-12
17.5.11	Earthquake Engineering Design .....	17-12
17.5.12	Environmental and Operational Engineering Design .....	17-12
17.5.13	Calculations .....	17-13
17.6	Supply Voltage .....	17-14
17.6.1	Medium and High Voltage Electrical Loads .....	17-14
17.6.2	Low Voltage Electrical Loads .....	17-14
17.7	New Electrical Utility Services .....	17-18
17.7.1	General Requirements for New Electrical Utility Services .....	17-18
17.7.2	Metering Requirements .....	17-20
17.7.3	Concessionaires .....	17-20
17.8	Emergency Power Supply Systems .....	17-20
17.8.1	Emergency Power Supply Equipment .....	17-21
17.9	Sustainable Power Sources .....	17-22
17.9.1	Photovoltaic and Wind Systems .....	17-22
17.10	Facility Power System Equipment .....	17-22
17.10.1	General .....	17-22
17.10.2	High Voltage Switchgear .....	17-24
17.10.3	High Voltage Transformers .....	17-24
17.10.4	High Voltage Motor Control Centers .....	17-24
17.10.5	Unit Substation .....	17-25
17.10.6	480/277 V ac Switchboards/Switchgear .....	17-25
17.10.7	Panelboards (480/277 V ac, 208/120 V ac, 250/125 V dc) .....	17-25
17.10.8	Low Voltage Transformers .....	17-25
17.10.9	Low Voltage Circuit Breakers/Switches .....	17-26
17.10.10	Motors, Starters and Controls .....	17-26
17.10.11	Receptacles .....	17-26
17.11	Specialized Electrical Equipment and/or Loads .....	17-27
17.11.1	Rolling Stock Maintenance Facilities .....	17-27
17.11.2	Elevators .....	17-27

17.11.3	Escalators.....	17-28
17.11.4	Sump Pumps.....	17-28
17.11.5	Tunnel Ventilation System .....	17-28
17.11.6	Ticket Vending Machines and Fare Gates.....	17-29
17.11.7	Panels .....	17-29
17.11.8	Interface Termination Cabinet.....	17-30
17.11.9	Vehicle Battery Charging Facilities .....	17-30
17.11.10	Specialized Equipment for First Responders (within Tunnels) .....	17-31
17.12	Lighting System Requirements.....	17-31
17.12.1	Quality of Illumination .....	17-32
17.12.2	Facility Illumination Levels .....	17-32
17.12.3	Emergency Lighting and Exit Signs .....	17-33
17.12.4	Circulation Roadway Lighting and Roadway Sign Lighting (within the Authority's Right-of-Way) .....	17-33
17.12.5	Street and/or Motorway Lighting .....	17-33
17.12.6	Parking Structure and/or Lot Lighting.....	17-34
17.12.7	Elevator/Escalator Pit Lighting .....	17-34
17.12.8	Lighting Control System .....	17-34
17.13	Specialized Lighting .....	17-35
17.13.1	Exterior Lighting at Heavy Maintenance Facility, Terminal Layup/Storage, Maintenance Facilities, and Maintenance of Infrastructure Facilities .....	17-35
17.13.2	Tunnel /Trench Lighting .....	17-35
17.13.3	Exterior Stairway Lighting .....	17-35
17.13.4	Access Roadway Lighting (to Trackside Core System Facilities).....	17-36
17.13.5	Blue Light Stations .....	17-36
17.13.6	Yellow Lights .....	17-36
17.13.7	White Lights.....	17-36
17.13.8	Line Storage, Tail Track, and Turnaround Lighting.....	17-36
17.13.9	Vehicle Inspection Pit Lighting .....	17-36
17.13.10	Security Lighting.....	17-37
17.13.11	Obstruction Lighting .....	17-37
17.13.12	Light Pollution Mitigation .....	17-37
17.13.13	Lighting Fixtures .....	17-37
17.13.14	Poles, Foundations, Bracket Arms and Miscellaneous Hardware .....	17-37
17.14	Raceway Systems.....	17-38
17.14.1	Conduit Types .....	17-38
17.14.2	Conduit Fill Ratio.....	17-39
17.14.3	Conduit Sizes .....	17-39
17.14.4	Tray .....	17-39
17.14.5	Ductbanks and Underground Structures.....	17-40
17.14.6	Raceway Systems and/or Cable Trough for the Core Systems .....	17-41
17.14.7	Conduits within Reinforced Structures .....	17-42
17.14.8	Conduits Routed Under the Track.....	17-42
17.14.9	Exposed Conduits and/or Boxes within Enclosed Spaces (e.g., Tunnels, Emergency Stairways, etc.).....	17-42
17.15	Wiring/Cabling Systems .....	17-42
17.15.1	Tunnels.....	17-43
17.15.2	Color Coding .....	17-43
17.16	Identification Requirements (Equipment, Conductors, Conduit, Tray).....	17-44
17.16.1	Equipment .....	17-44

17.16.2	Manholes, Handholes, Vaults and Boxes .....	17-45
17.16.3	Conduit/Tray .....	17-46
17.16.4	Conductors .....	17-46
17.16.5	Identification Requirements for the Core Systems and Facility Communication Systems 17-47	
17.17	Electrical Discrete and Serial I/O Point Requirements .....	17-47
17.18	Temporary Power and Lighting.....	17-48
17.19	Grounding, Bonding and Lightning Protection .....	17-48
17.20	Fire Detection System .....	17-48
17.21	Vehicular Traffic Management System .....	17-48
17.22	Facility Communication Systems .....	17-48
17.23	Testing .....	17-49
17.24	Spares.....	17-49

## Tables

Table 17-1:	Demand Factor .....	17-14
Table 17-2:	Rated Voltages of the Equipment .....	17-14
Table 17-3:	Rated Voltages for Yard and Shops Special Equipment.....	17-15
Table 17-4:	Minimum Fuel/Battery Storage Capacities.....	17-20
Table 17-5:	Equipment Numbering.....	17-45
Table 17-6:	Manholes, Handholes, Vaults and Boxes .....	17-45
Table 17-7:	Conduit/Tray.....	17-46
Table 17-8:	Conductors.....	17-47

## Appendix

Appendix 17.A: Minimum Lighting Levels

## Acronyms

ANSI	American National Standards Institute
ATC	Automatic Train Control
Authority	California High-Speed Rail Authority
BAS	Building Automation System
CBC	California Building Code
CEC	California Electric Code
CHSTP	California High-Speed Train Project
EMT	Electrical metallic tubing
EWLCP	Emergency Walkway Lighting Control Panel
FAA	Federal Aviation Administration
FPS	Facility Power System
GFCI	Ground-Fault Circuit Interrupters
GRS	Galvanized Rigid Steel
HV	High Voltage
HVAC	Heating, Ventilation and Air Conditioning
I/O	Input/Output
IEEE	Institute of Electrical and Electronic Engineers
IESNA	Illuminating Engineering Society of North America
LCP	Local Control Panel
LS	Lighting System
LV	Low Voltage
MOI	Maintenance of Infrastructure
NEC	National Electric Code
NEMA	National Electrical Manufacturers Association
NFPA	National Fire Protection Association
O/RCC	Operations/Regional Control Center
OCS	Overhead Contact System
PVC	Polyvinyl chloride
RSM	Rolling Stock Maintenance
SCADA	Supervisory Control and Data Acquisition
TES	Traction Electrification System
TVM	Ticket Vending Machine
UPS	Uninterruptable Power Supply

## 17 Facility Power and Lighting Systems

### 17.1 Scope

This chapter provides design criteria of Facility Power and Lighting Systems for the various trackside facilities and trackside equipment. The specified requirements apply to the entire California High-Speed Train Project (CHSTP).

The design requirements and criteria for these systems to be installed for the CHSTP address the following:

- Classifications of Electrical Loads
- General Requirements (Operational, Functional, Performance)
- General Design Standards and Codes of Practice
- Supply Voltage
- Electric Utility Services
- Emergency Power Supply Systems
- Sustainable Power Sources
- Facility Power System Equipment
- Specialized Electrical Equipment
- Facility Lighting
- Specialized Lighting
- Raceway Systems
- Wiring/Cabling Systems
- Identification Requirements for Electrical Infrastructure
- Supervisory Control and Data Acquisition (SCADA) and/or Building Automation System (BAS) requirements

The design requirements and criteria specified herein shall be coordinated with the requirements specified elsewhere in the Design Criteria and Contract Documents.

The criteria have been established to provide the basis to complete the design. The responsibility for the design integrity, as well as integration and coordination with other disciplines remains with the designer.



## 17.2 Regulations, Codes, Standards, and Guidelines

1 Refer to the *General* chapter for requirements pertaining to regulations, codes, standards, and  
2 guidelines.

- 3 • California Code of Regulations (CCR)
  - 4 – Title 8 – Occupational Safety and Health Act Standards
  - 5 – Title 19 – Public Safety
  - 6 – Title 24 – Parts 1, 2, 3, 6 and 9
- 7 • California Public Utility Commission (CPUC) General Orders (GOs)
  - 8 – CPUC GO 95 – Rules For Overhead Electric Line Construction
  - 9 – CPUC GO 128 – Construction of Underground Electric Supply and Communications
  - 10 Systems
- 11 • California Building Code (CBC)
- 12 • California Electric Code (CEC)
- 13 • American Concrete Institute (ACI) Standards
- 14 • American Institute of Steel Construction (AISC)
- 15 • American National Standards Institute (ANSI) Standards
  - 16 – ANSI C2 – National Electrical Safety Code (NESC)
  - 17 – ANSI Z535.1 – Safety Color Code
  - 18 – ANSI Z535.3 – Criteria For Safety Symbols
  - 19 – ANSI Z535.4 – Product Safety Signs and Labels
- 20 • American Society of Heating, Refrigerating and Air-Conditioning Engineers, Inc (ASHRAE)  
21 Standards
- 22 • Illuminating Engineering Society of North America (IESNA) Standards
  - 23 – Lighting Handbook Reference and Application
  - 24 – RP-20, Lighting for Parking Facilities
  - 25 – RP-22, Tunnel Lighting
  - 26 – TM-11, Light Trespass: Research, Results, and Recommendations
- 27 • Institute of Electrical and Electronic Engineers (IEEE) Standards
  - 28 – IEEE 141 – Recommended Practice for Electrical Power Distribution for Industrial Plants
  - 29 – IEEE 142 – Recommended Practice for Grounding of Industrial and Commercial Power
  - 30 Systems

- IEEE 241 – Recommended Practice for Electrical Power Service in Commercial Buildings
- IEEE 242 – Recommended Practice for Protection and Coordination of Industrial and Commercial Power Systems
- IEEE 383 – Standard for Qualifying Class 1E Electric Cables and Field Splices for Nuclear Power Generating Stations
- IEEE 399 – Recommended Practice for Industrial and Commercial Power System Analysis
- IEEE446 – Recommended Practice for Emergency and Standby Power Systems for Industrial and Commercial Applications
- IEEE 493 – Recommended Practice for Design of Reliable Industrial and Commercial Power Systems
- IEEE 519 – Recommended Practices and Requirements for Harmonic Control in Electrical Power Systems
- IEEE 693 – Recommended Practices for Seismic Design of Substations
- IEEE 1100 – Recommended Practice for Powering and Grounding Sensitive Electronic Equipment
- IEEE 1584 – IEEE Guide for Performing Arc Flash Hazard Calculations
- IEEE C37.010 – Application Guide for AC High-Voltage Circuit Breakers Rated on a Symmetrical Current Basis.
- National Fire Protection Association (NFPA) Codes and Standards
  - NFPA 70 – National Electric Code (NEC)
  - NFPA 70E – Standard for Electrical Safety in the Workplace
  - NFPA 101 – Life Safety Code
  - NFPA 110 – Standard for Emergency and Standby Power Systems
  - NFPA 111 – Standard on Stored Electrical Energy Emergency and Standby Power Systems
  - NFPA 130 – Standard for Fixed Guideway Transit and Passenger Rail Systems

New electrical utility services to CHSTP facilities and trackside equipment enclosures shall comply with the design standards of the applicable electric utility service provider. Refer to the *Utilities* chapter for additional requirements.

### 17.3 Description of the Facility Power System

The Facility Power System (FPS) comprises all of the equipment, cables, raceway systems, mounting hardware and appurtenances required to isolate, transform, distribute and/or disconnect the power source(s) to the facility equipment and Core Systems (e.g., system-wide communications, train control) where these are co-located within, or adjacent to, a facility.

The design of the FPS shall provide safe, reliable and durable power sources and incorporate features to account for single point of failures and ensure the availability of power to the connected electrical loads. The design of the FPS shall include the following:

- Normal power source
- Backup power source and/or
- Emergency power supply source

The normal and backup power sources can originate from the local electric utility service provider and/or the Traction Electrification System (TES). Each power source shall be sized for the connected electrical load (including prescribed spare capacity). During normal operations, the normal power source shall provide power to all of the connected electrical loads. During contingency operations, where the normal power source is no longer available, the connected electrical loads shall be automatically switched to the backup power source.

The emergency power supply sources are incorporated into the design of the FPS to provide additional power when normal and backup power (where provided) sources fail. An emergency power supply source can originate from the following:

- Emergency standby generator.
- Uninterruptable Power Supply (UPS) (typically provided for vital functions, e.g., train control, system-wide communications, or emergency lighting for compliance with California Building Code requirement and where interruptions cannot be tolerated)
- DC battery system

Where a permanent emergency standby generator is not provided at a facility, a generator receptacle shall be included in the FPS design to permit connection of a portable generator.

The UPS equipment shall be electrically connected to the permanent emergency standby generator 480 V ac bus (or generator receptacle where the former is not provided) and the normal/backup 480 V ac bus.

The following bus designations shall apply to the design of the FPS:

- Normal Bus – fed from the normal and/or backup power source
- Generator Bus – fed from the emergency standby generator

- Emergency Bus – fed from the UPS equipment (assigned to lighting only) or dc battery system
- Essential Bus – fed from the UPS equipment (assigned to train control, system-wide communications, etc.)

The following list summarizes the minimum power sources to be provided at key facilities and/or trackside equipment enclosures:

- Passenger Station – normal, backup and emergency power supply sources
- Rolling Stock Maintenance Facilities (RSM) (i.e., Heavy Maintenance Facility, Terminal Layup/Storage and Maintenance Facility) – normal, backup, and emergency power supply sources, with the facility power substation configured as double ended
- Maintenance of Infrastructure (MOI) – normal, backup, and emergency power supply sources
- Portal Facility – normal, backup, and emergency power supply sources
- Tunnel – normal, backup, and emergency power supply sources <sup>1</sup>
- Mid Tunnel Ventilation Structure – normal, backup, and emergency power supply sources
- Emergency Access Shaft – normal, backup, and emergency power supply sources
- Sump Pump – normal and backup power sources
- Viaduct/aerial structure <sup>2</sup>
- Train Control/Communication Equipment Rooms (within Facilities) – normal and backup or emergency power supply sources <sup>1</sup>

Alternative power source arrangements may be submitted as a Design Variance Request to the California High-Speed Rail Authority (Authority) for review and approval.

Refer to the *Traction Power Supply System*, *Automatic Train Control*, *Yard Signaling*, and *Communication* chapters for the design criteria associated with LV power distribution system at standalone pre-engineered buildings and/or remote trackside equipment.

---

<sup>1</sup> Provide a generator receptacle (at each tunnel) where the tunnel length is less than 2500'-0" and is located within 2 hours drive of a maintenance facility. A permanent generator shall be provided at a tunnel a) where the tunnel length exceeds 2500'-0", b) that is located in remote areas and can be accessed within 2 hours drive of the local maintenance facility, and c) comprising of a series of short length tunnels (back to back but spaced apart) where the combined length exceeds 2500'-0"

<sup>2</sup> Where a viaduct/aerial structure section is between tunnel sections the trackside facility power infrastructure can be routed between tunnels via the aerial-structure for convenience (subject to the Designer's overall FPS design).

## 17.4 Classifications of Electrical Loads

Electrical loads connected to the FPS shall be classified as follows:

- **Critical** – Load that cannot tolerate any interruption longer than 0.25 cycles, or that it must be maintained for some time after normal and backup power is lost and consequently shall be served by a UPS or battery-based (ac) power system. Critical loads include, but are not limited to, the following:
  - Alarm, supervision systems, and control power for operation of the FPS equipment, including medium-voltage and LV switchgear
  - Emergency lighting in passenger stations, tunnels, emergency egress, illuminated signs and maintenance and service facilities
  - Exits signs, variable message signs
  - Fire protection system and public address systems
  - Control power for emergency ventilation system
  - Control power for the TES
  - Control power for emergency communication system, and security systems
  - Control power for load shed panel
  - Normal electrical power for automatic train control systems
  - Normal electrical power for system-wide communication systems
  - Emergency electrical power for the functions at the Operations Control Center
  - Audio visual equipment for emergency exit stairs at areas of refuge
  - Power to field equipment needed for life safety systems
- **Essential** – Loads that can tolerate an interruption for the time required for a transfer switch or circuit breaker to operate. Such loads shall be backed up by an automatic transfer to an alternative power source and shall not be disconnected. Essential loads include, but are not limited to, the following:
  - Elevators
  - Tunnel lighting normal power source
  - Emergency ventilation fans and dampers
  - Station duplex sump pumps
  - Tunnel duplex sump pumps
  - Fare collection system
  - Normal power supply of the UPS systems

- Battery charger
- Heating, ventilation, and air conditioning (HVAC) infrastructure at rooms that house equipment associated with the TES, Train Control System, and Communication System.
- **Nonessential** – Includes all loads not classified essential or critical. Such loads may not be backed up by an automatic primary transfer to an alternative power source and may be disconnected if necessary during emergency conditions. Nonessential loads include, but are not limited to, the following:
  - Normal lighting
  - Escalators
  - Station roll-up grille
  - Tenant/concessionaire spaces
  - Equipment or other subsystems not required for evacuation or security surveillance
  - General purpose and convenience receptacles at stations and along tunnels

## 17.5 General Requirements

### 17.5.1 Safety Design

The FPS design shall incorporate the following as a minimum:

- Avoiding, eliminating, or reducing hazards identified by engineering analysis, design choices, material selection, or substitution
- Controlling and minimizing hazards (to personnel, the public, and to equipment and materials) that cannot be avoided or eliminated
- Incorporating fail-safe principles where failures would disable the system or cause human injury, damage to equipment, or inadvertent operation of critical equipment
- Locating equipment components so that access to them by the required personnel during operation, maintenance, repair, or adjustment shall not require exposure to hazards such as entrapment, chemical burns, electrical shock, cutting edges, sharp points, or toxic atmospheres
- Providing suitable warning and caution notes in operations, assembly, maintenance, and repair instructions; and distinctive markings on hazardous components, equipment, or facilities for personnel protection
- Venting components containing or generating obnoxious, flammable, and harmful gases to the outside

- 1 • Physically segregating/separating cables and wires of different systems, and high voltage  
2 (HV) and LV conductors from each other in accordance with the requirements specified in  
3 the NEC and CEC
- 4 • Interlocking features to prevent: the following:
  - 5 – Paralleling of power sources
  - 6 – Energization of FPS equipment and \or conductors during fault conditions
- 7 • The following safety considerations shall be included in the design:
  - 8 – Overcurrent elements that are: (1) designed to protect conductors serving emergency  
9 equipment, emergency lighting, Facilities Communication Systems and fire detection  
10 system; and (2) located in spaces other than electrical rooms, shall not depend upon  
11 thermal properties for operation.
  - 12 – Arc flash hazard warning labels shall be provided on all equipment, along with flash  
13 boundary, and incident energy values shall be provided in accordance with the NEC  
14 and CEC.
  - 15 – Ground fault protection shall be provided on branch circuits that have equipment or  
16 receptacles for which personnel protection is required by the NEC, CEC, and  
17 engineering judgment.

### **17.5.2 Uniformity**

18 Equipment enclosures, assemblies, sub-assemblies, and components that do not differ in  
19 operational, functional or performance characteristics shall be designed so that all components  
20 are positioned in the same location and internal wiring is routed in between components in a  
21 like manner.

22 Where identical installations exist, the following requirements shall be adhered to:

- 23 • Unless site conditions prevent it, equipment enclosures shall be mounted and installed in a  
24 like manner.
- 25 • Penetrations for conduit, grounding, and access panels shall be located in the same place.
- 26 • The location of equipment relative to adjacent equipment shall not differ.
- 27 • The routing of conduit, tray, and cables between equipment enclosures shall not differ.
- 28 • Termination hardware shall be located in like manner.
- 29 • Cables and wire terminations shall be located in like manner.

30 Standardization:

- 31 • Equipment providing like functions shall be of the same design and manufacture.

- Equipment or devices of the same rating and function shall be identical and interchangeable.

### 17.5.3 Reliability, Availability, Maintainability and Safety

The design of the FPS and Lighting System (LS) shall ensure that a single point of failure does not impact the operation of critical and/or essential electrical loads.

The reliability of the FPS and LS and its subsystems shall be consistent with the overall Reliability, Availability, Maintainability and Safety requirements and standards referenced in the *Automatic Train Control* chapter. The FPS and LS availability shall also be consistent with the availability requirements of the overall project high level thresholds as specified in TBD. FPS and LS availability shall be higher than TBD percent.

The FPS and LS Target System shall achieve the following Mean Time Between Failures (MTBF) quantitative requirements:

MTBF for Major Failures:  $MTBF_{Mj} \geq$  TBD h

MTBF for Minor Failures:  $MTBF_{Mi} \geq$  TBD h

Human factors contributions to Reliability, Availability, Maintainability and Safety shall be considered as follows:

Mean Time To Travel (MTTT) shall be a fixed value selected by the Authority, based on experience from other high-speed rail operators, reflecting the location of MOI facilities and highway access to FPS and LS wayside locations.

The supplier shall demonstrate that a Mean Time to Restore (MTTR) of TBD minutes is achievable. MTTR is a mean value for all FPS and LS units; it includes diagnosis of faults measured from the actual start of work on site, replacing of faulty components, testing, and placing the system back into service in the affected area. MTTR does not include MTTT.

### 17.5.4 Electrical Room/Yard Layout and Security Considerations

Ideally, HV and/or very heavy electrical equipment (e.g., switchgear, transformers) shall be located at-grade in secured yards that will permit easy installation and future replacement. Otherwise, FPS equipment shall be located in dedicated electrical rooms.

The electrical rooms/yards shall be sized to allow for the equipment, code clearances, equipment placement and/or removal, and future system expansion. The footprint of electrical rooms/yards shall include space for future expansion of at least 25 percent above the required footprint of the initial FPS equipment configuration.

Electrical rooms/yard shall be planned with clear circulation aisles and adequate access to all equipment. Layout shall be neat, and the electrical rooms/yards shall be easy to clean.



Horizontal clearances shall comply with requirements set forth by the NEC, CEC, and equipment manufacturer. Equipment placement, within electrical yards, shall not obstruct access gateways and/or roadways within yards.

The design of the perimeter site security barrier and associated gates shall consider the following:

- Site constraints and accommodate the placement and/or removal of FPS equipment
- Placement of and access to equipment owned and operated by the local electric utility service provider

FPS equipment at passenger stations shall be located inside electrical rooms. Where this is not practicable, the FPS equipment shall be located so it is inaccessible to and out of obvious view of the patrons.

Consideration shall be given to locating heat-producing FPS equipment, outside of enclosed rooms.

Electrical rooms shall have a clear height to the underside of the structure for compliance with requirements of the NEC and CEC and not less than 12 feet. Catwalks shall be provided for all equipment that cannot be maintained from floor level. Where maintenance or equipment replacement requires the lifting of heavy parts, hoists and hatchways (where applicable) shall be installed and the structure (to which the hoist is attached) shall be designed for the intended loading conditions.

Lighting at the electrical rooms/yards shall be laid out so as not to interfere with equipment. Switched emergency lighting shall be provided in electrical rooms.

The width and height of doorways/gates to electrical rooms/yards shall take into consideration the largest piece of equipment to be placed within the space and shall not necessitate non-standard equipment shipping splits or disassembly on-site of equipment (during placement/removal operations).

All doorways and/or gates at electrical rooms/yards shall be secured. Refer to *TBD* chapter for additional security requirements.

The number and location of electrical rooms shall be determined so that the electrical loads served are located within reasonable distances.

The space above suspended ceilings shall be adequate to run conduits and cable tray systems. Normally allow a 3-foot-minimum clearance from the top of light fixtures to the bottom of structural ceiling or steel girders (where girders are used to support the ceiling) for the installation of conduits and cable tray systems.

1 Utilities and/or facilities services, that support the operation of rooms/facilities other than the  
2 electrical room/yard, shall avoid being routed within and/or underneath the electrical  
3 room/yard.

4 Safety signage and graphics shall be provided at all electrical rooms and on FPS equipment in  
5 accordance with applicable codes and as specified in TBD.

6 Barriers shall be provided to protect FPS equipment and \or infrastructure where installed in  
7 locations subject to vehicle damage.

#### **17.5.5 Housekeeping Pads**

8 Housekeeping pads shall be provided for all floor-mounted FPS equipment. Housekeeping  
9 pads shall be at least 3 inches larger than the mounted equipment on all sides and at least 4  
10 inches high (above finished floor surface).

#### **17.5.6 Operating Instructions**

11 Posted operating instructions (that are required for manually operated electrical equipment)  
12 shall be provided in all electrical rooms/yards. The instructions shall consist of simplified  
13 instructions and diagrams of equipment, controls and operation of the systems, including  
14 selector switches, main-tie-main transfers, automatic transfer switch by-pass, UPS by-pass, etc.  
15 Instructions shall be framed and posted adjacent to the major equipment of the FPS.

#### **17.5.7 FPS Designs in Rooms Housing Sensitive Electronic Equipment**

16 Examples of rooms housing sensitive electric equipment include operations/regional central  
17 controls, train control rooms, and communications rooms.

18 Non-linear loads generate harmonic currents that are reflected into the neutral service  
19 conductors. Exercise caution when designing circuits and selecting equipment to serve non-  
20 linear loads.

21 Avoid running unshielded metallic signal or data lines parallel to power feeders.

22 Where metallic signal or data lines must be routed in noise prone environments, use shielded  
23 cables or install wiring in ferrous metal conduit or enclosed cable trays.

24 Metallic signal or data lines and equipment shall be located at a safe distance from arc-  
25 producing equipment such as line voltage regulators, transformers, battery chargers, motors,  
26 generators, and switching devices.

27 Isolation transformers, electronic power distribution panel boards or power conditioners shall  
28 be provided to serve critical electronics equipment loads within the room (where the equipment  
29 is housed).

1 Refer to the *Grounding and Bonding Requirements* chapter, and the *Electromagnetic Compatibility*  
2 *and Interference* chapter for additional design requirements to mitigate impacts to sensitive  
3 electronic equipment from FPS equipment and/or conductors.

#### **17.5.8 Enclosure Ratings**

4 All indoor (except as defined below) electrical equipment enclosures shall be rated as National  
5 Electrical Manufacturers Association (NEMA) 12 or 3R.

6 All outdoor electrical equipment enclosures located above ground shall be rated as NEMA 4.

7 All electrical equipment enclosures located underground (e.g., tunnel, cross passages,  
8 ventilation structures [e.g., shafts and stairways] and sump pump structures), elevators and  
9 escalator pits shall be rated as NEMA 4X (i.e., stainless steel) and comply with the requirements  
10 specified in NFPA 130.

11 Alternative equipment enclosure NEMA classifications to those noted above that are suitable  
12 for the environment to which the equipment is exposed may be considered but should be  
13 submitted as a Design Variance Request for the Authority's approval.

#### **17.5.9 Underwriters Laboratories Rating**

14 All FPS and LS infrastructure shall be Underwriters Laboratories (UL) listed for the intended  
15 operating environment and application.

#### **17.5.10 Future Equipment**

16 The FPS design shall include provisions for equipment anticipated to be added in the future,  
17 including, but not limited to, security and baggage screening equipment, elevators, escalators,  
18 fare gates, bus signs, and pay telephones. Provisions shall include electrical distribution  
19 capacity, space for future circuit breakers, and rough-in conduit/boxes to future equipment  
20 locations. Rough-in conduit shall be planned and installed in the same fashion as circuits  
21 provided, so that when future equipment is installed, surface-mounted conduit can be avoided.

#### **17.5.11 Earthquake Engineering Design**

22 Equipment design, including device mounting, supports, materials, and installation methods  
23 shall conform to the design requirements outlined in IEEE 693 and specified in the *Seismic*  
24 chapter.

#### **17.5.12 Environmental and Operational Engineering Design**

25 The facilities housing the FPS equipment and/or housekeeping pads/foundations (on which FPS  
26 equipment is mounted) shall be protected from the known 500-year flood levels.

27 Refer to the *Drainage* chapter for additional requirements.

- 1 Where the FPS equipment placement is exposed to winds, the associated structural bracing  
2 designs shall consider wind loading and compute it by using a basic wind speed of 80 miles per  
3 hour, Exposure D and a Wind Importance Factor (I) equal to 1.15.
- 4 Refer to the *Structures* chapter for additional requirements pertaining to slipstream affects.

### 17.5.13 Calculations

#### 17.5.13.1 Voltage Drop

5 The following voltage drop criteria shall apply to the FPS. Compliance to the requirements shall  
6 be demonstrated using the calculations specified in the contract documents.

##### A. Sensitive Electronic Equipment

- 7 • 1.5 percent or less on branch circuit conductors
- 8 • 2.5 percent or less on combined feeder and branch circuit conductors

##### B. Power and Lighting Circuits

- 9 • 3 percent or less on feeder circuit conductors at the farthest field device in the circuit
- 10 • 5 percent or less on combined feeder and branch circuit conductors at the farthest field  
11 device in the circuit

##### C. Tunnel Ventilation Fan Motors

- 12 • 10 percent or less on feeder circuit conductors to fan motor during initial start-up conditions
- 13 • 3 percent or less on feeder circuit conductors to fan motor during steady state conditions

##### D. LV Motors

- 14 • 10 percent or less on feeder circuit conductors to LV motor during initial start-up conditions
- 15 • 3 percent or less on feeder circuit conductors to LV motor during steady state conditions

##### E. Emergency Standby Generator System

- 16 • 5 percent or less on feeder circuit conductors to the 480 V switchboard during initial start-up  
17 conditions
- 18 • 3 percent or less on feeder circuit conductors to the 480 V switchboard during steady state  
19 conditions

#### 17.5.13.2 Demand Factor

20 The following demand factors shall be used when sizing equipment and/or conductors:

**Table 17-1: Demand Factor**

Lighting and Signs	1.0 x connected load
Emergency Lighting	1.0 x connected load
ATC Equipment	1.0 x connected load
System-wide Communications Equipment	1.0 x connected load
Facility Communications Equipment	1.0 x connected load
Escalators	0.85 x connected load
Elevators	0.5 x connected load
HVAC Equipment	0.8 x connected load
Tunnel and/or Emergency Ventilation Equipment	1.0 x connected load
Chiller Plant	0.8 x connected load
Fare Collection Equipment	0.5 x connected load
Drainage Pumps and Ejectors	0.5 x connected load
Convenience Receptacles	1.5 ampere each
Electric Utility Service/TES Service to FPS	1.0 x connected load

### 17.5.13.3 Harmonics

- 1 Facility power and lighting systems equipment shall comply with all criteria and design provisions of the
- 2 DCM Chapter 26 – Electromagnetic Compatibility and Interference, Section 26.10 (Station and Facility
- 3 Equipment EMC Design Criteria), including Section 26.10.5.6 (Current and Voltage Distortion Limits).

## 17.6 Supply Voltage

### 17.6.1 Medium and High Voltage Electrical Loads

- 4 Voltage rating of tunnel ventilation axial and/or jet fans above 100 horsepower shall be
- 5 4.16 kilovolts (kV) ac, 3 Phase, 60 Hertz (Hz) or greater.

### 17.6.2 Low Voltage Electrical Loads

- 6 AC power for general facilities shall be supplied at nominal 480 V ac, three-phase, 4-wire,
- 7 60 Hz. For loads 50 kVA and higher with feeder length of 1,500 feet or longer, an evaluation
- 8 shall be performed to determine the more cost-effective system voltage between 4,160 V ac and
- 9 480 V ac. Alternate voltage levels may be used where practical and safe for the equipment.
- 10 Other nominal voltages shall be obtained by use of dry-type transformers.
- 11 The rated voltages of the equipment shall be as follows:

**Table 17-2: Rated Voltages of the Equipment**

Item No.	Equipment	Electrical Requirements
----------	-----------	-------------------------

1	Advertising	120 V	single-phase (if any)
2	Area and parking lot lighting	277 V	single-phase
3	Auxiliary service to tie breaker stations	480 V	three-phase
4	Communication equipment and alarm systems	120/208 V	three-phase 4-wire, grounded neutral
5	Convenience receptacles	120 V	single-phase
6	Dry-type Transformers	480 V to 120/208 V	3-wire, delta primary, 4-wire, wye secondary
7	Electric clocks	120/208 V	three-phase 4- wire, grounded neutral
8	Exit signs	120 V	single-phase (if any)
9	Fare collection equipment	120/208 V	three-phase four wire
10	Fluorescent lighting	277 V	single-phase
11	Heaters for motors or electric equipment	120 V	2- or 3-wire single-phase
12	Heaters (> 5,500 Watts)	480 V	three-phase
13	HID lighting	277 V	single-phase
14	Incandescent lighting	120 V	single-phase
15	Motors	0.37 kW to 100kW Under 0.37 kW	480 V 120 V three-phase single-phase
16	Motor control	120 V	single-phase
17	Power receptacles	480 V 120 V	three-phase single-phase
18	Train control and signal systems	120/208 V	three-phase 4-wire, grounded neutral
19	Tunnel lighting	277 V	single-phase

1

2 Maintenance of Infrastructure and Rolling Stock Maintenance Facilities Special Equipment– The  
3 rated voltages of the equipment shall be as follows:

**Table 17-3: Rated Voltages for Yard and Shops Special Equipment**

Item No.	Equipment	Electrical Requirements	
1	20 ton hydraulic press	480 V	three-phase
2	89 kN overhead bridge crane	480 V	three-phase
3	Abrasive Blast	480 V	three-phase
4	Air compressor	480 V	three-phase
5	Analog converter	120 V	single-phase
6	ATC tester	120 V	single-phase

7	Audio visual equipment for emergency exit stairs in places of refuge	120 V	single phase
8	Axle cleaner	480 V	three-phase
9	Axle lathe	480 V	three-phase
10	Band saw	480 V	three-phase
11	Battery bench	120 V	single phase
12	Battery charger	480 V	three-phase
13	Belt grinder	480 V	three-phase
14	Belt sander	240 V	single-phase
15	Bench grinder	120 V	single-phase
16	Buffer grinder	480 V	three-phase
17	Card test console	240 V	three-phase
18	Chop saw	480 V	three-phase
19	Cleaning station	480 V	three-phase
20	Cleaning tank	480 V 240 V 120 V	three-phase three-phase single-phase
21	Coolant system	120 V	single-phase
22	DC TIG/MJG welder with trailer cart	480 V	three-phase
23	De-ionizer	120 V	single-phase
24	Double-ended grinder/dust collector	120/240 V 120V	single-phase single-phase
25	Drill bit grinder	480 V	three-phase
26	Drill grinder	480 V	three-phase
27	Drill sharpener	240 V	single-phase
28	Dust collector	480 V	three-phase
29	Electrode oven	480 V	three-phase
30	Electronic repair areas (electrical requirements will change, depending on systems purchased)	120 V	single phase
31	Engine lathe	480 V	three-phase
32	Event recorder	120 V	single-phase
33	Freon collection system	120 V	single-phase
34	Glass bead	120 V	single-phase
35	Grit blast	120 V	single-phase
36	"H" frame press	480 V	three-phase
37	Hazardous materials storage building	208/120 V	single-phase
38	High pressure washer	480 V	three-phase
39	High-pressure hot water generator cleaner	480 V	three-phase
40	Hi-pot tester	480 V	three-phase
41	Hi-volt power supply	480 V	three-phase

42	Honing machine	480 V	three-phase
43	HVAC	240 V	three-phase
44	Hydraulic pinch-nibbler	480 V	three-phase
45	Hydraulic press	120 V	single-phase
46	Jet spray washer	480 V	three-phase
47	Jib crane	480 V	three-phase
48	Jockey pumps	480 V	three-phase
49	Lathe	480 V 120 V	three-phase single-phase
50	Logic control	240 V	single-phase
51	Magna flux machine	480 V	three-phase
52	MOI building	480 V 120 V	three-phase single-phase
53	Master controller	120 V	single-phase
54	Material lift	480 V	three-phase
55	Metal shear	240 V	three-phase
56	Milling machine	480 V	three-phase
57	Mini fume extractor	120 V	single-phase
58	Oven	480 V	three-phase
59	Overhead bridge crane	480 V	three-phase
60	Overhead crane	480 V	three-phase
61	Paint booth	480 V	three-phase
62	Parts washer	120 V	single-phase
63	Pedestal grinder	480 V	three-phase
64	Pipe threading machine	208 V	single phase
65	Portable electrostatic fume processor	480 V	three-phase
66	Power brake	480 V	three-phase
67	Propulsion	120 V	single-phase
68	Radial drill	480 V	three-phase
69	Repair booth	480 V	three-phase
70	Repair stand	120 V	single-phase
71	Roller	480 V	three-phase
72	Sander	240 V	single-phase
73	Saw band	120 V	single-phase
74	Shear	480 V	three-phase
75	Shop drill press	480 V	three-phase
76	Small grinder	120 V	single-phase
77	Small parts paint booth	480 V	three-phase
78	Solvent rinse station	120 V	single-phase



79	Spot welder	208 V	single-phase
80	Surface grinder	480 V	three-phase
81	Table saw	480 V	three-phase
82	Test bench	120 V	single-phase
83	Test rack	120 V	single-phase
84	Test stand	240 V	single-phase
85	Tester	120 V	single-phase
86	Tire press	480 V	three-phase
87	Toilet control	120 V	single-phase
88	Toll and cutter grinder	480 V	three-phase
89	Tool grinder	480 V	three-phase
90	Train wash building	480 V	three-phase
91	Train wash building	120 V	single-phase
92	Trash compactor	Dependent on equipment purchased	
93	Truck hoists	480 V	three-phase
94	Truck repair hoist	480 V	three-phase
95	Truck unloading lift or dock leveler	480 V	three-phase
96	Turntables	Manually operated	
97	Ultrasonic cleaner	480 V	three-phase
98	Vehicle hoist system	480 V	three-phase
99	Vertical band saw	480 V	three-phase
100	Vertical mill	480 V	three-phase
101	Washer	240 V	three-phase
102	Welder	208 V	single-phase
103	Welding bench	480 V	three-phase
104	Wheel truing machine	480 V	three-phase
105	Wheel/axle press	480 V	three-phase
106	Wheel/hub boring machine	480 V	three-phase
107	Winding Machine	120 V	single-phase
108	Workbench	120 V	single-phase

## 17.7 New Electrical Utility Services

### 17.7.1 General Requirements for New Electrical Utility Services

The design and construction of new electrical utility services shall comply with the requirements of the local electrical utility service provider. Refer to the *Utility* chapter for additional requirements

The service entrance location for the FPS shall be determined concurrently with the development of design. Space-planning documents and standards for equipment furnished by the local electrical utility service provider shall be incorporated into the design. Locations for transformers, vaults, meters, and other electrical utility service items must be coordinated with the architectural design of the facility to avoid detracting from the facility's appearance.

The local electrical utility service provider shall own and maintain all local electrical utility service provided equipment and cables.

The location of the service entrance within the Authority's facilities and/or trackside equipment enclosures shall be assumed to be available to the local electrical utility service provider's personnel 24 hours a day/365 days a year. Accordingly, the location of the service entrance needs to consider the security guidelines that relate to limited access to secured areas/facilities by third parties. Where possible locate equipment requiring periodic inspection by the local electrical utility service provider at the perimeter of the Authority's facilities and/or trackside equipment. Refer to *TBD* chapter for additional security requirements with regards to limited access to secured areas.

Where the normal and backup power services to a facility are supplied by the local electrical utility service provider, the following requirements shall apply:

- The services shall be independent (i.e., originate from separate utility transformer/switchgear banks, routed on separate overhead or underground infrastructure for the majority of the route (between distribution network and the Authority's right-of-way).
- One of the power services shall be rated (e.g., Block 50) such that it is still energized and available to the Authority during roaming blackouts and/or load shedding by the electrical utility service provider.

Where adjacent facilities necessitate separate electrical utility services from the local electrical utility service provider, the electrical utility services shall be independent (i.e., originate from separate utility transformer/switchgear banks, routed on separate overhead or underground infrastructure for the majority of the route [between distribution network and the Authority's right-of-way]).

Electrical utility services for the Authority's facilities and/or trackside equipment enclosures shall be connected to the local electric utility service provider's nearest and available electrical line(s), which have adequate capacity for the anticipated electrical loads and meet the previously noted requirements.

The design of the FPS protection scheme shall prevent the "paralleling"/interconnection of the electrical utility services to other power sources at the Authority's facilities and/or trackside enclosures.

Refer to the *Utilities* chapter for additional requirements.

### 17.7.1.1 Tunnel Ventilation System

In the case of facilities and/or structures that house tunnel ventilation fan motors, the electrical utility services shall have sufficient reserve capacity to facilitate the designed operational modes of the fan motors during normal and emergency modes of operation. Refer to the *Mechanical* chapter for additional mechanical and operational mode requirements.

### 17.7.2 Metering Requirements

The design, manufacture and installation of the utility metering section shall be in accordance with the standard specifications of the Electric Utility Service Equipment Requirements Committee and the local electrical utility service provider.

#### 17.7.2.1 Facilities and Trackside Equipment Enclosures

Three-phase metering-power quality meters shall be provided with the following metering features: A, V, W, Wh, Wcost, Var, Varh, VA, Vah, Hz, and PF in true RMS and/or displacement (fundamental) qualities.

### 17.7.3 Concessionaires

Concession areas shall be provided with meter base, main circuit breakers, and panel boards for measurement of power consumption of future private concessions.

Electrical service for concession areas shall be independent of the electrical services to the facilities and shall be connected to the local electric utility service provider's nearest and available electrical line(s).

## 17.8 Emergency Power Supply Systems

Emergency power-supply system design shall conform to the NEC, CEC, NFPA 101, NFPA 110, NFPA 111, NFPA 130, and IEEE 446. Additionally the design and as-built installation of the emergency power supply system shall comply with all applicable codes, standards and ordinances as required by the local fire department.

Emergency power supply system electrical infrastructure shall include all equipment, wires, cables, conduit, and tray connected to an emergency standby generator, UPS and/or dc battery system, and associated with the tunnel ventilation system. This infrastructure shall be UL listed and certified as suitable for use in an emergency system.

The following minimum fuel/battery storage capacities shall be applied to the CHSTP:

**Table 17-4: Minimum Fuel/Battery Storage Capacities**

Emergency Standby Generator (fuel)	30 hours
------------------------------------	----------

Uninterruptable Power Supply [ATC/Communications/etc.] (battery)	4 hours
Uninterruptable Power Supply [Lighting] (battery)	90 minutes
DC Battery System (battery)	4 hours

Conductors associated with the emergency power supply system (including, but not limited to, from the power source to the switchboard/unit substation/panelboard and “downstream” to the field devices, including interconnections between field devices) shall be run in separate conduit, tray, boxes and so forth, independent of conductors associated with other electrical loads (e.g., non-essential).

Fuel tank and battery placement shall comply with the requirements specified in applicable building codes.

Battery chargers (associated with UPS and/or dc battery systems) shall be interlocked with the mechanical ventilation system for the respective battery room to prevent battery charging without ventilation.

### 17.8.1 Emergency Power Supply Equipment

#### 17.8.1.1 Emergency Standby Generators

Emergency standby generators shall be diesel-engine driven; output 480/277 Volts, 60 Hz, 3 phase and 4 wire; and be designed to start-up by manual initiation and automatically.

Emergency standby generators and associated fuel tanks shall be located at street level with roadway access.

#### 17.8.1.2 Uninterruptible Power Supplies

Separate UPS equipment shall be provided for each of the following electrical loads:

- Emergency Lighting
- Core Systems (i.e., TES, ATC and Communications)
- Facility Communication Systems and other facility essential electrical loads
- Management Information Systems

#### 17.8.1.3 DC Battery Systems

Separate dc battery system equipment shall be provided for each of the following electrical loads:

- Core Systems (i.e., TES, ATC and Communications)
- Facility Communication Systems and other facility essential electrical loads

## 17.9 Sustainable Power Sources

### 17.9.1 Photovoltaic and Wind Systems

The use of sustainable power sources is encouraged whenever feasible. This is especially advantageous for 'off-grid' trackside facility/equipment applications where bringing in a utility electrical service would be costly or the load does not justify it. Even when utility service would be economically feasible, photovoltaic or wind options should be considered.

These systems, if used, shall meet the following requirements:

- Photovoltaic panels or wind turbines shall be provided with substantial and efficient mounting assemblies and located to maximize energy capture. Complicated solar-tracking mechanisms for photovoltaic arrays shall be avoided unless they can be demonstrated to offer substantial operational or efficiency advantages that overcome higher maintenance costs.
- Photovoltaic panels and wind turbines shall be sunlight and weather-resistant and suitable for remote, unattended locations.
- Distribution system and loads shall be designed to operate on 12, 24, or 48 VDC power supply unless it is infeasible for the load to be served. This will avoid the added complexity and maintenance of an inverter power supply.
- System shall include a charge controller to manage battery charging and include appropriate circuit isolating disconnects for maintenance of various components.
- Batteries shall consist of maintenance-free, valve-regulated, pocket-plate nickel-cadmium cells sized to maintain power to equipment for the expected period of time that solar or wind power will not be available.
- System enclosures shall be designed to protect components from vandalism and weather and also protect from temperature extremes. Enclosures shall be ultraviolet ray-stabilized, fiberglass reinforced epoxy or stainless steel to resist corrosion.

## 17.10 Facility Power System Equipment

### 17.10.1 General

FPS equipment and conductors, providing electrical services to critical and essential electrical loads, shall continue to provide power to these loads during incidents (e.g., fires, seismic, etc.) as long as the local electric utility services are available and the minimum fuel and/or battery storage capacities are not exceeded.

FPS equipment providing power to sensitive electronic equipment (e.g., Communications, ATC, etc.) shall have built in protection against lightning surges introduced on the incoming electrical services.

Equipment monitored and/or controlled remotely during normal operations shall be capable of being operated locally when a failure occurs with infrastructure/equipment supporting the remote operations.

Individual pieces of equipment and/or integrated power centers are acceptable to the Authority.

No polychlorinated biphenyl (PCB), asbestos, or other materials with known toxicity posing health hazard upon handling and human contact, shall be used in any parts or products of the FPS equipment.

The design of FPS equipment and/or electrical rooms shall meet the specified maximum allowable noise levels (as specified below). Equipment design, enclosures with acoustic mitigation capability, and other measures, as appropriate, shall be employed to achieve compliance with the audible noise level criteria. The noise level outside the electrical rooms, measured at a distance of 15 feet from any equipment during operation at 100 percent electrical room load, shall not exceed 55 dB.

The noise level inside the pre-engineered enclosures, measured at a distance of 5 feet from any equipment during operation at 100 percent electrical load, shall not exceed 60 dB. Equipment height shall not exceed 9 feet. Control and indicator devices, and equipment that needs to be accessed by personnel shall be mounted at a height not to exceed 6 feet. Re-design equipment if the above noted constraints cannot be met without compromising the integrity of the standard design or clearance requirements.

The surfaces of the equipment shall be smooth, free of gaps, burrs, sharp edges, wrinkles, waves, and blemishes, and shall have uniformly rounded corners with constant radii.

Unless specified otherwise, all outdoor hardware such as hinges, bolts, nuts and screws shall be stainless steel.

Full height doors for equipment such as switchgear, transformers and cabinets shall have 3-point locking latches and 1/4-turn "tee" handles. For doors 4 feet or less in height, 2-point locking latches shall be permitted. Each door shall be provided with a cylinder lock or provision for padlock.

Openings and mounting holes for front mounted indicating and control devices shall be cut out, drilled, or punched without marring or distorting the exposed finished surfaces.

Surfaces to be welded shall be clean and free from dirt, rust and other foreign materials in accordance with industry standards. All welding shall be performed by electric arc method or resistance spot welding and the process shall conform to the standards of the American Welding Society.

Equipment openings shall be provided with 1/2-inch screen mesh to preclude the accidental entry of hand tools and other equipment.

1 Equipment enclosures shall be provided with ground lugs to meet CEC grounding  
2 requirements.

3 Pre-engineered and/or equipment enclosures shall have sufficient structural reinforcement to  
4 ensure that the surfaces remain plane and plumb and to provide necessary strength and rigidity  
5 during shipment, installation, and operation.

6 Equipment enclosures shall be designed to permit lifting by jacks or slings.

7 Devices not on the front panels shall be mounted on the surface of internal panels, accessed by  
8 opening the hinged front panel.

9 Each device shall be located and wired so that it can be serviced without removing the device or  
10 other devices.

11 The devices shall be plumb and square with the lines of the panel, and mounted as  
12 recommended by the manufacturer.

#### **17.10.2 High Voltage Switchgear**

13 The HV switchgear assembly and all components shall be designed in accordance with the  
14 latest applicable standards of ANSI, NEMA and the IEEE; and comply with the engineering and  
15 operating standards requirements of the local electrical utility service provider (e.g., Section 400  
16 of the Electric Utility Service Equipment Requirements Committee book). The HV switchgear  
17 assembly shall have sufficient short circuit and impulse withstand capability to operate safely.

18 Each switchgear assembly shall include a metering compartment consisting of voltmeter,  
19 ammeter, kilowatt meter, kilowatt-hour meter with peak demand indicator and pulse output,  
20 and kilovolt-ampere reactive hour meter with required switches, fuses and sensors.

21 Circuit breakers in switchgear shall operate with 125-VDC control power. The intent of the  
22 design shall be to keep the number of separate battery banks to a minimum and to remain in  
23 compliance with local electrical utility service provider requirements.

#### **17.10.3 High Voltage Transformers**

24 HV transformers shall be dry-type when used indoors and dry or liquid filled type when used  
25 outdoors. Construction shall be in accordance with applicable sections of ANSI and NEMA  
26 standards.

#### **17.10.4 High Voltage Motor Control Centers**

27 TBD

### 17.10.5 Unit Substation

Each electrical utility service entry distribution system shall contain a primary-fused disconnect switch and a medium voltage to 480/277 V dry-type cast resin transformer. The 480 V switchgear shall consist of main breakers, tie breakers, and switchboards with distribution feeder breakers. The tie breaker shall close automatically after either one of the main breakers opens on undervoltage. The tie breaker shall open after restoration of normal voltage to the main breaker. The transformers shall be equipped with forced-air cooling under thermostat control and shall be provided with a two-stage over-temperature device with alarm contacts.

The unit substation shall be designed to carry the forced-air-cooled rating of a single transformer, or the sum of the full load losses of both transformers multiplied by 0.80. Each unit substation 480 V switchgear will have approximately 20 percent spare spaces bussed and ready to accept future breakers, unless such provision requires an additional section, in which case space shall be provided for the additional section.

### 17.10.6 480/277 V ac Switchboards/Switchgear

Switchboards/switchgear shall consist of circuit breakers positioned in a metal-enclosed, free-standing enclosure together with all associated meters, relays, instrument transformers, heaters, and all other accessories as required to provide complete and operable switchboards/switchgear.

Where applicable, switchgear shall be arranged for attachment to a transformer enclosure as a component of a unit substation

### 17.10.7 Panelboards (480/277 V ac, 208/120 V ac, 250/125 V dc)

Each electrical load class shall be fed from its own panelboard and separate panelboards shall be provided for lighting applications.

Panelboards shall be circuit breaker type. The interrupting rating of the circuit breakers shall be selected so as to safely interrupt the calculated fault currents.

Panelboards located within finished spaces shall be flush mounted; panelboards located within unfinished spaces may be either flush or surface mounted.

Lighting panelboards shall contain addressable circuit breakers that are compatible with and controlled by a lighting programmable controller.

Panelboards shall be placed near or central to their loads. They shall be located in the electrical rooms or suitable ancillary rooms provided they are easily accessible to maintenance personnel.

### 17.10.8 Low Voltage Transformers

Transformers shall be two winding, LV, dry type suitable for indoor application and step down of the primary voltage (480 V ac) to a 208 Y/120 V ac secondary level.



### **17.10.9 Low Voltage Circuit Breakers/Switches**

#### **17.10.9.1 480 V ac Circuit Breakers/Fused Disconnect Switches**

- 1 Heavy duty circuit breakers or fused disconnect switches shall be provided where required by
- 2 the NEC and/or CEC as a means to disconnect equipment from its feeder, when the equipment
- 3 is not within the sight of either the feeder breaker or motor controller, or where it is
- 4 advantageous to separate feeder from electric loads to be supplied by others.
- 5 A circuit breaker and/or fused disconnect switch shall be provided in the same room as a
- 6 manual/automatic transfer device for each power source on the line side of the equipment.

#### **17.10.9.2 480 V ac Manual Transfer Device**

- 7 A manual transfer device shall be used where there is more than one power source feeding the
- 8 same electric load, and one of the power sources originates from a portable generator set. It
- 9 shall be designed such that only one power source can supply power without the possibility of
- 10 paralleling the two sources.

#### **17.10.9.3 480 V ac Automatic Transfer Device**

- 11 An automatic transfer device (i.e., circuit breaker) shall be used where there is more than one
- 12 power source feeding the same electric load. It shall be designed such that only one power
- 13 source can supply power without the possibility of paralleling the two sources. These shall be
- 14 electrically operated. All automatic transfer devices shall have a manual bypass feature.

### **17.10.10 Motors, Starters and Controls**

- 15 In general, motor control centers with motor-circuit-protector combination starters shall be used
- 16 for 480-volt motors. Control centers shall be equipped with either a main circuit breaker or a
- 17 fused circuit breaker with main busses adequately braced to withstand the available short-
- 18 circuit current. Individually mounted motor circuit protector combination starters may be used
- 19 where they can be located in a physically secure area.

### **17.10.11 Receptacles**

- 20 In public areas, receptacles shall be spaced not more than 100 feet apart and shall be located
- 21 flush in a wall or column. The circuit shall be energized only for operation by authorized
- 22 personnel. In public areas, no more than six outlets shall be connected to a branch circuit.
- 23 In non-public areas and non-office environments, receptacles may be surface mounted and shall
- 24 be spaced not more than 20 feet apart and shall be supplemented where needed for fixed
- 25 equipment. Receptacles in non-public areas in facilities shall be of the duplex 20-amp minimum
- 26 rating, grounded type. In ancillary or service areas, no more than 5 duplex outlets shall be
- 27 connected to a branch circuit.
- 28 Receptacles in public and trackside locations shall be ground-fault circuit interrupters (GFCIs)
- 29 and weatherproof. A Ground-Fault Circuit Interrupters (GFCI) receptacle shall be used where

- 1 ground fault protection is required and use of a GFCI circuit breaker for the receptacle circuit  
2 has not been provided.
- 3 Adjacent receptacles shall not be placed on the same feeder/branch circuit.
- 4 All other receptacles shall be rated for the intended working environment and comply with the  
5 requirements of the NEC and CEC.
- 6 The requirements for receptacles at TES, ATC, and Communications facilities shall be as  
7 specified elsewhere in the design criteria and/or contract documents.

## 17.11 Specialized Electrical Equipment and/or Loads

### 17.11.1 Rolling Stock Maintenance Facilities

#### 17.11.1.1 Facility Power Infrastructure

8 The design of the FPS at the RSM Facilities shall include electrical infrastructure (located below  
9 car floor level) that permits the power up of the stationary trainset onboard auxiliary systems  
10 when the Overhead Contact System (OCS) is de-energized. The electrical infrastructure shall be  
11 provided on all tracks within the noted facilities and and/or at storage tracks. The following  
12 features shall be included in their design:

- 13 • Utilization voltage shall be 480 V ac, 3 phase, 60 Hz.
- 14 • At least 4 connection points (spaced at intervals so as to permit connection to the rolling  
15 stock) shall be provided along each track.
- 16 • A local disconnection device shall be provided with each connection point to provide local  
17 isolation. The design of the disconnection device shall include visual indication of  
18 energization/de-energization and an interlock.
- 19 • Adjacent connection points and associated disconnection devices shall not be placed on the  
20 same feeder/branch circuit.
- 21 • Each extension cable, associated with each connection point, shall be sufficiently long  
22 enough to connect to rolling stock on the adjacent track(s).

23 Refer to the *Rolling Stock-Core Systems Interfaces* chapter for rolling stock interface requirements.

#### 17.11.2 Elevators

24 This equipment is considered essential during an emergency condition, and shall be fed from  
25 the nearest generator bus. The electrical power supply to the equipment shall consist of a  
26 nominal 480 V, three-phase, 60 Hz supply terminated in a fused disconnect switch in the  
27 elevator machine room.

1 A 208Y/120-volt, 3-phase, 20-amp, originating from the generator bus, shall be provided for  
2 hoistway lighting and receptacles in accordance with the applicable code requirements.

3 A 120-volt, 20-amp service shall be provided for cathodic protection circuit in the elevator  
4 machine room.

5 GFCI duplex weatherproof, 20 ampere, 120 V receptacles shall be provided in elevator pits.

### **17.11.3 Escalators**

6 The power supply for each escalator shall consist of a 480-volt, 3-phase supply terminated in a  
7 circuit breaker in the escalator machine space. For an escalator with a rise of 20 feet or less, the  
8 power supply shall be sized at 1 hp per foot of rise. For an escalator with a rise greater than 20  
9 feet, the power supply shall be sized at  $(20 + 1.5 (H - 20))$  hp, where H is the total escalator rise in  
10 feet.

11 A 208Y/120-volt, 3-phase, 20-amp service is required for each escalator for maintenance lights,  
12 light switch, and receptacle in the machine space. This service is not to be used for emergency  
13 power.

14 Escalator control connections to exit signs and roll-down gates shall be provided. Refer to *TBD*  
15 chapter for the local control and indication requirements.

16 GFCI duplex weatherproof, 20 ampere, 120 V receptacles shall be provided in escalator upper  
17 and lower pits.

### **17.11.4 Sump Pumps**

18 Each pump station shall be equipped with 2 or more pumps. These shall be fed from different  
19 buses (i.e., normal and backup) nearest to the pump station. Only 1 pump at a time may be out  
20 of service except during an area-wide power outage.

### **17.11.5 Tunnel Ventilation System**

21 Facilities (e.g., portal facilities, mid-tunnel ventilation structures) that contain a tunnel  
22 ventilation system shall have the following key features incorporated into the FPS design for  
23 these facilities (in addition to complying with the requirements of NFPA 130):

- 24 • An HV transformer per power source to transform the incoming power source to the  
25 utilization voltage (e.g., 4160 V ac)
- 26 • Main and tie circuit breaker arrangement to permit the tunnel fan motors to be powered by  
27 either incoming power source.
- 28 • Two feeder circuit breakers to the downstream transformation and distribution equipment  
29 to permit maintenance/testing of a feeder circuit breaker, while still keeping the  
30 downstream electrical loads energized

- Tunnel fan motors, that are used to ventilate the same tunnel, shall be directly fed from different 4.16 kV buses

#### 17.11.6 Ticket Vending Machines and Fare Gates

Each passenger station is equipped with ticket vending machines, fare gates and an associated ticket vending machine (TVM) control system. Two under-floor ducts for power and signal circuits shall be installed underneath the fare gate and ticket vending machines. Adequate junction boxes shall be provided wherever the raceways change direction. All other raceways for signal and power circuits shall be metallic conduits.

The power supply to TVM equipment shall be fed from the panelboard located in the nearest electrical room. Adjacent TVM equipment shall not be placed on the same feeder/branch circuit.

Refer to the *Stations* chapter for additional requirements pertaining to TVM equipment and fare gates.

#### 17.11.7 Panels

##### 17.11.7.1 Local Control Panel

A Local Control Panel (LCP) shall be provided at all electrical rooms where HV switchgear, HV transformers, HV motor control centers, unit substations, 480 V ac switchboards and so forth are located.

The design of the LCP shall include the following features:

- A mimic bus depicting the electrical interconnection of the equipment
- Visual indication on the position of circuit breakers and switches
- Control switches to operate circuit breakers and switches
- A touch screen annunciation panel, with pull-down menus, that permits operations and maintenance personnel to ascertain the status of all equipment (noted in Section 17.17, Electrical Discrete and Serial I/O Point Requirements) and undertake diagnostic checks

The LCP design shall include a local and remote control switch, which, during normal operation, shall be set to remote—thereby permitting the Operations/Regional Control Center (O/RCC) to monitor and control the local FPS equipment. Once the control switch is moved to the local position, the O/RCC display shall note the local control status, and the O/RCC shall not be permitted to operate the associated FPS equipment.

##### 17.11.7.2 Emergency Walkway Lighting Control Panels

An Emergency Walkway Lighting Control Panel (EWLCP) shall be provided at an electrical room that provides feeder/branch circuits to emergency walkway light fixtures.

The design of the EWLCP shall include the following features:

- A mimic bus depicting the electrical interconnection of the equipment and trackside distribution to light fixtures (include depiction of extent of lighting circuit and which track it applies to)
- Control switches to operate circuit breakers and switches

The requirements for control panels associated with access control, fire detection, tunnel ventilation and so forth are specified elsewhere in the design criteria and/or contract documents.

#### **17.11.8 Interface Termination Cabinet**

The interface wiring between the FPS and the SCADA and/or BAS systems (refer to Section 17.17, Electrical Discrete and Serial I/O Point Requirements) shall reside in an Interface Termination Cabinet (which will act as a demarcation between the communication and electrical disciplines/trades) located at each electrical room, within 10 feet-0 inches of an entrance doorway.

All functions that are monitored and/or controlled shall be uniquely identified at the Interface Termination Cabinet.

#### **17.11.9 Vehicle Battery Charging Facilities**

Vehicle battery charging facilities shall be provided at passenger stations, and rolling stock and infrastructure maintenance facilities

These facilities shall meet the following requirements:

- The power source to the vehicle battery facilities shall be separately revenue metered from the adjacent facility.
- The distribution cables, between the power source and each vehicle battery charging facility, shall be routed via an underground ductbank and structures. No other FPS or other system cables shall be routed in the ductbank.
- The placement of the vehicle battery charging facilities and routing of associated ductbanks shall be coordinated with site lighting, security/communication field devices, other subgrade facilities, site-work and landscaping.
- Enclosures shall be designed to protect components from vandalism and weather and also protect from temperature extremes. Enclosures shall be ultraviolet ray-stabilized, fiberglass reinforced epoxy or stainless steel to resist corrosion.
- At least TBD percent of parking spaces at each passenger station shall be equipment with vehicle battery charging facilities.

- At least TBD percent of parking spaces at each RSM facility shall be equipped with vehicle battery charging facilities.
- At least TBD percent of parking spaces at each MOI facility shall be equipped with vehicle battery charging facilities.
- A vehicle battery charging facility shall be provided to each designated parking space (i.e., it shall not be shared between adjacent parking spaces).

#### 17.11.10 Specialized Equipment for First Responders (within Tunnels)

The FPS shall be adequately sized and include provisions to permit connection of first responders' equipment at each portal, emergency access or egress shaft, cross passages throughout each tunnel and elsewhere. Refer to the *TBD* chapter for additional requirements.

The point of connection for first responders' equipment shall be designed to be electrically and mechanically compatible with the standardized equipment used by the first responders.

The following requirements shall apply:

- Conductors to any two adjacent points of connection shall originate from different panelboards.
- Panelboards that distribute power to points of connection shall be electrically connected to the normal, backup and emergency power supply sources.

### 17.12 Lighting System Requirements

The following general requirements shall be incorporated in the lighting design for each area:

- The lighting system design shall be designed to be relatively simple and minimize initial capital costs, as well as frequency and expense of maintenance.
- Lighting shall be designed to be functional, yet complementary to those other aesthetic features in the space that provide an atmosphere of relative comfort, pleasantness and cleanliness of surroundings, and a sense of personal safety and security.
- Adequate lighting levels, uniformity ratios, contrast, and other visibility attributes necessary to stimulate productivity, facilitate the use of facilities by patrons or the useful completion of tasks in a timely yet safe manner, and maintain the appropriate surveillance levels under all ambient lighting conditions shall be provided.
- Lighting designs shall be energy efficient and comply with California Code of Regulations Title 24, CBC Title 24, and all IESNA Recommended Practices, ASHRAE, and ANSI standards.

- 1 • Lighting shall be designed to satisfy security requirements and to provide a pleasant and  
2 bright environment. Refer to the *TBD* chapter for additional requirements
- 3 • Lighting fixture locations shall permit easy accessibility for re-lamping and periodic  
4 cleaning.
- 5 • Any required illumination shall be arranged and wired so that the failure of any single  
6 lighting unit or circuit shall not leave the area in total darkness.
- 7 • Lighting shall be designed to avoid “spill” light, objectionable glare, and light trespass.
- 8 • Lighting fixtures shall be vandal-resistant in spaces accessible to patrons or to the general  
9 public.
- 10 • Lighting designs shall effectively control glare or other extraneous reflections in the visual  
11 field.
- 12 • Lighting shall meet or exceed recommendations in this guide in accordance with all  
13 applicable IESNA Standard Practices.
- 14 • Manufacturers of lighting fixtures, poles, and accessories shall have a minimum of 10 years  
15 of reputable performance in the industry.

#### **17.12.1 Quality of Illumination**

16 Lighting designs shall be free from distracting and uncomfortable glare; care shall be exercised  
17 to prevent specular reflection on signage, direct glare from exposed lamps, high brightness  
18 areas of individual fixtures and reflections in glazing or other specular surfaces.

19 Uniformity ratios shall not exceed the recommended levels as directed by IESNA.

20 The color temperature of light fixtures shall be within the ranges specified in the contract  
21 specifications. Lamps of the same type within each area of illumination shall have the same  
22 color temperature. In areas where different types of light fixtures are required, the color  
23 temperature of each type shall be matched as close as practicable to ensure uniformity of color.

24 Special care shall be taken to avoid objectionable glare to streets, tracks, and adjacent properties.  
25 Light fixtures shall be positioned to minimize spill light. Light fixtures shall be provided with  
26 internal shielding and located to prevent spill light and glare in the direction of adjacent  
27 properties. Light fixtures within 50 feet-0 inches of elevated freeways and overpasses shall be  
28 designed to provide absolute cutoff in the direction of moving traffic.

#### **17.12.2 Facility Illumination Levels**

29 The average maintained illumination levels for various areas shall be provided as shown in  
30 Appendix 17.A (at the end of the chapter). The normal method for calculating these levels shall  
31 be in accordance with the IESNA Lighting Handbook with modifications or other requirements  
32 stated herein. The “point to point” method of computing illumination shall be used to verify  
33 and substantiate illumination and uniformity lighting levels.

### 17.12.3 Emergency Lighting and Exit Signs

Emergency lighting design shall conform to the NEC, CEC, NFPA 101, NFPA 130, and IESNA Lighting Handbook.

Emergency lighting shall be designed to maintain a minimum of 10 lux (1 fc) horizontal illumination at the finished floor elevation throughout the areas of pedestrian egress.

Exit lighting fixtures/signs shall be provided to illuminate the designated egress passageways, stairways, and the entrances/exits to stairways. Exit lighting fixtures shall be wall or bracket mounted at a mounting height visible to all occupants within the occupied spaces. Exit lighting fixtures/signs shall be separately wired from other electrical loads (to the emergency panelboard) and shall be connected to the UPS system.

Emergency lighting for stairways and escalators shall be designed to emphasize illumination on the top and bottom steps or landings. All escalator steps newel and comb lighting shall be on emergency feeder circuits in accordance with NFPA 130. A minimum of 10 lux (1 fc) shall be provided.

All lighting fixtures in the Operation/Regional Control Center, auxiliary spaces housing equipment to operate facilities and/or revenue services, and stairways shall be connected to the UPS system.

Emergency lighting fixtures and exit lights/signs in underground occupancies shall be located so as to minimize the possibility of being obscured by stratified smoke in a fire.

### 17.12.4 Circulation Roadway Lighting and Roadway Sign Lighting (within the Authority's Right-of-Way)

A uniform nighttime illumination shall be provided on the circulation roadways to enable motorists to quickly, distinctly, and comfortably view the roadway alignment, vehicles, obstacles, or obstructions at extended distances ahead, even during inclement weather. The lighting design shall comply with the latest applicable codes and IESNA Recommended Practices.

Lights shall be located above the sign face such that they are aimed down to minimize sky glow. The minimum lighting level for roadway signs shall be 280 lux (28 fc). The average to minimum uniformity ratio of the levels of illumination on the sign face shall be 6:1. The maximum illumination gradient produced on the sign face shall be 1.2:1.

### 17.12.5 Street and/or Motorway Lighting

Street and/or motorway lighting shall conform to the standards and/or design requirements of the local and/or State jurisdiction. Refer to the *Civil* chapter for additional requirements.



## 17.12.6 Parking Structure and/or Lot Lighting

### 17.12.6.1 Parking Structures (Not Applicable)

### 17.12.6.2 Parking Lot Lighting

- 1 Lighting at surface parking lots shall meet the recommendations in the latest edition of the
- 2 IESNA RP-20, *Lighting for Parking Facilities*.
- 3 Lighting poles shall be located inside of curbs, on traffic islands and walks, and along parking
- 4 lot perimeter. Placement shall present a minimum obstruction to movement and parking of
- 5 cars. Where poles are placed near automobile traffic or parking, they shall be protected from
- 6 physical damage by a concrete base of at least 2 feet-0 inch–diameter, which extends not less
- 7 than 2 feet-6 inches above grade.
- 8 Light pole positioning shall be as required to meet lighting uniformity ratios, be coordinated
- 9 with the proposed landscaping scheme and prevent off-property spill light.

### 17.12.7 Elevator/Escalator Pit Lighting

- 10 A compact fluorescent lighting fixture in an IP 65-rated enclosure with a wire guard shall be
- 11 provided at each elevator and escalator pit location, meeting all recommended illumination
- 12 levels in Table 17.A.

### 17.12.8 Lighting Control System

- 13 Artificial lighting shall be controlled automatically where practical by selecting from among
- 14 various lighting control devices. The most useful controls for increasing lighting energy
- 15 efficiency are timers, dimmers, photocells, and occupancy sensors.
- 16 Control strategies shall be selected based upon the following types of lighting situations:
- 17
  - Lighting shall be designed to continuously illuminate an area during a combination of hours
  - 18 of darkness and hours of operation. Examples of continuous lighting could be station stops,
  - 19 platforms, vehicle approaches, and parking lots. Suitable controls for continuous lighting
  - 20 are timers and photocells.
  - 21
    - Standby Lighting is similar in design to continuous lighting, except not continuously
    - 22 operated. Standby lighting is turned on automatically when activity is detected in the area
    - 23 or manually as necessary to occupy a space. Standby lighting is suitable only for “instant
    - 24 on” lamps. Examples of standby lighting could be utility closets, offices, or restrooms.
    - 25 Suitable controls for standby lighting include discrete “on/off” switches, occupancy sensors,
    - 26 and dimmers.
- 27 Refer to the *Stations* chapter for additional requirements.

## 17.13 Specialized Lighting

### 17.13.1 Exterior Lighting at Heavy Maintenance Facility, Terminal Layup/Storage, Maintenance Facilities, and Maintenance of Infrastructure Facilities

- 1 Lighting shall provide sufficient illumination to permit operations and maintenance activities to
- 2 be performed safely on a 24-hour, 365-day-a-year basis.
- 3 Light fixtures, towers, poles or stanchions shall be designed and located to maximize
- 4 maintenance accessibility, minimize shadows, glare to adjacent properties/neighborhoods, and
- 5 avoid interference with operations and other trackside infrastructure.
- 6 High mast poles used for area illumination shall be climbable and limited to 100 feet-0 inches.
- 7 High mast poles shall incorporate light fixture-lowering mechanisms.
- 8 The location of light fixtures, towers, poles or stanchions shall not interfere with operations,
- 9 OCS and/or other trackside Core Systems infrastructure or Rolling Stock movements.

### 17.13.2 Tunnel /Trench Lighting

- 10 Lighting shall be provided in all tunnels and conform to NFPA 101 and NFPA 130. Lighting
- 11 shall be provided in trenches, where the top of the trench wall exceeds 8 feet-0 inches above rail
- 12 level.
- 13 The lighting at walkway surfaces (including emergency exits, cross passages, thresholds,
- 14 stairways, walkway ramps, crosswalks and all other means of egress or walking surfaces) shall
- 15 not be less than 15 lux (1.5 fc) illumination.
- 16 The lighting at the trackway floor level shall not be less than 10 lux (1 fc) illumination in the
- 17 Interior Zone, 20 lux (2 fc) in the Transition Zone and 400 lux (40 fc) in the Threshold Zone.
- 18 During the nighttime hours, the illumination level should be 5 lux (0.5 fc) and 10 lux (1 fc) in the
- 19 Threshold Zone only.
- 20 The spacing of light fixtures inside tunnels and/or along trenches shall not be within the
- 21 annoyance range of stroboscopic flicker effect as defined in IESNA RP-22 Tunnel Lighting.
- 22 Walkway lighting fixtures shall be powered from two alternating electrical power sources (e.g.,
- 23 every other fixture powered from the same feeder circuit).

### 17.13.3 Exterior Stairway Lighting

- 24 The types of luminaires to be used are dependent on the location. Either pole mounted or wall
- 25 mounted luminaires shall be used whichever is more appropriate. Refer to the table in
- 26 Appendix 17.A for recommend illumination levels for stairways.

#### 17.13.4 Access Roadway Lighting (to Trackside Core System Facilities)

1 Not Used.

#### 17.13.5 Blue Light Stations

2 Blue light stations shall be provided at the locations as specified in the *TBD* chapter and be  
3 designed so as to conform to the requirements of NFPA 130.

4 Blue light stations and associated duplex outlets shall be supplied from alternate power sources,  
5 so that loss of power to a blue light station shall not result in power loss to adjacent blue light  
6 stations.

7 A 120 V ac duplex outlet shall be provided at each blue light station. No more than two outlets  
8 shall be fed from the same feeder circuit.

#### 17.13.6 Yellow Lights

9 Yellow light fixtures shall be installed above all tunnel emergency exit and cross passage  
10 doorways. Yellow lights shall have two lamps, each supplied from separate feeder circuits.

#### 17.13.7 White Lights

11 White light fixtures shall be installed above all fire isolation valves. White light fixtures shall be  
12 lit when the fire isolation valve is open.

#### 17.13.8 Line Storage, Tail Track, and Turnaround Lighting

13 Pathways for train operator access shall be illuminated to 25 lux (2.5fc) average at-grade with an  
14 average to minimum ratio not to exceed 3:1 for operations to be performed safely throughout a  
15 24-hour, 365-day-a-year period.

16 Lighting designs shall adopt mounting details as required to suit site-specific conditions. The  
17 location of light fixtures shall be selected to be easily accessible, to minimize shadows, and to  
18 not interfere with any OCS operations, and other trackside systems infrastructure and rolling  
19 stock movements.

20 The lighting shall permit the detection of trespassers, observation of switch points by a train  
21 operator, and observation of smoke emanating from a rolling stock fire.

22 The lighting design shall not have excessive spill light into surrounding areas.

#### 17.13.9 Vehicle Inspection Pit Lighting

23 Pits shall be provided with sufficient lighting to illuminate the floor and the underside of rolling  
24 stock (positioned over the pit) with a minimum of 50 lux (5 fc) in accordance with the IESNA  
25 Lighting Handbook.

#### **17.13.10 Security Lighting**

Lighting shall be provided such that all areas throughout the facility are illuminated to ensure against unwanted visitors. The fixtures shall be vandal resistant. Refer to Appendix 17.A for illumination levels.

#### **17.13.11 Obstruction Lighting**

Any structure that the Federal Aviation Administration (FAA) determines will impair the safety of air navigation in and around airports adjacent to the alignment shall be marked and lighted in accordance with the applicable FAA regulations.

#### **17.13.12 Light Pollution Mitigation**

The designer must design the exterior lighting using the latest IESNA-TM-11 documentation as a guideline to control the light trespass. All luminaires used shall have a U0 BUG rating (Full Cutoff) in accordance with the IESNA.

#### **17.13.13 Lighting Fixtures**

The luminaires for the project shall be constructed of the highest quality materials and designed to provide a minimum of 20 years of operation. All exterior luminaires, along with those interior luminaires that require an occasional wash down, shall be UL listed for Wet Locations and for Direct Spray. The chosen luminaires shall incorporate the most effective source for the application and shall be easily maintainable and removable without the use of tools. No interior luminaires shall be constructed using a fiberglass or polymer housing. All luminaires shall be cast or extruded aluminum with stainless steel hardware.

#### **17.13.14 Poles, Foundations, Bracket Arms and Miscellaneous Hardware**

All poles shall be galvanized round-tapered steel constructed to provide the ultimate security during inclement weather. All the poles shall include a grounding lug positioned in the pole opposite the reinforced hand hole. Additionally, all poles shall incorporate an internal vibration dampener to minimize vibration during steady winds or structural vibrations. The bracket arms shall be constructed using 2-inch schedule 40 tubing bent to the desired shape and completely galvanized after fabrication. Any holes that are required for mounting of accessories on the pole must be made during the manufacturing process and before any galvanizing or surface finishing is performed. All pole hardware shall be stainless steel.

## 17.14 Raceway Systems

### 17.14.1 Conduit Types

#### 17.14.1.1 Galvanized Rigid Steel/Polyvinyl Chloride-Coated GRS

- 1 Only galvanized rigid steel (GRS) conduits and accessories shall be provided for power, control,
- 2 and communications, except in underground or concrete encased ductbanks.
- 3 Only GRS conduit and accessories shall be provided for all installations in tunnels, stations
- 4 under platform chase, sump pump structures, and ventilation structures including tunnel
- 5 ventilation fans, lighting, and sump pumps, except when conduits are embedded in concrete.
- 6 Polyvinyl chloride (PVC)-coated GRS conduit sections shall be provided for transition between
- 7 embedded conduit and the above-ground metallic conduit. PVC within concrete
- 8 floors/walls/slabs shall be terminated with a male adapter and PVC-coated GRS.

#### 17.14.1.2 Electrical Metallic Tubing

- 9 Electrical metallic tubing (EMT) may be run exposed 10 feet-0 inches above finished floor or
- 10 concealed in walls and ceilings, where approved by the Authority. EMT may be used below
- 11 10 feet-0 inches above finished floor for circuits in exiting walls or ceilings but shall not have an
- 12 exposed length exceeding 3 feet-0 inches.
- 13 EMT shall not be installed in equipment rooms (i.e., rooms containing traction power,
- 14 communication, train control, electrical and mechanical equipment), tunnels, subways, utility
- 15 chases, under platform utility chases, under platforms, fire command post rooms, concourse
- 16 level ceilings, outdoor locations, and where exposed to weather.
- 17 All emergency power supply source power circuits, lighting circuits, security circuits,
- 18 communication circuits, fire alarms, service entrance, emergency system (vent fans, dampers,
- 19 pumps, generators), and critical loads for train control circuits shall not be installed in EMT.

#### 17.14.1.3 Liquid-tight Flexible Metal Conduit

- 20 Liquid-tight flexible metal conduit shall be provided only where required for flexibility such as
- 21 connections to vibrating equipment and across joints subject to differential movement, and
- 22 where required so that liquids tend to run off the surface and not drain toward fittings.
- 23 Liquid-tight flexible conduit shall be used in wet or damp locations.

#### 17.14.1.4 Fiberglass Reinforced Epoxy /PVC

- 24 Fiberglass reinforced epoxy and/or PVC schedule 40 electrical conduit shall be provided only
- 25 when embedded in concrete building or structure, parking structures, slabs and walls, pole
- 26 foundations, or where required in short sections for electrical isolation.

#### 17.14.2 Conduit Fill Ratio

1 Conduit fill ratio shall comply with the requirements as specified in the NEC and CEC.

#### 17.14.3 Conduit Sizes

2 The following minimum conduit size restrictions shall apply to the CHSTP:

- 3 • Galvanized Rigid Steel Conduit – 3/4-inch diameter or larger for exposed locations and 1-  
4 inch diameter or larger for embedded locations
- 5 • PVC Coated Galvanized Rigid Steel Conduit – 3/4-inch diameter or larger for exposed  
6 locations and 1-inch diameter or larger for embedded locations
- 7 • Electrical Metallic Tubing – 3/4-inch diameter or larger for exposed locations and 1-inch  
8 diameter or larger for embedded locations
- 9 • Liquid-tight Flexible Metal Conduit – 3/4-inch minimum diameter
- 10 • Fiberglass Reinforced Epoxy or PVC Schedule 40 Conduit – 3/4-inch diameter or larger for  
11 exposed locations and 1-inch diameter or larger for embedded locations. For concrete slabs  
12 less than 5 inches thick or elevated prestressed concrete decks, 3/4-inch conduits may be  
13 used.
- 14 • Innerduct – 1-inch minimum diameter

15 Conduit runs within finished areas (e.g., offices, concourses) shall be concealed in floor slabs,  
16 walls, ceiling slab or hung ceilings. Wiring devices shall be flush mounted.

#### 17.14.4 Tray

17 Tray systems shall be hot dipped galvanized, corrosion coated type and shall be engineered to  
18 comply with the following requirements:

- 19 • The tray system shall be fully enclosed metal cable trays with full system appendages.
- 20 • The drop-offs to all different points of utilization shall be conduit. Bushed conduit shall be  
21 used wherever possible.
- 22 • The tray system construction shall be secure and prevent inadvertent access by  
23 unauthorized parties.
- 24 • The tray system shall have suitable strength and rigidity to provide adequate support for all  
25 the contained cables.
- 26 • The tray system shall include a means to ventilate the enclosed cables, so the heat generated  
27 by the cables can be safely dissipated.
- 28 • The tray system shall include barriers to segregate cables of different systems and voltage  
29 ratings.

- The tray system shall provide adequate cross-sectional area to permit neat alignment of the cables and avoid crossing or twisting.

Tray systems may be provided in lieu of conduit where LV power and multi-conductor control cables are routed within electrical rooms, mechanical rooms, train control rooms, and so forth. Tray systems shall not be used for installation of medium voltage wires and cables in any locations, nor shall they be used for installation of any wires and/or cables within a tunnel environment.

#### **17.14.5 Ductbanks and Underground Structures**

The design of underground ductbanks associated with new electrical utility services (outside the Authority's right-of-way) shall comply with the design requirements of the local electric utility service provider. The design of all other underground ductbanks and associated structures shall comply with the NEC, CEC, and California Public Utility Commission GO 128.

The design of underground ductbanks and associated structures shall be carefully coordinated with other underground utilities and/or structural foundations to assure safe access within the Authority's right-of-way. Ductbanks shall be configured as required at the specific location. Manholes, pull boxes, junction boxes and cable vaults shall be spaced for ease of cable pulling, and shall meet applicable codes and operational requirements (without exceeding cable pulling tensions) but shall not exceed 500 feet. The design must be carefully coordinated with underground utilities to assure safe access within the Authority's right-of-way.

Where an underground ductbank is routed across seismic fault lines the following shall apply:

- A cable vault/manhole shall be located on both sides of the fault line.
- The cable vaults/manholes shall be within 100 feet of the fault line.
- Sufficient right-of-way is provided on and adjacent to the fault line to permit reconstruction of the trainway, and adjacent infrastructure and underground ductbanks after a seismic event.
- Cable vaults/manholes shall be sized to permit excess cable service loops.

Conduit transitions shall be provided when changing from one cable trough location or configuration to another and to provide conduit access from the cable trough to system wayside facilities.

Underground ductbanks shall be sloped toward the adjacent underground structure (e.g., vault, manhole, or box) from which water may be drained or pumped. Underground structures shall be equipped with sump area for drainage by gravity or application of portable or fixed pumps as required.

Vaults and manhole access covers and ladders shall conform to Cal/OSHA requirements.

1 Interior length, width, and depth of vaults and manholes shall be sufficient for cable pulling  
2 and splicing and for cable expansion and contraction.

3 The design of vaults, manholes, and pullboxes shall incorporate the following features:

- 4 • Non-metallic racks or fiberglass cable support insulators, which are spaced to avoid  
5 excessive weight or pressure on the cable insulation
- 6 • Cable pulling hooks
- 7 • Grounding provisions

8 Refer to the *Structures* chapter for additional requirements pertaining to loading considerations  
9 for undertrack ductbanks.

10 Underground Ductbanks and Structures within the Authority's Trackway

- 11 • The longitudinal underground ductbanks and structures (routed parallel to the trackway)  
12 shall be located between the Authority's right-of way security barrier and the outborne  
13 track.
- 14 • The side of the underground ductbank shall be clear of the trackform's TBD or at least 2 feet  
15 horizontally clear of the edge of the trackform tie/slab, whichever is greater.
- 16 • Underground ductbanks, routed across tracks, shall be routed perpendicular to the tracks.  
17 Structures associated with these ductbanks shall not be located between trackforms. Refer to  
18 the *Utilities* chapter for the minimum vertical clearance requirements, between the top of the  
19 underground ductbank and the rails.
- 20 • The edge of structure openings and the outer edge of the nearest rail shall be least 8 feet 6  
21 inches.
- 22 • The easement above the width of the underground ductbanks and structures, and within 3  
23 feet of the edge of an underground ductbank and/or structure shall be unobstructed by  
24 adjacent infrastructure and/or structures.
- 25 • The structures shall be drained into the trackway's drainage system.

#### 17.14.6 Raceway Systems and/or Cable Trough for the Core Systems

26 Core Systems include the traction electrification system, automatic train control and system  
27 communications and facility communication systems.

28 Refer to the *Traction Power Supply System*, *Automatic Train Control*, and *Communications* chapters  
29 for raceway system requirements for the Core Systems.

30 Refer to *TBD* chapter for raceway system requirements for the facility communication systems.



- 1 Trackside interbuilding power cables associated with the FPS shall not be routed in raceway  
2 systems and/or the cable troughs associated with the Core Systems and/or facility  
3 communication systems.

#### **17.14.7 Conduits within Reinforced Structures**

- 4 Conduit spacing, within load bearing structures, shall comply with the requirements of the  
5 American Concrete Institute code.
- 6 Conduit routing and reinforcement steel designs shall be coordinated to avoid conflicts.

#### **17.14.8 Conduits Routed Under the Track**

- 7 Unless otherwise specified, all conduits routed under the tracks shall be protected by  
8 encasement within reinforced steel concrete. The design of the encased conduits shall consider  
9 the loading conditions associated with the trackwork and substructure.
- 10 Refer to the *Structures* chapter for track loading conditions.
- 11 Refer to the *Utilities* chapter for minimum under track vertical clearance requirements.

#### **17.14.9 Exposed Conduits and/or Boxes within Enclosed Spaces (e.g., Tunnels, Emergency Stairways, etc.)**

- 12 Conduit, boxes, mounting hardware and appurtenances shall be low smoke, zero halogen and  
13 comply with the requirements as specified in NFPA 130.

#### **17.15 Wiring/Cabling Systems**

- 14 All conductors shall be enclosed in conduit, tray, and/or raceway.
- 15 All wires and cables shall be protected from mechanical impact, heat, or fire by any of the  
16 following:
- 17 • Suitable embedment or encasement
  - 18 • Routing external to the interior of a tunnel environment
  - 19 • Routing within exposed galvanized rigid steel conduit, located clear of the static and  
20 dynamic envelopes of the rolling stock, and fixed equipment envelope
  - 21 • Physical diversity in wire/cable routing
  - 22 • Use of listed fire-resistive cable systems
- 23 Wires and/or cables associated with the normal, backup, and emergency power supply sources  
24 shall be routed in physically separate raceway systems (i.e., conduit, trays, troughs, boxes) such

that the wires and/or cables of more than one power source shall not be damaged by the same localized impact or heat source.

The wire and cable specifications shall be suitable for the intended working environment and comply with the requirements specified in NFPA 130.

Minimum size of power conductors shall be AWG No. 12. Minimum size of control wires shall be AWG No. 14.

Temperature ratings of all wires and cables shall be not less than 90 degrees Celsius.

Insulated cables shall be appropriate for the voltage level and readily available sizes shall be used on the CHSTP.

The number and size of cables in a particular circuit shall be determined to provide adequate capacity, acceptable voltage drop and system fault level.

Insulated cables of different voltage classes shall not occupy the same conduit, tray, junction/pull box, manhole or vault.

Refer to the *Traction Power Supply System*, *Automatic Train Control*, and *Communications* chapters for requirements pertaining to associated cable infrastructure.

#### 17.15.1 Tunnels

Field wires and cables (except exposed bare ground conductors) installed in tunnel environments (including portals, tunnels, cross passages, access/egress shafts, ventilation structures, sump pump structures) shall have insulation ratings based upon the intended working environment and shall be in compliance with the NEC, CEC, and NFPA 130. Unless otherwise approved by the Authority, all factory wiring within trackside-mounted equipment enclosures shall comply with NFPA 130.

All wires and cables shall pass the flame propagation criteria of IEEE 383 and have a minimum circuit time of 5 minutes in the flame test of IEEE 383.

All wires and cables in assessable enclosures/boxes shall be completely fireproofed (2 hours minimum).

#### 17.15.2 Color Coding

All FPS and LS conductors shall be color coded by outer jacket or by vinyl tape according to the following requirements:

- Grounding conductor – Green
- 120/208 V ac ungrounded conductors – a. black; b. red; and c. blue
- Grounded conductor (Neutral) – white or natural grey

- 1     • 120 V dc – red (positive); black (negative)
- 2     • 277/480 V ac ungrounded conductors – a. brown; b. orange; c. yellow or purple
- 3         – Grounded conductor (Neutral) – white or natural grey
- 4     • HV 3-ph conductors – red, black, and blue, and mark with phase letter
- 5     Multi-conductor control/signaling cables shall be color coded in accordance with ICEA Method
- 6     1, K-2.

## **17.16 Identification Requirements (Equipment, Conductors, Conduit, Tray)**

### **17.16.1 Equipment**

- 7     Each piece of electrical equipment shall be numbered according to the number of the feeder
- 8     circuit breaker (except termination cabinets, which shall be numbered sequentially), and the
- 9     numbering shall be reflected on all applicable design and record drawings.
- 10    Equipment numbering shall be preceded by letter designations as follows:

**Table 17-5: Equipment Numbering**

1	Switchboard	A
2	Power Panelboard	P
3	Lighting Panelboard, 480/277 V	L
4	Facility Power Panel, 208/120 V	LA
5	LV Transformer	LVX
6	Local Control Panel	LCP
7	Emergency Walkway Lighting Control Panel	EWLCP
8	Disconnect Switch	Z
9	Motor	M
10	Motor Starter	MS
11	LV Motor Control Center	MCC
12	Intermediate Termination Cabinet	ITC
13	Automatic Transfer Switch	ATS
14	Manual Transfer Switch	MTS
15	Generator	G
16	Uninterruptible Power Supply	UPS
17	Unit Substation	US
18	HV Switchgear	HVSG
19	HV Transformer	HVX
20	HV Motor Control Center	HVMCC

Prefix all electrical equipment with “E” followed by the equipment designation to indicate that power originates from the emergency standby generator, UPS, and/or dc battery system. The equipment identifier shall be reflected on all applicable design and record drawings.

#### 17.16.2 Manholes, Handholes, Vaults and Boxes

All manholes, handholes, vaults, boxes, and so forth shall be identified by a 7-character reference designator: 4 letters followed by a 3-digit number. The 4 letters depend upon the electrical system as follows:

**Table 17-6: Manholes, Handholes, Vaults and Boxes**

1	MV (i.e., 12 kV and above) Facility Power	MVFPXXX
2	Tunnel Ventilation (4.16 kV) Facility Power	TVFPXXX
3	LV (480 V and below) Facility Power	LVFPXXX

The 3 digits shall increase sequentially, beginning with 001. The manhole, handhole, vault, and/or box identifier shall be reflected on all applicable design and record drawings.

### **17.16.3 Conduit/Tray**

All conduit/tray shall be identified by an 8-character reference designator and depend upon the electrical system as follows:

**Table 17-7: Conduit/Tray**

1	MV (i.e., 12 kV and above) Facility Power	MVFPXXXX
2	Tunnel Ventilation (4.16 kV) Facility Power	TVFPXXXX
3	LV (480 V and below) Facility Power	LVFPXXXX

The 4 digits shall increase sequentially, beginning with 0001. Interfacility conduit numbering shall be coordinated. The conduit/tray identifier shall be reflected on all applicable design and record drawings.

Interior raceways and tray systems shall be identified. Lettering shall be printed. Markers shall be placed on all exposed or accessible raceways and tray system within 18 inches of raceway/tray termination, wherever raceway and tray systems enter or leave concealed space, and every 50 feet along raceway and trays.

The following conventions shall apply:

- Power and lighting raceways and tray systems shall be identified as to the system Line to Line and Line to Ground voltages.
- Emergency power supply system raceways shall be identified “Emergency Power” in addition to system voltage.
- Grounding raceways shall be identified “Ground.”
- Empty raceways shall be identified as “Spare.”

Tray systems that house cabling of different subsystems shall be identified by the highest-rated voltage of the cables contained within it.

Conduits within ductbanks shall be identified at all locations where the conduits stub-up and/or into manholes. Identifiers shall be non-conductive, permanent, and follow the convention specified above.

### **17.16.4 Conductors**

All conductors shall be identified by an 8-character reference designator and depend upon the electrical system as follows:

**Table 17-8: Conductors**

1	MV (i.e., 12 kV and above) Facility Power	MVFPXXXX
2	Tunnel Ventilation (4.16 kV) Facility Power	TVFPXXXX
3	LV (480 V and below) Facility Power	LVFPXXXX

The 4 digits shall increase sequentially, beginning with 0001. Interfacility conductor numbering shall be coordinated. The conductor identifier shall be reflected on all applicable design and record drawings.

All conductors shall be identified within 1 foot of the termination point and at all vaults, manholes, pull boxes, junction boxes, and so forth. The identifier shall be non-conductive, permanent and specify the termination details that correspond to the associated termination hardware.

#### 17.16.5 Identification Requirements for the Core Systems and Facility Communication Systems

Refer to the *Traction Power Supply System*, *Automatic Train Control*, *Communications* and *Facility Communications* chapters for identifier requirements associated with the Core Systems.

Refer to *TBD* chapter for identifier requirements associated with the facility communication systems.

### 17.17 Electrical Discrete and Serial I/O Point Requirements

All control, alarm, and indication functions shall be available locally and remotely at the Operations/Regional Control Center, passenger stations, Heavy Maintenance Facility, Terminal Layup/Storage and Maintenance Facilities, and right-of-way Infrastructure Facilities. Summary alarms of equipment failures and equipment status indications shall be annunciated at the Operations/Regional Control Center. Refer to the *Supervisory Control and Data Acquisition Subsystems* chapter for SCADA requirements and *TBD* chapter for the Building Automation System (BAS) requirements.

All discrete and serial input/output (I/O) point designations shall be prefixed with the facility and equipment designations. Refer to *TBD* chapter for facility designation requirements

The FPS equipment/power sources that need to be monitored and/or controlled shall include, but not be limited to, the following:

- All power sources from the electric utility service provider and the traction electrification system
- All HV circuit breakers

- 1 • All HV transformers
  - 2 • All HV motor control centers
  - 3 • All automatic transfer switches
  - 4 • All main and tie circuit breakers at 480 V switchboards and/or unit substations
  - 5 • All emergency standby generators, uninterruptable power source equipment and dc battery
  - 6 system equipment
  - 7 • Facility lighting control panels
  - 8 • Tunnel and/or trench lighting circuits
  - 9 • Local controls within an electrical room activated
- 10 The design of the FPS equipment shall include all necessary equipment, termination blocks,  
11 termination hardware, factory wiring, etc. to interface to the SCADA system and the BAS.

### **17.18 Temporary Power and Lighting**

- 12 Temporary power and lighting shall be designed and provided for the CHSTP. This design of  
13 the temporary power and lighting shall be complete, and shall conform to the NEC, CEC, NFPA  
14 130, and all other applicable electrical and safety codes/orders.

### **17.19 Grounding, Bonding and Lightning Protection**

- 15 Refer to the *Grounding and Bonding Requirements* chapter for requirements.

### **17.20 Fire Detection System**

- 16 Refer to *TBD* chapter for requirements.

### **17.21 Vehicular Traffic Management System**

- 17 Refer to *TBD* chapter for requirements.

### **17.22 Facility Communication Systems**

- 18 Refer to *TBD* chapter for requirements.

### 17.23 Testing

For facility power equipment provided under contract to the Authority, the following conditions shall apply:

- At least one piece of HV equipment shall be design tested in accordance with applicable industry standards and/or publications.
- All equipment shall be undergo production testing in accordance with the manufacturer's recommendations, applicable industry standards and/or publications.
- All HV and Emergency Power Supply equipment shall be factory acceptance tested in accordance with manufacturer's recommendations, applicable industry standards and/or publications.
- All facility electrical equipment shall be field tested and commissioned in accordance with applicable industry standards and/or publications.

For facility power and lighting cables and wires provided under contract to the Authority, the following conditions shall apply:

- Non-standard HV cables shall be design tested in accordance with applicable industry standards and/or publications.
- Standard HV and LV cables and wires shall undergo production testing in accordance with the manufacturer's recommendations, applicable industry standards and/or publications.
- All HV cables shall be factory acceptance tested in accordance with manufacturer's recommendations, applicable industry standards and/or publications.
- All HV and LV cables and wires shall be field tested in accordance with applicable industry standards and/or publications.

### 17.24 Spares

The following spares requirements shall be incorporated into the design of the FPS:

- The design of the FPS shall include electrical distribution capacity, space for future circuit breakers, and rough-in raceway systems to permit future equipment placements in accordance with the Authority's requirements as specified in TBD. Rough-in raceway systems shall be planned and installed in the same fashion as circuits provided under this contract so that when the future equipment is installed, surface-mounted raceway can be avoided.



- 1 • HV switchgear, unit substations, switchboards, HV and LV transformers, and panelboards  
2 shall be equipped with a minimum of 25 percent spare circuits and bus capacity. Standard  
3 size equipment shall be used.
- 4 • The design of the Interface Termination Cabinet shall include at least 25 percent spare  
5 termination hardware.
- 6 • At least 25 percent or 1 spare duct (whichever is greater) per voltage level/system shall be  
7 provided in all interbuilding underground ductbank runs.
- 8 • At least 25 percent or 1 spare conduit (whichever is greater) per voltage level/system shall  
9 be provided in each conduit run.

## Appendix 17.A: Minimum Lighting Levels

Facility	ROOM/AREA	LIGHTING LEVELS		LIGHTING TYPE
		AVERAGE MAINTAINED (LUX)	AT (FT)	
At-Grade Passenger Station	- PLATFORM (EDGE, UNCOVERED)	200	0	MH
	- PLATFORM (GENERAL, UNCOVERED)	100	0	MH
	- PLATFORM (EDGE, UNDER CANOPY COVER)	200	0	MH/FLR
	- PLATFORM (GENERAL, UNDER CANOPY COVER)	100	0	MH/FLR
	- UNDER PLATFORM UTILITY CHASES	10	V.@ 3.0	HPS
	- STATION AGENTS BOOTH	150	2.5	FLR
	- CONCOURSE	150	0	MH/FLR
	- MEZZANINE	150	0	MH/FLR
	- FARE GATES	200	0	MH/FLR
	- VENDING MACHINE AREA	200	0	MH/FLR
	- PATRON WAITING AREA	250	0	MH/FLR
	- CONCESSIONS (IF ANY)	200	0	MH/FLR
	- ESCALATORS, STAIRWAYS AND PASSAGEWAYS	100 – 150	TREAD – ENTRANCE TO	MH/FLR
	- ELEVATORS	100	0	CFL
	- WALKWAYS/CORRIDORS	150	0	FLR
	- CONNECTIONS TO PUBLIC WALKWAYS	150	0	FLR
	- RAMPS LEADING TO WALKWAYS	150	0	FLR
	- RESTROOMS	200	0	FLR
	- OFFICES/STAFF ROOMS	300	2.5	FLR
	- STORAGE ROOMS	100	0	FLR
	- CLOSETS	100	0	FLR
	- TRASH ROOMS	100	0	FLR
	- BIKE LOCKER AREA	200	0	FLR

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

	- BAGGAGE STORAGE AREA	200	0	FLR
	- JANITORIAL ROOMS	200	0	FLR
	- VAULT AND PIT AREAS	300 <sup>1</sup>	0	FLR
	- TELCO ROOM	250	0	FLR
	- ELECTRICAL ROOMS	250	0	FLR
	- TRAIN CONTROL/COMMUNICATIONS ROOM	500	0	FLR
	- BATTERY ROOMS	200	0	MH/HPS
	- EMERGENCY STANDBY GENERATOR ROOM	200	0	FLR
	- ESCALATOR CONTROL ROOMS	100	0	FLR
	- ELEVATOR MACHINE ROOMS	100	0	FLR
	- HVAC ROOMS	250	0	FLR
	- CONDENSER ROOMS	100	0	FLR
	- VALVE ROOMS	100	0	FLR
	- SECURITY ROOMS	250	0	FLR
	- ROOF LIGHTING	10	0	HPS
	- PASSENGER STATION EXTERIOR	50	V. ON SURFACE	MH
Elevated Passenger Station	- PLATFORM (EDGE, UNCOVERED)	200	0	MH
	- PLATFORM (GENERAL, UNCOVERED)	100	0	MH
	- PLATFORM (EDGE, UNDER CANOPY COVER)	200	0	MH/FLR
	- PLATFORM (GENERAL, UNDER CANOPY COVER)	100	0	MH/FLR
	- UNDER PLATFORM UTILITY CHASES	10	V. @ 3.0	H[S
	- STATION AGENTS BOOTH	150	2.5	FLR
	- CONCOURSE	150	0	MH/FLR
	- MEZZANINE	150	0	MH/FLR
	- FARE GATES	200	0	MH/FLR
	- VENDING MACHINE AREA	200	0	MH/FLR
	- PATRON WAITING AREA	250	0	MH/FLR
	- CONCESSIONS (IF ANY)	200	0	MH/FLR
	- ESCALATORS, STAIRWAYS AND PASSAGEWAYS	100-150	TREAD-ENTRANCE TO	MH/FLR
	- ELEVATORS	100	0	CFL

	- WALKWAYS/CORRIDORS	150	0	FLR
	- CONNECTIONS TO PUBLIC WALKWAYS	150	0	FLR
	- RAMPS LEADING TO WALKWAYS	150	0	FLR
	- RESTROOMS	200	0	FLR
	- OFFICES/STAFF ROOMS	300	2.5	FLR
	- STORAGE ROOMS	100	0	FLR
	- CLOSETS	100	0	FLR
	- TRASH ROOMS	100	0	FLR
	- BIKE LOCKER AREA	200	0	FLR
	- BAGGAGE STORAGE AREA	200	0	FLR
	- JANITORIAL ROOMS	200	0	FLR
	- VAULT AND PIT AREAS	300 <sup>1</sup>	0	FLR
	- TELCO ROOM	250	0	FLR
	- ELECTRICAL ROOMS	250	0	FLR
	- TRAIN CONTROL/COMMUNICATIONS ROOM	250	0	FLR
	- BATTERY ROOMS	200	0	MH/HPS
	- EMERGENCY STANDBY GENERATOR ROOM	200	0	FLR
	- ESCALATOR CONTROL ROOMS	100	0	FLR
	- ELEVATOR MACHINE ROOMS	100	0	FLR
	- HVAC ROOMS	250	0	FLR
	- CONDENSER ROOMS	100	0	FLR
	- VALVE ROOMS	100	0	FLR
	- SECURITY ROOMS	250	0	FLR
	- ROOF LIGHTING	10	0	HPS
	- PASSENGER STATION EXTERIOR	50	V. ON SURFACE	MH
Exterior Passenger Loading/Circulation Areas/Other Areas Adjacent To The Passenger Station	- BUS LOADING/UNLOADING	75	0	MH
	- BUS LOOPS	25	0	MH
	- PATRON LOADING /UNLOADING	75	0	MH
	- KISS AND RIDE	50	0	MH
	- PLAZA (AT STATION ENTRANCES)	30	0	MH

	- PLAZA	25	0	MH
	- PEDESTRIAN WALKWAYS(OPEN)	30	V. @ 5.0	MH
	- OVERTRACK PEDESTRIAN WALKWAY BRIDGES	100	0	FLR
	- UNDERPASSES	100	0	FLR
	- LANDSCAPING AREAS	ACCENT	0	AS REQ.
Parking Lots	- CIRCULATION ROADWAY LIGHTING	30	0	HPS
	- PEDESTRIAN WALKWAYS	30	0	MH
	- PARKING SPACES	25	0	HPS
Parking Structures	- CIRCULATION ROADWAY LIGHTING	50	V. @ 5.0	HPS
	- PEDESTRIAN WALKWAYS	75	V. @ 5.0	MH
	- STAIRWAYS	100	TREADS	FLR
	- ELEVATORS	100	0	CFL
	- PARKING SPACES	50	V. @ 5.0	HPS
	- ELECTRICAL ROOMS	250	0	FLR
	- ELEVATOR MACHINE ROOMS	100	0	FLR
	- MECHANICAL ROOMS	250	0	FLR
	- BATTERY ROOMS	100	0	HPS
	- PARKING STRUCTURE EXTERIOR	50	V. ON SURFACE	MH
Heavy Maintenance Facility	TBD	TBD	TBD	TBD
Terminal Layup/Storage And Maintenance Facility	TBD	TBD	TBD	TBD
Maintenance of Infrastructure Maintenance Facility	TBD	TBD	TBD	TBD
Operations Control Center	- CENTRAL CONTROL ROOM	250	0	FLR
	- OFFICES/STAFF ROOMS	300	2.5	FLR
	- MEETING/TRAINING ROOMS	300	2.5	FLR
	- LUNCH/BREAK ROOM	250	2.5	FLR
	- STORAGE ROOMS	100	0	FLR
	- TELECOMMUNICATIONS COMPANY ROOM	250	0	FLR
	- ELECTRICAL ROOMS	250	0	FLR

	- TRAIN CONTROL/COMMUNICATIONS ROOM	250	0	FLR
	- BATTERY ROOMS	200	0	HPS
	- RESTROOMS			
Portal Facility	- ACCESS ROADWAY LIGHTING	20	0	HPS
	- EMERGENCY VEHICLE ASSEMBLY AND TURNAROUND AREA	50	0	HPS
	- RESCUE AREA/PASSENGER ASSEMBLY AREA	50-100 <sup>2</sup>	0	MH
	- HELICOPTER LANDING PAD	50 <sup>3</sup>	0	MH
	- PARKING	10	0	HPS
	-EMERGENCY EGRESS WALKWAYS	20	V. @ 5.0	MH
	- STAIRWAYS	100	TREADS	FLR
	- EMERGENCY COMMAND POST	50	0	MH
	- ELECTRICAL ROOMS/YARDS	250	0	MH
	- TRAIN CONTROL/COMMUNICATIONS ROOM	250	0	MH
	- BATTERY ROOM	200	0	HPS
	- EMERGENCY VENTILATION FAN ROOM	10	0	FLR
	- MECHANICAL CONTROL ROOMS	250	0	FLR
	- RESTROOMS	200	0	FLR
	- EMERGENCY STANDBY GENERATOR SITE	50	0	MH
	- PORTAL FACILITY EXTERIOR	50	0	MH
Mid-Tunnel Ventilation Structures	- ACCESS ROADWAY LIGHTING	20	0	HPS
	- EMERGENCY VEHICLE ASSEMBLY AND TURNAROUND AREA	50	0	HPS
	- RESCUE AREA/PASSENGER ASSEMBLY AREA	500-100 <sup>2</sup>	0	MH
	- HELICOPTER LANDING PAD	50 <sup>3</sup>	0	MH
	- PARKING			
	- EMERGENCY EGRESS WALKWAYS	20	V. @ 5.0	MH
	- STAIRWAYS	100	TREADS	FLR
	- EMERGENCY COMMAND POST	50	0	MH
	- ELECTRICAL ROOMS/YARDS	250	0	MH

	- TRAIN CONTROL/COMMUNICATIONS ROOM	250	0	MH
	- BATTERY ROOM	200	0	HPS
	- EMERGENCY VENTILATION FAN ROOM	10	0	FLR
	- MECHANICAL CONTROL ROOMS	250	0	FLR
	- RESTROOMS	200	0	FLR
	- EMERGENCY STANDBY GENERATOR SITE	50	0	HPS
	- MID-TUNNEL VENTILATION STRUCTURE EXTERIOR	50	0	MH
Emergency Access Shafts	- ACCESS ROADWAY LIGHTING	20	0	HPS
	- EMERGENCY VEHICLE ASSEMBLY AND TURNAROUND AREA	50	0	HPS
	- RESCUE AREA/PASSENGER ASSEMBLY AREA	50-100 <sup>2</sup>	0	HPS
	- HELICOPTER LANDING PAD	50 <sup>3</sup>	0	MH
	- PARKING	10	0	MH
	- EMERGENCY EGRESS WALKWAYS	20	V. @ 5.0	MH
	- STAIRWAYS	100	TREADS	FLR
	- ELECTRICAL ROOMS/YARDS	250	0	FLR
	- TRAIN CONTROL/COMMUNICATIONS ROOM	250	0	FLR
	- BATTERY ROOM	200	0	HPS
	- MECHANICAL CONTROL ROOMS	250	0	FLR
Sump Pumps (At-Grade)	- ACCESS ROADWAY LIGHTING	20	0	HPS
	- SITE LIGHTING	50	0	HPS
	- ELECTRICAL ROOMS	250	0	FLR
	- ROOM LIGHTING	200	0	FLR
Sump Pumps (Tunnel)	- ROOM LIGHTING	200	0	FLR
Standalone Traction Power Facilities		20	0	MH
	- PEDESTRIAN WALKWAYS	30	V. @ 3.0	MH
	- SITE/PERIMETER/SECURITY LIGHTING	10	V. @ 5.0	HPS
	- EXTERIOR LIGHTING AT PRE-ENGINEERED ENCLOSURE DOORWAYS	10	0	MH
	- INTERIOR LIGHTING AT PRE-ENGINEERED ENCLOSURES	150	0	FLR

Standalone Train Control/Communication Huts		20	0	HPS
	- PEDESTRIAN WALKWAYS	30	V. @ 5.0	MH
	- SITE/PERIMETER/SECURITY LIGHTING	10	V. @ 5.0	HPS
	- EXTERIOR LIGHTING AT PRE-ENGINEERED ENCLOSURE DOORWAYS	10	0	MH
	- INTERIOR LIGHTING AT PRE-ENGINEERED ENCLOSURES	150	0	FLR

Notes:

1. Maintenance facilities illuminations levels shall depend upon the task to be performed.
2. 50 lux under normal operations and 100 lux during an emergency.
3. Proper aviation lighting shall meet FAA requirements.

Abbreviations: V – Vertical, AVG – Average, REQ – Required, FT – Feet, MH – Metal Halide, HPS – High Pressure Sodium, FLR – Fluorescent, CFL – Compact Fluorescent Lamp





## Chapter 18

### Fire Protection

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	May 2013	Revision 1



## Table of Contents

18	FIRE PROTECTION (TBD) .....	18-1
----	-----------------------------	------

### Tables

No table of figures entries found.

### Figures

No table of figures entries found.

## Acronyms


## **18 Fire Protection (TBD)**

- 1 TBD. This topic will be included in a future revision of the Design Criteria.



## Chapter 19

### Building Automation and Management Systems

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	May 2013	Revision 1





## Table of Contents

19	BUILDING AUTOMATION AND MANAGEMENT SYSTEMS (TBD) .....	19-1
----	--	------

### Tables

No table of figures entries found.

### Figures

No table of figures entries found.

## Acronyms


## **19 Building Automation and Management Systems (TBD)**

- 1 TBD. This topic will be included in a future revision of the Design Criteria.



## Chapter 20

# Traction Power Supply System

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
0.1	Dec 2012	EXECUTION VERSION
1	Sept 2013	Rolling Stock Procurement
2	Feb 2014	Revision 2



## Table of Contents

<b>20</b>	<b>TRACTION POWER SUPPLY SYSTEM .....</b>	<b>20-1</b>
20.1	Scope .....	20-1
20.2	Regulations, Codes, Standards, and Guidelines .....	20-1
20.3	Overview and General Design Criteria .....	20-5
20.3.1	Traction Electrification System .....	20-5
20.3.2	Description of Traction Electrification System .....	20-5
20.3.3	RAMS Requirements .....	20-9
20.3.4	General Design Requirements .....	20-9
20.4	Key Design Criteria .....	20-11
20.4.1	Traction Power Substations .....	20-11
20.4.2	Switching Stations .....	20-13
20.4.3	Paralleling Stations .....	20-13
20.4.4	Traction Electrification System Sectionalizing Requirements .....	20-14
20.4.5	Ratings and Configuration for Each Type of Traction Power Facilities .....	20-15
20.4.6	High-Voltage Utility Connections .....	20-15
20.4.7	Regenerative Braking .....	20-15
20.4.8	Power Supply for Rolling Stock Maintenance Facilities .....	20-16
20.5	Related Documentation .....	20-17
20.6	System Voltage and Frequency .....	20-17
20.6.1	System Voltage .....	20-17
20.6.2	Voltage Related Requirements .....	20-17
20.6.3	Frequency .....	20-18
20.7	System Configuration .....	20-19
20.7.1	Verification of Configuration .....	20-19
20.7.2	Spacing of Traction Power Facilities .....	20-19
20.7.3	Additional Location Requirements .....	20-19
20.7.4	Continuity of Power Supply in Case of Disturbances .....	20-21
20.7.5	Electrical Protection Coordination .....	20-21
20.7.6	Protective Provisions of Traction Power Facilities .....	20-22
20.7.7	Harmonic Distortion Limits .....	20-23
20.7.8	Electromagnetic Compatibility .....	20-23
20.8	Traction Electrification System Interfaces .....	20-23
20.9	Traction Power Facilities and Associated Sites .....	20-23
20.9.1	General Site Requirements .....	20-23
20.9.2	Equipment Support Steel .....	20-24
20.9.3	Foundations .....	20-25
20.9.4	High-Voltage Transformers .....	20-26
20.9.5	Autotransformers .....	20-26
20.9.6	High-Voltage Switchgear .....	20-27
20.9.7	Prefabricated Enclosures for 25 kV Indoor Switchgear .....	20-27
20.9.8	25 kV Single-Phase Switchgear .....	20-28
20.9.9	Transformer Oil Containment .....	20-29
20.9.10	Utility Interface Equipment .....	20-30
20.9.11	Resistor Banks .....	20-30
20.9.12	Painting and Finishes .....	20-30



20.9.13	Auxiliary and Control Power .....	20-30
20.9.14	Lighting .....	20-31
20.9.15	Relay Protection .....	20-31
20.9.16	Protective Relaying Scheme for Catenary and NF Fault Detection .....	20-32
20.9.17	Fire and Life Safety .....	20-34
20.9.18	Test Cabinets .....	20-34
20.9.19	Communications.....	20-34
20.9.20	SCADA Equipment.....	20-34
20.9.21	Metering Equipment .....	20-34
20.9.22	Grounding, Bonding and Lightning Protection .....	20-35
20.9.23	Real Estate Requirements: Approximate Footprints for the TPF .....	20-35
20.10	Electrical Materials .....	20-36
20.10.1	Equipment Identification.....	20-36
20.10.2	Equipment Enclosures .....	20-36
20.10.3	Raceway.....	20-36
20.10.4	Cable Tray Systems.....	20-37
20.10.5	Electrical Manholes and Pullboxes .....	20-38
20.10.6	Cable Trenches for Power Cables .....	20-38
20.10.7	Conductors .....	20-39
20.10.8	Wiring Devices .....	20-40
20.10.9	Light Fixtures .....	20-40
20.11	Emergency Power .....	20-40
20.11.1	Emergency Electrical Loads.....	20-40
20.11.2	Emergency Power Requirements .....	20-41
20.12	Wayside Equipment .....	20-41
20.12.1	General.....	20-41
20.12.2	Wayside Power Control Cubicles .....	20-41
20.13	Calculations.....	20-42
20.14	New Power Utility Services.....	20-42
20.14.1	115 kV/230 kV ac, Single-Phase 60 Hz Services .....	20-42
20.14.2	Auxiliary Power Services.....	20-43
20.15	Spares.....	20-43

## Tables

Table 20-1:	Rated Impulse Voltage and Power Frequency Test Voltage .....	20-23
-------------	--	-------

## Figures

Figure 20-1:	Simplified Feeding Arrangement – 2x25 kV Autotransformer Feed System .....	20-19
--------------	--	-------

## Acronyms

ac	Alternating Current
AT	Autotransformer
ATC	Automatic Train Control
CHSTP	California High-Speed Train Project
CPUC	California Public Utility Commission
dc	Direct Current
HST	High-Speed Train
HV	High voltage
kA	kiloampere
kV	kilovolt
IEC	International Electrotechnical Commission
IEEE	Institution of Electrical and Electronics Engineers
MOD	Motor Operated Disconnect Switch
MVA	Mega Volt Ampere
N.C.	Normally Closed
N.O.	Normally Open
NEC	National Electric Code
NEMA	National Electrical Manufacturers Association
NESC	National Electrical Safety Code
NF	Negative Feeder
NFPA	National Fire Protection Association
O&M	Operations and Maintenance
OCC	Operations Control Center
OCS	Overhead Contact System
PS	Paralleling Stations
RCC	Regional Control Center
rms	root mean square
SCADA	Supervisory Control and Data Acquisition
SS	Traction Power Substations
SWS	Switching Stations
TES	Traction Electrification System
TPF	Traction Power Facilities
TPS	Traction Power Supply System
V	Volt
WPC	Wayside Power Control Cubicles

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015



## 20 Traction Power Supply System

### 20.1 Scope

This chapter provides the main design requirements for the Traction Power Supply System (TPS) and associated site works for the Project, which is based upon a 2x25 kilovolt (kV) autotransformer feed configuration. The design of the TPS and associated site works shall be coordinated with the following:

- The requirements of the power supply utility companies (both investor-owned and municipal utility companies) providing electrical power to the system
- The requirements of the state and local jurisdictions where the Traction Power Facilities (TPF) are located
- The technical and operational parameters and requirements of the system (e.g., trackwork, Overhead Contact System (OCS), rolling stock, operations, maintenance, train control system, communications system, electromagnetic compatibility, etc.)

This document shall be read in conjunction with the TPS-related Standard and Directive Drawings and other procurement documents.

Refer to the *Facility Power and Lighting Systems* chapter for additional design requirements for electrical equipment. In case of a conflict between requirements specified in this chapter and the *Facility Power and Lighting Systems* chapter, the requirements of this chapter shall prevail.

### 20.2 Regulations, Codes, Standards, and Guidelines

In addition to the references listed, refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.

- American Railway Engineering and Maintenance-of-Way Association (AREMA)
  - AREMA Manual for Railway Engineering, Volume 3 Infrastructure and Passenger, Chapter 33 Electric Energy Utilization
- California Code of Regulations (CCR)
  - Title 8 – Occupational Safety and Health Act Standards
  - Title 19 – Public Safety
- California Building Standards Code, Title 24 of California Code of Regulations (CCR) –
  - Part 2 – California Building Code
  - Part 3 – California Electrical Code

- 1       – Part 6 – California Energy Code
- 2       – Part 9 – California Fire Code
- 3       • California Public Utility Commission (CPUC) General Orders (GOs)
- 4       – GO 95 – Overhead Electric Line Construction
- 5       – GO 128 – Construction of Underground electric Supply and Communications System
- 6       • European Standards (EN for European Norms)
- 7       – EN 50121-1 – Railway applications – Electromagnetic Compatibility
- 8       – EN 50122-1 – Protective Provisioning Relating to Electrical Safety and Earthing
- 9       – EN 50124-1 – Railway applications – Insulation Coordination
- 10      – EN 50124-2 – Railway applications – Overvoltages and Related Protection
- 11      – EN 50152 – Railway Applications–Fixed Installations–Particular Requirements for ac
- 12      Switchgear Part 1: Single-phase circuit-breakers with  $U_m$  above 1 kV
- 13      Part 2: Single-phase disconnectors, earthing switches and switches with  $U_m$  above 1 kV.
- 14      – EN 50160 – Voltage Characteristics of Electricity Supplied by Public Distribution
- 15      Systems
- 16      – EN 50163 – Railway Applications – Supply Voltages of Traction Systems,
- 17      – EN 50329 – Railway Applications – Fixed Installations – Traction Transformers
- 18      – EN 50388 - Railway Applications – Power Supply and Rolling Stock – Technical Criteria
- 19      for the Coordination Between Power Supply (Substation) and Rolling Stock to achieve
- 20      Interoperability
- 21      • Insulated Cable Engineers Association (ICEA) Standards
- 22      – ICEA S-95-658/NEMA WC70 – Standard for Nonshielded Power Cables Rated 2000
- 23      Volts or Less for the Distribution of Electrical Energy
- 24      • International Electro-technical Commission (IEC) Standards
- 25      – IEC 60056 – A.C. High Voltage Circuit Breakers
- 26      – IEC 60076 – Power Transformers
- 27      – IEC 60099-4 – Surge Arresters
- 28      – IEC 60298 – A.C. Metal-Enclosed Switchgear and Controlgear for Rated Voltages Above
- 29      1 kV and Up to and Including 52 kV
- 30      – IEC 60694 – Specifications Common for High Voltage Switchgear and Controlgear
- 31      Standards
- 32      – IEC 62271 – High-Voltage Switchgear and Controlgear
- 33      • Institute of Electrical and Electronics Engineers (IEEE) Standards

- 1 – IEEE 80 – IEEE Guide for Safety in AC Substation Grounding
- 2 – IEEE 242 – IEEE Recommended Practice for Protection and Coordination of Power
- 3 Systems
- 4 – IEEE 485 – IEEE Recommended Practice for sizing Large Lead Storage Batteries for
- 5 Generating Stations and Substations
- 6 – IEEE 519 – IEEE Recommended Practices and Requirements for Harmonic Control in
- 7 Electrical Power Systems
- 8 – IEEE 980 – IEEE Guide for Containment and Control of Oil Spills in Substations
- 9 – IEEE 1189 – IEEE Guide for Selection of Valve-Regulated Lead-Acid (VRLA) Batteries
- 10 for Stationary Applications
- 11 – IEEE 1427 – IEEE Guide for Recommended Electrical Clearances and Insulation Levels
- 12 in Air-Insulated Electric Power substations
- 13 – IEEE C2 – National Electrical Safety Code
- 14 – IEEE C9.1 – Standard for Insulation Coordination
- 15 – IEEE C37.04 – Rating Structure for AC High-Voltage Circuit Breakers Rated on a
- 16 Symmetrical Current Basis
- 17 – IEEE C37.2 – Electric Power System Device Function Numbers and Contact Designations
- 18 – IEEE C37.20.2 – IEEE Standard for Metal-Clad Switchgear
- 19 – IEEE C37.20.4 – IEEE Standard for Indoor AC Switches (1 kV-38 kV) for Use in Metal-
- 20 Enclosed Switchgear
- 21 – IEEE C37.30 – IEEE Standard Requirements for High-Voltage Switches
- 22 – IEEE C37.90 – IEEE Standard for Relays and Relay Systems Associated with Electric
- 23 Power Apparatus
- 24 – IEEE C57.12.00 – Standard General Requirements for Liquid-Immersed Distribution,
- 25 Power, and Regulating Transformers
- 26 – IEEE C57.12.10 – American National Standards for Transformers – 230 kV and Below
- 27 833/958 through 8,333/10,417 kVA, Single-Phase, and 750/862 through
- 28 60,000/80,000/100,000 kVA Three-Phase, without Load Tap Changing; and 2,750/4,687
- 29 Through 60,000/80,000/1000,000 kVA with Load Tap Changing – Safety Requirements
- 30 – IEEE C57.12.80 – Standard Terminology for Power and Distribution Transformers
- 31 – IEEE C57.91 – Guide for Loading Mineral-Oil Immersed Transformers (Including
- 32 Corrigendum 1)
- 33 – IEEE C62 – IEEE Surge Protection Standards Collection

- 1       – IEEE C62.11 – IEEE Standard for Metal-Oxide Surge Arresters for Alternating Current
- 2       Power Circuits
- 3       – IEEE C80.1 – Rigid Steel Conduit, Zinc Coated
- 4       – IEEE C84.1 – Voltage Ratings for Electrical Power Systems and Equipment (60 Hz)
- 5       – IEEE Std C95.1 – Standard for Safety Levels with Respect to Human Exposure to
- 6       Electromagnetic Fields, 3 kHz - 300 GHz
- 7       – IEEE Std C95.6 – Standard for Safety Levels with Respect to Human Exposure to
- 8       Electromagnetic Fields, 0 – 3 kHz
- 9       • National Electrical Manufacturers Association (NEMA) Standards
- 10      – NEMA 250 – Enclosures for Electrical Equipment (1,000 Volts Maximum)
- 11      – NEMA BU 1 – Busways
- 12      – NEMA FG 1 – Fiberglass Cable Tray Systems
- 13      – NEMA LA 1 – Surge Arresters
- 14      – NEMA PB 1 – Panelboards
- 15      – NEMA PE 5 – Utility Type Battery Chargers
- 16      – NEMA RN1 – Polyvinyl-Chloride (PVC) Externally Coated Galvanized Rigid Steel
- 17      Conduit and Intermediate Metal Conduit
- 18      – NEMA SG 4 – AC High-Voltage Circuit Breakers
- 19      – NEMA SG 6 – Power Switching Equipment
- 20      – NEMA TC 2 – Electrical Polyvinyl Chloride (PVC) Tubing and Conduit
- 21      – NEMA TC 14 – Reinforced Thermosetting Resin Conduit (RTRC) and Fittings
- 22      – NEMA TR 1 – Transformers, Regulators and Reactors
- 23      – NEMA VE 1 – Metallic Cable Tray Systems
- 24      – NEMA WC 70 – Nonshielded 0-2 kV Cables (ICEA S-95-658)
- 25      – NEMA WD 1 – General Color Requirements for Wiring Devices
- 26      • National Fire Protection Association (NFPA) Codes and Standards
- 27      – NFPA 70 – National Electrical Code
- 28      – NFPA 101 – Life Safety Code
- 29      – NFPA 110 – Standard for Emergency and Standby Power Supply Systems
- 30      – NFPA 130 – Standard for Fixed Guideway Transit and Passenger Railway Systems
- 31      – NFPA 780 – Standard for Lightning Protection Systems
- 32      • Underwriters' Laboratories (UL) Publications

- 1 – UL 5 – Surface Metal Raceways and Fittings
- 2 – UL 6 – Rigid Metal Conduit
- 3 – UL 44 – Thermoset-Insulated Wires and Cables
- 4 – UL 50 – Enclosures for Electrical Equipment
- 5 – UL 67 – Panelboards
- 6 – UL 83 – Thermoplastic Insulated Wires and Cables
- 7 – UL 489 – Molded-Case Circuit Breakers, Molded Case Switches and Circuit-Breaker
- 8 Enclosures
- 9 – UL 651 – Schedule 40 and 80 Rigid PVC Conduit
- 10 – UL 924 – Emergency Lighting and Power Equipment
- 11 – UL 1008 – Transfer Switches
- 12 – UL 1059 – Terminal Blocks
- 13 – UL 1581 – Reference Standard for Electrical Wires, Cables and Flexible Cords

## 20.3 Overview and General Design Criteria

### 20.3.1 Traction Electrification System

14 A Traction Electrification System (TES) is the combination of the TPS, OCS, and the traction  
15 power return system, together with appropriate interfaces to the TES-related Supervisory  
16 Control and Data Acquisition (SCADA) System. It forms a fully functional 2x25 kV ac traction  
17 power supply and distribution system and provides the traction power to the electrically  
18 powered vehicles on the high speed railway line.

### 20.3.2 Description of Traction Electrification System

19 Terms used in this chapter include the following:

- 20 • **Traction Power Supply System (TPS)** – TPS is the railway traction distribution network  
21 used to provide energy to high-speed electric trains, which comprises 3 types of TPF  
22 (traction power substations, switching station, and paralleling stations) in addition to  
23 connections to the OCS and to the traction return and grounding system, and wayside  
24 power control cubicles for control and operation of wayside motor operated disconnect  
25 switches.
- 26 • **Traction Power Substations (SS)** – An electrical installation where power is received at  
27 high voltage and transformed to the voltage and characteristics required at the catenary and  
28 negative feeders for the nominal 2x25 kV system, containing equipment such as



transformers, circuit breakers and sectionalizing switches. It also includes the incoming lines from the power supply utility.

- **Switching Stations (SWS)** – This is an installation where the supplies from 2 adjacent SS are electrically separated and where electrical energy can be supplied to an adjacent, but normally separated electrical section during contingency power supply conditions. It also acts as a Paralleling Station.
- **Paralleling Stations (PS)** – An installation that helps boost the OCS voltage and reduce the running rail return current by means of the autotransformer feed configuration. The Negative Feeder (NF) and the catenary conductors are connected to the 2 outer terminals of the autotransformer winding at this location with the central terminal connected to the rail return system. OCS sections can be connected in parallel at PS locations.
- **Traction Power Facilities (TPF)** – TPF is a general term that encompasses substations, switching stations, and paralleling stations.
- **Traction Power Return System** – A traction power return system refers to all conductors including the grounding system for the electrified railway tracks, which form the intended path of the traction return current from the wheel-sets of the traction units to the substations under normal operating conditions and the total return current under fault conditions. The conductors may be of the following types:
  - Running rails
  - Impedance bonds
  - Static wires, and buried ground or return conductors
  - Rail and track bonds
  - Return cables, including all return circuit bonding and grounding interconnections
  - Earth
  - Negative feeders (as a consequence of the configuration of the autotransformer connections)
- **Overhead Contact System (OCS)** – OCS comprises the following:
  - The aerial supply system that delivers 2x25 kV traction power from substations to the pantographs of high-speed electric trains, comprising the catenary system messenger and contact wires, stitch wires and hangers, associated supports and structures (including poles, portals, headspans and their foundations), manual and/or motor operated isolators, insulators, phase breaks, conductor termination and tensioning devices, downguys, and other overhead line hardware and fittings
  - Portions of the traction power return system consisting of the negative feeders and aerial static wires, and their associated connections and cabling

- 1 • **Catenary** – An assembly of overhead wires consisting of, as a minimum, a messenger wire,  
2 carrying vertical hangers that support a solid contact wire (which is the contact interface  
3 with operating electric train pantographs), and which supplies power from a central power  
4 source to an electrically-powered vehicle.
- 5 • **Messenger Wire** – In catenary construction, the OCS Messenger Wire is a longitudinal bare  
6 stranded conductor that physically supports the contact wire or wires either directly or  
7 indirectly by means of hangers or hanger clips and is electrically common with the contact  
8 wire(s).
- 9 • **Contact Wire** – A solid grooved, bare aerial, overhead electrical conductor of an OCS that is  
10 suspended above the rail vehicles and supplies the electrically powered vehicles with  
11 electrical energy through roof-mounted current collection equipment (pantographs), and  
12 with which the current collectors make direct electrical contact.
- 13 • **Negative Feeder (NF)** – Negative Feeder is an overhead conductor supported on the same  
14 structure as the catenary conductors, is at a voltage of 25 kV with respect to ground, but is  
15 180 degrees out-of-phase with respect to the voltage on the catenary. Therefore, the voltage  
16 between the catenary conductors and the negative feeder is 50 kV nominal. The NF connects  
17 successive feeding points and is connected to 1 terminal of an autotransformer in the TPF  
18 via a circuit breaker or disconnect switch. At these facilities, the other terminal of the  
19 autotransformer is connected to a catenary section or sections via circuit breakers or  
20 disconnects.
- 21 • **Static Wire** – A wire (usually installed aerially adjacent to or above the catenary conductors  
22 and negative feeders) that connects OCS supports collectively to ground or to the grounded  
23 running rails to protect people and installations in case of an electrical fault. In an ac  
24 electrification system, the static wire forms a part of the traction power return circuit and is  
25 connected to the running rails at periodic intervals and to the traction power facility ground  
26 grids. If mounted aerially, the static wire may also be used to protect the OCS against  
27 lightning strikes. It is sometimes termed “aerial ground wire.”
- 28 • **Wayside Power Control Cubicle (WPC)** - WPC is an enclosure for power supply equipment  
29 for operation of motorized disconnect switches and the associated SCADA equipment  
30 located at the wayside.
- 31 • **Electrical Section** – This is the entire section of the OCS, which, during normal system  
32 operation, is powered from an SS circuit breaker. The SS feed section is demarcated by the  
33 phase breaks of the supplying SS and by the phase breaks at the nearest SWS or line end. An  
34 electrical section may be subdivided into smaller elementary electrical sections.
- 35 • **Elementary Electrical Section** – The smallest section of the OCS power distribution system  
36 that can be isolated from other sections or feeders of the system by means of disconnect  
37 switches and/or circuit breakers.

- **Rail Potential** – Rail Potential is defined as the voltage between running rails and ground occurring under operating conditions when the running rails are utilized for carrying the traction return current or under fault conditions.

### 20.3.2.1 System Configuration

The TES shall have a 2x25 kV autotransformer feed type configuration.

The TES configuration shall utilize Traction Power Substations (SS) with main transformers, and Switching Stations (SWS) and Paralleling Stations (PS), both with autotransformers, which provide 25 kV (nominal) voltage to the catenary with respect to remote ground, and also 25 kV to along-track Negative Feeders (NF) with respect to remote ground. Both of these voltages are 180 degrees out-of-phase with each other; therefore, the catenary is at 50 kV with respect to the NF.

Traction power will be supplied to the trains from wayside TPF through the catenary, which distributes the power to the train pantographs. The pantographs, mounted on the roof of the rolling stock, collect the traction power from the catenary through mechanical contact by running/sliding under the contact wire. The electrical circuit is completed back to the source SS via multiple return paths, including running rails, static wires, ground, and the NF. The power for the trains will be distributed along the tracks by the OCS from the SS, which transform 2 phases of the high-voltage (HV) (115 kV or 230 kV as applicable), 3-phase, utility power to the 2x25 kV single-phase power of the autotransformer feed system. Some of the high-voltage incoming feed points are at 230 kV and some are at 115 kV. Along the open route, the NF shall be bare overhead conductors, 1 NF per main track, attached to the catenary structures with brackets and insulators. The catenary shall consist of a messenger wire and a contact wire. The contact wire will be suspended from the messenger wire by the means of hangers, and tied electrically to the messenger wire by means of jumper wires. The details of OCS design criteria are presented in the *Overhead Contact System and Traction Power Return System* chapter.

Autotransformers will be provided along the line at locations spaced generally 5 miles apart, interconnecting catenary, NF and rails. The autotransformer turns ratio shall be 2:1 of primary (catenary-to-NF) to secondary (catenary-to-rails) windings, in order to step down the 50 kV distribution voltage between catenary and NF, to 25 kV nominal between catenary and rails, considered suitable for the trains.

### 20.3.2.2 TPS Performance Requirements

The TPS and OCS shall be designed to meet the following performance requirements:

- The line speed for the sections of the California High-Speed Train Project (CHSTP) dedicated to very high speed is a maximum operating speed of 220 mph and a maximum design speed of 250 mph.
- The design peak frequency of trains for Phase I is 12 trains in each direction with 9 trains consisting of 2 trainsets (1,312 feet or 400 m) and 3 trains consisting of 1 trainset (656 feet or 200 m).

- There shall be no degradation of train performance in case of single electrical contingency conditions and no stranding of trains in case of double electrical contingency conditions. Refer to Section 20.7.4 for additional details.
- The system will support the maximum train current, the tractive effort and braking effort available/missible at different speeds, the train acceleration/deceleration/adhesion characteristics, and other relevant parameters of a typical modern high-speed train (HST) system. (This information shall be provided by the Rolling Stock Contractor.)

### 20.3.3 RAMS Requirements

These are specified elsewhere in the contract documents.

### 20.3.4 General Design Requirements

**Safety Design** – The design of the TPS and associated site works shall incorporate the following principles:

- Avoid, eliminate, or reduce hazards (identified by engineering hazard analysis) through design choices, material selection, or substitution.
- Incorporate fail-safe principles where failures could disable the system, cause human injury, damage to equipment, or inadvertent operation of critical equipment.
- Locate equipment components so that access to them by the required personnel during operation, maintenance, repair, or adjustment shall not require exposure to hazards such as entrapment, chemical burns, electrical shock, cutting edges, sharp points, or toxic atmospheres.
- Provide measures designed to prevent or discourage unauthorized persons from entering hazardous areas. Refer to the *Electromagnetic Compatibility and Interference* chapter for additional requirements.
- All components containing or generating obnoxious, flammable, and harmful gases shall be vented to the outside.
- Cables and wires of different systems and/or high- and low-voltage conductors shall be physically segregated/separated from each other and rated in accordance with the requirements specified in CEC, NEC and IEEE-1100, as applicable. Refer to Section 20.10.7 for additional requirements.

**Product Selection** – All prescribed equipment, materials, cables and appurtenances shall be either certified by a nationally recognized testing laboratory, or compliant with relevant ANSI/IEEE, EN or local standards.

- All prescribed equipment, materials, cables and appurtenances shall be designed, constructed and have a “proven track record” of operating within the intended application and operating environment.

- All equipment, materials, cables and appurtenances shall be produced by manufacturers that have at least 5 consecutive years of experience in the production of such products.
- All prescribed equipment, materials, cables and appurtenances shall adhere to applicable practices recommended by AREMA.

**Uniformity** – Equipment enclosures, assemblies, sub-assemblies, and/or components that do not differ in operational, functional, and/or performance characteristics shall be designed so that all components are positioned in the same location and internal wiring is routed between components in a like manner.

Where identical installations exist, the following requirements shall be adhered to (unless site conditions prevent it):

- Equipment enclosures shall be mounted and installed in a like manner.
- Penetrations for conduit, grounding, and access panels shall be located in the same place.
- The location of equipment relative to adjacent equipment shall not differ.
- The routing of conduit, cable tray, and cables between equipment enclosures shall not differ.
- Termination hardware shall be located in a like manner.
- Cables and wire terminations shall be located in a like manner.

**Accessibility and Equipment Arrangement** – Working clearances on all sides of equipment shall be provided (per the equipment manufacturer's recommendations, power utility requirements, CEC, NEC and/or NESC); as well as horizontal and vertical clearance for equipment removal, replacement and/or maintenance (without impacting other energized equipment), and door openings/hatches.

Each prefabricated enclosure for 25 kV indoor switchgear shall have adequate area to accommodate the electrical equipment, raceways/cable trays and ancillary components, and personnel, and allow for easy removal and replacement of any equipment item.

All switchgear enclosures shall have front- and rear-access doors and/or removable panels.

**Environmental Conditions** – The TPS design shall ensure minimum electric energy loss in the equipment and maximum possible utilization of regenerated energy. The TPS design shall be suitable for the environmental conditions that have been specified in other chapters of this Design Criteria (e.g., the *General*, *Seismic*, *Structures*, and *Facility Power and Lighting Systems* chapters) and in other contract documents.

## 20.4 Key Design Criteria

### 20.4.1 Traction Power Substations

**General** – Based on the results of a traction power study involving load flow analysis, it has been determined that the needs of the project can be served by SS spaced approximately 30 miles apart. The SS locations have been determined in consideration of the results of the load flow simulation analysis, proximity to high-voltage transmission facilities and feasibility of drawing the required HV power, and availability of real estate.

**HV Connection Scheme** – At each SS, 2 separate 3-phase HV circuits will be drawn from the power utility network. These circuits shall be originating from different utility substations, at least from different bus systems. These may be carried on the same transmission towers. Two equally sized HV traction power transformers shall be provided at each SS, with each transformer supplied from a separate incoming circuit. Both transformers shall be energized under normal TES configuration—1 of them supplying power to the feed section north of the SS and the other to the section to the south—with the 2 feed sections separated by a phase-break at the SS. Both HV power transformers shall be individually capable of supplying the full normal load of the SS. As a special case, a traction power substation located at the junction of 2 lines merging into 1 line will have 3 HV power transformer configuration. Refer to Standard and Directive Drawings for details.

The HV transformers shall be single-phase, with their primary windings connected to 2 phases of the utility 115 (or 230) kV 3-phase system. The secondary winding of the HV transformer shall either be a single winding with a grounded midpoint connected also to the running rails, or comprising 2 separate counter-phase secondary windings connected in series, with the common point grounded and connected to the running rails.

For HV transformer details refer to Section 20.9.4.

**Impact on the HV Utility Grid** – The load imposed by the railway's traction power substations on the electric utility's 3-phase, 115 (or 230) kV system will be single-phase, nonlinear and rapidly variable over time. Since each HV transformer will draw power from only 2 phases of a 3-phase system, some current and voltage imbalances will inevitably be caused in the HV supply grid. As a rule, the railway load is characterized by 3 factors:

- Phase unbalance caused by the single-phase nature of the load. Of the current and voltage imbalances, the voltage imbalance is of greater concern, as it affects the power quality of other utility customers.
- Voltage flicker, caused by the highly variable nature of the load.
- Harmonic distortion, produced by the power convertors on the trains.

In order to mitigate the effects of the unbalanced loading, the single-phase connections of the HV transformers shall be alternated from 1 pair of phases feeding 1 transformer to a different

1 pair of phases feeding the other transformer at the same SS; the phase connections shall be  
2 changed between substations. This will tend to partially balance the load between the 3 phases  
3 regionally.

4 The TPS design shall address power quality issues, such as voltage imbalance, voltage flicker,  
5 and harmonic distortion caused by the railway load on the HV supply system, arising from  
6 operation of the HST. The mitigation measures shall be submitted to the Authority for review.

7 **Configuration and Operational Flexibility** – The traction power substation's single-line  
8 diagram and reconfiguration capabilities shall be such that in the event of a power loss to 1 of  
9 the incoming 115 (or 230) kV feeder lines, or temporary outage of 1 of the transformers, or  
10 transformer-related equipment, or outage of a 25 kV bus section, the remaining transformer  
11 shall be able to supply power to the feed sections north and south of the SS.

12 The "positive" bus of the SS (the bus supplying power to the catenary) shall be split into 2  
13 sections interconnected via a normally open (N.O.) motorized tie circuit breaker, with each bus  
14 section supplied by a different transformer under normal conditions. Each section of the  
15 positive bus shall feed 2 different catenary electrical sections in normal TES configuration for 2  
16 main tracks. (Similar requirements apply to 4 main track sections.) The "negative" bus of the SS  
17 (the bus supplying power to the along-track NF) shall be sectionalized likewise. Tie-breakers of  
18 the catenary and the NF buses shall be interlocked with each other so that they open and close  
19 together; these shall be, in effect, double-pole circuit breakers. The tie-breakers shall also be  
20 interlocked with their associated disconnect switches and the main transformer circuit breakers  
21 to prevent inadvertent bridging of 2 incoming supplies.

22 The outer terminals of the secondary winding of each HV transformer shall be connected to the  
23 positive and negative buses (the bus sections corresponding to the particular transformer)  
24 through a double-pole circuit breaker. The positive and negative buses in turn shall be  
25 connected to the catenary and NF, respectively, through single-pole circuit breakers and in-  
26 series connected no-load motorized disconnect switches.

27 Jumper-type motorized, N.O., load-break disconnect switches shall also be provided, connected  
28 between each pair of in-phase, same-side, single-pole circuits to allow for 1 25 kV circuit to feed  
29 both track sections under emergency conditions of feed extension because of complete failure of  
30 any SS. Furthermore, N.O. trackside, motorized, load-break switches shall be installed at the  
31 substation's phase break, to provide for electrical continuity in emergency conditions between  
32 the catenary and NF, respectively, on either side of the phase break. The flexibility and re-  
33 configuration capability of the single line diagram of the SS on the 50/25 kV side shall be such  
34 that a loss of 1 single-pole circuit breaker, or disconnect switch, or interconnecting cable still  
35 allows the SS to feed the whole feed zone of the SS without having to de-energize 1 of the HV  
36 transformers. Refer to the Standard and Directive Drawings for details.

## 20.4.2 Switching Stations

**General** – The SWS is a facility interfacing the feeding sections of adjacent SS. This is an installation at which electrical energy can be supplied to an adjacent, but normally separated electrical section during contingency power supply conditions.

**Connection** – The SWS shall include a phase break because the ac voltages on either side of any SWS will be of different phases (or even if these have the same phase sequence, the angular displacement may be different). In normal operations, the phase break shall be open, isolating the 2 feed sections. Autotransformers (ATs) shall be connected on either side of the phase break, 1 AT per side, serving as the last AT of the respective feed section. As a special case, a switching station located at the junction of 2 lines merging into 1 line will have a 3 autotransformer configuration. Refer to the Standard and Directive Drawings for details.

The catenary and NF buses shall be connected in turn to the primary winding terminals of the AT via a double-pole circuit breaker. The AT winding center tap shall be connected to a neutral bus, which shall be locally grounded and connected to the running rails of both tracks (through impedance bonds as required) and to the static wires.

**Configuration** – The SWS equipment shall include switchgear (25 kV circuit breakers) and motorized disconnect and bypass switches in a configuration that allows isolation of an AT in case of problems, and as required for maintenance. The SWS design shall provide for electrical continuity across the phase break in contingency operations, in the event the SS on 1 side is out-of-service. In such instances, the SS on the other side of the SWS shall be used to provide power to the sections normally served by the outaged SS. This shall be achieved by interconnecting the catenary and NF on both sides of the SWS, which is achieved by closing N.O. tie circuit breakers. Furthermore, N.O. trackside, motorized, load-break disconnect switches shall be installed at the SWS phase break, to provide for electrical continuity in emergency conditions between the OCS and NF, respectively, on either side of the phase break. Suitable interlocking shall be provided between the tie-breakers and the phase-break bridging disconnect switches at the SWS with the circuit breakers of adjacent substations to prevent inadvertent bridging of feeds from the 2 substations. Refer to the Standard and Directive Drawings for details.

## 20.4.3 Paralleling Stations

**General** – The Paralleling Station (PS), known also as an AT station, is a facility featuring an AT as part of the 2x25 kV TES, and associated switchgear and disconnect switches. The PS helps boost the OCS voltage and reduce the running rail return current by means of the autotransformer feed configuration. The AT installed along the line in the PS steps down the 50 kV nominal voltage between catenary and negative feeder to the 25 kV level between catenary and running rails. Similar to the SS, the number and locations of the PS shall be determined based on the results of a traction power study, and by taking into account environmental and real estate considerations.



1 **AT Connection** – One AT serving all tracks shall be installed at each PS, along with a line-up of  
2 medium voltage switchgear containing separate buses for connections to the catenary and NF  
3 circuits. The switchgear shall include single-pole 25 kV catenary circuit breakers and NF circuit  
4 breakers, and double-pole 50 kV AT circuit breakers. The catenary and NF conductors shall be  
5 connected to the switchgear buses via no-load type motorized disconnect switches and the  
6 switchgear. The catenary and NF buses shall be connected in turn to the primary winding  
7 terminals of the AT via a double-pole circuit breaker. The AT winding center tap shall be  
8 connected to a neutral bus, which shall be locally grounded and connected also to the running  
9 rails of both tracks (through impedance bonds as required) and to the static wires.

10 **PS Configuration** – The PS shall provide a booster connection to the OCS, without OCS  
11 sectionalizing gap. There shall be no sectionalizing of the catenary and NF circuits at the PS. The  
12 catenary of each main track shall be connected via single-pole circuit breaker to the catenary bus  
13 of the PS. Likewise, the NF wire of each track shall be connected via tap connection and single-  
14 pole circuit breaker to the NF bus. For a PS serving only 2 main tracks this configuration results  
15 in 4 single-pole circuit breakers overall—2 for the OCS bus and 2 for the NF bus. Refer to the  
16 Standard and Directive Drawings for details.

#### 20.4.4 Traction Electrification System Sectionalizing Requirements

17 **Main Tracks** – The catenary shall be sectionalized between the tracks and longitudinally on the  
18 same track along the route in order to limit the extent of an outage zone due to faults or  
19 maintenance. Longitudinal sectionalizing of the catenary shall be provided at the SS, SWS, and  
20 at all track interlockings and track turnouts. The sectionalizing at the SS and SWS shall be of the  
21 phase break type; elsewhere it shall be a regular sectioning gap (insulated overlap or air gap  
22 type on the main tracks, with section insulators permitted on crossover and turnout tracks).

23 At TPF, the sectionalizing gaps shall be provided with N.O. no-load type motorized disconnect  
24 switches that can be closed during contingency operations if the catenary on both sides of the  
25 sectionalizing gap needs to be electrically continuous.

26 At track interlockings, the longitudinal sectionalizing gaps shall be provided with normally-  
27 closed (N.C.) circuit breakers and/or motorized disconnect switches, which can be opened  
28 during contingency operations to isolate a smaller segment of 1 track between adjacent  
29 interlockings (contained within an electrical section) or within an interlocking and the adjacent  
30 SS or SWS (contained within an electrical section), and permit single-track operations on the  
31 other track. At back-to-back crossovers, the sectionalizing arrangement shall be such that the  
32 catenary of any track on either side of the interlocking can be isolated selectively.

33 Concerning the negative phase, 2 parallel along-track NF shall be provided along the route (1  
34 per main track) regardless of the number of parallel tracks at a given location. Longitudinally,  
35 the NF system shall be sectionalized at the SS and SWS.

36 **Power Supply to Sidings and Extra Terminal Tracks** – As a rule, the power supply to short  
37 segments of track sidings not used for regular train service along the main line shall be derived

from the adjacent main track via N.C. no-load type motorized disconnect switch across the sectionalizing gap at the turnout. If the siding has turnouts from the main track at both ends, sectionalizing gaps and switches shall be provided at both ends, with 1 of the disconnect switches being N.O. and the other an N.C. type. Such feeding arrangement shall be used regardless of whether the track siding is close to a TPF or not. At terminals with more than 2 tracks, traction power for the additional tracks shall be derived from the main tracks in similar fashion (i.e., using N.C. motorized disconnect switches across sectionalizing gaps at the turnouts). Both sidings and extra terminal tracks shall be radially fed from the adjoining main track through a single connection point. Exceptions to the above rule shall be submitted to the Authority for review.

**Overhead Line Switches** – The N.O. disconnect switches at sectionalizing gaps at TPF, and the N.C. sectionalizing or power supply switches at track interlockings shall be load-break type, motorized, featuring remote status monitoring and remote control capabilities.

Refer to the *Overhead Contact System and Traction Power Return System* chapter for additional requirements.

#### 20.4.5 Ratings and Configuration for Each Type of Traction Power Facilities

Ratings and configuration for each type of TPF shall be standardized to the extent practical. In general, each SS shall have 2 60 MVA power transformers, each SWS shall have 2 20 MVA AT and each PS shall have 1 20 MVA autotransformer. The locations of the SS and the SWS are fixed because of the location of HV power supply points.

#### 20.4.6 High-Voltage Utility Connections

Different feeding arrangements on the HV side include direct feed from a utility substation or loop-in-loop-out feeding arrangement from its HV transmission network. The specific feeding arrangement at any SS depends on site-specific conditions, the configuration of the power utility company's network at that location, and the policies/procedures of the power utility company.

At some locations, the concerned power utility company is required to install an HV switching station adjacent to the SS location for supplying HV power to the SS. The space requirements for this utility switching station depends upon the HV system voltage, the feeding arrangement and the configuration of the power utility's network, and varies from site to site. These sites have been identified and environmentally cleared. Refer to Section 20.9.23 for additional information.

#### 20.4.7 Regenerative Braking

The TPS and the associated OCS shall be designed to permit the use of regenerative braking as a service brake and as an emergency brake.

1 The use of regenerative braking shall be facilitated by 1 or more of the following:

- 2 • Transfer of braking energy back into the OCS for use by another train(s) that is (are)  
3 drawing power from the OCS and is (are) located in the same electrical section as the  
4 braking train
- 5 • Transfer of braking energy back to the power supply utility company's network in case  
6 trains in the same electrical section do not draw the full regenerated power
- 7 • Provision of rheostatic braking resistors or other electrical energy absorbing units onboard  
8 the trains
- 9 • Provision of automatic assured receptivity unit braking resistors within TPS substations

10 The SS control and protection devices shall be configured to allow regenerative braking.

11 Refer to the *Rolling Stock – Core System Interfaces* chapter for additional details.

#### **20.4.8 Power Supply for Rolling Stock Maintenance Facilities**

12 In addition to the traction power substations required for electrification of the main track, 4  
13 more substations will be required at the rolling stock maintenance facilities proposed in the  
14 Central Valley, the San Francisco area, the San Jose/Gilroy area, and the Los Angeles/Anaheim  
15 area. These substations will provide power supply for the following:

- 16 • Traction Purposes – for operation of trains in the storage yard/siding and maintenance  
17 tracks and for feeding auxiliary power to trains standing therein with raised pantographs
- 18 • Facility Power Services – for buildings, machinery and plant, illumination, HVAC, water  
19 pumps, control power, etc.

20 The TPS of the maintenance facilities and their yards will be conventional 1x25 kV ac system.  
21 Two main traction power transformers of 30 MVA each shall be planned for each facility  
22 substation, 1 supplying the traction load in the facility, and the other acting as 100 percent  
23 standby. Refer to the *Facility Power and Lighting Systems* chapter for design of the facilities power  
24 system. Two independent HV circuits will be brought from the power supply utility network at  
25 each maintenance facility location; 1 circuit normally feeding the traction load, and the other the  
26 facility power load. The substation switchgear and other equipment shall be so configured that  
27 either of the incoming HV circuits is able to feed the traction and facility power loads  
28 simultaneously.

29 Normally, the TPS of the rolling stock facility will be segregated from the traction power supply  
30 of the main track. The traction power system design shall provide for suitable and safe  
31 interconnection between the 2 adjacent systems under contingency conditions.

## 20.5 Related Documentation

- 1 Related documentation refers to traction power load flow studies, impact assessment studies,  
2 and interconnection feasibility studies, etc. These documents have not been finalized.

## 20.6 System Voltage and Frequency

### 20.6.1 System Voltage

3 The system voltage ( $U$ ) shall be the potential at the train's current collector or elsewhere on the  
4 catenary, measured between the catenary and the rail return circuit. It shall be the root mean  
5 square (rms) value of the fundamental ac voltage and its values shall be as follows (refer to  
6 European Standard EN 50163 – 2004: Railway Applications – Supply Voltages of Traction  
7 Systems):

- 8 • The nominal voltage ( $U_n$ ), (the designated value for the system voltage) shall be 25 kV.
- 9 • The highest permanent voltage ( $U_{\max1}$ ) (the maximum value of the voltage likely to be  
10 present indefinitely) shall be 27.5 kV.
- 11 • The highest non-permanent voltage ( $U_{\max2}$ ) (the maximum value voltage likely to be present  
12 for a limited period of time as defined below) shall be 29.0 kV.
- 13 • The lowest permanent voltage ( $U_{\min1}$ ) (the minimum value of voltage likely to be present  
14 indefinitely) shall be 19.0 kV.
- 15 • The lowest non permanent voltage ( $U_{\min2}$ ) (the minimum value of voltage likely to be  
16 present for a limited period of time as defined below) shall be 17.5 kV.

### 20.6.2 Voltage Related Requirements

17 The following voltage related requirements shall be fulfilled:

- 18 • The duration of voltages between  $U_{\min1}$  and  $U_{\min2}$  shall not exceed 2 minutes.
- 19 • The duration of voltages between  $U_{\max1}$  and  $U_{\max2}$  shall not exceed 5 minutes. If voltage  
20 between  $U_{\max1}$  and  $U_{\max2}$  is reached, it shall be followed by a level below or equal to  $U_{\max1}$  for  
21 an unspecified period. Voltages between  $U_{\max1}$  and  $U_{\max2}$  shall only be reached for non  
22 permanent conditions such as regenerative braking.
- 23 • The voltage at the busbar of the substation at no-load conditions shall be less than or equal  
24 to  $U_{\max1}$ .
- 25 • Under normal operating conditions, voltages shall be within the range of  $U_{\min1} \leq U \leq U_{\max2}$ .
- 26 • Under abnormal operating conditions, the voltage in the range of  $U_{\min2} \leq U \leq U_{\min1}$  shall not  
27 cause any damage or failure, and shall permit continuing vehicle operation with some  
28 significant degradation. Rated vehicle power and performance may not be available, but  
29 reduced operation may be possible assuming onboard logic will automatically degrade the

performance of the traction system (rolling stock) and auxiliaries. Typical abnormal operating conditions could include substation outage, loss of 1 or more transformers at the substation, utility supply problems, etc. Refer to Section 20.7.4 for details.

- The setting of under-voltage relays in fixed installations or onboard rolling stock shall be from 85 percent to 95 percent of  $U_{min2}$ .
- Both the following acceptance criteria for “Quality Index of Power Supply” for ac 2x25 kV autotransformer feed configuration shall be satisfied (refer to Section 8 of EN 50388:2005 – Railway Applications: Power Supply and Rolling Stock – Technical Criteria for the coordination between power supply [substations] and rolling stock to achieve interoperability):

$$U_m = > 22.5 \text{ kV}$$

$$U_i \Rightarrow 19 \text{ kV } (U_{min1} - \text{Lowest permanent voltage})$$

Where:

Mean Useful Voltage ( $U_m$ ) is the mean value of all rms voltages analyzed in the system simulation study, and gives an indication of the quality of the power supply for the entire system during the peak traffic period in the timetable, and

$U_m = \Sigma U_i / N$  where  $U_i$  is the rms ac voltage over the  $i^{\text{th}}$  second during the peak period for all trains in the system, and  $N$  is the total number of observations.

The TPS Designer shall verify this by a traction power simulation study using the specific CHSTP design parameters.

### 20.6.3 Frequency

The nominal frequency of the supply voltage shall be 60 Hz.

Unless the requirements of the power supply utilities are more stringent, for systems with synchronous connection to an interconnected system, under normal operating conditions the mean value of the fundamental frequency measured over 10 seconds shall be within a range of the following:

60 Hz +/- 1 percent (i.e., 59.4 Hz ... 60.6 Hz) for 99.5 percent of a year

60 Hz +4 percent/-6 percent (i.e., 56.4 Hz ... 62.4 Hz) for 100 percent of the time

## 20.7 System Configuration

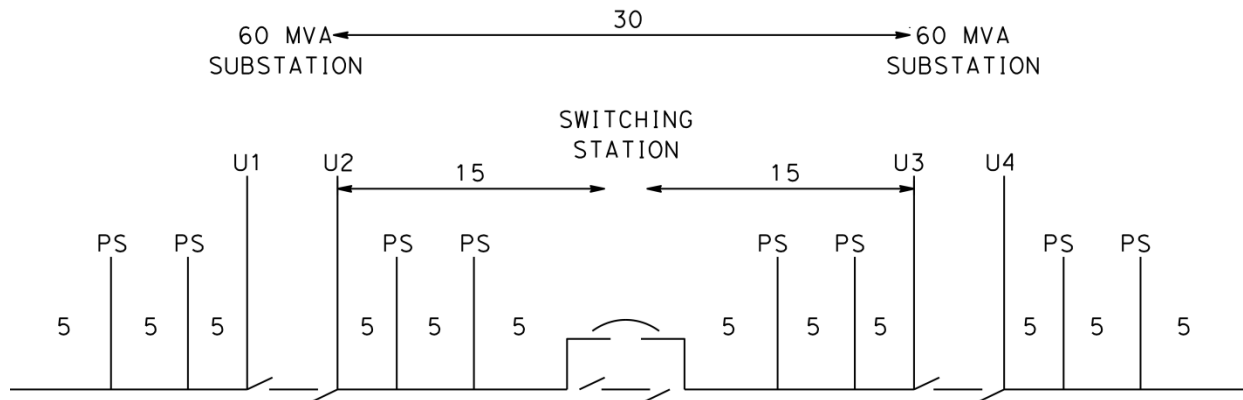
### 20.7.1 Verification of Configuration

- 1 The configuration of the TPS and distribution system shall be confirmed by a computer-based
- 2 traction power load flow study.

### 20.7.2 Spacing of Traction Power Facilities

- 3 SS sites shall be located at approximately 30-mile intervals along the Authority's right-of-way.
- 4 SWS sites shall be located approximately midway between adjacent SS sites.
- 5 PS sites should be located at approximately 5-mile intervals between switching stations and
- 6 substation sites.

7 **Figure 20-1: Simplified Feeding Arrangement – 2x25 kV Autotransformer Feed System**



- 9 At some locations because of site-specific alignment, geographical or other constraints,
- 10 deviations from the criteria may be permitted where the guideline pertaining to location of PS
- 11 sites is not practically achievable. Modified spacing of the TPF shall be proved by a computer-
- 12 based traction power load-flow analysis and details submitted to the Authority for review. The
- 13 locations of the SS and the SWS shall not be changed.

- 14 With a view to preventing stalling of trains at the phase-breaks, the phase breaks of SS and SWS
- 15 shall not be located within a 4500 feet distance from the nearest end of any railway station,
- 16 wayside crossover, or tunnel portal. The track grades at the phase breaks and to 4500 feet on
- 17 either side shall not be greater than 0.6 percent. Any exception shall be submitted to the
- 18 Authority for review.

### 20.7.3 Additional Location Requirements

- 19 In addition to details provided in Standard and Directive Drawings, the following location
- 20 requirements shall be met:

- 1 • If the TPF is located in the proximity of or beneath HST tracks located on aerial trackway, it  
2 shall be ensured that that the main power transformers and AT and their outdoor  
3 switchgear are located in an open area (shall not be located underneath structures), and that  
4 proper clearances are available for the main gantry.
- 5 • Access shall be provided between the TPF and the HST track. If this cannot be provided, an  
6 alternate method of providing vehicular access to the trackside shall be provided to the  
7 Authority for review. Access to the track at TPF sites is required for service and  
8 maintenance vehicles access. Refer to the *Civil* chapter for additional requirements.
- 9 • If the TPF is located adjacent to the HST track, the main (catenary feeding) gantry shall be  
10 located within the TPF fence. If the TPF is located away from the track, the main gantry shall  
11 be located within the trackway, parallel to and towards the TPF side of the track. In this  
12 case, ductbanks and manholes for laying power cables from the TPF to the main gantry shall  
13 be laid on the strip of land provided for this purpose.
- 14 • There shall be a strain gantry located within the trackway parallel to and on the opposite  
15 side of the track away from the TPF, with footprints exactly equal to that of the main gantry.
- 16 • At locations where track alignment is on retained fill, the TPF shall be located on ground,  
17 and power and traction return cables shall be routed from the TPF to the gantries located on  
18 the retained fill structures through ductbanks and manholes and then onto the walls of the  
19 retained fill structures. The gantries may be located on ground if the retained fill wall height  
20 is not more than 10 feet at that location.
- 21 • As a rule main and strain gantries shall not be located on top of MSE (mechanically  
22 stabilized earth) walls. Any exception to this requirement shall be submitted to the  
23 Authority for review.
- 24 • At locations where track alignment is on aerial structures, the TPF shall be located on  
25 ground, and power and traction return cables shall be routed from the TPF to the gantries  
26 located on the aerial structures through ductbanks and manholes and then onto the vertical  
27 columns of the aerial structures. Refer to Section 20.9.3 and the *Structures* chapter for  
28 additional requirements.
- 29 • At locations where track alignment is in trenches, the TPF shall be located on ground, and  
30 power cables shall be routed from the TPF to the motor operated disconnect switch (MOD)  
31 assemblies located adjacent to the trench alignment through ductbanks and manholes and  
32 then onto the OCS. If the TPF is located adjacent to the trench alignment, the MODs shall be  
33 located within the TPF fence.
- 34 • Phase-breaks shall not be located in tunnels.
- 35 • If an AT connection to the OCS must be located in a tunnel, the respective PS shall be  
36 located at or close to the entrance of the tunnel, and feeder cables shall be carried along the  
37 walls of the tunnel to provide connection to the OCS. Alternatively, at locations where  
38 tunnels have vent shafts and/or adits, the respective PS may be located at the mouth of the

vent shaft/adit of the tunnel (if feasible) and the feeder cables routed through the vent shafts to provide connection to the OCS.

- In all cases, the traction gantries, 25 kV feeders, traction return cables, motor operated disconnect switches, wayside power control cubicles, manholes, ductbanks, and other traction power supply related equipment shall be located along the track alignment at-grade, on retained fill or aerial structures, in trenches and in the tunnels (as applicable) conforming to the structure gauge, vehicle gauge and track clearance requirements. Refer to the *Trackway Clearances*, *Utilities*, *Structures*, and *Tunnels* chapters for additional requirements.

#### 20.7.4 Continuity of Power Supply in Case of Disturbances

The TPS and the OCS shall be designed to enable continuity of operation in case of disturbances such that all of the following are achieved:

- There shall be no degradation of train performance in case of single electrical contingency conditions, i.e., isolation of any 1 main power transformer in a substation, 1 autotransformer in a PS/switching station or of NF for any 1 electrical section.
- There shall be no stranding of trains in case of double electrical contingency conditions (simultaneous occurrence of more than 1 single electrical contingency condition in any electrical section).
- The minimum permanent voltage at the train's pantograph (both under normal as well as single contingency conditions) is 19 kV.
- The acceptance criteria of Quality Index of Power described in Section 20.6.2.7 are satisfied.
- The minimum non-permanent voltage at the train's pantograph (under double electrical contingency conditions) is 17.5 kV.

Refer to Section 20.6.2 for additional information.

#### 20.7.5 Electrical Protection Coordination

The protection system for the TES shall be designed for a maximum catenary – rails short-circuit fault current of 15 kA.

Compatibility of protective systems between traction unit (rolling stock) and SS shall be verified for the following:

- When any internal fault occurs within the traction units (rolling stock), the SS feeder circuit breaker and the traction unit circuit breaker may trip immediately. However, the traction unit circuit breaker should trip in order to avoid the substation circuit breaker tripping.
- After the substation circuit breakers have tripped, these breakers shall be capable of being reclosed either automatically or manually only after a lapse of at least 3 seconds.



- 1 • The traction unit circuit breakers shall trip automatically within 3 seconds after loss of line  
2 voltage.
- 3 • On re-energization, the traction unit circuit breaker shall not reclose within 3 seconds of the  
4 line being re-energized.
- 5 Refer to the *Rolling Stock-Core Systems Interfaces* chapter for additional requirements.

#### 20.7.6 Protective Provisions of Traction Power Facilities

6 Additional information is found in the *Grounding and Bonding Requirements* chapter.

7 The TES design presupposes running rails electrically insulated from ground, but connected to  
8 ground at intervals, at least at TPF locations, through the neutral points of impedance bonds. A  
9 part of the return current will flow through the running rails because they are part of the  
10 traction return system. Because of the impedance of the rails, this return current flow will cause  
11 a voltage with respect to the ground especially at locations away from the ground connections.

12 Electrical safety of the TPS shall be achieved by the following means:

- 13 • The installations shall be designed and tested such that the permissible touch voltages  
14 caused by the traction system under fault conditions or in operating conditions shall not  
15 exceed values specified in the *Grounding and Bonding Requirements* chapter.
- 16 • A direct connection shall be made between the return circuit and the grounding system of  
17 the TPF (SS, SWS, and PS).
- 18 • Each TPF shall be connected to the running rails and the aerial ground wire by at least 2  
19 return cables. Each return cable shall be of sufficient size to carry the maximum load  
20 current, thereby allowing for the failure of 1 return cable. The connection to the running  
21 rails is through impedance bonds.
- 22 • Fuses, non-lockable switches, and joint straps that can be released without a tool shall not be  
23 installed in the return circuit.

24 The rated impulse voltage ( $U_{Ni}$ ) and the short-duration power-frequency (ac) test level voltage  
25  $U_A$  (kV rms) shall be as shown in Table 20-1. (Refer to European Standard EN 50124-1 - 2001:  
26 Railway Applications – Insulation Coordination – Part 1: Basic Requirements – Clearances and  
27 Creepage Distances for all Electrical and Electronic Equipment.)

**Table 20-1: Rated Impulse Voltage and Power Frequency Test Voltage**

	Rated Impulse Voltage $U_{Ni}$ (kV <sub>crest</sub> ) <sup>a</sup>	Short-Duration Power- Frequency (ac) Test Level Voltage $U_A$ (kV rms) <sup>b</sup>
Between Catenary/Negative Feeder and Ground	200	95
Between Catenary and Negative Feeder	250	95

Notes:

1. Based on Table A.2, EN 50124-1:2001 (against Rated Insulation Voltage 27.5 kV – Higher Values for Fixed Installations)

2. Based on Table B.1, EN 50124-1:2001 (against  $U_{Ni}$  of 200 kV and 250 kV, respectively)

All TPF shall be fenced against unauthorized access. At locations where TPF are located adjacent to the Authority's right-of-way, a fence shall be installed for the complete length of the TPF site between the TPF and the trackside. Refer to the *Civil* chapter for additional requirements.

The grounding of TPF shall be integrated into the general grounding system along the route to comply with the requirements for mitigating electric shock as specified above.

#### 20.7.7 Harmonic Distortion Limits

The traction power system current and voltage distortion, including the effects of rolling stock in regular operation, shall comply with all criteria and design provisions of the *Electromagnetic Compatibility and Interference* chapter.

#### 20.7.8 Electromagnetic Compatibility

The TPS installations shall conform to the criteria in the *Electromagnetic Compatibility and Interference* chapter.

### 20.8 Traction Electrification System Interfaces

Refer to other Contract Documents for systems interface details.

### 20.9 Traction Power Facilities and Associated Sites

#### 20.9.1 General Site Requirements

**Access/Egress** – Access to each TPF site shall be required both during construction and for operation and maintenance purposes. Access roads leading to/from the TPF shall be designed in accordance with the criteria noted in the Technical Memorandum TM 2.8.1. Refer to the *Civil* chapter for additional requirements.

1 Access roads and gates at TPF shall be sized in accordance with the criteria noted in the  
2 Technical Memorandum TM 2.8.1.

3 The design of the access roadways and equipment arrangement within the TPF shall ensure that  
4 equipment owned by and/or maintained by the local power utility company is located within a  
5 distance from the access roadway, as specified by the relevant power supply utility.

6 Every TPF shall have a communications enclosure for locating communications interface  
7 equipment relating to TES SCADA and other associated equipment. Separate access shall be  
8 provided for the communications staff to the communications enclosure located in the TPF.

9 **Security** – Refer to the *Civil* and *Safety and Security* chapters for the requirements.

10 All equipment enclosures shall have an Authority-approved locking device. Refer to TBD for  
11 additional details.<sup>1</sup>

12 The doorways at the prefabricated equipment enclosures shall include Authority-approved  
13 intrusion detection hardware, which shall be remotely monitored. Refer to TBD for additional  
14 details.<sup>2</sup>

15 **Parking Spaces** – Parking spaces shall be designed and sized in accordance with the  
16 requirements specified in the *Civil* chapter.

17 **Aesthetic Treatments** – TPF and their sites shall be designed to minimize the adverse visual  
18 impact on the areas in which they are located and to be in compliance with the appropriate  
19 architectural and environmental guidelines. Refer to TBD for requirements.<sup>3</sup>

20 **Drainage** – Refer to the *Drainage* chapter for the requirements.

21 **Equipment/Enclosure Safety Signage** – Safety signage shall be provided at all TPF in  
22 accordance with applicable codes and standards (e.g., Cal/OSHA, NEC, NESC, CPUC GOs,  
23 etc.). Refer to the *Civil* chapter for additional requirements.

24 **Protective Barriers** – Protective barriers shall be provided at TPF that are subject to road vehicle  
25 damage. Refer to the *Safety and Security* chapter.

## **20.9.2 Equipment Support Steel**

26 Equipment support steel (steel members for supporting TPS equipment) shall be designed to be  
27 assembled on site using bolted connections.

---

<sup>1</sup> This information is not available at present.

<sup>2</sup> This information is not available at present.

<sup>3</sup> This information is not available at present.

- 1 Equipment support steel shall have a hot-dipped galvanized finish, suitable for the intended  
2 working environment.
- 3 Structural steel shall include a cleat/bracket with 2 holes to permit 2-hole grounding/bonding  
4 lug connections.
- 5 Refer to the *Structures* chapter for additional requirements.

### 20.9.3 Foundations

6 The design of the foundations for all of the equipment and structural steel located at the TPF  
7 and wayside power control cubicles (WPC) shall meet the following requirements:

- 8 • Conform to established civil and structural engineering practices: California Building Code,  
9 ASTM, ACI, AISC, NESC, and other applicable codes and standards
- 10 • Be structurally capable of withstanding the live loads and dead loads occurring during  
11 installation, operation, and maintenance
- 12 • Consider, amongst other issues, the local flood, soil, and seismic conditions at each TPF and  
13 WPC site. Refer to the *Drainage* chapter for establishing the floor height above the 100-year  
14 and 500-year flood elevations. Refer to the *Seismic* and *Structures* chapters for specific  
15 requirements.

16 The design of the foundations shall ensure that water drains to the site drainage system and  
17 prevents standing water at/under equipment and/or structural steel.

18 Anchor bolts (hold-down bolts) shall be galvanized.

19 The foundations of each prefabricated enclosure for 25 kV indoor switchgear shall include the  
20 following features:

- 21 • A concrete slab, extending 6 inches beyond the outside walls of the enclosure (excluding  
22 doorways, which shall be provided with landing pad)
- 23 • Subject to the height between the finished grade and the enclosure floor, a concrete staircase  
24 at doorways used for personnel access/egress designed in accordance with federal, state,  
25 and/or local codes
- 26 • A ramp at doorways used for equipment placement and removal (in addition to personnel  
27 access/egress), designed in accordance with federal, state and/or local codes
- 28 • The interface of the indoor switchgear room with the 25 kV cable ducts shall be via cable  
29 trench with removable covers. If the connections between power cables and circuit breakers  
30 are not near the edge of the foundation, a cable vault underneath the switchgear enclosure  
31 shall be provided, as part of the foundation structure. Access to the cable vault shall be via  
32 bulkhead entrance/exterior doorway, which shall be large enough (of adequate width, depth  
33 and height) to provide convenient working space to maintenance personnel for installation

1 and maintenance of medium voltage cables, terminations, surge arrestors and other  
2 equipment required by the system design. (Refer to Sections 20.9.4 to 20.9.9 for additional  
3 requirements.)

4 The design of the foundations associated with the HV transformers and AT shall prevent oil  
5 from entering the site drainage system and shall contain fluids in accordance with federal, state,  
6 and local codes.

7 Refer to the *Seismic* and *Structures* chapters for additional slipstream and seismic requirements.

#### **20.9.4 High-Voltage Transformers**

8 HV transformers shall be outdoor type, mineral-oil-insulated, and self-cooled, with a nominal  
9 rating of 60 MVA ONAN (oil-insulated, natural circulation, air-cooled, natural circulation). The  
10 transformers shall be furnished and installed without cooling fans. However, transformer  
11 design shall incorporate provisions for possible installation of cooling fans in the future.

12 The HV transformers shall conform to the appropriate duty class as specified in the European  
13 Standard EN 50329:2003+A1:2010, corresponding to the load curves based on the traction power  
14 load flow study.

15 The HV transformers shall be single-phase, with the primary winding connected between  
16 2 phases of the incoming 115 kV (or 230 kV) line of the local utility company. The secondary  
17 winding may be constructed as either 1 winding with its center point brought out and  
18 grounded, or it may comprise 2 separate windings such that their voltages are in counter-phase  
19 (180 degrees apart). The no-load voltage on the secondary side, assuming nominal  
20 115 kV/230 kV on the primary and a neutral tap, shall be 52.5/26.25 kV. Transformer impedance  
21 shall be approximately 10 percent on a 60 MVA basis.

22 For TPF with more than 1 transformer, sufficient space or masonry fire barriers between the  
23 transformers shall be provided to preclude a transformer fire from damaging other  
24 transformers.

25 The design of the transformers shall be such as to minimize the generation of acoustic noise; the  
26 noise levels produced shall not exceed the requirements as specified in the *General* chapter.  
27 Refer to IEEE Standard C.57.12.00 and NEMA Standard TR1 for additional requirements.

28 Each HV transformer shall be equipped with no-load tap changer on the primary side, and  
29 automatic on-load tap changer on the secondary side.

#### **20.9.5 Autotransformers**

30 AT shall be outdoor type, mineral-oil-insulated, and self-cooled, with a 20 MVA ONAN  
31 nominal rating.

1 The AT shall conform to the appropriate duty class as specified in the European Standard EN  
2 50329:2003+A1:2010, corresponding to the load curves based on the traction power load flow  
3 study.

4 The AT shall be single-phase, with the primary winding connected between the catenary and  
5 NF circuits, and the center tap grounded and connected to the running rails (and to the static  
6 wires). The nominal voltage of the primary winding shall be 50.0 kV between the winding  
7 terminals, and 25.0 kV to ground. The turns ratio between OCS side and center tap to center tap  
8 and NF side shall be 1:1. Autotransformer design shall minimize leakage reactance, and the  
9 autotransformer impedance shall be approximately 1.2 percent.

### 20.9.6 High-Voltage Switchgear

10 Each HV transformer shall be connected to the incoming utility line via outdoor HV switchgear  
11 of the same voltage class as the utility supply line (nominal 115 kV or 230 kV). The switchgear  
12 shall include a circuit breaker, motorized gang-operated air isolation switches, instrument  
13 transformers and other accessories. The basic impulse level rating of the outdoor HV switchgear  
14 shall be 550 kV for 115 kV line and 900 kV for 230 kV line.

15 The power circuit breaker shall be outdoor type, 121 kV rms maximum operating voltage for  
16 115 kV line (or 242 kV for 230 kV line), 550 kV (or 900 kV) basic impulse level, double-pole, SF6  
17 insulated, free standing. Short-circuit interrupting current capability shall be 40 kA. The circuit  
18 breaker shall be rated for operation on a nominal 115 kV (or 230 kV), effectively grounded  
19 utility transmission system. It shall be similar to a 3-phase circuit breaker of the same voltage  
20 rating, except that 1 phase/pole shall not be used.

### 20.9.7 Prefabricated Enclosures for 25 kV Indoor Switchgear

21 The 25 kV phase-to-ground class switchgear and associated control and protection systems of  
22 the SS, SWS, and PS shall be indoor type. The medium-voltage switchgear and related  
23 protection control, and auxiliary systems at each TPF site shall be housed in a prefabricated  
24 walk-in, climatized, and transportable metal enclosure (or in 2 separate enclosures). The  
25 enclosure shall be fabricated from sheet steel, mounted on a structural steel base, and provided  
26 with internal and external high durability paint finishes designed to prevent corrosion over the  
27 life of the enclosures. Non-painted steel surfaces shall be hot-dipped galvanized after  
28 fabrication. Damage to galvanized areas caused by the fabrication process shall be repaired in  
29 accordance with ASTM A780/A780M: Standard Practice for Repair of Damaged and Uncoated  
30 Areas of Hot-Dip Galvanized Coatings.

31 At least 2 doorways, located at diametrically opposite ends of the enclosures, shall be provided  
32 to permit an unobstructed means of egress in accordance with the California Building Code.

33 One of the doorways shall be sized and located to allow removal or replacement of the largest  
34 piece of equipment in the room and shall be located such that the equipment can be moved  
35 through the enclosure to the outside for transporting off-site.

1 The design of the enclosures shall ensure a dry internal environment within the specified  
2 temperature and humidity limits.

3 The enclosures shall be designed to withstand the level of structural, wind, and seismic loading  
4 as specified in the *General*, *Seismic*, and *Structures* chapters.

5 The floor and walls of each enclosure shall be designed to support the equipment, raceway, and  
6 cable tray systems installed and to provide openings to cable trenches, without buckling,  
7 bending, or sagging.

### **20.9.8 25 kV Single-Phase Switchgear**

8 The design of the 25 kV, single-phase, 60 Hz, ac switchgear shall include at a minimum the  
9 following features:

- 10 • The 25 kV class single-phase circuit breakers shall be designed per applicable European  
11 standards for railway applications, shall be suitable for indoor installation, and shall have  
12 rated maximum operating voltage to ground 29 kV. Circuit breakers protecting catenary or  
13 NF circuits shall be single pole. Circuit breakers for the AT or on the secondary side of the  
14 HV transformers shall be double-pole. Nominal phase-to-ground voltage for both single-  
15 pole and double-pole circuit breakers shall be 25 kV in all the TPF. The pole-to-pole nominal  
16 voltages for the double-pole circuit breakers shall be twice the respective voltages to  
17 ground.
- 18 • The circuit breakers shall be either of the sealed vacuum or SF6 type. If SF6 circuit breakers  
19 are proposed, an evaluation shall be provided to indicate that such medium-voltage indoor  
20 switchgear does not conflict with state or local regulations concerning the SF6 gas and its  
21 byproducts from arc extinguishing as hazardous materials.
- 22 • The ac switchgear shall be either metal-clad or metal enclosed, and of the same voltage  
23 rating as the circuit breakers. If metal-enclosed, disconnect switches shall be provided  
24 between the circuit breakers and the common bus to allow the isolation of any circuit  
25 breaker for maintenance or repairs. In case of metal-clad switchgear with draw-out circuit  
26 breakers, the stationary contacts of the circuit breakers shall be connected directly to the  
27 common bus without a disconnect switch. The basic impulse level of the switchgear shall be  
28 200 kV or higher.
- 29 • Power connections from the catenary and NF of each track to the 25 kV buses of SS, SWS,  
30 and PS shall be through single-phase circuit breakers (forming part of the indoor switchgear  
31 lineups) and outdoor gantry-mounted disconnect switches. The latter shall be motorized  
32 and connected in series with the circuit breakers, to provide visible circuit isolation means  
33 between tracks and TPF. As noted, if metal-enclosed switchgear is used, an additional set of  
34 in-series disconnect switches shall be provided between the ac circuit breakers and the  
35 common bus. The disconnects in series with the circuit breakers shall be no-load type and  
36 shall be interlocked with the respective circuit breaker so that the switch cannot be operated  
37 unless the circuit breaker is open.

- Switchgear in the TPF and outdoor-mounted disconnect switches shall be appropriately interlocked with the associated circuit breakers to ensure the safety of O&M personnel and equipment for all possible circuit configurations, and to avoid inadvertent paralleling of different electrical sections (which will be supplied by out-of-phase voltage systems).
- Circuit breakers equipped with protective relaying for catenary and NF circuit protection shall feature multi-stage auto-reclosing capability.
- Equipment interconnecting buses shall be copper. Buses shall be sized to limit temperature rise in accordance with the applicable codes and standards. Buses shall be adequately supported to withstand the forces from short-circuit currents matching the ratings of the circuit breakers.
- If draw-out circuit breakers are selected, the design of the switchgear shall include automatic shutters to protect personnel from accidental contact with live power circuits when the truck-mounted circuit breaker is removed from the cubicle.
- The design of the circuit breaker shall include means for physical (padlocking) lockout/tag-out when the circuit breaker is in the disconnected position.
- Visual indication of the status of the circuit breaker (i.e., closed or open) shall be displayed on the front door of the circuit breaker cubicle by indicator lights and by mechanical flag indicators.
- All circuit breakers and motorized disconnect switches shall be locally and remotely controlled. Control means to operate all switchgear remotely from the Operations Control Center (OCC) or the Regional Control Center (RCC), and locally via a mimic annunciation panel, graphical user interface, and/or control switches on the equipment.
- A mimic annunciation panel shall be provided at each TPF that permits O&M personnel to monitor and control circuit breakers and/or disconnect switches located at the traction power facility and its vicinity. The design of the mimic annunciation panel shall include a sectionalization plan on the exterior of the panel, complete with control switches and status indication lights located adjacent to the control switches (of the respective circuit breakers and disconnection switches).
- Equipment space heaters, which are thermostatically and humid-statically controlled, shall be provided in the ac switchgear cubicles and control equipment cabinets.
- Feeder cable terminations shall be designed to prevent accidental contact and alleviate voltage stress zones. If no-load break elbows are available for this voltage class from suppliers of elbow style terminations, the cable terminations shall be via elbows.

Refer to Section 20.9.15 for circuit breaker protection requirements.

### 20.9.9 Transformer Oil Containment

The transformer oil containment system shall have open area covered with non-skid galvanized steel grating on all sides of the transformer concrete pad as shown in the Standard and Directive



Drawings and shall conform to IEEE-980 and other applicable codes/standards/guidelines including 40 CFR, Part 112 – Oil Pollution Prevention Regulations – published by the Environmental Protection Agency. Refer to the *Grounding and Bonding Requirements* chapter for grounding of steel grating.

Typically, at 1 corner of this structure, a sump shall be provided. All 4 sides shall slope 1 percent minimum towards the sump.

Interior surface of the containment basin shall be painted with epoxy primer and polyurethane finish coat.

Waterstops shall be provided at the construction joints.

Structural steel beams shall support the galvanized steel grating.

A system for reclamation and/or disposal of spilled oil shall be designed such that it shall conform to the applicable codes, standards, and regulations.

An active fire suppression system shall be designed by the TPS Designer.

#### **20.9.10 Utility Interface Equipment**

Ancillary equipment may be mandated by third party utilities to protect their 115 kV (or 230 kV) transmission networks. These third parties include PG&E, SCE, LADWP, City of Anaheim, etc. Refer to the general requirements specified by the relevant power supply utilities. Site-specific requirements are indicated elsewhere in the Contract Documents.

#### **20.9.11 Resistor Banks**

Requirements are specified elsewhere in the Contract Documents.

#### **20.9.12 Painting and Finishes**

All electrical equipment enclosures, materials, and appurtenances shall have a corrosion resistant finish and shall be suitable for use in the environment in which they are installed.

#### **20.9.13 Auxiliary and Control Power**

Auxiliary power for lighting, receptacles and so forth, at prefabricated equipment enclosures, can be derived from any 1 of the following:

- Two local power utility services
- Locally by tapping of each 25 kV bus and transforming through auxiliary transformers, to the utilization voltage
- One local power utility service and 1 tapping of the 25 kV bus (which is transformed to the utilization voltage)

1 The primary auxiliary power source shall be switched to the secondary auxiliary power via an  
2 automatic transfer switch.

3 The auxiliary transformer shall be sized based upon the demand electrical load. Auxiliary  
4 transformers may be indoor or outdoor with suitable enclosures according to their location

5 Refer to Section 20.14.2 for additional requirements.

6 Control power at TPF shall be 125 V dc and originate from a battery and battery charger.

7 Refer to Section 20.11.2 for additional requirements for control power.

#### **20.9.14 Lighting**

8 Exterior lighting layouts shall relate to the equipment locations and access/egress routes of both  
9 pedestrians and road vehicles.

10 Exterior lighting shall be activated manually by photo cell or astronomical clock controls.

11 Exterior lighting shall provide at least a uniform lighting level of 10 foot-candles within the  
12 barriered area of the TPF sites and 5 foot-candles within the 10-foot perimeter outside each site's  
13 barrier.

14 Interior lighting levels at each prefabricated equipment enclosure shall be at least 20 foot-  
15 candles.

16 Emergency lighting capable of illuminating exit paths with at least 1 foot-candle shall be  
17 provided at each prefabricated equipment enclosure.

#### **20.9.15 Relay Protection**

18 The design of the relay protection system shall achieve the following:

- 19 • Protect the TES equipment and cables within the TPF, catenary, and NF against short-circuit  
20 faults, overloading, and subcomponent failures.
- 21 • Include fault location and discrimination capabilities, including automatic circuit breaker  
22 reclosing for catenary and NF circuits, as well as manual local and remote re-closure  
23 management.
- 24 • Provide proper coordination and selectivity for rapid fault clearance to only the affected  
25 area of the system, preventing as much as possible the loss of power to the healthy sections  
26 of the TES.
- 27 • Adequately discriminate between short-term high loads and fault conditions.

28 HV relay protection equipment on the primary side of the traction power substations shall be  
29 coordinated with the respective utility company as applicable.

Each HV transformer and autotransformer shall be provided with protective devices, including but not limited to the following:

- Overcurrent relays on the primary side (HV transformers only)
- Differential relays
- Ground overcurrent relay on the secondary side
- Over-temperature protection
- Oil-level and oil-pressure detection relays and alarms

Catenary and NF circuit breakers shall be provided with electronic, microprocessor-based protective relays and devices to protect against short-circuits and conductor overloading conditions. The number and type of protective devices for a particular circuit breaker shall be based on the overall relay protection scheme for the TES. Typically, the protective relays for catenary and NF circuits employed shall include, but not be limited to, the following:

- Instantaneous and time delay overcurrent relays (for backup short-circuit protection)
- Multi-zone distance relays (for primary short-circuit protection). The distance relays shall feature at least 3 zones with separately adjustable parameters.
- Undervoltage /overvoltage relay
- Thermal overload relay, for protection of the downstream catenary or NF circuit against overheating. This relay shall be thermal replica type and shall include an ambient temperature probe located outside the traction power facility, with a feedback loop to the relay.
- Phase directional overcurrent relay
- Multi-shot automatic reclosing relay (maximum number of auto-reclosing attempts and pause durations to be determined in coordination with the overall protective relaying scheme)

The design of the relay protection scheme shall be presented to the Authority for review.

#### **20.9.16 Protective Relaying Scheme for Catenary and NF Fault Detection**

Circuit breakers equipped with distance relays shall feature multi-stage auto-reclosing capability, and catenary and NF circuit breakers shall have separate protective relaying. The preferred relay protection scheme shall be based on the following general principles:

- The distance relays shall be located in the SS, and possibly also in the SWS (for use during contingency operations with 1 SS completely out-of-service). The distance relays shall be set to protect the entire feed section from the SS to the SWS, or to the end-of-line (in case of the first and last electrical sections). Upon fault detection by 1 of the distance relays, initially both tracks shall be tripped. During the pause before the first auto-reclosing, the circuit

breakers that tie electrically the 2 tracks at the PS and SWS in the affected feed section shall open, isolating electrically the 2 tracks. Opening of these circuit breakers shall be by local undervoltage relays.

- Upon reclosing of the circuit breakers in the SS, if the fault has been transient, both tracks will remain energized. In such case, following time delay (presence of voltage indicating power has been restored), all circuit breakers in the PS and SWS shall reclose, restoring the TES to its normal configuration. If the fault is still present, power will be restored to the healthy track only, as the breaker feeding the fault will be tripped again.
- Auto-reclosing on the faulted track shall be attempted several times, and if the circuit breaker is tripped at the last attempt, the conclusion will be that the fault is permanent. The next stage consists of the following actions:
  - Approximating the location of the fault between a pair of track interlockings (or between an SS and its adjoining SWS, or between a track interlocking and the adjacent TPF [SS or SWS], depending on the location of the fault and the TES configuration)
  - Isolating the faulted subsection by opening (the N.C.) disconnect switches at the interlockings (or SS and the adjoining SWS, or the interlocking and the TPF) bracketing the fault
  - Restoring power to the healthy segments of the affected track.
- The procedure for fault isolation (i.e., limiting the power loss between adjacent interlockings on 1 track) can be either manual—conducted by operators in the OCC/RCC using remote control of circuit breakers and motorized switches—or it can be automated (driven by PLC-based logic). Both alternatives shall be investigated and presented to the Authority for review.

The protective relaying scheme outlined above shall be analyzed for both normal and contingency configurations of the TES. The contingency configurations shall include, at a minimum, the following:

- Operational loss of 1 HV transformer of any substation (with the second transformer supplying both feed sections of the affected substation)
- Loss of a PS
- Loss of 1 catenary or NF circuit breaker in an SS, SWS or a PS
- Complete loss of an SS, with the adjacent healthy SS providing feed to the extended feed sections
- Design a detailed protection scheme based on the above principles. Suggest any alternative relay protection scheme bringing out its benefits. Submit details of both the alternative options to the Authority for review.

### 20.9.17 Fire and Life Safety

1 A fire alarm control system shall be installed at each prefabricated 25 kV switchgear room in  
2 accordance with NFPA 72 and CFR Title 19.

3 Fire alarm devices, initiating devices, notification appliances and signaling line circuits shall be  
4 designated as Class A, as defined in NFPA 72.

5 The fire alarm system shall be electrically supervised and shall be furnished with emergency  
6 backup power.

7 A portable emergency eye-wash unit shall be provided at a location adjacent to the station  
8 battery.

9 A portable fire extinguisher, sized per federal, state, and local code requirements shall be  
10 provided in each prefabricated 25 kV switchgear room.

11 Refer to the *System Safety and Security* and *Fire and Life Safety* chapters for additional  
12 requirements. Conform to all applicable local codes and regulations.

### 20.9.18 Test Cabinets

13 If withdrawable circuit breakers (metal-clad switchgear) are selected, a test cabinet shall be  
14 provided at each prefabricated 25 kV switchgear room for the testing of the truck-mounted  
15 25 kV circuit breakers.

### 20.9.19 Communications

16 Refer to the *Communications* chapter for these requirements.

### 20.9.20 SCADA Equipment

17 Refer to the TES SCADA System related sections in the *Supervisory Control and Data Acquisition*  
18 *Subsystems* chapter for these requirements.

### 20.9.21 Metering Equipment

19 • Conform to the metering equipment requirements of the relevant power supply utilities.  
20 The relevant power supply utilities include: Pacific Gas and Electric, Southern California  
21 Electric, Los Angeles Department of Water and Power, City of Anaheim, etc. Refer to other  
22 Contract Documents for additional requirements.

23 • Refer to the *Supervisory Control and Data Acquisition Subsystems* chapter for metering  
24 equipment requirements specific to the CHSTP.

25 Metering instrumentation shall be provided on the primary side of the HV transformers, AT,  
26 and on each individual catenary and NF circuit.

### 20.9.22 Grounding, Bonding and Lightning Protection

1 Refer to the *Grounding and Bonding Requirements* chapter for details.

### 20.9.23 Real Estate Requirements: Approximate Footprints for the TPF

2 The size of the TPF sites has been determined considering the following requirements:

- 3 • Site shall accommodate all of the equipment necessary for the level of service, associated  
4 roadway, and right-of-way requirements.
- 5 • Design requirements imposed by utility companies and/or the local jurisdictional entities
- 6 • Space provisions for future equipment (normally 20 percent), as approved by the Authority<sup>4</sup>
- 7 • Space requirements for the placement and removal of equipment

8 Where practical, the footprints of different TPF (considering the above requirements), shall be  
9 limited as follows:

- 10 • SS (2 power transformers, each of 60 MVA capacity) with 2 HV utility supply circuits – 200  
11 feet x 160 feet
- 12 • SS (3 power transformers, each of 60 MVA capacity) with 2 HV utility supply circuits – 200  
13 feet x 210 feet
- 14 • SWS with 2 20 MVA, 2x25 kV ATs – 160 feet x 90 feet
- 15 • SWS with 3 20 MVA, 2x25kV ATs – 160 feet x 145 feet
- 16 • PS with 1 20 MVA, 2x25 kV AT – 120 feet x 80 feet

17 These are typical footprints of different TPF. Orientation of the TPF with respect to tracks,  
18 locations of utility supply circuits, equipment, and road access shall be determined on a site-by-  
19 site basis. Additional space may be required at the TPF sites for vehicle parking, access road,  
20 setbacks, landscaping, and construction.

21 In general, all SS shall be configured with 2 power transformers where practical. Some locations  
22 shall require an SS configuration with 3 power transformers. Similarly, in general, all SWS shall  
23 be configured with 2 autotransformers where practical. Some locations shall require an SWS  
24 configuration with 3 autotransformers. Refer to Standard and Directive Drawings.

25 Provision shall be made for locating a power utility switching station close to each SS. Typically  
26 the utility's switching station will have a footprint of 220 feet x 160 feet. This utility switching  
27 station shall be, to the extent practical, located adjacent to the SS. If the utility's switching

---

<sup>4</sup> This requirement may be common to many subsystems, and may be shifted to another contract document with a reference at this location.

1 station is located away from the SS, easements shall be provided for installing the HV line  
2 connection from the utility's switching station to the SS.

3 The design and installation of the TPF, power utility switching stations and the HV line  
4 connections shall conform to the requirements of all applicable codes, standards, guidelines,  
5 and the structure gauge, vehicle gauge and track clearance requirements.

6 **TPF Sites** – The TPS detailed design, including interconnections to the OCS/power utility  
7 network, shall be carried out within the limits of the land plots and easements earmarked for  
8 this purpose. If additions/alterations are required to be made to these identified land  
9 plots/easements because of the results of the traction load flow studies/calculations conducted  
10 by the Contractor and/or because of additional requirements of the power supply utilities,  
11 justifying data for the additional requirements shall be provided to the Authority for review.

12 Also refer to Sections 20.4.6 and 20.7.3.

## **20.10 Electrical Materials**

### **20.10.1 Equipment Identification**

13 All equipment shall be uniquely identified in accordance with the Authority's requirements to  
14 indicate geographical location, function, feeding point (if applicable), and ownership.

15 The equipment identifiers shall be permanent and located on the front and rear (where rear  
16 access is provided) of each equipment enclosure.

17 Refer to the Contract Documents for the Authority-approved identification and nomenclature  
18 system.

### **20.10.2 Equipment Enclosures**

19 Equipment enclosures shall be of NEMA classification suitable for the environment in which the  
20 equipment is operating.

### **20.10.3 Raceway**

21 Exposed conduits shall be rigid galvanized steel and shall be minimum 3/4-inch trade size.

22 Raceways shall be installed parallel or perpendicular to the building members of the TPF.

23 The number of bends in any 1 conduit run shall not exceed the limit specified in the CEC and  
24 NEC.

25 The bend radius of exposed and/or underground raceway systems shall be sufficient to  
26 maintain the cable side pressures within manufacturer's recommendations during cable pulling  
27 activities and shall conform to applicable codes and standards.

1 Refer to the *Structures* chapter for provisions for the installation of conduit embedment on  
2 specific columns of aerial structures and site specific walls of earth retaining structures.

3 All exposed raceways shall be supported /secured to the walls and/or ceiling of the  
4 prefabricated 25 kV switchgear, and Control and Relay Room equipment enclosures in  
5 accordance with standard industry practice and the CEC and NEC.

6 Emergency circuits (e.g., fire detection, emergency power) shall not share the same raceway or  
7 enclosures with other systems.

8 Embedded conduits shall be minimum 1-inch trade size. Refer to the *Utilities* chapter for  
9 conduits crossing under the trackbed.

10 All underground raceways shall utilize PVC schedule 40, reinforced thermosetting resin  
11 conduit or similar conduits, be encased in steel-reinforced concrete (with red pigment for high  
12 voltage carrying ductbanks), and comply with the requirements of the CEC, NEC, NESC, CPUC  
13 General Order 128, and applicable codes and standards. Refer to the *Structures* chapter for  
14 additional requirements with regard to ductbanks.

15 Underground raceways shall slope away from the TPF and towards manholes, pullboxes, etc.,  
16 at a minimum rate of 3 inches per 100 feet. Raceway entrances in manholes, pullboxes, etc. shall  
17 be sealed against entry of silt, debris, rodents, etc. into raceways.

18 Tracer/detectable tape shall be installed in accordance with Standard Specifications Section 33  
19 05 28: Trenching and Backfilling for Utilities.

20 Ductbanks shall run as directly as practicable and shall be located to avoid interference with  
21 civil structures and sub-grade utilities.

22 All ductbanks shall be designed and laid conforming to all the applicable codes, standards, and  
23 guidelines. Refer to the *Utilities* and *Facility Power and Lighting System* chapters for additional  
24 requirements.

#### 20.10.4 Cable Tray Systems

25 Cable tray systems shall comply with NEMA VE 1.

26 Cable tray systems shall be engineered to comply with the following requirements:

- 27 • The cable tray system shall be fully enclosed metal cable trays hot-dipped galvanized after  
28 fabrication, with full system appendages.
- 29 • The drop-offs to all different points of utilization shall be conduit. Bushed conduit shall be  
30 used wherever possible.
- 31 • The cable tray system construction shall be secure and prevent inadvertent access by  
32 unauthorized parties.



- 1 • The cable tray system shall have suitable strength and rigidity to provide adequate support  
2 for all the contained cables.
- 3 • The cable tray system shall include a means to ventilate the enclosed cables, so the heat  
4 generated by the cables can be safely dissipated.
- 5 • The cable tray system shall include barriers to segregate cables of different systems and  
6 voltage ratings.
- 7 • The cable tray system shall provide adequate cross-sectional area to permit neat alignment  
8 of the cables and avoid crossing or twisting.
- 9 • Refer also the *Facility Power and Lighting Systems* chapter for additional requirements.

#### 20.10.5 Electrical Manholes and Pullboxes

10 Manholes and pull-boxes shall be designed and laid conforming to all the applicable codes,  
11 standards, and guidelines. All manholes and manhole covers shall be traffic-rated, meeting the  
12 requirements of AASHTO M306. Refer to the *Utilities* and *Facility Power and Lighting Systems*  
13 chapters for additional requirements.

#### 20.10.6 Cable Trenches for Power Cables

14 The interface of the 25 kV ductbanks with the prefabricated 25 kV switchgear houses shall be  
15 through cable trenches. Cable trenches shall be equipped with sump area for drainage by  
16 gravity or application of portable or fixed pumps as required

17 Cable trenches shall be sized to accommodate the number and size of 25 kV circuits, with  
18 “positive/catenary” and “negative” cables separated by solid barrier, or installed on the  
19 opposite sides of the same trench (design to be submitted to the Authority for review).

20 At the foundation of the 25 kV switchgear house, the cable trenches shall transition into a cable  
21 vault of dimensions depending on the locations of the cable terminations at the switchgear. The  
22 cable vault shall have sufficient depth and height to provide for ease of installation and  
23 maintenance of the cable terminations and other equipment, such as surge arresters. A staircase,  
24 external to the prefabricated 25 kV switchgear equipment enclosures, shall be provided to  
25 permit access to the cable vault.

26 The design of the cable trenches shall include removable covers, extending a suitable distance  
27 from the edge of the switchgear house.

28 If SF6 switchgear is used, specific requirements for the design of the trenches directly below the  
29 switchgear shall be developed. Provide for all such requirements per existing law, codes,  
30 standards, and regulations in the TPS design.

## 20.10.7 Conductors

### 20.10.7.1 General

1 All electrical conductors shall be copper. Conductors and cables interconnecting equipment  
2 and/or cabinets shall be enclosed in raceways or cable tray systems.

3 Insulated traction power cables shall be single-conductor, with concentric neutral, shielded, and  
4 with external non-metallic jacket that is low smoke and sunlight resistant. The cables shall be  
5 suitable for installation in wet or dry locations, in underground conduit or exposed to the  
6 weather. The cables shall be rated for 30 kV phase-to-ground, and have 133 percent insulation  
7 level. Refer to NFPA 130 for requirements of conductors when routed through tunnels.

8 The cables shall be rated for 90°C continuous conductor temperature; 130°C for emergency  
9 short-term operation; and 250°C for short circuit conditions. The conductors shall be copper,  
10 with Class C stranding. The shield and concentric neutral shall be grounded at 1 end only, at  
11 the station ground bus, to avoid circulating ground return currents through the shield and  
12 neutral wires.

13 Traction power cables, connecting the 25 kV ac feeder breakers to the catenary and negative  
14 along-track feeders, and the running rails to the return bus, shall be sized to carry the maximum  
15 rms load currents, with due consideration for the installation environment. Cables shall be de-  
16 rated for installations in common underground ductbanks or cable trays.

17 Positive and negative 25 kV feeders and neutral return feeders shall be standardized in  
18 multiples of a single copper conductor size to achieve the required circuit ampacity, especially if  
19 using only 1 cable requires sizes larger than 500 kcmil. The cables shall have sufficient ampacity  
20 to carry the maximum rms current imposed by the worst-case operating scenario on a  
21 continuous basis, without exceeding the 90°C conductor temperature limit.

22 Low voltage ac and dc power and control cables shall be copper conductors, rated for 600 V ac,  
23 with maximum conductor temperature of 90°C, and shall be suitable for installation in conduits,  
24 ducts, cable troughs, and cable trays. Cables exposed to the outdoor environment shall have a  
25 weather resistant jacket.

26 Instrumentation cable shall be 600 V insulated, multiple shielded, certified for installation in  
27 conduits, ducts, cable troughs, and cable trays. For multi-pair twisted cable, each pair shall be  
28 individually shielded and the cable shall have an overall shield insulated from the individual  
29 pair shields.

30 No cable splices shall be permitted.

### 20.10.7.2 Conductor Routing and Segregation

31 Refer to the *Communications* chapter for conductor routing and cabling segregation for core  
32 systems equipment.

### 20.10.7.3 Sizes of Low Voltage Power and Control Cables

- 1 LV power and control cables shall be sized on the basis of the maximum expected load current
- 2 and acceptable voltage drop. In addition, conductor size shall not be less than the prescribed
- 3 minimum, for mechanical integrity purposes.
- 4 Minimum size of LV power conductors shall be No. 12 AWG.
- 5 Minimum size of LV control conductors shall be No. 14 AWG.
- 6 Minimum size of analog indication cables shall be No. 16 AWG.

### 20.10.7.4 Color Coding

- 7 Refer to the *Facility Power and Lighting Systems* chapter for requirement of color coding of all LV
- 8 conductors. The color coding of HV and medium-voltage TPS conductors is specified in other
- 9 Contract Documents.<sup>5</sup>

### 20.10.8 Wiring Devices

- 10 All light switches shall be 20 ampere grade.
- 11 All receptacles shall be 20 ampere grade, duplex, grounded type and have ground-fault circuit-
- 12 interrupter protection for O&M personnel.
- 13 Receptacles shall be provided in accordance with the CEC and NEC at each prefabricated
- 14 equipment enclosure.
- 15 Receptacles shall not be on lighting circuits.
- 16 Refer to the *Facility Power and Lighting Systems* chapter for further details.

### 20.10.9 Light Fixtures

- 17 The voltage rating of fluorescent and high intensity discharge type lighting fixtures shall be
- 18 277 V ac. The voltage rating of incandescent fixtures or compact fluorescent fixtures shall be
- 19 120 V ac or 277 V ac. Refer to the *Facility Power and Lighting Systems* chapter for further details.

## 20.11 Emergency Power

### 20.11.1 Emergency Electrical Loads

- 20 Emergency electrical loads are those ac and dc electrical loads required to be in operation
- 21 during a disruption in the normal power supply to a TPF. These electrical loads include, but are
- 22 not limited to, the following:

---

<sup>5</sup> This is yet to be done.

- 1 • Fire Alarm Control System
- 2 • Supervisory Control and Data Acquisition System
- 3 • Intrusion Detection System
- 4 • Control Power
- 5 • Emergency Lighting System
- 6 • Refer to the *Facility Power and Lighting Systems* chapter for additional requirements.

### 20.11.2 Emergency Power Requirements

- 7 An emergency power source (i.e., battery and charger system), rated for at least 8 hours  
8 connected electrical load, shall be provided for all emergency lighting, exit signs, and other vital  
9 equipment located at TPF. In addition to the noted electrical loads, the emergency power source  
10 shall be able to support at least 3 operating cycles (where trip and close operation constitute 1  
11 cycle) of all circuit breakers simultaneously.
- 12 The design of the batteries shall be Lithium-ion or VRLA or approved equivalent, and contain a  
13 life expectancy of at least 20 years, and be low maintenance.
- 14 Transfer from the normal LV power source to the emergency power source shall be automatic.
- 15 The design of each TPF shall include a receptacle and associated switching equipment to permit  
16 the connection of a portable diesel generator during abnormal operating conditions.

## 20.12 Wayside Equipment

### 20.12.1 General

- 17 The secure mounting of all traction power supply system enclosures and equipment along the  
18 right-of-way shall be accomplished in accordance with Section 20.9.3.

### 20.12.2 Wayside Power Control Cubicles

- 19 In addition to the TPF, Wayside Power Control Cubicles (WPC) will be located at railway  
20 stations including the universal crossovers at both ends, and on the wayside at universal  
21 crossovers, at tunnel portals, at rolling stock maintenance facilities, at wayside infrastructure  
22 maintenance facilities, and in tunnels longer than 3 miles. WPC is an enclosure for power  
23 supply equipment for operation of motorized disconnect switches and the associated SCADA  
24 equipment located at the wayside. Every WPC shall have, in general, a footprint of 10 feet x  
25 8 feet. For WPC located in tunnels and trenches the equipment layouts shall be suitably  
26 modified to fit within the Fixed Equipment Envelope (refer to the *Trackway Clearances* chapter).  
27 Refer to Automatic Train Control (ATC) Standard and Directive Drawings for requirements of  
28 WPC at railway stations and at wayside universal crossovers. WPC at railway stations and

wayside universal crossovers will be located in close vicinity of ATC installations with common access roads, parking and drainage facilities, and low voltage power supply, and will use some common equipment (e.g., ductbanks, under-track crossings, etc.), hence these should be designed and installed in consultation with the ATC System. Low voltage power supply for WPC shall be drawn from auxiliary transformers which will step down 25 kV ac NF supply to 480V ac. The number of WPC at each site will depend upon the site conditions, the layout of track (including crossovers and MODs), the location of the auxiliary power source, and the routing of cables. The requirement and locations of WPC shall be suitably optimized in consultation with the OCS, ATC, and Communications Systems.

The design of each WPC shall include:

- All equipment provided therein
- Its grounding system
- SCADA interface with the communications system
- Auxiliary power and SCADA interface with the OCS system at the operating panel of the MOD
- Foundations

## **20.13 Calculations**

These requirements have been specified in other Contract Documents.

## **20.14 New Power Utility Services**

### **20.14.1 115 kV/230 kV ac, Single-Phase 60 Hz Services**

The design of the traction power substations and the associated interfaces to the power utility service provider shall comply with the latest version of the codes and standards specified elsewhere in this chapter.

At some locations, the TPS design shall also include full (or partial) design of the HV power utility switching stations and/or the HV interconnections to the traction substations. Refer to the Contract Documents for these requirements.

The power utility services are identified and described in the Facility Interconnection Studies by the local power utility service providers (refer to Section 20.5).

The following requirements shall apply:

- The capacity of each incoming power service shall be sufficient to support the following:

- Full service for the ultimate design headway under normal operation and single electrical contingency mode of operation (refer to Section 20.3.2.2).
- No disruption to (but reduced) services under double contingency mode of operation that includes the outage of an entire substations (refer to Section 20.3.2.2).
- The incoming power services to the SS shall meet the following conditions:
  - Two reliable HV power services, as independent as practical, supplied from separate utility transformers or buses
  - The power services to adjacent SS are to be from different transmission lines, supplied from different utility substations
- Where 1 or more of these conditions cannot be met due to capital costs, and/or existing limitations on the concerned power utility transmission network, the issue shall be submitted to the Authority for review

#### **20.14.2 Auxiliary Power Services**

The design of the TPS and the associated interfaces to the power utility shall comply with the latest version of the codes and standards specified elsewhere in this chapter.

Refer to Section 20.9.13 for additional requirements.

#### **20.15 Spares**

All control, signal, and communication installations shall include at least 10 percent spare conductors and/or fiber strands.

All panel-boards and termination cabinets shall include at least 20 percent spare capacity for future growth.

All ductbank systems shall include at least 1 spare conduit or 20 percent spare conduits, whichever is greater. All spares shall be capped with a pull line/rope secured inside.

Cable tray system shall be sized to include 20 percent spare capacity.

The minimum level of spares for all major equipment and general consumables shall be prescribed by the TPS Designer during the design phase.

Spare provisions, associated with incoming power services, shall be coordinated with the respective power supply utility, and submitted to the Authority for review.



## Chapter 21

### Overhead Contact System and Traction Power Return System

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
0.1	Dec 2012	Execution Version
1	Sept 2013	Rolling Stock Procurement
2	Feb 2014	Revision 2





## Table of Contents

<b>21</b>	<b>OVERHEAD CONTACT SYSTEM AND TRACTION POWER RETURN SYSTEM....</b>	<b>21-1</b>
21.1	Scope .....	21-1
21.2	Regulations, Codes, Standards, and Guidelines .....	21-2
21.3	Definitions .....	21-3
21.4	Overhead Contact System Description and General Performance Requirements...	21-8
21.5	Environmental Conditions and Climatic Loading Requirements .....	21-9
21.5.1	Humidity .....	21-10
21.5.2	Ice.....	21-10
21.5.3	Wind .....	21-10
21.5.4	Atmospheric Pollution .....	21-11
21.5.5	Ambient Temperatures Range .....	21-11
21.5.6	Conductor Wire Temperature Range .....	21-11
21.5.7	Conductor Tensioning .....	21-12
21.6	Overhead Contact Line Design .....	21-12
21.6.1	Geometry of the Overhead Contact Line .....	21-13
21.6.2	Geometry of the Pantographs .....	21-14
21.6.3	Compliance of the Overhead Contact Line System with the Infrastructure Gauge .....	21-16
21.7	Conductor Tensions .....	21-16
21.8	Catenary Conductors .....	21-17
21.8.1	Contact Wire.....	21-17
21.8.2	Messenger Wire .....	21-17
21.8.3	Stitch Wire .....	21-17
21.8.4	Hanger Wire .....	21-17
21.8.5	Alternate Conductors .....	21-17
21.9	Other Overhead Conductors and Cables .....	21-18
21.9.1	Parallel Negative Feeder.....	21-18
21.9.2	Static (Ground) Wire.....	21-18
21.9.3	Insulated 25 kV Cable .....	21-18
21.9.4	Insulated Return Cable .....	21-19
21.10	Dynamic Behavior and Quality of Current Collection .....	21-19
21.10.1	Requirements .....	21-19
21.10.2	Contact Wire Wave Propagation Speed .....	21-20
21.10.3	Static Contact Force.....	21-21
21.10.4	Mean Contact Force.....	21-21
21.10.5	Contact Wire Uplift or Vertical Movement of the Contact Point .....	21-23
21.10.6	Conformity Assessment .....	21-23
21.11	Current Capacity of Overhead Contact System .....	21-24
21.12	Sectionalizing and Switching, and Pole-Mounted Equipment.....	21-25
21.12.1	Pantograph Spacing for Design of the Overhead Contact Line .....	21-25
21.12.2	Phase Breaks .....	21-26
21.12.3	OCS Sectionalizing in Tunnels.....	21-26
21.12.4	Disconnect Switches .....	21-27
21.12.5	Auxiliary Step-down Transformers.....	21-27
21.12.6	Protection against Lightning Strikes.....	21-28
21.13	Insulation Coordination Requirements for OCS Installations.....	21-28

21.14	OCS Clearances and Protection against Electric Shock .....	21-28
21.14.1	Overhead Contact Line Zone and Pantograph Zone .....	21-29
21.14.2	Protection by Clearances from Standing Surfaces .....	21-30
21.14.3	Protective Screening and Barriers for Standing Surfaces in Public Areas .....	21-31
21.14.4	Protective Screening and Barriers for Standing Surfaces in Restricted Areas .....	21-33
21.14.5	Additional Requirements for Protection Barriers and Screens .....	21-36
21.14.6	Protection Against Climbing .....	21-36
21.14.7	Warning Signs .....	21-37
21.14.8	Clearances for Utility Lines Crossing over the Electrified Railroad.....	21-37
21.14.9	Electrical Clearances to Rail Vehicles and Structures .....	21-37
21.14.10	Clearance Envelope at Fixed Structures .....	21-39
21.14.11	Applicable Pantograph and OCS Clearance Envelopes.....	21-41
21.14.12	Clearances to Vegetation.....	21-42
21.15	OCS Structural Requirements .....	21-42
21.15.1	General.....	21-42
21.15.2	OCS Pole and Foundation Requirements.....	21-43
21.15.3	OCS Poles.....	21-45
21.15.4	OCS Foundations.....	21-45
21.16	Traction Power Return System.....	21-46
21.17	OCS Interfaces with Other Disciplines .....	21-47
21.17.1	Traction Power Supply System .....	21-48
21.17.2	Rolling Stock .....	21-48
21.17.3	Train Control System .....	21-48
21.17.4	Communications System .....	21-48
21.17.5	Bridge and Aerial Structure/Earth Retained Structure Design .....	21-48
21.17.6	Trackwork.....	21-49

## Tables

Table 21-1:	Traction Power System Voltages.....	21-8
Table 21-2:	Maximum Permissible Contact Wire Gradient versus Operating Speed .....	21-15
Table 21-3:	Requirements for Dynamic Behavior and Current Collection Quality .....	21-20
Table 21-4:	Maximum Permissible Bare Conductor Temperatures.....	21-25
Table 21-5:	25 kV ac Electrical Clearances .....	21-38

## Figures

Figure 21-1:	Combined Pantograph - Maximum Static Geometry (mm).....	21-15
Figure 21-2:	Target Fm Values (lbf-mph).....	21-22
Figure 21-3:	Target Fm Values (N-km/hr).....	21-22
Figure 21-4:	The Long Phase Break.....	21-26
Figure 21-5:	Overhead Contact Line Zone and Pantograph Zone .....	21-30
Figure 21-6:	Minimum Required Safety Clearances – Unconstrained Access .....	21-31

Figure 21-7: Clearances from Protective Screens and Barriers for Standing Surfaces in Public Areas .....	21-33
Figure 21-8: Clearances from Protective Screenings and Barriers for Standing Surfaces in Restricted Areas .....	21-35
Figure 21-9: Vertical Clearance Envelope at Fixed Structures.....	21-41
Figure 21-10: Typical Proportional Current Distribution in a 2x25 kV Autotransformer System for a Train Current of 200 Amps.....	21-47

## Acronyms

ACSR	Aluminum Conductor Steel Reinforced
EMC	Electromagnetic Compatibility
HST	High-Speed Train
NESC	National Electrical Safety Code
OCS	Overhead Contact System
PS	Paralleling Station
SCADA	Supervisory Control and Data Acquisition
SS	Substation
SWS	Switching Station
TES	Traction Electrification System
TPF	Traction Power Facility

## 21 Overhead Contact System and Traction Power Return System

### 21.1 Scope

1 These criteria detail the overhead contact system (OCS), and the traction return system,  
2 including the parallel negative feeders.

3 The OCS is a system in which electrical conductors are supported aerially above the Authority's  
4 right-of-way, generally by means of insulators and appropriate mechanical support arms or  
5 brackets, and which supplies electrical energy from the traction power supply facilities to rail  
6 mounted, electrically-powered vehicles through onboard, roof-mounted current collection  
7 equipment (pantographs). The OCS comprises the following:

- 8 • All overhead wiring, including the messenger wires, stitch wires, and contact wires,  
9 mounted on OCS support structures or brackets
- 10 • The foundations, supporting structures, and any components supporting, registering,  
11 terminating or insulating the conductors
- 12 • Insulators, neutral-sections, auto-tensioning devices, and other overhead line hardware and  
13 fittings
- 14 • Equipment mounted on the supports for feeding, switching, detection or protection
- 15 • Overhead conductor rails and their insulated support arrangements (if used) in very  
16 restricted clearance locations

17 The traction return system is the means by which traction current is returned from the wheel-  
18 sets of traction units to the traction power facilities of the electrified railway track, comprising  
19 the negative feeders (due to the configuration of the autotransformer connections), the  
20 grounded running rails, aerial static wires (and buried ground conductors), together with all  
21 return current bonding and grounding interconnections. Grounding and bonding and lightning  
22 protection for the electrified railway is covered in the *Grounding and Bonding Requirements*  
23 chapter.

24 In an auto-transformer feed system, the feeder (often termed the negative feeder) is a paralleling  
25 conductor that is electrically separate from the catenary conductors over the tracks. This parallel  
26 (negative) feeder connects successive feeding points, and is connected via circuit breakers  
27 and/or disconnect switches to 1 terminal of a main power supply transformer or  
28 autotransformer in the traction power facilities. At these facilities, the other terminal of the  
29 transformers is connected to an OCS section (or sections) via circuit breakers or disconnect  
30 switches.

## 21.2 Regulations, Codes, Standards, and Guidelines

- 1 Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.
- 2 • American Railway Engineering and Maintenance-of-Way Association (AREMA) Manual for  
3 Railway Engineering, Chapter 33 Electrical Energy Utilization
  - 4 • European Committee for Electrotechnical Standardization (CENELEC) Standards
    - 5 – EN 50119, 2001, Electric Traction Overhead Contact Lines
    - 6 – EN 50122-1 Part 1, 1998, Protective Provisions Relating to Electrical Safety and Earthing
    - 7 – EN 50124-1, 2001, Insulation Coordination: Part 1 – Basic Requirements
    - 8 – EN 50149, 2001, Electric Traction: Copper and Copper Alloy Grooved Contact Wires
    - 9 – EN 50206, 1999, Pantographs: Characteristics and Tests
    - 10 – EN 50317, 2002, Requirements for and Validation of Measurements of the Dynamic  
11 Interaction between Pantograph and Overhead Contact Line
    - 12 – EN 50318, 2002, Validation of Simulation of the Dynamic Interaction between  
13 Pantograph and Overhead Contact Line
    - 14 – EN 50367, 2006, Technical Criteria for the Interaction between Pantograph and  
15 Overhead Line
  - 16 • California Code of Regulations (CCR) Title 8, Division 1, Chapter 4, Subchapter 5: Electrical  
17 Safety Orders
  - 18 • Institute of Electrical and Electronics Engineers (IEEE)
    - 19 – IEEE C2, National Electrical Safety Code (NESC)
    - 20 – IEEE 80, Guide for Safety in AC Substation Grounding
    - 21 – IEEE 142, Recommended Practice for Grounding of Industrial and Commercial Power  
22 Systems
  - 23 • California Public Utilities Commission (CPUC) General Orders (GOs)
    - 24 – CPUC GO 26-D, Regulations Governing Clearances on Railroads and Street Railroads  
25 with Reference to Side and Overhead Structure Parallel Tracks, Crossings of Public  
26 Roads, Highways and Streets
    - 27 – CPUC GO 95, Rules for Overhead Electric Line Construction
    - 28 – CPUC GO 143-B, Safety Rules and Regulations Governing Light Rail Transit
  - 29 • Technical Specification for Interoperability (TSI) Energy, 2008, Technical Specifications for  
30 the Interoperability of Electrical Energy Subsystems

## 21.3 Definitions

<b>Agency</b>	The railroad or other jurisdictional entity that is responsible for the operation and maintenance of the railroad
<b>Barrier</b>	Equipment provided to prevent entry by an unauthorized person to a restricted area, structure or building, which also provides physical protection against direct contact with energized parts from non-normal directions of access
<b>Bond</b>	A bond is an electrical connection from 1 conductive element to another for the purpose of maintaining a common electrical potential (equi-potential).
<b>Bonding Conductor</b>	A conductor for ensuring equi-potential bonding
<b>Collector Head</b>	That part of the pantograph that runs under and in contact with, and collects current from, the overhead contact wire or conductor rail.
<b>Cross Bond</b>	An electrical bond that interconnects the running rails, which in signalized territory, must be connected through impedance bonds.
<b>De-energized</b>	<p>Electrical apparatus, such as overhead wires, substation conductors, cables, switches and circuit breakers, which is disconnected from its electrical power source(s), but is not necessarily grounded.</p> <p>Note: This does not imply or ensure a safe state.</p>
<b>Direct Contact</b>	Contact with energized parts
<b>Direct Feed System</b>	A traction power feeding system in which the transformers are fitted with a single secondary winding having 2 terminals. One terminal is connected to the running rails/ground and the other to the catenary conductors over the tracks.
<b>Direct Traction System Grounding</b>	<p>The direct connection between conductive parts and the traction system ground</p> <p>Note: Grounding via impedance bonds, required by reason of signaling system track circuit considerations, is considered to be direct grounding.</p>
<b>Effectively Grounded</b>	Intentionally connected to earth through a ground connection or connections of sufficiently low impedance and having sufficient current-carrying capacity to limit the build-up of voltages to levels below which undue hazards to persons or to connected equipment may result.



<b>Electrical Section or Feed Section</b>	This is the entire section of the OCS, which during normal system operation, is powered from an SS circuit breaker. The SS feed section is demarcated by the phase breaks of the supplying SS and by the phase breaks at the adjacent SWS or line end. An electrical section maybe subdivided into smaller elementary electrical sections.
<b>Elementary Electrical Section</b>	This is the smallest section of the OCS traction power distribution system that can be isolated from other sections or feeders to the system by means of disconnect switches and/or circuit breakers.
<b>Electric Shock</b>	The effect of an electric current passing through the human body
<b>Energized</b>	Electrical apparatus, such as overhead wires, substation conductors, cable, disconnect switches, and circuit breakers, that are connected to an electric power source.
<b>Energized Part</b>	An energized part is a conductor or conductive part that is energized under normal service conditions, but does not include the running rails or parts connected to them. Energized parts include roof-mounted equipment on electric vehicles, such as pantographs, train line conductors, and resistor units. The full length of insulators connected to energized parts shall be classified as energized when considering electrical clearance requirements.
<b>Fault Condition</b>	The presence of an unintended and undesirable conductive path in an electric power system
<b>Feeder</b>	<p>A current-carrying electrical connection, energized at high voltage (HV), between a traction power facility (substation, paralleling station or switching station) and the catenary conductors, which is energized at high voltage (HV) – 25 kV nominal, and is supported on the same structure as the catenary and static wire.</p> <ol style="list-style-type: none"><li>1. In an auto-transformer feed system, the feeder (often termed the negative feeder) is a paralleling conductor that is electrically separate from the catenary conductors over the tracks. This parallel (negative) feeder connects successive feeding points and is connected, via a circuit breaker(s) and/or disconnect switch(es), to 1 HV terminal of a main power supply transformer or an auto-transformer in the traction power facilities. At these facilities, the other HV terminal of the main power supply transformer or auto-transformer is connected to a catenary section (or sections) via circuit breakers or disconnect switches.</li><li>2. In a direct feed system, the feeder is a paralleling conductor that can be connected at frequent intervals to the OCS to provide localized electrical reinforcement of the circuit by increasing the effective cross-sectional area of the electrical system in that section.</li></ol>

<b>Grounding Conductor</b>	A conductor that is used to connect equipment or wiring systems to a ground electrode or ground grid
<b>High Voltage (HV)</b>	A nominal voltage of 600 Volts or more
<b>Leakage Current</b>	A current that flows to ground or to extraneous conductive parts, following a path or paths other than the normal intended path, but which is not of sufficient magnitude to create a fault.
<b>Non-Current-Carrying Parts</b>	Metallic parts within the Authority's right-of-way which do not normally carry load currents or return currents
<b>Overhead Conductor Rail</b>	A rigid metallic conductor, which substitutes for the contact wire and is mounted on insulators under a fixed overhead structure
<b>Paved Areas</b>	<p>In selected areas of maintenance facilities, yards and shops, the trackway may be paved to the upper level of the running rails to provide for the crossing of maintenance vehicles over the tracks and under the overhead conductors.</p> <p>Note: Where railroads support high-speed operations, at-grade crossings of any description are not permitted.</p>
<b>Rail Joint Bond</b>	A conductor that ensures the electrical continuity of a running rail at an uninsulated, bolted rail joint
<b>Rail Potential</b>	The voltage between running rails and ground occurring both under operating conditions when the running rails are utilized for carrying the traction return or under fault conditions
<b>Rail to Ground Resistance</b>	The electrical resistance between the running rails and the earth
<b>Railroad or Railway Environment</b>	The area adjacent to the running rails that is subject to the noise, vibration and air pressure of trains operating at high speed, and to the effects of the voltages, currents and electric fields associated with a 25 kV ac TES
<b>Rake</b>	A preset lean of an OCS pole from vertical
<b>Regenerative Braking</b>	A system in which the drive motors of the electric vehicles operate as generators and provide dynamic braking of the vehicle, while at the same time returning power to the OCS that can be used by receptive vehicles on the system or can be returned to the utility at the traction power substations.
<b>Return Cable</b>	A conductor that forms part of the TES return circuit, and which connects the rest of the return circuit to the substation

<b>Right-of-Way classification</b>	<p>Right-of-Way (ROW) classifications are defined in CPUC GO No. 143-B Rule 9.04</p> <p>1. Exclusive -- a railroad or railway right-of-way without at-grade crossings, which is grade-separated or protected by a fence or substantial barrier, as appropriate to the location (including subways and aerial structures).</p> <p>Note: This is the only type of right-of-way that is acceptable for the operation of trains at speeds in excess of 125 mph.</p> <p>2. Semi-Exclusive -- exclusive right-of-way with at-grade crossings protected between crossings by a fence or substantial barrier, as appropriate to the location.</p> <p>Note: This type of right-of-way is not suitable for high-speed operations in excess of 125 mph, since at-grade crossings of the high-speed tracks cannot be permitted in accordance with FRA regulations.</p>
<b>Running Rails</b>	<p>The steel rails on which the rail vehicles run and which, in an electrified system, form part of the traction return circuit. The running rails may also be used for signal system track circuits, in which case special measures must be implemented to permit joint use with electrification.</p>
<b>Screen</b>	<p>A barrier that prevents unintentional direct contact with energized parts but will not totally prevent direct contact by deliberate action.</p>
<b>Short Circuit</b>	<p>A conductive path between energized and grounded components which may result in a high fault current.</p> <p>Note: Any such conductive path whether between conductors or between a conductor and ground is regarded as a short circuit.</p>
<b>Short Circuit Current or Fault Current</b>	<p>The electric current flowing through the short circuit or fault path.</p>
<b>Standing Surface</b>	<p>Any point on a surface where persons may stand or walk.</p>
<b>Step Potential or Voltage</b>	<p>The difference in surface potential experienced by a person bridging a distance of 3 feet-3 inches with their feet without contacting any grounded object.</p>

<b>Stitch wire</b>	The stitch wire is a supplementary tensioned conductor that is attached to the messenger wire and positioned at the supports with hangers supporting the contact wire. The spring effect of the stitch wire and hanger arrangement enhances the elasticity of the catenary at the support and provides for a better match with the mid-span elasticity of the catenary, thereby providing improvement in the quality of the current collection.
<b>Stray Current</b>	A current which follows a path or paths other than the intended electrical path (see Leakage Current).
<b>Supports</b>	The structural elements that support the conductors and their associated line hardware and insulators in an OCS.
<b>Surge Arrester or Surge Suppressor</b>	<p>A protective device for limiting surge voltages on equipment by discharging or by-passing surge current; it limits the flow of power follow-on current to ground, and is capable of repeating these functions.</p> <p>Note: Sometimes referred to as a Lightning Arrester.</p>
<b>Touch Voltage:</b>	The potential difference between the ground potential rise (GPR) and the surface potential at the point where a person is standing while at the same time having a hand in contact with a grounded structure (Per IEEE-80).
<b>Traction Power Substation (SS)</b>	An electrical installation where power is received at high voltage and transformed to the voltage and characteristics required at the catenary and negative feeders for the nominal 2x25 kV system, containing equipment such as transformers, circuit breakers and sectionalizing switches. It also includes the incoming HV lines from the power supply utility.
<b>Traction Return Current</b>	The sum of the currents returning to the supply source (i.e., the substation).
<b>Traction System Ground</b>	The traction system ground consists of the running rails, the aerial static wires and all conductive parts connected thereto and which are solidly connected to ground.
<b>Traction System Grounding</b>	Connection between non-energized metallic parts and the traction system ground.
<b>Tunnel Ground</b>	<p>The electrical interconnection of the reinforcing steel in reinforced concrete tunnels and, in the case of other modes of construction, the conductive interconnection of the metallic parts of the tunnel.</p> <p>Note: In the case of single-phase ac traction systems, the tunnel ground is connected to the running rails and thus forms part of the traction system ground which may be supplemented by external ground connections to earth.</p>

**Voltage-limiting Device** A protective device which operates to prevent the permanent existence of a dangerously high step or touch voltage.

## 21.4 Overhead Contact System Description and General Performance Requirements

- 1 In order to minimize the number of substations and Electromagnetic Compatibility (EMC)
- 2 problems along the alignment, the line will be fed by a 2x25 kV, 60 Hz autotransformer power
- 3 supply system, utilizing traction power substations, switching stations and paralleling stations.
- 4 The Traction Power Substations (SS) will be connected to HV utility supplies and spaced
- 5 approximately every 30 miles, while the Switching Stations (SWS) will be spaced at
- 6 approximately mid distance between SS, i.e., at about 15 miles from each SS, and the Paralleling
- 7 Stations (PS) will be spaced at approximately 5 mile intervals. At the PS and SWS locations, the
- 8 autotransformers will parallel the Track 1 and Track 2 power supplies and balance the 2 25 kV
- 9 supplies (longitudinal parallel negative feeder and catenary) with respect to each other.
- 10 The OCS shall support voltage variations in accordance with IEC 60850 "Supply Voltages of
- 11 Traction Systems", as shown in Table 21-1.

**Table 21-1: Traction Power System Voltages**

Voltage Condition	Symbol	Voltage
Operating nominal system voltage		25.0 kV
Highest permanent voltage	$U_{\max 1}$	27.5 kV
Highest non-permanent voltage	$U_{\max 2}$	29.0 kV
Lowest permanent voltage	$U_{\min 1}$	19.0 kV
Lowest non-permanent voltage	$U_{\min 2}$	17.5 kV

- 12 In addition, the maximum short circuit current shall be 15 kA for protection measurement
- 13 purpose and accordingly for specification of the electrical equipment.
- 14 At all traction power supply stations the center tap of the respective supply transformer or
- 15 auto-transformer will be connected to and referenced to the running rails, which will nominally
- 16 be at ground potential.
- 17 The OCS will provide electric traction power to the pantographs of the electric trains using the
- 18 route and will, therefore, be configured as a 25 kV-0-25 kV arrangement with the catenary at a
- 19 nominal voltage of 25 kV to ground and the longitudinal parallel negative feeder also at a
- 20 nominal voltage of 25 kV to ground, but in phase opposition to the catenary. There is a
- 21 180 degree phase difference between the voltages of the parallel negative feeders and the OCS,
- 22 giving a 50 kV phase-to-phase voltage difference between these conductors. The OCS shall
- 23 transfer electric power from the Traction Power Substations to the trains under all operating

conditions and shall provide for reliable operation under the environmental conditions detailed in Section 21.5.

Except at Phase Breaks, the OCS shall provide for uninterrupted traction power collection at the maximum operating speed of 220 mph.

To allow bi-directional working, enabling trains to continue operation under emergency conditions and to facilitate routine OCS maintenance, the OCS shall be divided into electrical sections and sub-sections. The OCS shall be sectionalized as indicated in Section 21.12.

To facilitate operations and maintenance activities, the OCS shall typically be equipped with non-load break motor operated disconnect switches at feeding points, which can be operated both locally on site and remotely through a supervisory control and data acquisition (SCADA) system. The switches shall be fitted with OCS voltage detection circuitry that will provide for remote monitoring of the system.

The OCS phase break arrangements shall be located at SWSs and, as required, at SS to electrically separate 2 successive catenary electrical sections fed from different 25 kV ac sources, i.e., not of the same phase. The electric trains shall pass through each phase break arrangement without establishing an electrical connection between the successive electrical sections which are fed from different phases. This shall be achieved at the designated maximum operating speed with the train pantographs raised and in contact with overhead contact wire, but with the pantograph breakers off.

Rail return shall primarily be through the running rails, but a static wire (ground wire) shall be provided that interconnects all OCS support structures (poles, portal structures, wall brackets, tunnel drop pipes, etc.), which shall be connected via impedance bonds to the running rails and to the ground grid at each traction power facility (TPF). Other cross-bonding connections may be required to minimize rail potential rise, and the frequency and location of these connections and of the impedance bonds shall be determined under the TP system design and coordinated with the ATC System design. Refer to the *Grounding and Bonding Requirements* chapter. In electrical sections remote from sections in which trains are operating, the parallel negative feeder effectively carries much of the return current and minimizes the amount that flows through the rails and static wires.

For a more comprehensive description of the traction power supply system and its associated facilities, refer to the *Traction Power Supply System* chapter.

## 21.5 Environmental Conditions and Climatic Loading Requirements

Information on climatic and environmental conditions in the corridor is given in the *General* chapter with data listed on a segment-by-segment basis. The OCS shall be designed on a system-wide basis to provide for reliable operation under the following environmental and climatic conditions.

### 21.5.1 Humidity

The OCS shall operate without failure or deterioration in all humidity conditions found in California. These include 100 percent humidity, including rain, heavy fog and salt-laden atmospheres in sections of the route near the ocean, and 100 percent humidity in tunnels.

### 21.5.2 Ice

Reference to Figure 7.1 “Ground Snow Loads” of the ASCE Standard “Minimum Design Loads for Building Structures” indicates limited snow falls and formation of ice along the alignment. In accordance with Table 250-1 and Figure 250-3(a) of the National Electrical Safety Code (NESC), the OCS design shall not consider ice loading.

### 21.5.3 Wind

The ASCE Standard “Minimum Design Loads for Building Structures” defines the basic wind speed corresponding to the wind load for wind force resisting structures as a 3 second gust speed at 33 feet above ground for open terrain, Exposure C, associated with an annual probability of 0.02 (50 year mean recurrence interval) of being equaled or exceeded. This basic wind speed, in accordance with Figure 6-1 of the ASCE Standard, is  $V_{bws} = 85$  mph for the State of California. This 3 second gust speed corresponds to a mean maximum hourly wind speed of  $V_{bws}/1.52 = 56$  mph approximately.

In accordance with Section 4.2.2 of Chapter 33 of the AREMA Manual, 2 different wind speeds, the operational wind speed and the design wind speed shall be used for OCS design:

- The operational wind speed shall be used to compute catenary support loading, catenary wire displacement for pantograph security, and permissible maximum span lengths, and will be taken as  $V_{op} = 60$  mph.
- The design wind speed shall be used to determine the ultimate strength requirements of the OCS and shall be taken as  $V_{bws} = 85$  mph corresponding to the ASCE and NESC basic wind speed for the route.

The wind velocity pressure  $q_z$  shall be calculated by the NESC formula:

$$q_z = 0.00256 V^2 K_z G_{RF} I C_f A \quad \text{in pounds/square foot}$$

Where:

- 0.00256 is the velocity pressure numerical coefficient reflecting the mass density of air for the standard atmosphere
- $K_z$  is the velocity pressure exposure coefficient
- $V$  is the basic wind speed = 3 second gust wind speed at 33 feet above ground for open terrain, Exposure C; i.e.,  $V_{bws}$  in mph

- $G_{RF}$  is gust response factor
- $I$  is the importance factor ( $I$  being equal to 1.00 for OCS)
- $C_f$  is the force coefficient shape factor
- $A$  is the projected wind area

Note:  $K_z$ ,  $V$  and  $G_{RF}$  are based on open terrain with scattered obstructions (Exposure Category C as defined by ASCE, and are used as the basis for the NESC extreme wind criteria). For very exposed areas, the wind velocity pressure shall be increased by the ASCE factor  $K_{zt}$ .

For OCS structural calculations, loads due to wind shall be multiplied by the load factors given in NESC Table 253-1. The effects of wind pressure on OCS poles, due to slipstream effects caused by the proximity to high-speed trains operating at speeds in excess of 125 mph shall also be considered.

#### 21.5.4 Atmospheric Pollution

The OCS equipment shall be resistant to polluted atmospheres, such as may occur in highly industrialized areas, salt-laden marine atmospheres near the ocean, and persistent fog. In addition, the OCS equipment shall be resistant to the corrosive atmospheres that may be found in tunnels and cut-and-cover structures.

#### 21.5.5 Ambient Temperatures Range

**General:** In developing the OCS arrangements, the designer shall take into consideration the typical and extreme ambient temperatures, as recorded on a segment-by-segment basis along the route.

**Tunnels:** For long tunnels, only the first 1300 feet of catenary from each portal shall be considered subject to external ambient temperature variations. For the balance of the tunnel length inside the 1300-foot limits, the Designer shall confirm the probable ambient temperature range, which may differ from the external range, and shall use the identified range for the tunnel OCS design.

#### 21.5.6 Conductor Wire Temperature Range

**General:** In developing the settings for the auto-tensioning devices (balance weight anchor assemblies [BWAs]), the designer shall take into consideration the extreme operational OCS conductor temperatures. Based on the initial analyses, the messenger and contact wires and the parallel feeder conductors are likely to reach a maximum operating temperature of 176°F in above grade sections and for the first 1300 feet in tunnels.



### 21.5.7 Conductor Tensioning

1 **General:** The mechanical tensions in the messenger and contact wires shall be maintained  
2 automatically throughout the temperature ranges specified above.

3 The designer shall confirm the probable maximum conductor temperatures in the tunnels,  
4 which shall be used for OCS tensioning and support system design.

## 21.6 Overhead Contact Line Design

5 The OCS shall be of a proven design that is capable of sustaining satisfactory current collection  
6 for train operations at 220 mph.

7 The OCS System Designer shall be cognizant of and shall incorporate into the OCS design the  
8 fundamental design data and performance instructions, as defined in these Design Criteria,  
9 which include the following:

- 10 • Service and operations information
- 11 • Infrastructure characteristics
- 12 • Vehicle characteristics
- 13 • Pantograph characteristics
- 14 • Traction power system design
- 15 • Environmental conditions
- 16 • Safety requirements, including external limitations on contact wire height, uplift, system  
17 height, and/or clearances
- 18 • Life expectancy and desired maintenance/renewal philosophy for all components, plus  
19 allowable grooved contact wire wear
- 20 • Specification of EMC limitations

21 It is vital that the OCS design be coordinated with the pantograph and rolling stock designs  
22 because 1 of the most critical aspects in the development of the OCS design is the dynamic  
23 performance of the overhead contact line and the need to achieve good quality current  
24 collection at high operating speeds. Refer to the *Rolling Stock-Core Systems Interface* chapter. The  
25 principal measure of this aspect is loss of contact between the pantograph strip and the contact  
26 wire, which must be minimized. In order to minimize contact loss and the creation of arcs, the  
27 Designer shall comply with the requirements for dynamic behavior and quality of current  
28 collection as detailed in the following sections.

29 Other important aspects of the design are the provision of adequate clearances or provision of  
30 protective barriers and screens, together with effective grounding and bonding of the electrical  
31 system and wayside metallic objects, which are required to minimize safety hazards.

1 The recommended minimum clearances between energized parts and grounded parts are  
2 detailed in Section 21.14.8. The location of OCS equipment, including poles and downguys,  
3 shall be coordinated with clearances defined in the *Utilities* chapter.

#### 21.6.1 Geometry of the Overhead Contact Line

4 The OCS shall consist of a simple, stitched auto-tensioned catenary system, using a bare hard-  
5 drawn copper, bronze or other copper alloy messenger wire supporting a nominally level (no  
6 pre-sag), solid copper alloy contact wire by means of copper alloy current carrying hangers.

7 In general, the catenary shall be supported by pole mounted cantilever frames, which shall be  
8 designed to provide the required system height and to register the correct stagger of the wires  
9 relative to the track centerline. The messenger wire shall be positioned vertically (plumb) above  
10 the contact wire. Back-to-back cantilevers, supported on single poles centered between tracks,  
11 shall not be used for the high-speed main tracks, except in station areas where their use will be  
12 permitted between a through-line and a station platform track.

13 An aerial static wire (ground wire), connected at regular intervals to the track via impedance  
14 bonds, shall be run alongside the catenary to interconnect each OCS support structure and  
15 bracket, such that all OCS non-live metallic supports are at the same ground (and track)  
16 reference potential.

17 The longitudinal negative feeder shall be supported near the top of the OCS poles, preferably  
18 on the track side, but may be positioned on the field side where the Authority's right-of-way  
19 width or overhead structure configuration dictates.

20 The aerial parallel negative feeders, and the aerial static/ground wires that connect all OCS  
21 supporting structures, shall both be fixed termination bare ACSR (Aluminum Conductor Steel  
22 Reinforced) conductors, except where local site conditions (reduced clearances, etc.) dictate the  
23 need to use insulated cables for the negative feeders.

24 The method of auto-tensioning the messenger wire and contact wire shall be by balance weight  
25 and pulley tensioning devices. The tensions shall be applied to the contact and messenger wires  
26 individually, using separate balance weights, tensioning devices and anchoring positions.

27 The Designer shall evaluate overlap arrangements but the initial analyses indicate that the  
28 overlaps should comprise a 5-span configuration.

29 Maximum tension lengths from anchor to anchor shall not exceed 4,600 feet in open route  
30 sections and 4,000 feet in tunnels and adjacent to traction power substations and switching  
31 stations. Exceptions up to 5,000 feet may be allowed on a case-by-case basis. At approximately  
32 mid-distance between auto-tension terminations, mid-point anchor arrangements shall be  
33 installed, such that the maximum half tension lengths do not exceed 2,300 feet in open route  
34 sections and 2,000 feet in tunnels and at power supply stations and switching stations.

1 The maximum permissible span length between supports shall be determined using  
2 appropriate computer software programs, which shall take into consideration the permissible  
3 working range of the pantograph and allowable lateral displacement of the contact wire under  
4 the designated operating conditions, including dynamic movement of the vehicle and  
5 pantograph. The programs shall be fully validated and back-up hand calculations shall be  
6 furnished. The maximum permissible span differential (difference in adjacent span lengths)  
7 shall be no more than 33 feet with the proviso that, if the dynamic pantograph-OCS simulation  
8 results (detailed below) indicate other values may be more appropriate, these shall be adopted.

9 At overlap locations where sectionalizing is not required, uninsulated mechanical overlaps shall  
10 be installed that permit the pantographs to transition smoothly from 1 tension length to the next  
11 under power.

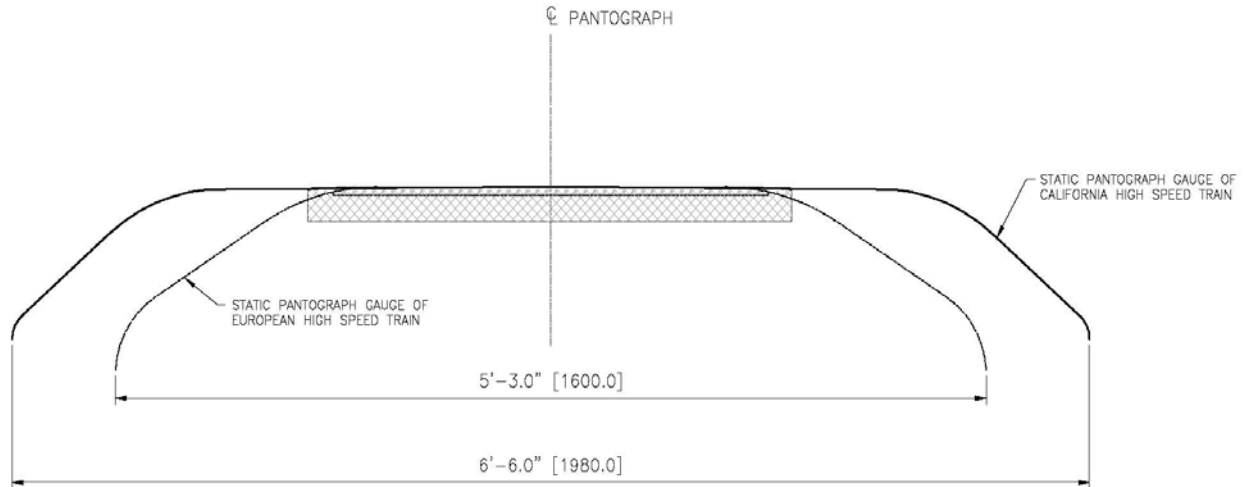
12 Wherever practicable, the OCS shall be free running under overhead bridges, i.e., no OCS or  
13 feeder support attachments under the structure. New bridges shall be designed to  
14 accommodate a free-running clearance height. Existing bridge clearances, if any, shall be  
15 reviewed to determine whether free-running OCS arrangements can be accommodated, which  
16 is the goal. The Designer shall secure permits for attachments to any third party owned bridges  
17 or structures where it is determined that OCS support, registration or termination attachments  
18 will be required.

19 In tunnels, the OCS, feeders and static wires shall, wherever possible, be supported by  
20 cantilevers attached to soffit-mounted drop pipes, or from wall-mounted support brackets. The  
21 OCS system height and cantilever geometry will be dictated by the available headroom. In  
22 limited clearance locations, particularly at low headroom bridges or in cut-and-cover tunnels,  
23 resilient arms, supporting and registering both messenger and contact wires, may have to be  
24 utilized. If any extremely restricted clearance locations are identified, it may be necessary to  
25 adopt the use of conductor rail arrangements, but these should be avoided wherever possible.

### **21.6.2 Geometry of the Pantographs**

26 The Overhead Contact Line shall be designed to accommodate pantographs with a pantograph  
27 head profile, as depicted on Figure 21-1, which shall be based on the geometry detailed in EN  
28 50367: 2006 Figure B.3, but having a maximum pantograph head width of 78 inches (1980 mm),  
29 and with horns made of insulating material. Based on the dynamic OCS-Pantograph  
30 simulations and other dynamic OCS analyses, the Designer may determine that a narrower  
31 width pantograph is acceptable. Regardless, it shall not be smaller than the 5 feet 3 inch (1600  
32 mm) European standard profile as shown in Figure A.7 in EN 50367: 2006.

1 **Figure 21-1: Combined Pantograph - Maximum Static Geometry (mm)**



2  
3 Pantograph heads fitted with contact strips, having independent suspensions, shall remain  
4 compliant to the overall profile with a static contact force of 15.75 pounds force (lbf) applied to  
5 the middle of the head.

6 The contact wire shall be installed and maintained at a nominal constant height of  
7 17 feet 5 inches at the supports with a construction tolerance of  $\pm 0.5$  inches subject to the  
8 proviso that the contact wire height difference at adjacent structures shall be less than 1/2 inch  
9 to ensure the near-constant contact wire height that is required for satisfactory current collection  
10 by pantographs at high speed.

11 To satisfy the clearance requirements above paved areas in maintenance facilities, yards and  
12 workshops, as detailed in Clause 21.14.12, the contact wire shall be installed and maintained at  
13 a nominal constant height of 20 feet 6 inches at the supports with a construction tolerance of  $\pm 1$   
14 inch subject to the proviso that the contact wire height difference at adjacent structures shall be  
15 not more than 1 inch. To make the transition from the mainline contact wire height of  
16 17 feet 5 inches to the yard height of 20 feet 6 inches, the contact wire height shall be increased  
17 in accordance with the gradients specified in Table 21-2.

18 The maximum permissible contact wire gradients and the corresponding maximum gradient  
19 changes shall not exceed, according to the maximum speed, the following values:

**Table 21-2: Maximum Permissible Contact Wire Gradient versus Operating Speed**

Maximum Speed (mph)	Maximum Contact Wire Gradient	Maximum Contact Wire Gradient Change
> 125	0	0
125	2/1000	1/1000
100	3.3/1000	1.7/1000

75	4/1000	2/1000
60	6/1000	3/1000
45	8/1000	4/1000
30	13/1000	6.5/1000

On tangent track (straight track), the contact wire shall be staggered at each location to alternate sides of the pantograph center line, and the stagger shall normally be set at  $\pm 12$  inches. On curved track, the staggers shall be calculated on a case by case basis taking into account the track superelevation, radius of curvature, and wind speed, but shall not exceed 15 inches. Registration elements (steady arms, contact wire clips, etc.) shall be as light as possible to minimize the possibility of creating a hard spot in the contact wire.

For pantograph security purposes, the permissible lateral deflection of the contact wire under the action of crosswind (defined as the maximum operational wind speed for which unrestricted train operations will be permitted) shall be the smaller of  $\leq 15 \frac{3}{4}$  inches (400 mm) or 55-L2 inches ( $[1.4-L2]$  m) for the 5 feet 3 inch wide pantograph, as specified in ENE 4.2.9.2, where L2 is the half-width of the dynamic envelope of the pantograph passage (as defined in Appendix A.3 of EN 50367: 2006). If a pantograph width other than 5 feet 3 inch is to be used, the 55 inch dimension shall be adjusted.

### 21.6.3 Compliance of the Overhead Contact Line System with the Infrastructure Gauge

The OCS design shall comply with the static and dynamic envelopes, as defined in the *Trackway Clearances* chapter, for vehicles that will run on the dedicated high-speed train tracks.

The design of civil structures shall take into account the space necessary for the passage of pantographs in contact with the overhead line equipment and for installation of the OCS itself, as detailed in Section 21.14.10 and as depicted in Figure 21-9, and the Standard and Directive Drawings. The dimensions of tunnels and other structures shall be mutually compatible with the geometry of OCS and the dynamic envelope of the pantograph, the static profile of which is depicted on Figure 21-1.

## 21.7 Conductor Tensions

The permissible tensile loading of the wires and ropes to be used shall consider the weighted parameters, as indicated in EN 50119: 2001, clauses 5.2.4, 5.2.5, and 5.2.6, which include maximum working temperature (excluding short circuit loading), allowable wear, wind and ice loads, tensioning accuracy and efficiency, termination fitting effects, welded or soldered joint effects, and creep, as applicable. In addition, the current heating effects of short circuit faults occurring during peak operations shall be assessed to ensure that the maximum permissible conductor temperatures are unlikely to be exceeded.

## 21.8 Catenary Conductors

1 Initial analyses have shown that the following conductors form a viable catenary. The Designer  
2 shall confirm conductor sizes and material selection as defined in these criteria. The same  
3 conductor and cable types and sizes shall be used throughout the entire system.

### 21.8.1 Contact Wire

4 The proposed contact wire is a 150 mm<sup>2</sup> (approximately 300 kcmil equivalent) grooved copper-  
5 magnesium alloy wire, designated CuMg 0.5 that shall comply with the requirements of EN  
6 50149: 2001 Clauses 4.1, 4.2, 4.5, 4.6 and 4.7, regarding the material designation and  
7 composition, conductor appearance and condition, clamping groove, electrical characteristics  
8 (resistivity, resistance per mile [km]), tensile strength and percentage elongation after fracture,  
9 breaking load, and mass of the wire. Joins shall be permitted in drawing stock or intermediate  
10 rod stock, as detailed in EN 50149: 2001 Clause 4.8, but no joints shall be made in the completed  
11 wire.

### 21.8.2 Messenger Wire

12 The proposed messenger wire is a 300 kcmil, 37-strand, hard drawn copper conductor  
13 conforming to ASTM B-1 and ASTM B-8 requirements. Substitutes that can meet the electrical  
14 and mechanical requirements could be accepted, as detailed in Section 21.8.5 of these criteria.

### 21.8.3 Stitch Wire

15 The proposed stitch wire is a 76 kcmil, 7-strand, hard drawn bronze conductor, Alloy 55,  
16 conforming to ASTM B-8 and ASTM B-105 requirements.

### 21.8.4 Hanger Wire

17 The Designer shall select a suitable flexible conductor for the hanger wire, which together with  
18 the messenger wire and contact wire clips, shall provide an electrical connection between the  
19 messenger and contact wires.

### 21.8.5 Alternate Conductors

20 If the Designer opts to use conductors other than those indicated above, the Designer shall  
21 confirm conductor sizes and material selection as defined in these criteria. In addition, the  
22 Designer shall require the manufacturer to provide conformity verifications, as detailed in EN  
23 50149, during the production phase of all catenary wires.

## 21.9 Other Overhead Conductors and Cables

1 Insulated cables and conductors required by other disciplines such as signal cable, signal-power  
2 cables, control wires and communications cables, will generally be installed underground but  
3 may occasionally have to be mounted aerially on the OCS poles.

4 These aerial conductors shall be mounted and spaced on the OCS support structures in  
5 accordance with the more stringent requirements of either the NESC or CPUC General Order  
6 rules, as they apply to each system classification. Mounting arrangements shall provide for the  
7 safety of maintenance personnel. These cables and conductors shall be mounted and profiled in  
8 such a manner as to avoid the Overhead Contact Line Zone and Pantograph Zone (Figure 21-5)  
9 to the greatest extent practicable. Loading calculations and structural designs for the support of  
10 these cables and conductors shall comply with these design criteria.

11 Insulated cables and bare conductors (other than the catenary conductors identified above) that  
12 are associated with the OCS may parallel or cross the Authority's right-of-way, including the  
13 parallel negative feeders, and static (ground) wires. The following selections have been made  
14 based on preliminary analyses. However, each shall be confirmed by the Designer.

### 21.9.1 Parallel Negative Feeder

15 In general, the parallel negative feeder shall be a bare stranded 556 kcmil ACSR "Eagle"  
16 conductor for use throughout the system. Since the mainline will be 2-track, with platform  
17 tracks at intermediate stations, 2 negative feeders are to be installed; 1 on each side of the  
18 Authority's right-of-way.

19 At locations where a bare conductor cannot be installed, appropriately sized insulated 25 kV  
20 cables with appropriate sealing ends shall be substituted and spliced into the bare conductor,  
21 which may or may not have to be terminated on a dead-end anchor pole.

### 21.9.2 Static (Ground) Wire

22 In general, the static wire shall be a bare stranded 4/0 ACSR "Penguin" for use throughout the  
23 system. Two static wires are to be installed, 1 on each side of the Authority's right-of-way,  
24 interconnecting all metallic OCS support structures, including OCS poles and bridge and tunnel  
25 drop pipes and wall brackets, to provide a continuous ground connection.

### 21.9.3 Insulated 25 kV Cable

26 Power feeder cables, where used, shall be insulated with a black, low-smoke, flame-retardant,  
27 ozone-resistant, ethylene-propylene compound jacket and the conductor shall be coated, soft-  
28 drawn stranded copper, covered with a double-wrapped separator tape or extruded semi-  
29 conducting ethylene propylene rubber (EPR) screen. Cables shall be sized to suit the identified  
30 ampacity requirements and installation location conditions.

#### 21.9.4 Insulated Return Cable

Return cables, where used, shall be insulated with a black, low-smoke, flame-retardant, ozone-resistant, ethylene-propylene compound jacket and the conductor shall be coated, soft-drawn stranded copper, covered with a double-wrapped separator tape or extruded semi-conducting EPR screen. Cables shall be sized to suit the identified ampacity requirements and installation location conditions.

### 21.10 Dynamic Behavior and Quality of Current Collection

Good quality interactive dynamic performance with minimum wear can be assured by consideration of the quality of current collection, which has a fundamental impact on the life of the contact wire and pantograph components. Compliance with several measurable parameters, as detailed below, shall be achieved.

#### 21.10.1 Requirements

The number of pantographs in service per train and the spacing between multiple pantographs is necessary to confirm the OCS phase break design arrangement. These factors have a significant impact on the quality of current collection, since each pantograph interacts dynamically through the OCS with the performance of other pantographs. This interaction is also affected by the wave propagation speed. The overhead contact line shall be designed for operation at the maximum line speed with 2 adjacent operating pantographs spaced at 656 feet apart, as indicated in Section 21.12.2. The 656 feet spacing shall be used in the OCS dynamic simulations, which shall be considered to be the conformity assessments for verifying compliance with the requirements for dynamic behavior and quality of current collection, as shown in Table 21-3.

It is possible that more than 1 type of pantograph and current collector head may be supplied, particularly if rolling stock is procured from more than 1 supplier. In all cases, the pantograph shall be of proven design for very high speed performance and shall be equipped with a fail-safe device that will detect any failures of the contact strips or collector head, which will trigger automatic lowering of the pantograph. The pantograph shall also be equipped with an uplift limiting device (pantograph stop) and with insulated horns. It is recommended that a carbon-based material be selected for the collector strips to minimize wear of the contact wire. The supplier shall be required to demonstrate the compatibility of the collector strip material with the contact wire.

To achieve good quality current collection, loss of contact between the pantograph strip and the contact wire shall be minimized, since loss of contact can generate electric arcs, which will cause rapid wear of both the contact wire and the pantograph head and collector strips, and may result in the creation of radio frequency interference and in the tripping of feeder circuit breakers (in the event of large arcs with excessive current draws for protracted durations due to significant contact loss). As indicated in EN 50367: 2006 Table 6, for sections dedicated to very



high speed, the on-site measured arc percentage (NQ) shall be  $\leq 0.2$  percent at maximum line speed of 220 mph. For any given vehicle speed, the minimum arc duration that is to be considered shall be 5 milliseconds (ms); the arcing percentage characteristic (NQ), which is also known as the contact loss percentage, is given in percent by the EN 50367: 2006 Clause 3.16 formula:

$$NQ = \frac{\sum t_{\text{arc}}}{t_{\text{total}}} \times 100$$

Where:

$t_{\text{arc}}$  is the duration of an arc lasting longer 5 ms;

$t_{\text{total}}$  is the measuring time with a current greater than 30 percent of the nominal current.

The goal of assessing the interactive dynamic behavior and its impacts on current collection is to ensure there is a continuous and uninterrupted power supply to the electric vehicles with minimal disturbances.

At the design stage, the quality of the OCS-pantograph current collection shall be assessed at the maximum operational speeds for all proposed combinations of rolling stock and pantograph by means of computerized dynamic simulation models. The output from these simulations shall provide determinations of the dynamic effects on the OCS, including values of the simulated contact forces, mean contact force ( $F_m$ ), standard deviation ( $\sigma$ ), statistical value  $F_m - 3\sigma$ , contact loss percentage (NQ), and vertical movement of the contact point (contact wire uplift -  $S_0$ ). The permissible allowances for these factors at maximum line speed are detailed in Table 21-3.

**Table 21-3: Requirements for Dynamic Behavior and Current Collection Quality**

Operating Requirement	> 125 mph
Allowance for Steady Arm Uplift	$2 S_0$
Mean Contact Force $F_m$ (N)	See target curve below
Standard Deviation $\sigma_{\text{max}}$ (N)	$\leq 0.3 F_m$
Percentage of Arcing - NQ (minimum duration of arc 5 ms)	$\leq 0.2$ percent

### 21.10.2 Contact Wire Wave Propagation Speed

The speed of wave propagation in contact wires is a characteristic parameter that is used to assess the suitability of an overhead contact line for high-speed operation. This parameter depends upon the specific mass and the tensile stress in the contact wire.

Based on the recommendations of Clause 4.2.12 of the Energy TSI, the maximum operational line speed shall be not greater than 70 percent of the wave propagation speed. Therefore, the wave propagation speed shall be greater than 314 mph for the 220 mph maximum operating speed.

### 21.10.3 Static Contact Force

The pantograph static contact force is the mean vertical force exerted upward on the contact wire by the pantograph collector head, and is caused by the pantograph raising mechanism when the pantograph is raised and the vehicle is at a standstill.

The pantograph static force shall be adjustable between 9 and 27 lbf (40 and 120 N). The nominal static force shall be 15.75 (+4.5/-2.25) lbf (70 +20/-10 N), and the OCS shall be designed to suit this permissible range in the static contact force from 13.5 to 20.2 lbf (60 to 90 N). Only pantographs designed and proven for very high speed performance shall be permitted.

### 21.10.4 Mean Contact Force

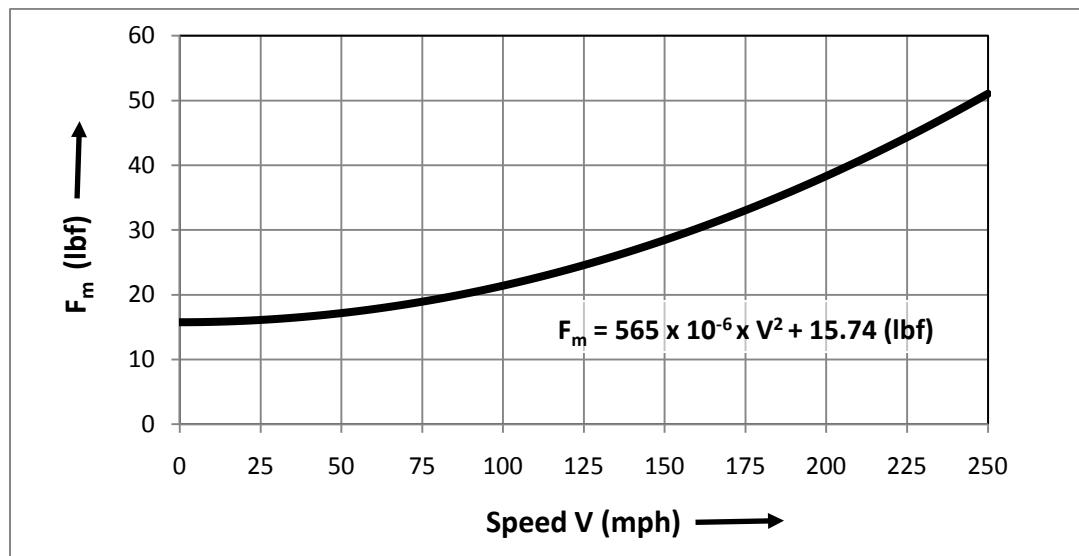
The mean contact force is the dynamically corrected statistical mean value of the forces due to static and aerodynamic effects, which depend on the design of the pantograph and the nature of the current collector strips on the pantograph head. It is equal to the sum of the static contact force and the aerodynamic force, which is caused by airflow on the pantograph elements at the considered speed. The mean uplift force is a characteristic of the given rolling stock/pantograph combination. In this context,  $F_m$  represents a target value that should be achieved to ensure current collection without undue arcing, but which should not be exceeded to limit wear and hazards to the contact wire and the current collection strips.

The design of the overhead contact line equipment must allow for the maximum and minimum contact forces that occur between the pantograph and the contact wire, at the maximum permissible speed of the vehicle, while taking into account the aerodynamic effects. The minimum contact force shall always be positive to ensure no loss of contact between the pantograph and the overhead contact line. Force values vary with different combinations of rolling stock/pantograph and OCS.

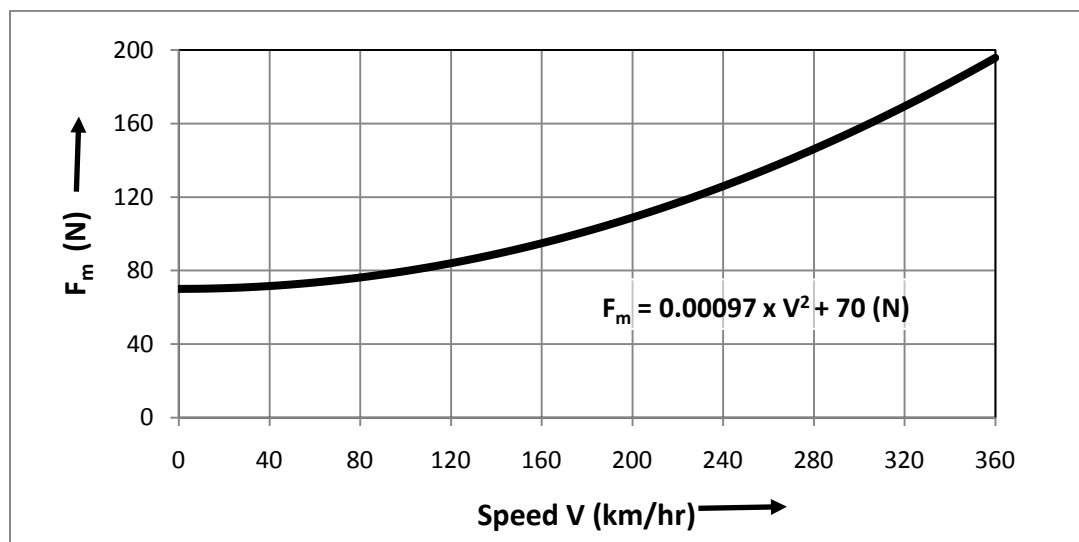
The overhead contact line shall be designed to be capable of sustaining this level of force for all pantographs on a train.

In the case of trains with multiple pantographs simultaneously in operation, the mean contact force  $F_m$  for any pantograph shall be not higher than the target value, since the current collection criteria shall be met by each individual pantograph. The target value for the mean contact force  $F_m$  as a function of the running speed for ac systems is depicted in Figure 21-2 and Figure 21-3.

**Figure 21-2: Target  $F_m$  Values (lbf-mph)**



**Figure 21-3: Target  $F_m$  Values (N-km/hr)**



The maximum contact force ( $F_{max}$ ) is normally within the range of  $F_m + 3\sigma$  (3 standard deviations) for level grade open route sections. Higher values may occur elsewhere, but shall not exceed 79 pounds (350 N) at speeds greater than 125 mph per EN 50119: 2001 Table 1.

The minimum permissible contact force is the force value at which loss of contact between the pantograph and contact wire is probable, and is represented by the statistical value  $F_m - 3\sigma$ , which is a measure that permits the assessment of the consistency of contact between the pantograph and the OCS. The value  $F_m - 3\sigma$  must be positive to avoid contact loss.

### 21.10.5 Contact Wire Uplift or Vertical Movement of the Contact Point

The contact point is the point of mechanical contact between the pantograph contact strip and the contact wire.

The vertical height of the contact point above the track shall be as uniform as possible along the span length; this is essential for high-quality current collection. The maximum difference between the highest and the lowest dynamic contact point height within 1 span shall be less than 3.15 inches (80 mm) at the maximum operating speed of 220 mph. This value has been derived from the Energy TSI Clause 4.2.17 and Table 4.2.17 for Category I lines (as defined in the Energy TSI Clause 1.1).

During the design phase, the projected contact wire uplift (variation in dynamic contact point height) shall be verified by simulations in accordance with EN 50318: 2002. Uplift values shall be presented as a graph of the contact point vertical position against distance in the span to evaluate the extent of the vertical movement:

- For the maximum line speed of the overhead contact line
- For the longest span length
- Using the mean contact force  $F_m$  (as detailed above)

After installation, uplift shall be validated by measurements in accordance with EN 50317:2002.

The variation in contact point height need not be verified for uninsulated or insulated overlap spans or for spans above track turnouts or crossovers.

In order to maximize safety under all operating conditions (including strong wind conditions and slight mis-adjustments of the pantographs), the dynamic pantograph envelope at the maximum operating speed shall consider twice the value of the estimated or simulated uplift  $S_0$  at the support point. The design of the OCS cantilever and registration shall allow the uplifted steady arm to clear the dynamic pantograph envelope. For initial design purposes, a minimum uplift of 10 inches shall be assumed.

Uplift values shall be confirmed by simulation results (as indicated below) and, if multiple pantograph-train consists are furnished, the designs shall accommodate, as a minimum, the greatest simulated uplift values.

### 21.10.6 Conformity Assessment

Conformance with the above criteria shall be confirmed by the OCS supplier by means of dynamic interactive OCS-pantograph simulations and through equivalent records of on-site testing results for speeds above 220 mph. Notwithstanding the above, the OCS shall be a proven system capable of sustaining satisfactory current collection for train operations at 220 mph. The simulation program shall meet the validation requirements detailed in EN 50317: 2002 and EN 50318: 2002.

1 The final designs and specifications shall require that measurements (in accordance with EN  
2 50317: 2002) of the interaction between the pantograph and the OCS shall be performed on the  
3 high-speed line during the testing and commissioning phase to check for correct installation  
4 and to prove the safety and the quality of the current collection system. These measurements  
5 shall be carried out with an approved pantograph, exhibiting the mean contact force  
6 characteristics for the envisaged design speed, installed on approved rolling stock. The installed  
7 overhead contact line shall be accepted if the measurement results comply with the  
8 requirements stipulated in Table 21-3.

9 To check the performance capability of the current collection system, the following data, as a  
10 minimum, shall be measured:

- 11 • The contact force
- 12 • The contact wire uplift at the support as the pantograph passes
- 13 • The percentage of arcing and duration of arcs longer than 5 ms

14 In addition to the measured values, the operating conditions (train speed, location, etc.) shall be  
15 recorded continuously, and the environmental conditions (rain, temperature, wind, tunnel, etc.)  
16 and details of the test configuration (parameters and arrangement of pantographs, type of OCS,  
17 etc.) during the measurement tests shall be recorded in the test report.

18 When or if changes from the accepted and approved equipment are proposed, such as the use  
19 of a pantograph of proven design that is to be installed on a new type of rolling stock, or a new  
20 OCS design for additions to, or substitution of, existing sections of the system, or a new  
21 pantograph design that is to be installed on the approved rolling stock, conformity assessment  
22 testing shall be carried out in accordance with EN 50317: 2002 and/or EN 50206-1: 1999, with  
23 particular emphasis on the mean contact force and loss of contact requirements. If the tests are  
24 passed successfully, the new OCS design, or the specific proposed pantograph/rolling stock  
25 combination, will be approved for use on the high-speed line.

## **21.11 Current Capacity of Overhead Contact System**

26 As a minimum, the current-carrying capacity of the OCS shall comply with the current-draw  
27 requirements specified for the trains.

28 The OCS, including parallel feeders, return circuit conductors and feeder connections, shall be  
29 designed to cater to the electrical current loading under steady state peak period operating and  
30 fault conditions, as defined by the system design, under the environmental and climatic  
31 conditions defined in Section 21.5, with reference to the advisories contained in Annex A to EN  
32 50119: 2001. In addition, the current heating effects of short circuit faults and durations resulting  
33 from automated circuit breaker closure sequences (if adopted) occurring during peak  
34 operations shall be assessed.

- 1 The maximum temperature rise in the conductors caused by the load currents shall not lead to  
2 conductor temperatures to the point at which the mechanical properties are impaired. The  
3 maximum permissible temperatures for bare conductors are shown in Table 21-4.

**Table 21-4: Maximum Permissible Bare Conductor Temperatures**

Conductor Material	Max. Temperature (°F)	Max. Temperature (°C)
Normal and high strength, high conductivity Copper	176 °F	80 °C
Silver Copper alloys	212 °F	100 °C
Cadmium Copper alloys	176 °F	80 °C
ACSR	212 °F	100 °C

- 4 The melting point of any grease used in the strands of the conductors shall be higher than the  
5 temperature limits specified above.
- 6 The Designer shall undertake a design study to confirm the OCS complies with the specified  
7 requirements. Conformity assessment shall be carried out by design review.

## 21.12 Sectionalizing and Switching, and Pole-Mounted Equipment

- 8 To allow bi-directional working, enabling trains to continue operation under emergency  
9 conditions and to facilitate routine OCS maintenance, the OCS shall be divided into electrical  
10 sections and sub-sections. On the main tracks, only phase breaks (utilizing insulated overlaps)  
11 and insulated overlaps shall be used for power supply sectionalizing purposes. Mechanical  
12 section insulators will not be permitted except when used in the OCS above slow speed track  
13 turnouts and in the yard and shop areas.
- 14 To form the insulated overlaps, insulation shall be cut into the out-of-running sections of the  
15 messenger wire and contact wires of the 2 overlapping catenaries, having between them a  
16 limited air gap electrical clearance. The insulated overlap thus provides a sectionalizing point in  
17 the OCS as required for operational and maintenance reasons, but allows pantographs to  
18 transition smoothly from 1 energized electrical sub-section to the next under power.

### 21.12.1 Pantograph Spacing for Design of the Overhead Contact Line

- 19 The overhead contact line design shall be based on rolling stock operating with 2 raised  
20 pantographs spaced at 656 feet apart for dynamic simulation assessments

### 21.12.2 Phase Breaks

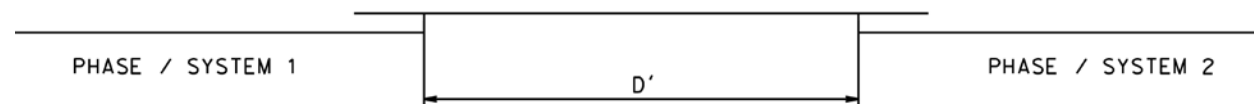
The design of the OCS phase breaks shall permit approved trains to move at all speeds up to the designated maximum operating speeds from 1 electrical section to an adjacent electrical section without bridging between 2 electrical phases or 2 separate utility supply systems.

Trains shall traverse the entire phase break with pantographs raised and in contact with overhead contact wire, but with the pantograph breakers open. The main circuit breaker onboard each power unit shall be opened automatically by an input from the ATC system before the train enters a phase break, and shall be similarly closed automatically after the train clears the phase break section in the OCS. Adequate means shall be provided to allow a train that is stopped within the phase break neutral section to be restarted and bidirectional movements shall be supported. The neutral section shall be configured such that it can be connected to, and energized from, either of the adjacent electrical sections by remotely controlled disconnect switches with the provision of appropriate interlocks to ensure the different phases cannot be inter-connected under any circumstances.

The geometry of the phase break elements shall prevent pantographs short-circuiting or bridging between power systems. Provision shall be made in the rolling stock design to avoid bridging of adjacent power supply systems should opening of the onboard circuit breaker(s) fail. Train consists with multiple pantographs shall have no electrical connection between the pantographs that are in service.

For high speed sections over 65 mph, the Long Phase Break (Figure 21-4) has been selected. The long phase break design will allow all pantographs of the longest compliant trains to lie within the neutral section. The length of the neutral section shall be at least 1,320 feet.

**Figure 21-4: The Long Phase Break**



Conditions:  $D' > 1,320$  feet

### 21.12.3 OCS Sectionalizing in Tunnels

The Designer shall coordinate the sectioning of the power supply system in each tunnel with the pertinent agency that will be responsible for development of the Tunnel Emergency Evacuation Plan. The sectioning shall be designed to support the overall strategy for evacuation from the tunnel.

For tunnels that are greater than 3 miles in length, in which the signaling system permits the simultaneous presence of more than 1 train on each track, the OCS shall be divided into sections that do not exceed 16,500 feet (as indicated in Section 4.2.3.1 of the Safety in Railway Tunnels TSI).

Grounding devices shall be provided at tunnel portals and at tunnel access points, and close to the separation points between electrical sections. These shall be in the form of 3-position disconnect switches, providing for Closed (inter-connection between adjacent electrical sections), Open (no electrical inter-connection), and Closed to Ground.

The disconnect switches shall be motorized and shall be both remotely and locally controlled fixed installations. Switch indication panels shall be provided at or adjacent to each switching location, indicating for the benefit of emergency response personnel the status of each switch and whether the OCS is energized / de-energized / grounded.

Procedures and responsibilities for grounding the OCS in tunnels by the Power Director or Power Dispatcher shall be defined in the emergency plan.

#### 21.12.4 Disconnect Switches

To facilitate maintenance work and emergency operations, the OCS shall be equipped with disconnect switches at all primary feeding and by-pass feeding locations.

Where feasible, the OCS disconnect switches shall be pole-mounted at trackside and shall be single pole motorized switches capable of remote operation and also of local motorized or manual operation. The switches shall provide for isolation of discrete sections of the OCS (track), such that segments of the OCS can be de-energized for maintenance purposes. The disconnect switches shall also provide for by-pass feeding arrangements that can be implemented during emergency conditions to permit contingency modes of operation. Remote operation shall be performed from the Operations Control Center (OCC) and shall be accomplished using the SCADA system. OCS disconnect switches shall be of the no-load-break type and shall be rated for the system voltage and anticipated current loads, and shall be designed to carry the worst-case overload and short circuit currents without overheating. As a safety precaution, the switch operating mechanism shall be fitted with a locking bar that will permit the attachment of maintainer locks.

In general, the disconnect switches shall be of the 2-position type, providing for Closed (inter-connection between adjacent electrical sections), or Open (no electrical inter-connection). For locations where solid grounding of the OCS is required, the OCS disconnect switches shall be of the 3-position type, providing for Closed, Open, and Closed to Ground connections.

Where motorized disconnect switches are located in the vicinity of a traction power facility, the 125 V dc motor power shall be supplied from that facility. At remote locations, such as interlockings, the 125 V dc motor power shall be supplied from a wayside power control cubicle (WPC).

#### 21.12.5 Auxiliary Step-down Transformers

To provide power to remote wayside power control cubicles (WPC), the traction power negative feeder can be tapped. At selected locations, an auxiliary step-down transformer will be



1 mounted on the back (field-side) of an OCS pole on both sides of the right-of-way to step down  
2 the 25 V negative feeder power to 480 V ac to provide redundant feeds to the WPC. The feeder  
3 tap shall be equipped with a lightning arrester, and a drop-out fuse assembly to facilitate  
4 maintenance work on the transformer.

#### 21.12.6 Protection against Lightning Strikes

5 The OCS Designer shall investigate the incidence of lightning storms on a project section-by-  
6 section basis and shall determine appropriate lightning protection measures, based upon the  
7 incidence of lightning strikes in each area. Refer to the *Grounding and Bonding Requirements*  
8 chapter. Regardless, all cabled connections to the OCS and Negative Feeders shall be equipped  
9 with lightning arresters, preferably installed at disconnect switch locations. In addition,  
10 between TP facilities, the OCS and Negative Feeders shall be equipped with lightning arresters  
11 at a spacing not to exceed 2 miles.

#### 21.13 Insulation Coordination Requirements for OCS Installations

12 Insulation requirements for railroad electrification systems are covered by EN 50124-1: 2001  
13 Railway Applications – Insulation Coordination – Part 1: Basic Requirements. Insulation  
14 coordination implies selection of the electrical insulation characteristic of the equipment with  
15 regard to its application and in relation to its surroundings. Insulation coordination can only be  
16 achieved if the design of the equipment is based on the stresses to which it is likely to be  
17 subjected during its anticipated lifetime. In accordance with EN 50124-1: 2001 Clause 2.2.2.1, the  
18 OCS falls into either the OV3 or OV4 overvoltage categories for circuits that are powered by or  
19 from the overhead contact line, which are not protected against external or internal  
20 overvoltages, and which may be endangered by lightning or switching overvoltages. For a  
21 25 kV OCS, the rated insulation voltage is given as 27.5 kV in Table D.1, and for fixed  
22 installations, the rated impulse voltage is given as either 170 kV or 200 kV for the OV3 and OV4  
23 overvoltage categories respectively. The Designer shall determine the category applicable to the  
24 OCS and shall furnish justification for the selection. Pollution categories and associated  
25 creepage distances are also covered in EN 50124-1; insulation for the system shall be designed  
26 accordingly.

#### 21.14 OCS Clearances and Protection against Electric Shock

27 Protection against electric shock can be achieved by establishing adequate safety clearances that  
28 minimize the possibility of direct contact by persons with energized parts, and/or by erecting  
29 suitable barriers or screens to prevent direct contact, and installing appropriate signs warning of  
30 the potential dangers, and by implementing comprehensive project-wide bonding and  
31 grounding systems. Refer to the *Grounding and Bonding Requirements* chapter of these criteria.

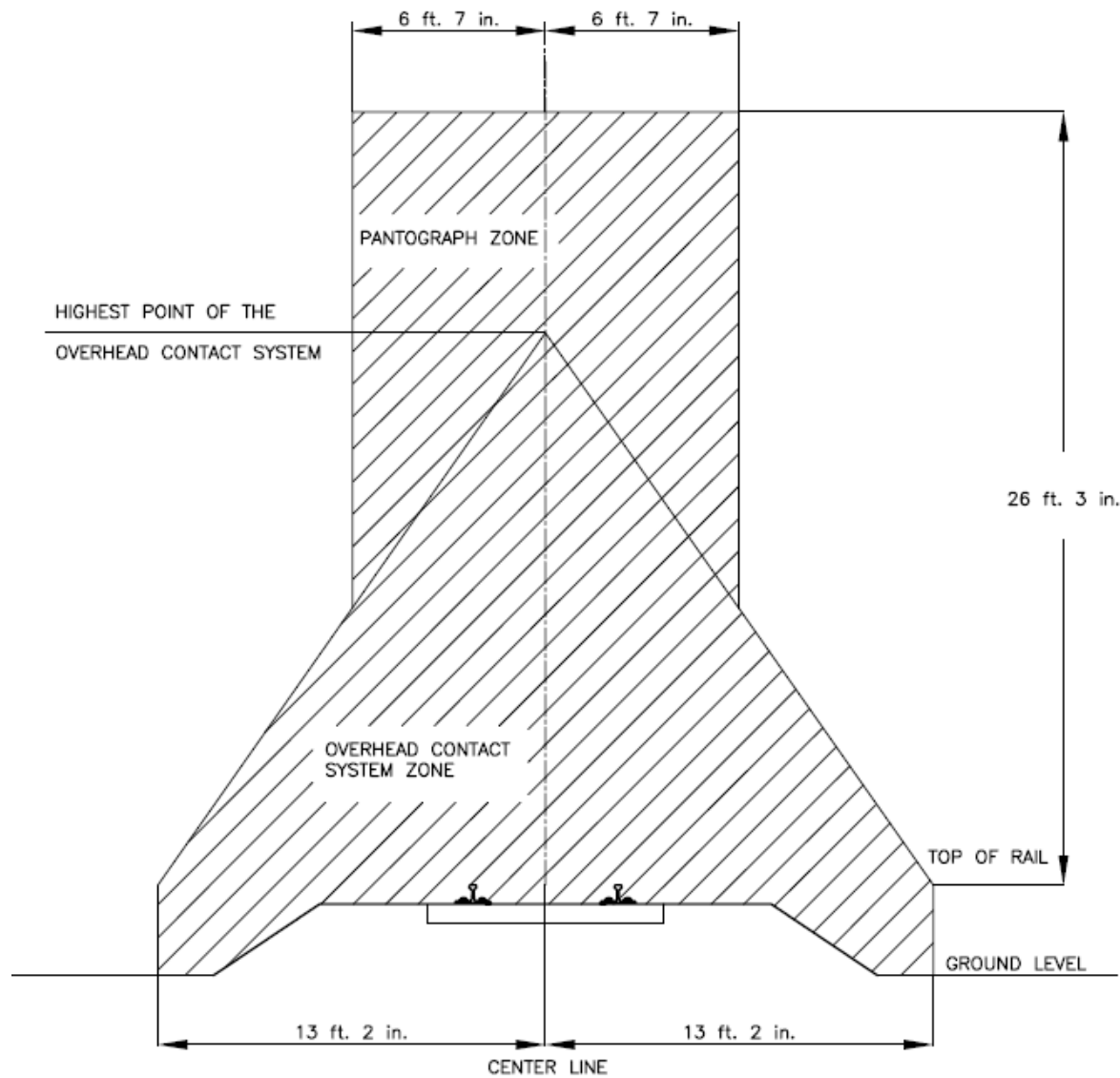
#### 21.14.1 Overhead Contact Line Zone and Pantograph Zone

1 Structures and equipment may accidentally come into contact with a live broken contact line, or  
2 with the live parts of a broken or de-wired pantograph or energized fragments. Figure 21-5 has  
3 been derived from EN 50122-1: 1997 Figure 1 and defines the zone inside which such contact is  
4 considered probable but which limits are unlikely to be exceeded by a broken overhead contact  
5 line or damaged energized pantograph, or energized fragments thereof.

6 Note that the damaged pantograph may be live, even though it is not in contact with the  
7 overhead line, because it is inter-connected with other energized pantographs or because the  
8 train is in regenerative braking mode.

9 The limits of the overhead contact line zone below top of rail extend vertically down to the  
10 earth surface, except where the tracks are located on an aerial structure where they extend  
11 down to the deck of the aerial structure. In the case of out of running OCS conductors, the  
12 overhead contact line zone shall be extended accordingly.

1 **Figure 21-5: Overhead Contact Line Zone and Pantograph Zone**



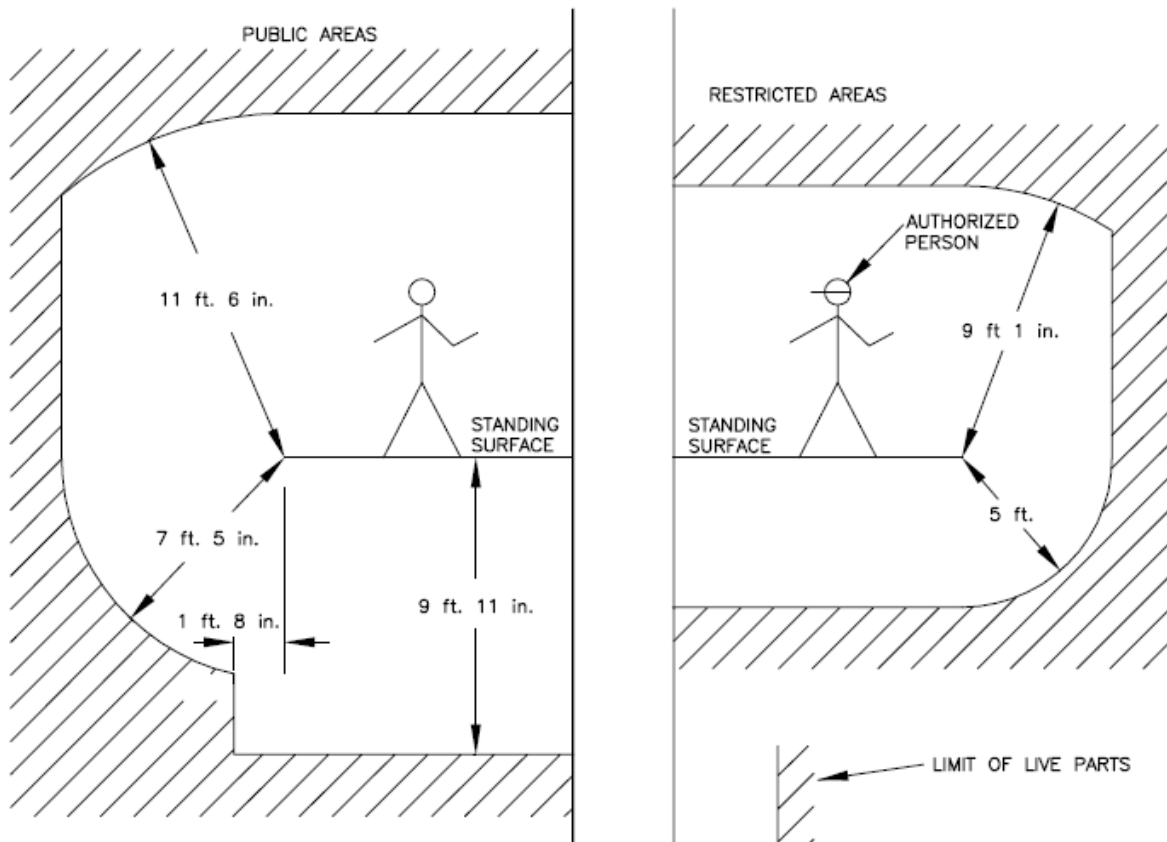
Source: Derived from EN 50122-1: 1997 Figure 1

## 21.14.2 Protection by Clearances from Standing Surfaces

The minimum unconstrained clearances from energized parts to generally accessible areas (no barriers, screens or other physical restrictions to movement) for 25 kV systems have been derived from EN 50122-1: 1998 Figure 14 and values are depicted in Figure 21-6 for both public areas and restricted areas. The values shown are based on touching in a straight line without the use of tools or other objects, and shall be achieved under all climatic and loading conditions. These requirements apply to clearances from standing surfaces used by people to accessible live parts on the outside of vehicles as well as to live parts of the OCS.

- 1 Placing energized parts over walkways shall be avoided wherever practical.
- 2 Safe working clearances and approach distances for qualified employees shall be developed by
- 3 the applicable agency for inclusion in the Safety Manual and in the appropriate work practices
- 4 and procedures documents.

**Figure 21-6: Minimum Required Safety Clearances – Unconstrained Access**



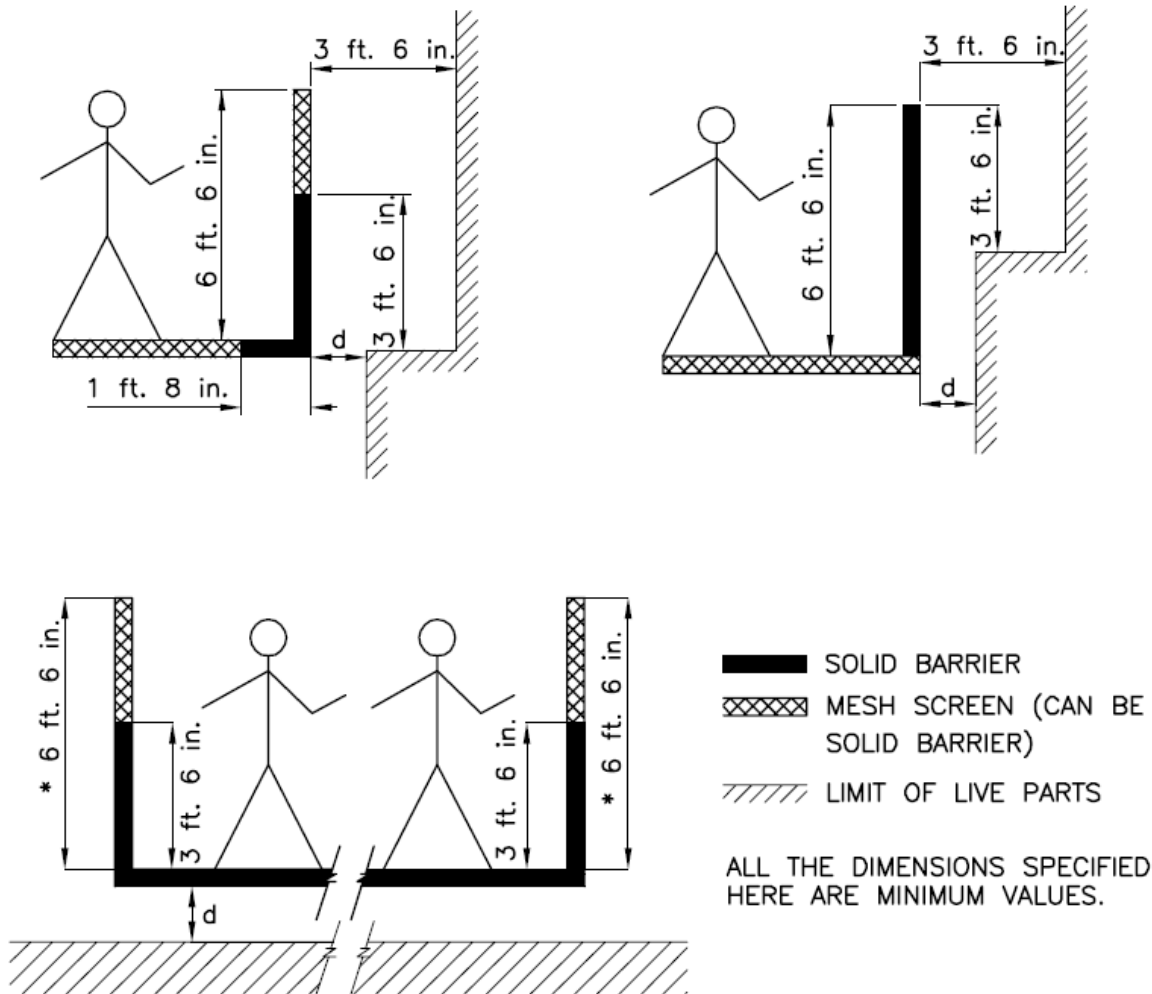
### 21.14.3 Protective Screening and Barriers for Standing Surfaces in Public Areas

The following criteria address OCS requirements only. For CHSTP fencing requirements, refer to the *Civil* chapter. The requirements for protective screening and barriers for use in public areas for protection against direct contact with adjacent live parts on the outside of vehicles or adjacent live parts of an overhead contact line system for normal voltages up to 25 kV ac to ground, where clearances are less than those shown in Figure 21-6, have been derived from EN 50122-1: 1997 Figure 18 (herein depicted on Figure 21-7) and are summarized as follows:

- Where the energized parts are located below the standing surface, protection of the standing surface shall be by means of a solid barrier.

- 1 • The minimum height of the protective barrier (solid barrier or a combination of solid barrier  
2 plus mesh screen, as shown) shall be not less than 6 feet-6 inches.
- 3 • Protective barriers of greater height may be required in areas where vandalism is prevalent.
- 4 • The value of Dimension “d” between the protective screen or barrier and live parts shall be  
5 determined from Table 21-5. Where mesh screens are used, 4 inches shall be added to the  
6 value of Dimension “d” and where buckling or warping of solid barriers is likely,  
7 1.25 inches shall be added, in accordance with EN 50122-1 Clause 5.1.3.1.2.
- 8 • The length of the protective screening and/or barrier on structures that cross over the  
9 electrified railroad, which protect publicly accessible standing surfaces, shall be extended  
10 laterally 30 feet from the centerline of the outermost track (refer to Civil Standard and  
11 Directive Drawings) or a minimum of 10 feet beyond the outermost live parts of the  
12 overhead contact line (conductor or component). In the case of energized conductors not  
13 being used for current collection (e.g., line feeders, reinforcing feeders, out of running  
14 overhead contact lines), the barrier shall extend for a width of at least 10 feet on each side of  
15 the conductor, with the proviso that movements due to dynamic and thermal effects shall be  
16 taken into account.

**Figure 21-7: Clearances from Protective Screens and Barriers for Standing Surfaces in Public Areas**



Note: \* for the height of Access Restriction (AR) fencing along public access areas, refer to the *Civil* chapter.

Source: Derived from EN 50122-1: 1997 Figure 18

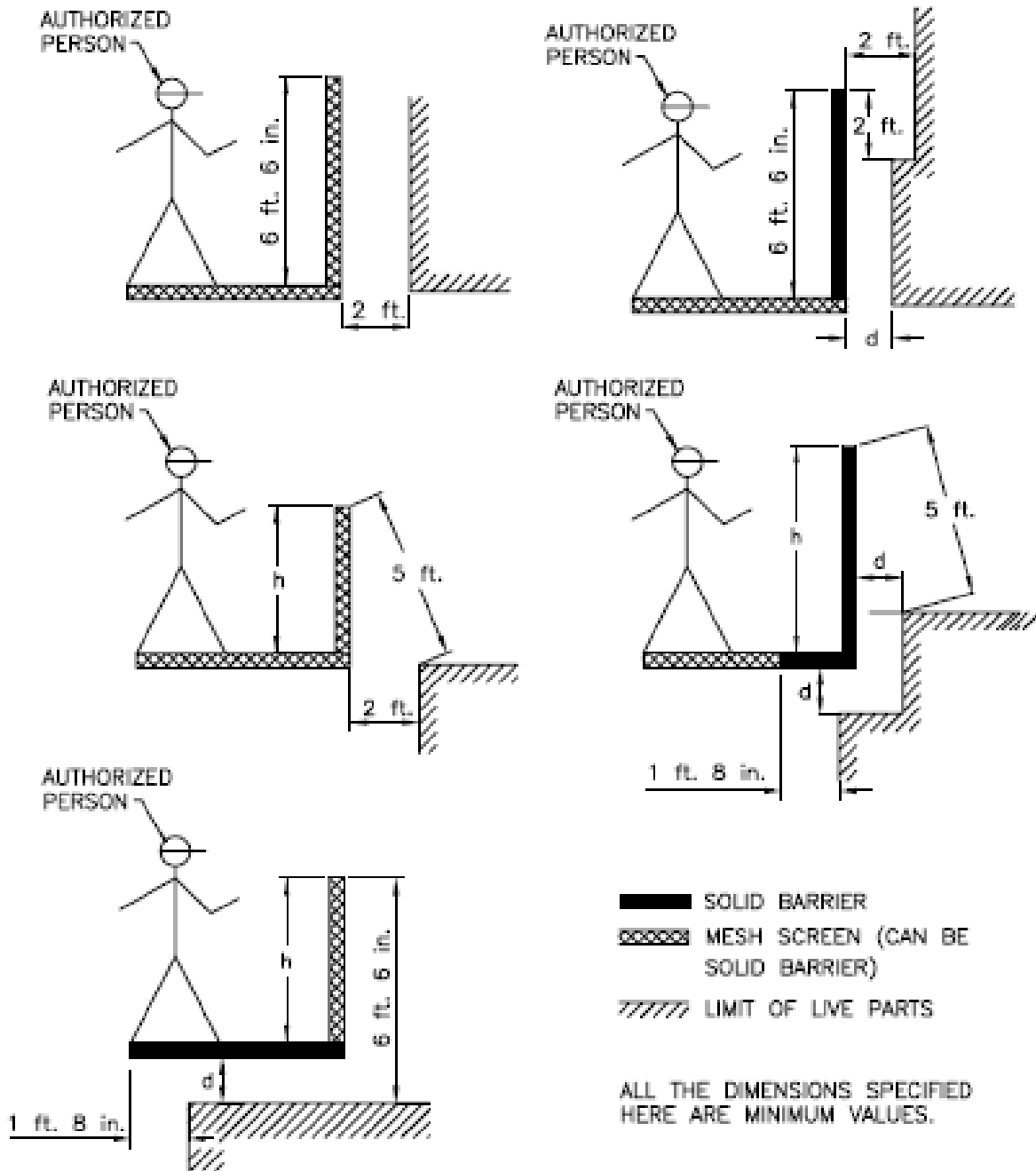
#### 21.14.4 Protective Screening and Barriers for Standing Surfaces in Restricted Areas

The requirements for clearances from protective screening and barriers for standing surfaces in restricted areas for protection against direct contact with adjacent live parts on the outside of vehicles or adjacent live parts of an overhead contact line system for normal voltages up to 25 kV ac to ground, where clearances are less than those shown in Figure 21-6, have been

1 derived from EN 50122-1: 1997 Figure 16 and 17, and are shown in Figure 21-8 and are  
2 summarized as follows:

- 3 • For standing surfaces above live parts on the outside of vehicles or above live parts of an  
4 overhead contact line system, the protection shall be of solid barrier construction.
- 5 • The length of the solid barrier, protecting the standing surface, shall correspond to the  
6 pantograph zone and shall extend beyond the live parts of an overhead contact line by at  
7 least 1 foot 8 inches. In the case of energized conductors not being used for current  
8 collection (e.g., line feeders, reinforcing feeders, out of running overhead contact lines), the  
9 barrier shall extend for a width of at least 1 foot 8 inches on each side of the conductor, with  
10 the proviso that movements due to dynamic and thermal effects shall be taken into account.
- 11 • The height “h” of the protective screening and barrier shall be such that a clearance of 5 feet  
12 from the top of the protective screening and barrier shall be maintained (refer to Figure  
13 21-8).
- 14 • The height of the side protective screenings and barriers shall correspond to the height of  
15 the required safety railing but should be a minimum of 3 feet 6 inches.
- 16 • The value of Dimension “d” between the protective screen or barrier and live parts shall be  
17 determined from Table 21-5. Where mesh screens are used, 4 inches shall be added to the  
18 value of Dimension “d” and where buckling or warping of solid barriers is likely,  
19 1.25 inches (30 mm) shall be added, in accordance with EN 50122-1: 1998 Clause 5.1.3.1.2.

**Figure 21-8: Clearances from Protective Screenings and Barriers for Standing Surfaces in Restricted Areas**



Source: EN 50122-1 Figure 16 and 17



**21.14.5 Additional Requirements for Protection Barriers and Screens**

1 Protection barriers or screens shall be of sufficient strength and shall be supported rigidly and  
2 securely enough to prevent them from being displaced or dangerously deflected by a person  
3 slipping or falling against them.

4 Barriers and screens shall be permanently fixed, and shall be removable only with tools.  
5 Barriers in public areas shall employ non-removable, captive fasteners.

6 Barriers shall be of solid construction and fabricated from either conductive or non-conductive  
7 materials.

8 • Non-conductive barriers shall be surrounded by a grounded, bare conductor that is inter-  
9 connected with the traction system ground, preferably at not less than 2 locations.

10 • Conductive barriers shall be bonded and grounded by inter-connection with the traction  
11 system ground, preferably at not less than 2 locations.

12 Screens shall be of grounded, conductive, open mesh materials, and shall be grounded by inter-  
13 connection with the traction system ground, preferably at not less than 2 locations. Non-  
14 conductive mesh or plastic-coated metal mesh shall NOT be used.

15 Conductive mesh screens shall be constructed such that a cylinder, greater than 1/2 inch in  
16 diameter, cannot be pushed through the mesh. Mesh screen construction shall be such that  
17 required clearances to energized parts are maintained.

18 The style of barrier to be employed is dependent upon the type of standing surface and its  
19 proximity to the energized parts, and whether the surface provides for public or restricted  
20 access, as detailed above.

21 The size of the barrier or screen shall be such that energized parts cannot be touched in a  
22 straight line by persons on a standing surface.

23 The design of the protective screens and barriers shall minimize the loading on the existing  
24 structures and the adverse visual impact.

25 The metallic parts of overhead bridge screens and barriers shall be bonded to the static wires.  
26 Other metallic items under overhead bridges, within a lateral distance of 10 feet of any  
27 energized and uninsulated equipment below the structure, shall be directly or indirectly  
28 bonded to the static wires.

**21.14.6 Protection Against Climbing**

29 Where there is public access or trespass is likely, anti-climbing protection shall be provided at  
30 buildings and other structures supporting energized parts of the OCS. The anti-climbing  
31 protection shall include signs warning of the dangers of high voltage.

Access to fixed ladders, particularly at signal poles and signal gantries, and the means of access to any roof or other place, which could allow non-authorized persons to approach energized parts, shall be secured or otherwise protected.

#### 21.14.7 Warning Signs

Permanent High Voltage Warning signs, as detailed in the *Civil* chapter, shall be installed as follows:

- Displayed in conspicuous places at all entrances to locations containing exposed current carrying parts
- Located on all enclosures that provide access to conductors, equipment, and apparatus that are energized at high voltage
- Displayed at all anti-climbing locations

The warning signs shall be posted in a consistent manner throughout the electrified route and shall be clearly visible to persons on or near the electrified lines.

#### 21.14.8 Clearances for Utility Lines Crossing over the Electrified Railroad

The minimum clearance for overhead power, communications or other utility lines, which are not part of the Traction Electrification System (TES), shall be in accordance with CPUC General Order No. 95 Rule 38 Table 2 and shall be measured from the highest energized point on the TES.

For any crossing of the high-speed lines, the utility shall comply with the requirements of CPUC General Order No. 95 with regard to the conductor suspension arrangements and strength of the structures immediately adjacent to the crossing point.

For other utility requirements, refer to the *Utilities* chapter.

#### 21.14.9 Electrical Clearances to Rail Vehicles and Structures

Clearances are classified as either Static or Passing.

Static Clearance is the physical air clearance between energized parts of a vehicle or OCS when not subjected to dynamic conditions or climatic influences or pantograph pressure, and an adjacent fixed structure or the grounded parts of a vehicle, while the vehicle is stationary.

Passing (or Dynamic) Clearance occurs under dynamic operating conditions that exist during the passage of a train, or when the OCS is affected by extreme climatic conditions, such as wind and/or ice loading. Passing (or Dynamic) Clearance is the physical air clearance between energized parts of either the vehicle or OCS and the grounded vehicle, or between energized parts of either the vehicle or OCS and an adjacent fixed structure.

Electrical clearances, shown in Table 21-5 and depicted in Figure 21-9, from energized parts to grounded parts of rail vehicles or structures are categorized as Normal and Minimum and are applicable, as noted, in Non-Polluted and Polluted atmospheric locations. Typical polluted conditions are detailed in Section 21.5.4 and the Designer shall determine their applicability. Polluted locations/areas shall be so noted in the designs, so all users are aware that increased clearances must be employed and maintained.

**Table 21-5: 25 kV ac Electrical Clearances**

Atmospheric Condition	Normal Clearance		Minimum Clearance	
	Static (C <sub>A</sub> )	Passing/ Dynamic (P <sub>A</sub> )	Static (C <sub>A</sub> )	Passing/ Dynamic (P <sub>A</sub> )
Non-Polluted	10.5 inches*	8 inches *	8 inches *	6 inches*
Polluted	12.5 inches **	10 inches**	10 inches **	8 inches **

\* These clearance values are as stated in AREMA Table 33-2-4 (2010)

\*\* For polluted atmospheres, 2 inches has been added as stated in AREMA Table 33-2-4 (2010)

The designated normal clearances shall be adopted at all locations, wherever practicable. Where it can be demonstrated that it is not practicable to provide normal clearances, adoption of the minimum clearances shall be permissible. However, prior to their adoption, the following factors shall be further evaluated:

- Fault current resulting from a breakdown of the electrical clearance
- Vulnerability of the OCS and railroad infrastructure to damage should a breakdown of the electrical clearance occur
- Consequences for the safety of persons should a breakdown of the electrical clearance occur
- Application and maintenance of tolerances of the OCS and railroad infrastructure
- Economic and technical considerations

The minimum clearance from bare energized ancillary conductors (the 25 kV negative feeders) to grounded structures under worst case conditions in non-polluted areas shall be 10.5 inches and 12.5 inches in polluted locations, as specified in the AREMA Manual Chapter 33 Table 33-2-2. These values shall be adopted for the project.

In a 2x25 kV ac system, there is a 180 degree phase difference between parts common to the energized negative feeder and parts common to the energized catenary system. The minimum clearance between these elements shall be 21.5 inches under static conditions or 12 inches under worst case dynamic conditions.

Enhanced clearances or other protective measures shall be provided at locations where there is a high probability of incidents due to birds, animals, icicles, or vandalism, or for particularly

vulnerable structures. The maximum practicable value of electrical clearance shall be provided at all locations.

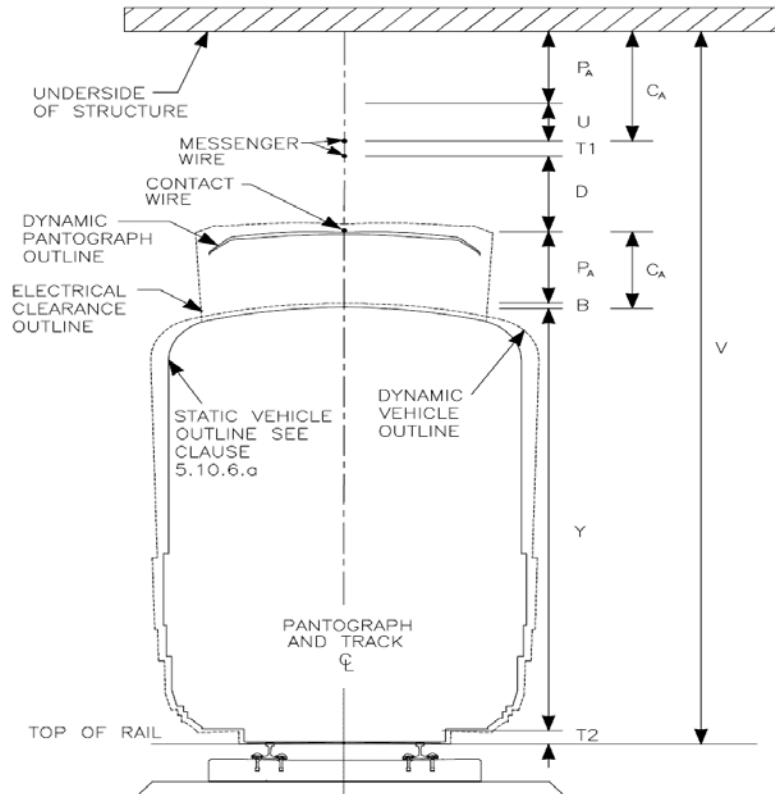
#### 21.14.10 Clearance Envelope at Fixed Structures

In determining the minimum vertical clearance envelope at fixed structures, including OCS support structures and signal bridges, the following factors shall be assessed, as depicted on Figure 21-9:

- The static vehicle outline, which shall be based on the size of the high-speed rail vehicles.
- The dynamic vehicle outline, which shall take into consideration the dynamic swept envelope, track position and maintenance tolerances, including railhead side wear, and the effects of vertical and horizontal curvature, including track super-elevation. Refer to the *Trackway Clearances* chapter for further information regarding vehicle clearance requirements.
- The position of energized parts on the rail vehicles, including the dynamic pantograph envelope, allowing for pantograph carbon wear and dynamic movements and deflections of the pantograph frame, and vehicle construction and maintenance tolerances. The pantograph envelope shall include an allowance for chording effects, if the pantograph is offset longitudinally on the vehicle from a truck centerline.
- The position and size of energized parts of the OCS allowing for installation and maintenance tolerances, uplift and other dynamic movements, including those due to wind, temperature and loading conditions.
- Electrical clearance values as applicable for non-polluted or polluted areas.
- For minimum vertical clearances for new and existing structures, refer to the *Trackway Clearances* chapter and the Standard and Directive Drawings.
- In assessing the minimum vertical clearance of the overhead structure, the vertical clearance between the energized bare negative feeder cable shall also be considered.
- At locations where the bare negative feeder has to be mounted on the back or field side of the OCS poles, lateral clearances to adjacent structures shall take into consideration the position of this pole-mounted equipment and the conductor under worst case wind conditions.
- At phase break locations, positive feeders will be run from the phase break switching gantry to the OCS feeder disconnect locations outside the limits of the phase break. These bare positive feeders will be mounted on the back or field side of the OCS poles, and lateral clearances to adjacent structures shall take into consideration the position of this pole-mounted equipment and the conductor(s) under worst case wind conditions.
- To provide for safe clearances from OCS pole-mounted equipment, no structure that is more than 10 feet high above top of rail shall be constructed within 9 feet from the field-side of the

centerline of the OCS pole. This requirement shall also apply where OCS poles are mounted on top of retaining walls or trench walls. Where OCS poles are mounted inside trench structures or retaining walls, this spatial requirement shall be evaluated on a site specific basis.

1 **Figure 21-9: Vertical Clearance Envelope at Fixed Structures**



2 Source: AREMA Figure 33-2-3 and Figure 33-2-4

- 3  
4  
5  $V$  = Total Vertical Clearance Required for Electrification  
6  $P_A$  = Passing (Dynamic) Electrical Clearance – see Note below  
7  $U$  = OCS Uplift  
8  $T_1$  = OCS Construction Tolerance  
9  $D$  = OCS Depth  
10  $B$  = Vehicle Bounce  
11  $Y$  = Static Vehicle Load Height  
12  $T_2$  = Track Maintenance Tolerance  
13  $C_A$  = Static Electrical Clearance

14 Notes: The diagram depicts the dynamic condition. For static situations, the Static Electrical Clearance ( $C_A$ ) shall be  
15 not less than  $P_A + U$  or  $P_A + B$  - refer to Table 21-5.

16 The minimum lateral clearance at fixed structures, including OCS poles and other OCS support  
17 structures and signal bridges, shall comply with the clearance requirements detailed in the  
18 *Trackway Clearances* chapter.

#### 21.14.11 Applicable Pantograph and OCS Clearance Envelopes

19 In assessing clearances along the alignment to accommodate the OCS and Pantograph, 2  
20 Clearance Envelopes have been developed, which incorporate electrical clearances based on the

polluted area clearances shown in Table 21-5. These envelopes are detailed in the OCS Directive Drawings and shall be adopted.

For non-polluted areas, the diagrams can be reduced in size by incorporating the non-polluted electrical clearance values indicated in Table 21-5.

#### 21.14.12 Paved Areas in Maintenance Facilities, Yards and Workshops

The normal clearance of the lowest energized part of the electrification system above paved areas in maintenance facilities, yards and workshops, is specified in Table 21-6. Warning signs shall be provided, as detailed in the *Civil* chapter.

At pedestrian and vehicle crossings in maintenance facilities or yards where vehicles over 8 feet in height are prohibited, a restricted clearance is permitted, as specified in Table 21-6.

**Table 21-6: Minimum Clearances above Track Crossings in Paved Areas of Maintenance Facilities, Yards and Workshops**

Normal Clearance	Restricted Clearance
20 feet 4 inches	18 feet 4 inches

#### 21.14.13 Clearances to Vegetation

Based on the requirements stipulated in EN 50122-1: 1997 Clause 5.1.2.5, trackside vegetation shall be managed, such that there is no overhanging vegetation and that a minimum clearance of 8 feet-3 inches is maintained between the vegetation and energized parts of the OCS and/or negative feeders at all times and under all climatic conditions.

### 21.15 OCS Structural Requirements

#### 21.15.1 General

In the above ground, at-grade sections, the OCS poles shall be either galvanized H-section wide flange beams or galvanized round, tapered tubular steel sections. In residential areas and at passenger stations, round, tapered tubular steel poles shall be used. All poles shall be of the bolted base type and shall be designed and manufactured to relevant U.S. steel standards.

Where multiple OCS equipments are to be supported in the above ground, at-grade sections, such as at overlaps and turnouts, multiple cantilevers may be attached to a single structure, which shall be of a heavier section such that the applied loads shall not cause twisting of the structure by more than 5 degrees.

For multi-track areas where independent poles cannot be installed between tracks, portal structures with bolted base support poles and with drop tubes to support the OCS equipment

related to individual tracks shall be used, thereby providing for mechanical independence of the individual equipments.

In general, OCS poles in station areas shall be located between tracks. For situations where OCS poles must be located on station platforms, they shall be placed in a manner that minimizes the visual impact and obstruction to passengers, and shall be integrated with platform architecture design. The minimum distance from platform edge to face of poles shall be 7 feet. Counterpoise grounding shall be used within passenger stations and the aerial static wire shall be electrically isolated from the OCS structures and components connected thereto. Refer to the *Grounding and Bonding Requirements* chapter.

Wall brackets and drop pipe supports in tunnels, cut-and-cover box structures and trench structures shall be of galvanized steel, and shall be attached using either C-channels or anchor expansion bolts of the undercut type. Refer to the *Structures* chapter and Standard and Directive Drawings for details. Where bracket installation requires drilling of reinforced concrete, the specifications shall require that specialized equipment be used to locate the reinforcing bars before drilling commences. The minimum distance from a reinforcing bar to a drilled hole shall be 2 inches.

For OCS poles mounted on top of trench walls or retained fill walls, the anchor (hold-down) bolts shall be field drilled and anchored in place with epoxy resin grout (with appropriate loading tests being performed). Refer to the *Structures* chapter and the Standard and Directive Drawings for details. OCS poles shall not be placed on top of or attached to intrusion protection walls.

For OCS poles on aerial structures, refer to the *Structures* chapter and Standard and Directive Drawings for the embedded anchor bolt sleeve detail. Unused sleeves on aerial structures shall be filled in by the OCS Designer. For OCS poles on retained fill, mechanically stabilized earth (MSE) wall structures, provisions shall be made by the Structures Contractor for the future installation of the OCS foundations. Refer to the *Geotechnical* and *Structures* chapters and Standard and Directive Drawings for construction and space allocation details.

### 21.15.2 OCS Pole and Foundation Requirements

The pole and foundation locations shall be designed in a manner that avoids conflicts with existing or planned overhead or underground obstructions. For existing revenue service locations, the foundation shall be constructed in a manner that does not disturb the existing tracks under revenue service.

The loading assumptions and strength requirements shall meet or exceed the requirements of NESC rules. The general design loads include dead load, live loads such as wind and ice, and earthquake load. However, as noted in NESC Rule 250A4, the structural capacity provided by meeting the loading and strength requirements of NESC Sections 25 and 26 will provide sufficient capability to resist earthquake ground motions.



1 In addition to the load conditions indicated in NESC, a 100 mph wind plus 10 percent gust  
2 allowance shall be evaluated to prove no failure. The Designer shall also evaluate the local  
3 extreme climatic conditions and adjust the load combinations for worst case loads, including the  
4 effects of wind pressure on OCS poles due to slipstream effects per the *Structures* chapter.

5 All structures, poles, brackets, foundations and anchors shall be capable of handling  
6 construction loads imposed during erection and during catenary assembly and wire  
7 installation, and of withstanding a broken-wire failure, including breakage of both the static  
8 wire and parallel feeder conductor in any 1 span, without exhibiting major, catastrophic  
9 damage. These support structures shall also be capable of handling the loads due to breakage of  
10 other parts of the OCS. Pole and foundation loadings and structural designs shall be developed  
11 in accordance with the criteria defined herein. To facilitate aerial structure design, maximum  
12 loads for design of OCS pole foundation on aerial structures are specified in the *Structures*  
13 chapter.

14 All steel materials, related processes and manufacturing methods shall be specified in  
15 accordance with ASTM standards, wherever applicable and deemed appropriate, including  
16 requirements for hot-dip galvanizing of steelwork and hardware.

17 The design of bolted steelwork connections shall conform to AISC requirements and shall  
18 specify materials and methods in accordance with ASTM standards.

19 Anchor bolts (hold-down bolts) shall be galvanized.

20 OCS foundations and structures shall be designed so that their deflection under the loads  
21 imposed during normal operating conditions shall not cause a contact wire displacement that  
22 could prejudice acceptable tracking and performance of the pantograph current collector. To  
23 this end, the maximum allowable live-load operating deflection of the pole and foundation  
24 structure together shall be limited to 2 inches at the normal design contact wire height. For the  
25 purposes of structural design, this live loading shall be considered a dynamic operating  
26 condition, and the structure shall fully recover from its displacement due to the live loading.

27 For all non-operating loading conditions, excluding seismic conditions, the maximum total  
28 deflection of the pole and foundation together (measured at the pole top) shall not exceed  
29 2.5 percent of the total pole length due to both static (dead) loads and live loads combined.

30 The foundation and steel pole, or vertical members of the support structure, shall be designed  
31 to enable the pole to be raked during installation. This rake shall allow for the static dead loads  
32 that are imposed on the structure by the cantilevers, equipment and along-track conductors.  
33 Rake installation shall provide for a visually plumb and vertical pole after application of the full  
34 static loading. This position shall serve as the design reference datum for the calculation of the  
35 live-load operating deflection. All OCS alignment and wire layout designs shall utilize this  
36 static, plumb, dead load position as the true pole-face reference datum.

1 The OCS foundations and poles shall be designed in a manner to minimize the number of types  
2 and sizes to simplify constructability, to avoid disturbing existing adjacent structures, to  
3 provide flexibility for pole rake adjustment, and to minimize future maintenance inventory and  
4 costs.

5 Anchor bolt patterns shall be selected to provide coordinated relationships between poles and  
6 foundations. The coordination shall be based on matching strengths and minimizing the  
7 number of required configurations.

8 Particular attention shall be given to the provision of a high level of protection against  
9 atmospheric pollution and contamination to maintain the design life without frequent  
10 maintenance cycles.

11 OCS support locations shall be individually numbered for ease of identification on site.  
12 Structure number plates shall be fitted to the structure at a height of 6 feet 6 inches  
13 approximately above rail level. For supports located in tunnels, the number plate shall be  
14 attached to the wall using suitable fixings.

### **21.15.3 OCS Poles**

15 Poles shall be designed as free-standing structures, except for poles carrying wire terminations,  
16 which shall be down-guyed, typically in the along track direction. The lateral offset from  
17 centerline of tangent track to centerline of pole shall be 10 feet-8 inches. Offsets shall be  
18 increased as needed to satisfy curved track situations and/or signal sighting requirements.

19 Aerial structures will be designed in a manner such that OCS poles can be located at any  
20 position along the structure. Alternatively, working in close coordination with the OCS  
21 Designer, aerial structures can be designed to provide site-specific locations for OCS pole  
22 installations.

23 The OCS supporting structures shall be calculated in accordance with relevant American  
24 standards (ACI, AISC, ANSI, ASCE, NESC). The allowance for a one-third increase in allowable  
25 stress for wind combined loading shall be waived.

26 The design of structural and fabrication welding shall conform to the AWS, Standard D.1.1,  
27 "Structural Welding Code".

28 Painted poles shall not be precluded from use in passenger stations, within any urban design  
29 area, or in other special circumstances. Painting shall be specified to conform to the Steel  
30 Structure Painting Council, "Steel Structure Painting Manual," Volumes 1 and 2.

### **21.15.4 OCS Foundations**

31 The OCS foundations shall be capable of meeting the structural loading requirements, and shall  
32 be designed for each individual location. The structural dimensions will be dependent on the  
33 following:

- Loads on the poles due to the OCS conductors, feeder cables, tensioning equipment, insulators, mid-point anchor ties, and all other necessary equipment
- Wind loads on the poles and associated OCS conductors and equipment
- Soil conditions
- Earthquake loads
- Operational requirements the applicable speed

OCS foundation designs shall be in accordance with ACI, AISC, and ASTM standards, other applicable codes, and proven foundation engineering and anchoring methods. Foundation designs shall consider buoyancy effects where applicable. For corrosion control requirements, refer to the *Corrosion Control* chapter for details.

Augered, cast-in-place concrete foundations with a nominal diameter of 3 feet shall be adopted for all normal situations. Site-specific conditions or unusual loading combinations may dictate the adoption of other types or sizes of foundations. The permissible increase in soil resistance values, as defined in the CBC as being applicable to free-standing structures, shall be taken into consideration in the design of OCS foundations, in accordance with the CBC formulae.

The OCS foundations shall be designed to exceed the maximum design capability of the pole or structure being supported by the foundation by not less than 25 percent to ensure the foundation will not experience failure under the specified operating and non-operating conditions. The overturning moment shall not exceed 85 percent of the stability moment.

Where fragmented rock is encountered, excavation shall be required for the installation of standard foundations. Where solid rock is encountered below grade (i.e., with soil cover), epoxy-grouted dowels shall be anchored to the rock (with appropriate pull-out tests being performed), and the upper portion of a standard anchor-bolt foundation cast into the soil. Where solid rock is encountered at-grade (no soil cover), the anchor bolts shall be epoxy-grouted into the rock (with appropriate pull-out tests being performed) and with a small foundation-top cast around the bolts, primarily for aesthetic effect.

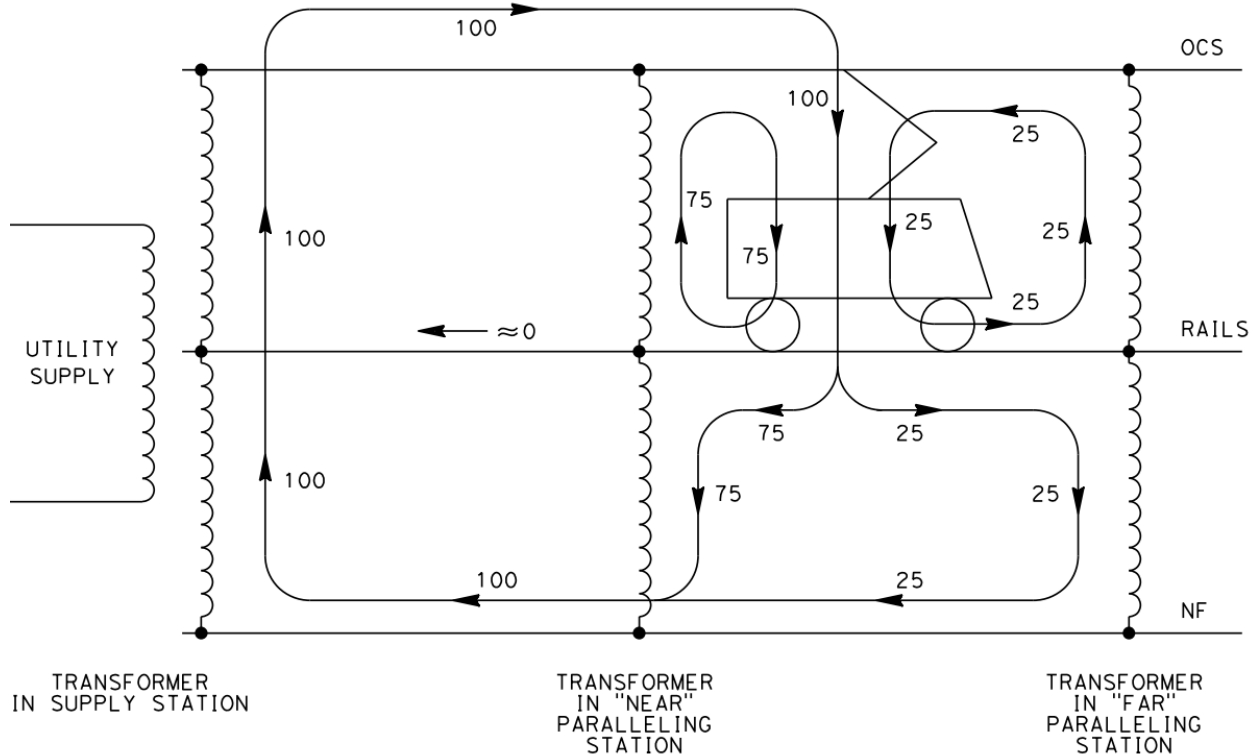
## 21.16 Traction Power Return System

The Rail Return System comprises the running rails, impedance bonds, static or ground wires, return cables, and the earth, each of which provides a part of the electrically continuous return path for the traction currents (refer to Figure 21-10). The Traction Power Return System comprises the Rail Return System together with the Parallel Negative Feeders, through which the whole traction current is returned from the wheel sets of the traction units to the substations.

The whole traction return current of a train operating between any 2 adjacent autotransformers flows through the rail return system within the bounds of these 2 autotransformers. These

autotransformers, however, “force” a major portion of the traction return current to flow into the negative feeders, thereby minimizing the flow of return current in the rails in sections away from the train operating section. This is a safety related benefit of the autotransformer feed system, the other benefit being reduced electromagnetic interference produced in this system as compared to the direct feed system.

**Figure 21-10: Typical Proportional Current Distribution in a 2x25 kV Autotransformer System for a Train Current of 200 Amps**



It is recognized that Figure 21-10 is a simplified diagram and is not an accurate representation of all current flows, since portions of the return current flow from the train location back to the substation via the static wires and earth. Further, a portion also remains in the track rails.

Refer to the *Communications* chapter for conductor routing and cabling segregation for core systems equipment.

## 21.17 OCS Interfaces with Other Disciplines

To achieve satisfactory performance of the OCS and current collection by the electrically-powered HSR vehicles, it is essential that the OCS Designer work closely with other disciplines. The following sections highlight some of the major issues that shall be addressed during the final design process. It is not a comprehensive list, but provides general guidance to the OCS Designer.

**21.17.1 Traction Power Supply System**

- 1 • Confirmation of OCS and ancillary conductor sizes based on the traction power load flow  
2 studies
- 3 • Confirmation of insulated cable sizes for both aerial and underground applications based on  
4 the load flow studies
- 5 • Confirmation of traction power facility locations, and particularly of the SS and SWS where  
6 phase breaks are required
- 7 • Recommended frequency of static wire to rail connections based on the rail potential rise  
8 calculations
- 9 • Confirmation of proposed OCS sectionalizing scheme based on coordination with  
10 Operations and Maintenance requirements

**21.17.2 Rolling Stock**

- 11 • Confirmation of the selected vehicle and pantograph operating characteristics for input into  
12 the OCS-Pantograph dynamic simulation program analyses
- 13 • Confirmation of pantograph spacing, on train consists using multiple pantographs, for input  
14 into the OCS-Pantograph dynamic simulation program analyses and for development of  
15 OCS phase break designs
- 16 • Confirmation that trains with multiple in service pantographs have no electrical inter-  
17 connection between the pantographs

**21.17.3 Train Control System**

- 18 • Coordination of locations of impedance bonds
- 19 • Coordination of signal sighting requirements
- 20 • Coordination of wayside power cubicle requirements and locations

**21.17.4 Communications System**

- 21 • Coordination of wayside power cubicle requirements and locations
- 22 • Coordination of Broadband Radio System access point locations and clearance
- 23 • Coordination of OCS disconnect switch RTU (Remote Terminal Unit) and interface  
24 requirements

**21.17.5 Bridge and Aerial Structure/Earth Retained Structure Design**

- 25 • Coordination of location of OCS poles and pole loadings for any conflicts, coordinate with  
26 infrastructure and restore any associated components.
- 27 Refer to the *Structures* chapter for the allowable loading and space allocation requirements.

#### **21.17.6 Trackwork**

- 1 • Coordination of locations of impedance bonds.
- 2 • Confirmation of space requirements below rails for the installation of return cables and
- 3 connections to impedance bonds.

## Chapter 22

### Grounding and Bonding Requirements

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	Sept 2013	Rolling Stock Procurement
2	Feb 2014	Revision 2
3	Feb 2016	Revision 3





## Table of Contents

<b>22</b>	<b>GROUNDING AND BONDING REQUIREMENTS .....</b>	<b>22-1</b>
22.1	Scope .....	22-1
22.2	Regulations, Codes and Standards.....	22-2
22.3	General Grounding and Bonding Requirements .....	22-3
22.3.1	Exceptions for Small Conductive Elements .....	22-4
22.3.2	General Facility Grounding .....	22-4
22.4	Maximum Permissible Step and Touch Potential .....	22-8
22.5	Grounding and Bonding Requirements for Facilities/Buildings and Structures .....	22-8
22.5.1	General Requirements .....	22-8
22.5.2	Facilities/Buildings.....	22-9
22.5.3	Grounding and Bonding of Structures - General.....	22-11
22.5.4	Aerial Structures (Viaducts and Underbridges).....	22-12
22.5.5	Track Support Structure .....	22-13
22.5.6	Passenger Station and Service Siding Platforms .....	22-15
22.5.7	Existing Overhead Structures .....	22-16
22.5.8	New Overhead Structures.....	22-17
22.5.9	Trenches, Retaining Walls, and Retained Fill Structures.....	22-17
22.5.10	Mechanically Stabilized Earth Walls .....	22-18
22.5.11	Tunnels.....	22-19
22.5.12	Screen/Noise/Wind/Safety Barriers.....	22-20
22.5.13	Use of Non-Reinforced Concrete and Non-Metallic Materials .....	22-20
22.5.14	Fence and Gate Grounding.....	22-20
22.5.15	Third-Party Grounding Interface.....	22-21
22.6	Grounding and Bonding Systems for the Traction Electrification System Equipment and Structures .....	22-21
22.6.1	General.....	22-21
22.6.2	Traction Power Facilities .....	22-22
22.6.3	Overhead Contact System .....	22-23
22.6.4	Return System.....	22-25
22.6.5	DC Traction Systems Adjacent to the Authority's Right-of-Way .....	22-25
22.7	Grounding and Bonding Protection Systems for the Automatic Train Control System Equipment and Structures .....	22-26
22.7.1	Automatic Train Control Houses and Automatic Train Control Rooms.....	22-26
22.7.2	Trackside ATC Equipment and Structures.....	22-26
22.7.3	Cross Bonding.....	22-27
22.8	Grounding and Bonding Protection Systems for the Communications System Equipment and Structures .....	22-30
22.8.1	General Requirements .....	22-30
22.8.2	Communication Equipment within Rooms at Stations and Facilities (located within a Shared Automatic Train Control and Communications Room and/or Dedicated Communications Room) .....	22-31
22.8.3	Communications Equipment and Structures.....	22-32
22.9	Grounding and Bonding Requirements for Facility Power Systems and Lighting Systems.....	22-32

22.10	Grounding Requirements for Raceway, Cable Tray, Underground Ductbanks, and Structures .....	22-34
22.11	Cables .....	22-34
22.11.1	General .....	22-34
22.11.2	Automatic Train Control System .....	22-34
22.11.3	Communications System .....	22-35
22.11.4	Facility Power System and Lighting System .....	22-35
22.11.5	Cable Trough and Outside Plant .....	22-35
22.12	Grounding and Bonding Requirements for Utilities .....	22-36
22.13	Lightning Protection .....	22-36

## Tables

Table 22-1:	Maximum Dimensions of Small Conductive Elements .....	22-4
Table 22-2:	Durations of Maximum Permissible Touch Voltages .....	22-8

## Figures

Figure 22-1:	Overhead Contact Line Zone and Pantograph Zone .....	22-24
--------------	--	-------

## Acronyms

ac	Alternating Current
ATC	Automatic Train Control
CEC	California Electrical Code
CIC	Communications Interface Cabinet
EMI	Electromagnetic Interference
IEC	International Electrotechnical Commission
MOI	Maintenance of Infrastructure
NFPA	National Fire Protection Association
O&M	Operations and Maintenance
OCS	Overhead Contact System
OSP	Outside Plant Cable
RGB	Rack Grounding Busbar
SRG	Signal Reference Grid
STP	Shielded Twisted-Pair
TBB	Telecommunications Bonding Backbone
TBBIBC	Telecommunications Bonding Backbone Interconnecting Bonding Conductor
TGB	Telecommunications Grounding Busbar
TES	Traction Electrification System
TPF	Traction Power Facility
UL	Underwriters Laboratories, Inc.
WPC	Wayside Power Cubicles



## 22 Grounding and Bonding Requirements

### 22.1 Scope

Grounding, Bonding, and Lightning Protection shall be designed to address 3 purposes: (1) personal safety; (2) equipment, cabling and building protection; and, (3) equipment noise reduction. This chapter addresses the first two items, and equipment noise reduction is addressed in the *Electromagnetic Compatibility and Interference* chapter. Grounding and bonding design shall be compatible with the requirements of the *Electromagnetic Compatibility and Interference* chapter.

In principle, to ensure the integrity of the grounding and bonding systems and the longevity of the system components, particularly for buried or encased elements, the bonding and grounding designs shall create duplicate electrical continuity paths and provide for redundancy in jumpers and bonds.

This chapter also provides criteria for the electrical separation of outside utility lines from the traction return and grounding systems.

Grounding is the establishment of a common reference voltage (typically 0 V) between power sources and/or electrical equipment. Electrical ground faults, short circuits, lightning, and transients can occur in electrical power supply and distribution systems or the facilities power systems. These design criteria specify requirements for the protective provisions relating to electrical safety in structures associated with the alternating current (ac) traction system and to any structures that may be endangered by the traction power supply and distribution systems or the facilities power system, and to any automatic train control (ATC), communications, or other electronic equipment that must be protected from electrical shocks. The grounding of ATC, communications, and other electronic equipment sensitive to high currents and/or voltages is also covered in this chapter. Grounding systems are intended to help clear faults in the quickest possible manner by providing a low impedance path for fault currents.

Grounding, bonding, and lightning protection is multi-disciplinary in nature. The design shall consider and mitigate the negative effects of lightning, ground potential rise, contact with electrical power circuits, and induction. The various discipline designers must collaborate with one another to coordinate the overall grounding and bonding design, so that a consistent approach is used and applied by each discipline in the development of the electrical, power and structural grounding and bonding and lightning protection. Refer to Standard and Directive Drawings for additional requirements.

Where multiple codes address the same issue, but specify differing approaches or values, the most stringent requirement shall be met.

In addition, this chapter provides criteria for designs that will minimize the touch voltage and ground return currents created by the electrification system and facilities electrical systems that will provide for the safety of passengers and operating personnel and minimize the hazards of electrical shock. The grounding and bonding system designs shall provide the means to carry electric currents into the earth under both normal and fault conditions without exceeding any operating and equipment limits or adversely affecting continuity of service.

For ac traction systems, grounding is the preferred method for reducing potentials of the electrical system both during normal operations and under fault conditions to protect equipment and to provide safety for employees and the general public. Adequate bonding shall be designed and installed throughout the entire electrified system to provide proper return circuits for the normal traction power currents and fault currents, with grounding connections as detailed in these criteria.

Design documents shall identify each type of ground connection, consistent with the ground categories identified in the *Electromagnetic Compatibility and Interference* chapter and as indicated in the following sections.

## 22.2 Regulations, Codes and Standards

- California Code of Regulations, Title 24 (California Building Standards Code which includes the California Electrical Code)
- California Public Utilities Commission (CPUC) General Order (GO) 95
- International Electrotechnical Commission (IEC) 60479: Effects of Current on Human Beings and Livestock – Part 1 General Aspects
- Institute of Electrical and Electronics Engineers (IEEE)
  - IEEE Std. 80: IEEE Guide for Safety in AC Substation Grounding
  - IEEE Std. 81: IEEE Guide for Measuring Earth Resistivity, Ground Impedance, and Earth Surface Potentials of a Ground System (Part 1)
  - IEEE Std. 142: IEEE Recommended Practice for Grounding of Industrial and Commercial Power Systems (IEEE Green Book)
  - IEEE Std. C2: National Electrical Safety Code
  - IEEE Std. 1100: “Recommended Practice for Powering and Grounding Sensitive Electronic Equipment.”
  - IEEE Std. C62.41: “IEEE Recommended Practice for Surge Voltages in Low-Voltage AC Power Circuits”
- American National Standards Institute (ANSI): ANSI-TIA-EIA-607-A “Grounding and Bonding Requirements for Telecommunications in Commercial Buildings”

- National Fire Protection Association (NFPA)
  - NFPA 70: National Electrical Code
  - NFPA Std. 780: Standard for Installation of Lightning Protection Systems
- European Standards (EN)
  - EN 50119 - 2001: Railway Applications – Fixed Installations – Electric Traction Overhead Contact Lines
  - EN 50122-1 - 2011: Railway Applications – Fixed Installations - Part 1. Protective provisions relating to electrical safety and earthing
  - EN 50124-1 - 2001: Railway Applications – Insulation Coordination – Part 1. Basic requirements – Clearances and creepage distances for all electrical and electronic equipment
- The Manual for Railway Engineering of the American Railway Engineering and Maintenance-of-Way Association (AREMA Manual)
- Underwriters Laboratories (UL)

### 22.3 General Grounding and Bonding Requirements

A uniform/standardized grounding and bonding system design shall be adopted to provide for protection of personnel and equipment. All grounding and bonding designs shall be coordinated with the various discipline designers, including civil, architectural, electrical and electronic, mechanical, plumbing and systems, such as traction power supply and distribution, communications, ATC, etc. All grounding and bonding designs shall be coordinated with the stray current and corrosion control measures, and with the electromagnetic compatibility and interference requirements, so that the respective designs do not conflict and render other systems ineffective.

Non-current carrying conductive parts, such as conduit, cable trays, handrails, trackside fencing, etc., shall be electrically bonded to provide a continuous electrical path, and shall be permanently and effectively grounded. Grounding system designs may include the grounding of individual items, and/or dividing the length of non-current-carrying conductive entities into sections with each section grounded at only 1 point. Grounding and bonding conductor sizes shall be selected in accordance with the latest version of the applicable code. Ground resistance at each grounding location shall be less than or equal to the value specified in the applicable code.

An electrical safety analysis shall be undertaken to assess which metallic parts need to be grounded and bonded, and the appropriate methodology for implementation identified.

### 22.3.1 Exceptions for Small Conductive Elements

At locations other than passenger station and service siding platforms, small conductive or partially conductive elements, such as signal posts, individual poles or masts that are not part of the OCS, fences or metallic structures, which are located outside the Overhead Contact Line and Pantograph Zone – Figure 22-1, and satisfy the dimensional limits stipulated in Table 22-1, and also meet the following requirements, do not need to be grounded or bonded to the traction return system:

- The unit does not contain or support electrical equipment
- A person approaching the unit from any direction can see if an energized conductor is touching it
- The distance between adjacent such units is greater than 8 feet.

For equipment on passenger station and service siding platforms, refer to Section 22.5.6.

**Table 22-1: Maximum Dimensions of Small Conductive Elements**

Type of Conductive Element	Parallel to Track	Perpendicular to Track
Fully Conductive	10 feet	6 feet
Partially Conductive	50 feet	6 feet

### 22.3.2 General Facility Grounding

The design of each large facility/building shall include a ground grid. Wayside houses shall be grounded by means of 1 or more interconnected ground rods. Where ground grids are used, the design shall adhere to the following requirements:

- Ground grid design shall be based on local soil resistivity and the calculations shall be in compliance with IEEE standards (e.g., 80, 142, and 1100) and the CEC/NEC/GO 95/NESC rules as applicable. Ground grids shall be constructed from an assembly of driven ground rods and bare copper conductors, installed adjacent to but not under the building, and shall achieve a ground resistance of not more than 5 ohms. A continuous loop of the grounding conductor(s) shall surround the perimeter of each facility/building, to which the perimeter fence and gates (where provided) shall be effectively bonded at frequent intervals, and within this loop the conductors should be laid in the form of a grid. At the cross-connections, the conductors should be securely bonded together by means of exothermic welds. Ground rods shall be installed at grid corners, at junction points along the perimeter, and at major equipment locations with the ground rods being driven vertically into the ground to not less than the minimum depth specified in NESC (2007) Rule 094B2a. Horizontal ground rod configurations may be required where subsurface rock or other obstructions interfere with the placement of vertical ground rods. The ground conductors may be made of copper or other



metals/alloys that will not corrode excessively during the expected service life. Ground rods may be of stainless steel, copper-clad or stainless steel-clad steel. The ground conductors shall be securely bonded to the ground rods and to the equipment (including busbars) to be grounded. Joints shall be exothermically welded.

- The ground rods shall be driven to stable soil where constant conductivity properties apply.
- At least 2 ground test stations shall be provided at each ground grid.
- Ground test stations shall be incorporated into the design of the ground grids. Each ground test station shall be connected to the ground grid by at least 2 grounding conductors.
- Ground test stations shall be located so that they are accessible to Operations and Maintenance (O&M) personnel. Locations shall be chosen that minimize the bonding conductor length.
- Supplemental electrodes (at equipment) shall not be permitted.
- The ground grid shall be bonded to the ac service ground electrode and the structural steel of the building structure.
- Ground grid locations shall be coordinated with landscaping plans to avoid conflicts with tree roots, and with underground utilities and sewer installations to avoid any direct electrical connection to these systems.
- In areas where soil resistivity is high or the space for installing a ground grid is limited, alternative methods shall be considered for obtaining low impedance grounding, such as connections to remote ground grids or wire mesh, deep-driven ground rods or drilled ground wells, plus the use of environmentally acceptable additives and soil treatment methods, etc. The effects of transferred potentials, which can result from interconnection to remote ground grids, shall be considered.
- Ground grids shall be designed to allow up to 50 percent additional ground rods to be driven and attached to the grid in the future.

#### 22.3.2.1 Ground Busbars

Unless otherwise specified, a single ground busbar shall be provided in each equipment room and pre-engineered enclosure (e.g., traction power facility [TPF] equipment houses, communications rooms, ATC/signal houses, and wayside power control cubicles). Ground busbars shall comprise a solid copper grounding busbar with insulated standoffs, and each ground busbar shall be drilled with rows of holes according to National Electrical Manufacturers Association (NEMA) standards, for attachment of bolted compression fittings.

Additional ground busbars for equipment shall be provided such that no potential equipment location within an auxiliary space (housing the equipment) is more than 30 feet from the nearest ground busbar.

1 Ground conductors at ground busbars shall be identified as to which system they are  
2 connected.

3 The ground busbars shall be connected to the ground electrodes or ground grids as detailed in  
4 Section 22.5.

#### **22.3.2.2 Non-buried Grounding Conductors**

5 Non-buried conductors between the ground grid or ground busbar and the grounded  
6 equipment shall be insulated copper wire or cable in non-metallic conduit. Grounding and  
7 bonding conductors shall be sized in accordance with the applicable code, so they can pass the  
8 maximum ground fault current without melting or fusing before the circuit breakers or  
9 protective relays disconnect the source of the fault current.

10 Non-buried grounding conductors shall be protected against physical damage and accordingly  
11 routed in conduit, cable tray systems, cable troughs, and/or ductbank systems under the  
12 following scenarios:

- 13 • Between buried ground grids and ground test stations
- 14 • Between ground test stations
- 15 • Between ground test stations and grounding busbars
- 16 • Between equipment enclosures and grounding busbars
- 17 • Between equipment enclosures
- 18 • Along the trackside

#### **22.3.2.3 Publicly Accessible Locations**

19 For locations that are accessible to the public, the following constraints shall apply to the  
20 grounding and bonding design:

- 21 • Anchor bolts and ground lugs shall not protrude in a manner that could result in injury  
22 or property damage.
- 23 • Materials shall be concealed wherever possible.
- 24 • Ground test stations in public areas shall be avoided.
- 25 • Tamper proof hardware shall be used.

#### **22.3.2.4 Grounding Connections**

26 Exposed grounding and bonding connections at equipment, enclosures, ground busbars,  
27 ground test stations, and so forth shall be visible and accessible. Two-hole compression-type  
28 termination lugs shall be used to connect bonding conductors to equipment enclosures.

29 Buried/underground joints in grounding conductors and connections shall be exothermically  
30 welded. Connections to reinforcement steel shall be exothermically welded. The steel

reinforcement in the structure elements shall be interconnected electrically by means of exothermic welds and shall be electrically continuous. Rather than relying on the interconnected rebar to provide the conductive paths, the Contractor may opt to install separate dual embedded grounding conductors, which shall be not less than 4/0 AWG Copper equivalent, to provide the primary grounding paths to which the rebar shall be electrically inter-connected as detailed in the various sections below. Epoxy coated rebar cannot be used as a grounding conductor. Where epoxy coated rebar is used as the only type of reinforcement, alternate grounding measures, such as connecting grounding plates directly to a series of buried ground rods or a ground grid, shall be adopted to achieve the required ground resistance of 25 ohms. Where epoxy coated rebar is used in combination with black rebar, the black rebar shall be interconnected to provide an electrically continuous path, with connection(s) to grounding plate(s), but with no connection to the epoxy bar. The required ground resistance of 25 ohms shall be achieved, if necessary, by connecting the grounding plate(s) directly to buried ground electrodes. Splices in grounding conductors will not be permitted.

Equipment enclosure doors shall be bonded with flexible metal bonding straps, instead of reliance on hinges for electrical continuity.

Where identical installations exist, the following requirements apply wherever practicable:

- The routing of conduit and conductors between structures and enclosures shall not differ.
- Conductor terminations shall be located in like manner.

Prescribed materials, cables and appurtenances shall be compliant with Underwriters Laboratories, Inc. (UL) standards.

The color of the insulation jacket of insulated ground conductors shall be green.

Water, gas or other piping shall not be utilized as a ground electrode or ground conductor.

#### **22.3.2.5 Electromagnetic Compatibility**

For electromagnetic compatibility considerations, provide the following:

- Proper grounding and bonding of apparatus, conductor shields, and raceways to maximize shielding and to minimize circulating currents in shields
- Surge protection against lightning and other natural sources of Electromagnetic Interference (EMI)

Additional requirements are specified in the *Electromagnetic Compatibility and Interference* chapter and in the Standard and Directive Drawings and Performance Specifications.

## 22.4 Maximum Permissible Step and Touch Potential

- 1 Step and touch potential at the facility substations shall be governed by the requirements of
- 2 IEEE 80: Guide for Safety in AC Substation Grounding. Step, and touch potential at facility
- 3 power electrical rooms shall be governed by the requirements of NFPA 70 and the CEC.
- 4 The bonding and grounding of current carrying equipment, enclosures and associated
- 5 structures, including the Overhead Contact System (OCS), rails, and other trackside equipment,
- 6 shall be designed such that the touch voltages do not exceed the values indicated in Table 22-2,
- 7 which has been derived from EN 50122-1: 2011 Section 9.2.2.

**Table 22-2: Durations of Maximum Permissible Touch Voltages**

Duration of Current Flow (sec)	Permissible Voltage in V (rms)
0.02	865
0.05	835
0.1	785
0.2	645
0.3	480
0.4	295
0.5	220
0.6	180
< 0.7	155
0.7	90
0.8	85
0.9	80
1.0	75
≤ 300	65
> 300 (where accessible to the public under all power supply feeding conditions)	60
> 300 (in workshops and similar locations)	25

8

## 22.5 Grounding and Bonding Requirements for Facilities/Buildings and Structures

### 22.5.1 General Requirements

- 9 Structure grounding and bonding shall create a conductive path that will achieve potential
- 10 equalization of the grounded elements of the railway system. Grounding connections provide

for tying wayside metallic parts to the return circuit and for the electrical interconnection of reinforcing rods in concrete structures, and in case of other modes of construction, the conductive interconnection of the metallic parts. The structure grounding system provides grounding connections for the following:

- High/medium-voltage protective ground
- Low-voltage protective ground
- Communication and signaling systems
- Lightning protection ground

Any non-energized component of structures within the overhead contact line and pantograph zone (refer to Section 22.6.3) shall be either directly grounded or be bonded to the static wire to provide for personnel safety.

In addition to the following requirements, grounding provisions for particular facilities, buildings and structures are indicated in more detail in the Standard and Directive Drawings and Performance Specifications.

## **22.5.2 Facilities/Buildings**

### **22.5.2.1 Service Entrance and Building Grounding**

The ac grounding electrode system (otherwise known as “building ground”, “service entrance ground”) shall be designed to provide the following:

- Establish a common reference voltage for ac electrical power systems
- Provide a safe dissipation path for lightning and/or accidental high-voltage contact
- Provide a safe dissipation path for electrostatic discharge

The components that make up the ac grounding electrode system include the following:

- Grounding electrode system (ground rod or ground grid)
- Grounding electrode conductor
- Bonding Conductor connects equipment grounding systems to the ac grounding electrode

A ground grid (refer to Section 22.3.1 for requirements), in direct contact with the earth at a depth below the earth surface of at least 3 feet, shall be provided at each building. The ground grid shall extend at least 2 feet beyond the foundation footer and at least 1 foot 6 inches outside the roof drip line.

1 The metal frame of buildings shall be bonded to the ground grid. Connections to the ground  
2 grid shall be exothermically welded. Where exothermic welding is impractical, UL listed  
3 connection hardware may be used.

4 For steel-frame buildings, alternate vertical columns shall be bonded to the ground grid.

#### **22.5.2.2 Building Exterior and Interior Bonding and Grounding**

5 Each grounding conductor that passes through a structure, foundation or wall shall be provided  
6 with a waterstop.

7 Multiple separate grounding systems are not permitted within the same building. Where a  
8 building is supplied by 2 or more services, the grounding electrodes for the 2 services shall be  
9 bonded together.

10 In multi-floor buildings, the grounding conductor shall be extended to each floor.

11 Provide a grounding electrode conductor sized in accordance with the applicable code between  
12 the service equipment ground bus and metallic water and gas pipe systems, building steel, and  
13 supplemental or made electrodes. Jumper insulated joints and bolted (non-welded) joints in the  
14 metallic piping.

15 Bond the steel columns to the reinforced steel within the building foundation.

16 Conductive piping systems shall be bonded to the building grounding system. Bonding  
17 connections shall be made as close as practical to the equipment ground bus.

18 Within a building, the grounding cable shall, where possible, be embedded in or underneath the  
19 floor slabs. Attach and bond the grounding electrode system to non-current-carrying  
20 conductive entities within the building.

#### **22.5.2.3 Raised Floor Systems**

21 A signal reference grid (SRG) shall be provided for raised floor systems in compliance with  
22 IEEE 1100 and with the requirements of the *Electromagnetic Compatibility and Interference* chapter.

23 Connect the SRG to the facility grounding electrode system.

24 Where raised floor systems are of bolted metal stringer construction and are electrically  
25 continuous, 2 connections only to facility grounding electrode system shall be required.

#### **22.5.2.4 Trackside Facilities Including Sump Pump Structures, Ventilation Structures and Portal Facilities**

26 Refer to Sections 22.5.2.1 and 22.5.2.2 for the grounding and bonding requirements for these  
27 facilities. For the grounding and bonding requirements at the tunnel portals for reinforced  
28 concrete paved areas of train surface evacuation and fire control zone refer to Section 22.5.6, and  
29 for the emergency walkways refer to Section 22.5.3.

### 22.5.2.5 Rolling Stock Maintenance Buildings Including Heavy Maintenance Facility and Terminal Layout/Storage and Maintenance Facilities

Refer to Sections 22.5.2.1 and 22.5.2.2 for requirements.

Where the catenary passes through the rolling stock maintenance buildings, catenary grounding devices shall be provided at building entrance points. These shall be in the form of 3-position disconnect switches, providing for Closed (to interconnect to the adjacent elementary electrical section), Open (no electrical interconnection), and Closed to Ground, with 1 disconnect per shop track elementary electrical section and with inter-linked indication lights on a per elementary electrical section basis. The inter-linked lights shall be positioned above each shop catenary section and shall permit maintenance personnel to be able to see clearly which sections of the OCS are energized and which are grounded. The disconnect handles shall be provided with bars that will accept multiple locks so that more than 1 maintainer can apply his/her personal lock to ensure the switches cannot be operated while someone is working.

The catenary disconnect switch grounding shall be electrically isolated from the building and associated facility electrical grounding system. The OCS static wire shall not be electrically interconnected to any conductive facility structure or to the facility grounding system.

### 22.5.2.6 Maintenance of Infrastructure Facilities

Refer to Sections 20.5.2.1 and 20.5.2.2 for the grounding and bonding requirements for Maintenance of Infrastructure (MOI) facilities.

### 22.5.3 Grounding and Bonding of Structures - General

Except for passenger station and service track platforms, metallic items on structures crossing over, under or immediately adjacent to the electrified tracks shall be bonded either directly or indirectly to a static wire and/or to a trackside grounding plate for personnel safety and lightning protection. The steel reinforcement in the structure elements discussed in the following sections, including the viaduct parapets, shall be interconnected electrically by means of exothermic welds and shall be electrically continuous. Rather than relying on the interconnected rebar to provide the conductive paths, the Structures Contractor may opt to install separate dual embedded grounding conductors, which shall be not less than 4/0 AWG Copper equivalent, to provide the primary grounding paths to which the rebar shall be electrically inter-connected as detailed in these criteria. Epoxy coated rebar cannot be used as a grounding conductor. Where epoxy coated rebar is used as the only type of reinforcement, alternate grounding measures, such as connecting grounding plates directly to a series of buried ground rods or a ground grid, shall be adopted to achieve the required ground resistance of 25 ohms. Where epoxy coated rebar is used in combination with black rebar, the black rebar shall be interconnected to provide an electrically continuous path, with connection(s) to grounding plate(s), but with no connection to the epoxy bar. The required ground resistance of 25 ohms shall be achieved, if necessary by connecting the grounding plate(s) directly to buried ground electrodes. Where dissimilar metals are connected, appropriate protective measures shall be employed. The grounding and bonding of the emergency walkway area and other publicly

accessible areas, as well as grounding and bonding of the track structure (where appropriate), shall be designed to avoid inadmissible touch and step voltages and also to meet the requirements of the signaling system.

#### **22.5.4 Aerial Structures (Viaducts and Underbridges)**

##### **22.5.4.1 Concrete Structures**

For concrete aerial structures, over which the high-speed trains travel, the following criteria shall apply. The static wire shall be electrically grounded through all aerial structure columns and abutments. The ground resistance from any grounding plate installed in concrete aerial structures shall be 25 ohms or less. The reinforcement steel within the individual concrete elements comprising a concrete aerial structure, including parapet walls, shall be bonded together to achieve an equipotential structure. A sufficient number of reinforcement bars shall be bonded together to create a 4/0 AWG copper equivalent electrical path for each grounding circuit. Jumpers, both internal between rebar and grounding plates and external between grounding plates, shall be a minimum size of 4/0 AWG copper, but alternate materials of equivalent electrical capacity may be adopted. Jumpers shall be flexible and shall be of sufficient length to accommodate structure movements. Appropriate measures shall be adopted where dissimilar metals are interconnected. Surface-mounted grounding plates shall be a minimum of 6 inches x 6 inches in size and shall have 4/0 AWG copper equivalent electrical capacity, shall resist corrosion and staining of the concrete structures and accommodate exothermic welding of the copper jumpers. Structural grounding and bonding equipment shall be designed to meet the design life of the structure in which it is embedded.

##### **Concrete Columns, and Abutments and Foundations**

The reinforcement steel in all column foundations and abutment foundations shall be electrically connected to the reinforcement steel in the columns and abutments, respectively. For abutments, an exothermically welded 4/0 AWG copper jumper shall connect the abutment reinforcement to a surface-mounted grounding plate near the top and bottom of the abutment to provide a minimum of 4 grounding plates per abutment. For columns, an exothermically welded 4/0 AWG copper jumper shall connect the column reinforcement to a surface-mounted grounding plate near the top and bottom of, and on transverse faces of, the column to provide a minimum of 4 grounding plates per column.

##### **Concrete Superstructure Units**

A “superstructure unit” is defined as a singular, horizontally-installed, reinforced concrete structure of any length that is supported by one or more columns and/or abutments. The reinforcement steel in concrete aerial superstructure unit shall be electrically interconnected and similarly jumper-connected to surface-mounted grounding plates on both sides of and near the bottom of the superstructure unit, such that an external jumper can be installed to electrically connect the superstructure grounding plates to the grounding plates at the top of each supporting column and abutment. The number and location of the superstructure unit surface-



mounted grounding plates shall be coordinated and aligned with the grounding plate locations on supporting columns and abutments – refer to Overhead Contact System Directive Drawings.

For superstructure units that are 150 feet or less in length, grounding plates that are electrically bonded to the reinforcing steel shall be installed at or close to the midpoint of each superstructure unit on the surface of the deck slab near both trackside cable trough walls and on the surface of both parapet walls for connection to the track slab grounding plates. The parapet grounding plates shall be used for exothermically welded grounding connections to the OCS poles and to other systems elements, such as ATC and communications cubicles or houses, and wayside power control cubicles, and shall be sized to accommodate not less than 4 separately-installed exothermically welded jumpers. For superstructure units that are more than 150 feet in length, additional grounding plates shall be installed such that the along-track spacing between deck-slab and parapet plates is a maximum of 150 feet.

#### 22.5.4.2 Steel Structures

For steel girder structures, over which the high-speed trains run, the static wire shall be electrically grounded by means of jumpers with bolted lug connections to the OCS pole baseplates and exothermic weld connections to the grounding plates on the structure columns and/or abutments. The steel bridge girders shall be interconnected at the “fixed ends” with exothermically welded flexible bonds (sized as indicated above), which shall be connected to the reinforcement steel (within the bridge piers or abutments) at 2 grounding plates, each with a ground resistance of 25 ohms or less, similar to the requirements indicated above for the concrete structures. To provide electrical continuity at the “sliding end”, the 2 outer girders shall each be connected by an exothermically welded flexible jumper (of sufficient length to allow for expansion and contraction of the girders and sized as indicated above) to a grounding plate that is connected to the reinforcement steel (within the bridge pier or abutment). Where non-ballasted track is installed on the steel support structures, flexible jumpers shall be exothermically welded between the track slab grounding plates and the steel support girders.

#### 22.5.5 Track Support Structure

Steel reinforcing bar (rebar) loops in concrete supporting running rails can cause inductive loading or undesired coupling between track circuits. To avoid these adverse effects, all rebar in any structure that is within 1 foot of a CHST running rail shall be grounded in 1 of the following configurations. The Contractor shall determine the most appropriate configuration and shall use the same configuration in all similar applications. Structures to be treated include track slabs, viaduct decks, etc.:

- **Long comb** – In each affected structure, longitudinal rebars (those parallel to the running rails) are electrically connected in a 'comb' pattern to a single lateral (perpendicular to the running rails) connecting rebar at 1 end only of the longitudinal rebars. The lateral connecting rebar shall be connected to an external connection to the reinforcement that is

connected to a grounding plate. The other lateral rebars shall be ungrounded and insulated from the longitudinal rebars at their intersections.

- **Short comb** – In each affected structure, lateral rebars shall be electrically connected in a 'comb' pattern to a single longitudinal connecting rebar at 1 end only of the lateral rebars. The longitudinal connecting rebar shall be connected to an external grounding plate. The other longitudinal rebars shall be ungrounded and insulated from the lateral rebars at their intersections.

- **Double comb** – In each affected structure, longitudinal rebars shall be electrically connected in a 'comb' pattern to a single lateral connecting rebar at 1 end of the longitudinal rebars. The lateral connecting rebar shall be connected to an external grounding plate. The other lateral rebars shall be connected in a 'comb' pattern to a single longitudinal rebar. The longitudinal connecting rebar shall be connected to an external grounding plate. The longitudinal and lateral rebars shall be insulated at their intersections, and shall be only connected together through their grounding connection.

If a structure contains multiple layers of rebar within 1 foot of a running rail, each layer shall be treated in 1 of the above configurations.

If a structure contains multiple layers of rebar, layers that are greater than 1 foot from any running rail can be grounded in any pattern that satisfies the safety requirements for grounding and bonding.

#### **22.5.5.1 Non-Ballasted Independent Dual Block Track**

In this form of track construction, the independent blocks, which are supported by non-reinforced track concrete, provide no electrical interconnection through which traction current would flow. However, the reinforcement in the cast-in-place concrete slab could become part of the return circuit and shall be grounded and bonded in a manner similar to that for the aerial structure beams/segments.

The grounding design shall ensure the maximum permissible touch voltages, as specified in Table 22-2, are not exceeded.

#### **22.5.5.2 Non-Ballasted Precast Concrete Slab Track**

In this form of track construction, the reinforced precast segmental slabs, which are supported by non-reinforced asphaltic material, could provide an electrical path through which traction current may flow. In addition, the reinforcement in the cast-in-place concrete slab could become part of the return circuit. Therefore, the reinforcement in the precast segmental slabs shall be inter-connected to the reinforcement in the cast-in-place concrete slab and shall be grounded and bonded in a manner similar to that for the aerial structure beams/segments.

The grounding design shall ensure the maximum permissible touch voltages, as specified in Table 22-2, are not exceeded.

## 22.5.6 Passenger Station and Service Siding Platforms

### 22.5.6.1 Passenger Stations and Service Siding Platforms in At-grade, Cut-and-Cover Tunnel, or Trench Locations

For at-grade platform grounding, a counterpoise shall be installed along the entire length of each platform with the conductor buried in earth and extending a minimum of 50 feet beyond the ends of the platform, with intermediate ground rods driven and interconnected at not more than 50-foot intervals. For cut-and-cover tunnel or trench located platform grounding, where the structure is protected by waterproof membranes, an in-ground counterpoise cannot be installed. For these locations, a bare grounding conductor shall be installed along the entire length of each platform with intermediate connections to the platform reinforcement at not more than 500-foot intervals and with the conductor extending a minimum of 50 feet beyond the ends of the structure buried in earth and connected to driven ground rods. The grounding conductor shall be a minimum size of 4/0 AWG copper, but alternate materials, such as aluminum of comparable electrical capacity, may be adopted. Appropriate measures shall be adopted where dissimilar metals are interconnected. One end of the counterpoise or grounding conductor shall be terminated in a handhole, which will permit the counterpoise or grounding conductor to be connected to the rails via an impedance bond, with the location being coordinated with the ATC System Designer. Metallic structures and miscellaneous metallic items within 8 feet from the edge of the platform (including platform reinforcement steel and any OCS poles) shall be isolated from the static wire and shall be bonded directly or indirectly to the counterpoise or grounding conductor. The counterpoise- or grounding conductor-bonded metallic items shall be isolated from steel building grounds and particularly from utility grounds.

The grounding design shall ensure the maximum permissible touch voltages, as specified in Table 22-2, are not exceeded and, without exception, the resistance to ground shall not exceed 5 ohms. Subject to field testing during construction, it may be necessary to install supplemental ground rods at the ends of the counterpoise to achieve the 5 ohm value, and the Designer shall incorporate this requirement in the design.

Refer to Sections 22.5.2.1 and 22.5.2.2 for the grounding and bonding requirements for facility power installations in passenger stations.

### 22.5.6.2 Passenger Station Platforms in Aerial Structures

The station buildings, including the platforms, will be constructed to be structurally independent from the aerial trackway. The facility electrical system will utilize insulated cable grounding connections to the electric utility supply point. Refer to Sections 22.5.2.1 and 22.5.2.2 for the grounding and bonding requirements for facility power installations in passenger stations.

To minimize step and touch potentials between the station platform and the vehicles, the platform reinforcement shall be electrically interconnected and shall be connected to the rails through an impedance bond at 1 end only, as detailed above for the at-grade platforms. The

reinforcement in the platform shall be electrically interconnected to the reinforcement in the supporting structure and its foundation reinforcement. If the platform is constructed using pre-cast panels, grounding plates (as detailed above for the aerial structures) shall be used to provide the means for interconnecting the reinforcement in the panels and for attachment of the impedance bond connection. Provisions shall be incorporated into the platform design such that all metallic structures and miscellaneous metallic items within 8 feet from the edge of the platform (including any OCS poles) shall be isolated from the static wire and shall be bonded directly to the platform reinforcement. The platform reinforcement-bonded metallic items shall be isolated from the facility and utility grounds.

The grounding design shall ensure the maximum permissible touch voltages, as specified in Table 22-2, are not exceeded and, without exception, the resistance to ground shall not exceed 5 ohms. Subject to field testing during construction, it may be necessary to install supplemental ground rods outside the limits of the platform, which can be attached to the platform grounding system to satisfy the touch voltage requirements, and the Designer shall incorporate this requirement into the design.

#### **22.5.6.3 Elevated Access Platforms at Service Sidings**

Metallic parts of elevated access platforms for rooftop equipment inspection and maintenance at service sidings shall be electrically interconnected and shall be connected to the rails through an impedance bond at 1 end only to minimize step and touch voltages between the access platform and the vehicles. Provisions shall be incorporated into the platform design such that all metallic structures and miscellaneous metallic items within 8 feet from the edge of the platform (including any OCS poles) shall be isolated from the static wire and shall be bonded directly to the platform. The platform-bonded metallic items shall be isolated from any facility or utility grounds.

The grounding design shall ensure the maximum permissible touch voltages, as specified in Table 22-2, are not exceeded. Subject to field testing during construction, it may be necessary to install supplemental ground rods outside the limits of the platform, which can be attached to the platform grounding system to satisfy the touch potential requirements, and the Designer shall incorporate this requirement into the design.

#### **22.5.7 Existing Overhead Structures**

If components of existing overhead structures, as detailed below, lie within the overhead contact line and pantograph zone, refer to Figure 22-2, the following special grounding provisions may be required to afford protection to adjacent third party installations:

- Abutments or Piers – galvanized steel strip on each bridge wall or attached to columns of piers
- Bridge Face – galvanized steel strip or angle section above the overhead line at each bridge face, if the bridge soffit is within the pantograph zone

1 The above measures shall be provided at existing structures if an analysis determines the need  
2 for them.

3 When the vertical clearance between OCS conductors and concrete overhead structures is less  
4 than 3 feet, protection panels (flash plates) shall be installed above the OCS, attached to the  
5 underside of the structure, and interconnected to the static wire at not less than 2 locations. For  
6 steel overhead structures, the steel girders shall be interconnected and bonded to the static wire  
7 at not less than 2 locations.

8 Publicly accessible overhead structures shall be protected by screening and/or barriers which  
9 shall be at least 8 feet high and extend laterally beyond the live parts of the overhead contact  
10 line by at least 10 feet on each side. The metallic portion of the screening and barriers shall be  
11 bonded to the static wire at not less than 2 locations. All other metallic items on the overhead  
12 structure, within the lateral range of not less than 10 feet beyond any energized and uninsulated  
13 equipment passing below the structure, shall be directly or indirectly bonded to the static wire.

#### 22.5.8 New Overhead Structures

14 Grounding and bonding for concrete beam and steel beam structures, and their supporting  
15 abutments or piers, shall be as detailed above for the aerial structures, and shall include  
16 provision of grounding plates for connections to the static wire(s).

17 Where the headroom at new overhead structures is less than the preferred minimums for  
18 structures over either dedicated or shared use segments of the route, as shown in the Contract  
19 Documents, some of the measures detailed in Section 22.5.7 may have to be implemented. If  
20 components of new overhead structures lie within the overhead contact line and pantograph  
21 zone, refer to Figure 22-2, the provision of flash plates may be required when the vertical  
22 clearance between OCS conductors and concrete overhead structures is less than 3 feet, attached  
23 to the underside of the structure, and interconnected to the static wire at not less than 2  
24 locations. For steel overhead structures, the steel girders shall be interconnected and bonded to  
25 the static wire at not less than 2 locations.

26 Publicly accessible overhead structures shall be protected by screening and/or barriers which  
27 shall be at least 8 feet high and extend laterally beyond the live parts of the overhead contact  
28 line by at least 10 feet on each side. The metallic portion of the screening and barriers shall be  
29 bonded to the static wire at not less than 2 locations. All other metallic items on the overhead  
30 structure, within the lateral range of not less than 10 feet beyond any energized and uninsulated  
31 equipment passing below the structure, shall be directly or indirectly bonded to the static wire.

#### 22.5.9 Trenches, Retaining Walls, and Retained Fill Structures

32 The reinforcement steel in the trench walls shall be electrically connected to the reinforcement  
33 steel of the base slab, and successive trench segments shall be connected together to provide  
34 electrical continuity. For trench structures, where the structure is protected by waterproof  
35 membranes, a bare grounding conductor shall be installed along the entire length of both sides

1 of the structure with intermediate connections to the structure reinforcement at not more than  
2 500-foot intervals. At these connection points, the grounding conductor shall be connected to  
3 buried electrodes, driven into the ground outside the trench structure, and each grounding  
4 point shall achieve a resistance of 25 ohms or less. To satisfy this requirement, the Contractor  
5 may opt to install additional grounding plates on the top of the trench walls. The grounding  
6 conductor shall be extended a minimum of 50 feet beyond the ends of the structure (buried in  
7 earth) and connected to driven ground rods. The grounding conductor shall be a minimum size  
8 of 4/0 AWG copper, but alternate materials, such as aluminum angle of comparable electrical  
9 capacity, may be adopted. Appropriate measures shall be adopted where dissimilar metals are  
10 interconnected. The reinforcement steel in retaining walls or retained fill structures shall be  
11 similarly electrically connected to the reinforcement steel of the footing, and successive  
12 retaining wall or retained fill structure segments shall be interconnected to provide electrical  
13 continuity. The static wire shall be electrically grounded through the trench or retaining wall or  
14 retained fill structure reinforcement via surface-mounted grounding plates, with a ground  
15 resistance of 25 ohms or less, as detailed for the aerial structures. To facilitate grounding of the  
16 track slab, grounding plates, electrically interconnected to the structure reinforcement, shall be  
17 installed in the face of the walkway. Grounding plates, electrically bonded to the reinforcing  
18 steel, shall be provided at no more than 10 feet from each end of, and in each niche/recess in,  
19 trench and retaining wall structures, and at intervals not to exceed 200 feet +/- 10 feet for all  
20 structure types. Systems elements, including ATC and communications cubicles or houses,  
21 wayside power control cubicles, and facilities electrical equipment, shall be grounded to these  
22 plates.

23 For long trenches or retaining wall segments, particularly those cut into rock, the Systems  
24 Designer shall determine whether additional grounds and/or an additional along-track ground  
25 wire(s) may be required to meet the ground resistance requirements and/or to minimize the  
26 possibility that rail potentials may cause unacceptable touch voltages and shall incorporate such  
27 into the design as needed. If any additional ground conductors are laid at low level—adjacent to  
28 the track or along the walkways—these conductors will also supplement the grounding  
29 capability of the system and enhance fault detection and control in the event of a broken wire  
30 condition. In order to provide a sufficiently low ground resistance, it may be necessary to install  
31 a ground grid at or near to the end(s) of the trench or retaining wall segment.

#### **22.5.10 Mechanically Stabilized Earth Walls**

32 Typically, Mechanically Stabilized Earth (MSE) structures comprise a number of discrete  
33 concrete facing panels that are mechanically inter-connected and attached to lateral reinforcing  
34 strips which are embedded in compacted, high resistivity, granular fill material. Refer to the  
35 *Geotechnical* and *Structures* chapters for details. Individual reinforcing strips are short and are  
36 generally installed perpendicular to track, so, even if the reinforcing strips are metallic, they are  
37 not subject to induced currents from the overhead electrification system. Metallic reinforcing  
38 strips, where used, must not be electrically interconnected to the reinforcement in the facing  
39 panels, which must not be electrically connected to one another. Since these elements are all of

relatively small size, MSE walls do not constitute continuous electrically conductive elements and, therefore, do not need to be bonded and grounded. For stray current and corrosion issues, refer to the *Corrosion Control* chapter. Where parapet walls, safety handrails or barriers, noise and wind walls or barriers are constructed on top of the MSE structure, using reinforced concrete or metallic components, they must be bonded and grounded as detailed in Sections 22.5.3, 22.5.4 and 22.5.12 and other parts of these criteria.

### 22.5.11 Tunnels

For tunnel sections, metallic drop pipes or brackets, which support the OCS cantilevers, shall be bonded to the static wire and, dependent on tunnel length and the requirements of the ATC system, the static wire shall be connected to rail through impedance bonds in the same manner as for surface sections. For tunnels, particularly those cut into rock, the Systems Designer shall determine whether additional grounds and/or an additional along-tunnel ground wire(s) may be required to meet the ground resistance requirements and/or to minimize the possibility that rail potentials may cause unacceptable touch voltages and shall incorporate such into the design as needed. Any required additional ground conductors shall be laid at low level—adjacent to the track or along the outer edges of the walkways—so these conductors will supplement the grounding capability of the system and enhance fault detection and control in the event of a broken wire condition. Based on the rail potential analysis and in order to provide a sufficiently low ground resistance, it may be necessary to install a ground grid at or near 1 or both tunnel portals.

Where the reinforcing rods in the tunnel structures can be interconnected longitudinally, such as in cut-and-cover construction, or the tunnel is built in sections (with gaps along the length), each section shall be connected to the return circuit static wire, or the sections shall be connected together and then connected to the static wire, utilizing grounding plates where appropriate—mounted at the interior surface of the tunnel—as detailed above for the aerial structures. To facilitate grounding of the track slab, grounding plates, electrically interconnected to the structure reinforcement, shall be installed in the face of the walkway.

Grounding plates, which shall be electrically bonded to the static wire, shall be provided at no more than 10 feet from each end of, and in each niche/recess in, tunnels and at intervals not to exceed 200 feet +/- 10 feet.

Ground connections for facility services within tunnels shall provide an exposed ground conductor (sized per Code [NEC and CEC]) parallel to each track for the complete length of the tunnel and interconnected (by at least 2 connections) to each building (e.g., ventilation structures, portal facilities, sump pump structures) ground grid associated with the tunnel structure. Where the tunnel structure design includes cross passages, emergency shafts and so forth, the ground conductor shall be extended into these spaces. The ground conductors shall be located to avoid accidental contact by personnel.

### 22.5.12 Screen/Noise/Wind/Safety Barriers

1 The reinforcement steel of screen, noise, wind and/or safety barriers shall be electrically  
2 connected to the reinforcement steel of the associated structure (e.g., aerial structure, trench) in  
3 a similar manner to that detailed above for aerial structures.

4 Conductive safety barriers shall be electrically bonded to the static wire at not fewer than 2  
5 locations. Independent conductive safety barriers, such as hand rails, safety screens, etc., which  
6 are constructed in panels, shall be electrically bonded together and shall be electrically bonded  
7 to the static wire at not fewer than 2 locations.

### 22.5.13 Use of Non-Reinforced Concrete and Non-Metallic Materials

8 Due to the lack of metallic components, trackside elements, such as cable troughs, cable trough  
9 covers, sound walls, parapets, screens, and safety railings or barriers that are to be constructed  
10 within the OCS and Pantograph Zone (Figure 22-1) using non-reinforced mass concrete or non-  
11 metallic materials, cannot be bonded or grounded. However, to protect these elements in the  
12 event that an energized aerial conductor breaks and comes to rest on top of these items and to  
13 ensure rapid de-energization of the broken conductor, a grounding strip or stranded grounding  
14 conductor shall be laid along and attached to the top of these units and shall be electrically  
15 bonded to the static wire and segmented (electrically insulated) at intervals such that the touch  
16 voltage does not to exceed the limits specified in Table 22-2.

### 22.5.14 Fence and Gate Grounding

17 The Designer shall evaluate touch voltages on metallic fences and/or gates, including inter-track  
18 fences, which lie within the Overhead Contact Line and Pantograph Zone (Figure 22-1). Ground  
19 electrodes shall be installed on either side of a gate or other opening in the fence. Fence posts at  
20 openings in the fence shall be bonded to form a continuous path, and gates shall be bonded to  
21 support posts with flexible metal bonding straps to eliminate reliance on hinges for electrical  
22 continuity. Fences shall be made electrically continuous and grounding conductors shall be  
23 exothermically welded to fence posts and to any fence material support members (top and  
24 bottom) between posts. Metallic fences inside the Overhead Contact Line and Pantograph Zone  
25 (Figure 22-1) shall be electrically bonded to the static wire and segmented (electrically  
26 insulated) at intervals such that the touch voltage does not to exceed the limits specified in  
27 Table 22-2.

28 Metallic fences outside the Overhead Contact Line and Pantograph Zone (Figure 22-1), up to a  
29 distance of 45 feet from an outside track centerline, shall be bonded to form a continuous path  
30 in the same manner as detailed above. Ground electrodes shall be installed on either side of a  
31 gate or other opening in the fence, and at intermediate locations, based on local soil resistivity  
32 and worst case projected potentials. Grounding conductors shall be exothermically welded to  
33 fence posts and to driven ground electrodes.



### 22.5.15 Third-Party Grounding Interface

Due to the danger of voltage propagation, third-party grounding installations in the vicinity of the right-of-way shall not be connected to the railway grounding system. For third-party pipework, non-conducting materials shall be used or an insulating segment or insulated joint shall be inserted at the site boundary. Where the public network grounding system cannot be separated from the railway grounding system due to lack of space for separation, the traction power return circuit shall be interconnected with the neighboring grounding system of the public network.

To minimize the possibility of shock hazards outside the fence line, the Systems Designer shall evaluate touch potentials on third-party metallic fences/gates and/or pipelines that parallel the right-of-way, or other metallic structures. The Systems Designer shall provide designs for grounding and/or segmenting the conductive feature using insulating measures for these elements, such that touch potentials are controlled to levels that do not exceed the limits detailed in Table 22-2. Fences, and/or segments shall be made electrically continuous, but shall not be connected to railway ground grids, grounding conductors, static wires, or the rails, and shall be independently grounded by means of driven ground rods. Grounding conductors shall be exothermically welded to fence posts and to driven ground electrodes.

In cases where fences are purposely electrified to inhibit livestock or wildlife from crossing the fence, site-specific insulating measures shall be designed and implemented.

Connections to third-party infrastructure shall be coordinated and approved by the third party. Refer also to Section 22.12 and to the *Utilities* chapter.

## 22.6 Grounding and Bonding Systems for the Traction Electrification System Equipment and Structures

### 22.6.1 General

The rail return path of the 2x25 kV ac autotransformer feed Traction Electrification System (TES) consists of the static wires, the running rails, cable connections from static wires/running rails to the TPF (all of which are grounded as detailed below) and the earth. The static wire is connected at regular intervals to the running rails, via impedance bonds, and to the (grounded) center tap of the secondary winding of each traction power transformer and the (grounded) center tap of the winding of each autotransformer, using 2 independent connections. The static wire runs alongside the catenary to interconnect the OCS supporting structures and brackets, such that all normally non-current-carrying metallic supports of the OCS are at the same ground (and track) reference potential.

The traction return current causes a voltage rise in the running rails and static wires, due to the impedance of these conductors, for both normal operations as well as under short circuit conditions, thereby resulting in a voltage between the running rails and static wires and the

surrounding ground or other grounded metallic parts (touch voltage). These touch voltages shall be limited to acceptable values. Hazards due to touch voltages shall be minimized by means of adequate grounding and bonding measures, and as required, mitigation measures. For a more detailed description of the (TES), refer to the *Traction Power Supply System* and *Overhead Contact System and Traction Power Return System* chapters.

The overall grounding and bonding protection for the TES shall consist of the OCS aerial static (ground) conductors, connections from the aerial ground conductors to any buried ground conductor/ground rods and the impedance bonds connected to the track, with connections between these elements and each TPF ground grid. In addition to the impedance bond connections at the TPFs, additional connections between the static wires and the rails through impedance bonds may be needed, based on the traction power load flow simulation results and the step and touch analysis. The Designer shall determine the required spacing of the impedance bonds and interconnections to the rails, which must be coordinated with the operating requirements of the ATC System.

Where insulated cables are used within the TES, they shall be specified and manufactured in accordance with the appropriate electrical standards that are applicable to the working environment—voltages, operating and fault currents—to which they will be subjected.

The OCS grounding and bonding system shall interconnect all OCS normally-non-current-carrying metallic parts and shall bond metallic components of overhead bridges, tunnels and other structures.

The grounding and bonding system for the TES shall not be electrically connected to any non-traction power facility electrical grounding system.

### **22.6.2 Traction Power Facilities**

Refer to Section 22.5 for general ground grid requirements.

The ground grid for each TPF shall be in direct contact with earth at a depth of 12 to 18 inches below grade, as stipulated in IEEE-80: 2000, Section 9.2. The ground grid shall be extended at least 2 feet beyond the fence of the TPF. The ground grid for each TPF shall be designed per the specifications of IEEE-80, NESC, CEC and NEC.

The metal fence of the TPF shall be bonded to the TPF ground grid as specified in the applicable code.

A layer (3 to 6 inches thick) of high resistivity material, such as gravel, shall be spread on the earth's surface above the ground grid to increase the contact resistance between the soil and the feet of persons in the TPF. The grounding system shall be designed so that the step and touch potentials under fault conditions are within the designated limits.

Wayside power cubicles (WPC) in at-grade locations shall be grounded by separately driven ground rods at opposite corners, which are to be connected to grounding pads on the enclosure.

For aerial locations and in tunnels, trenches, retaining wall and retained fill structures, the WPC shall be grounded by 2 interconnections to a grounding plate (as detailed above for the aerial structures).

### 22.6.3 Overhead Contact System

OCS structure grounding and bonding should create a conductive path that will achieve potential equalization of the grounded elements of the railway system. Grounding connections provide for tying wayside metallic parts to the return circuit and for the electrical interconnection of reinforcing rods in concrete structures, and in case of other modes of construction, the conductive interconnection of the metallic parts. The structure grounding system provides grounding connections for the following:

- High/medium-voltage protective ground
- Low-voltage protective ground
- Grounding of telecommunication and signaling systems
- Lightning protection ground

The OCS poles shall be grounded through interconnection of the pole to the static wire so that the ground resistance of the interconnected poles is kept low. Reinforced concrete and anchor bolt foundations, where the concrete is in good contact with the adjacent soil, are recognized as being good earth electrodes. Where the ground resistance of individual OCS poles exceeds 25  $\Omega$ , individual ground rods or other grounding solutions shall be applied. All other OCS structural supports (e.g., wall brackets, drop pipes, feeder wire brackets, etc.) shall be interconnected to the static wire.

Ground connections to disconnect switches and ground leads from surge arresters shall have a maximum ground resistance of 5 ohms. Ground rods or a ground mat may be utilized to obtain the required ground resistance. If the metallic OCS pole at the disconnect switch location is to be used as part of the grounding system, the pole must be bonded to the steel reinforcement in the concrete pier foundation, and the reinforcing steel bonded to 1 or more driven ground rod(s) and/or the ground mat.

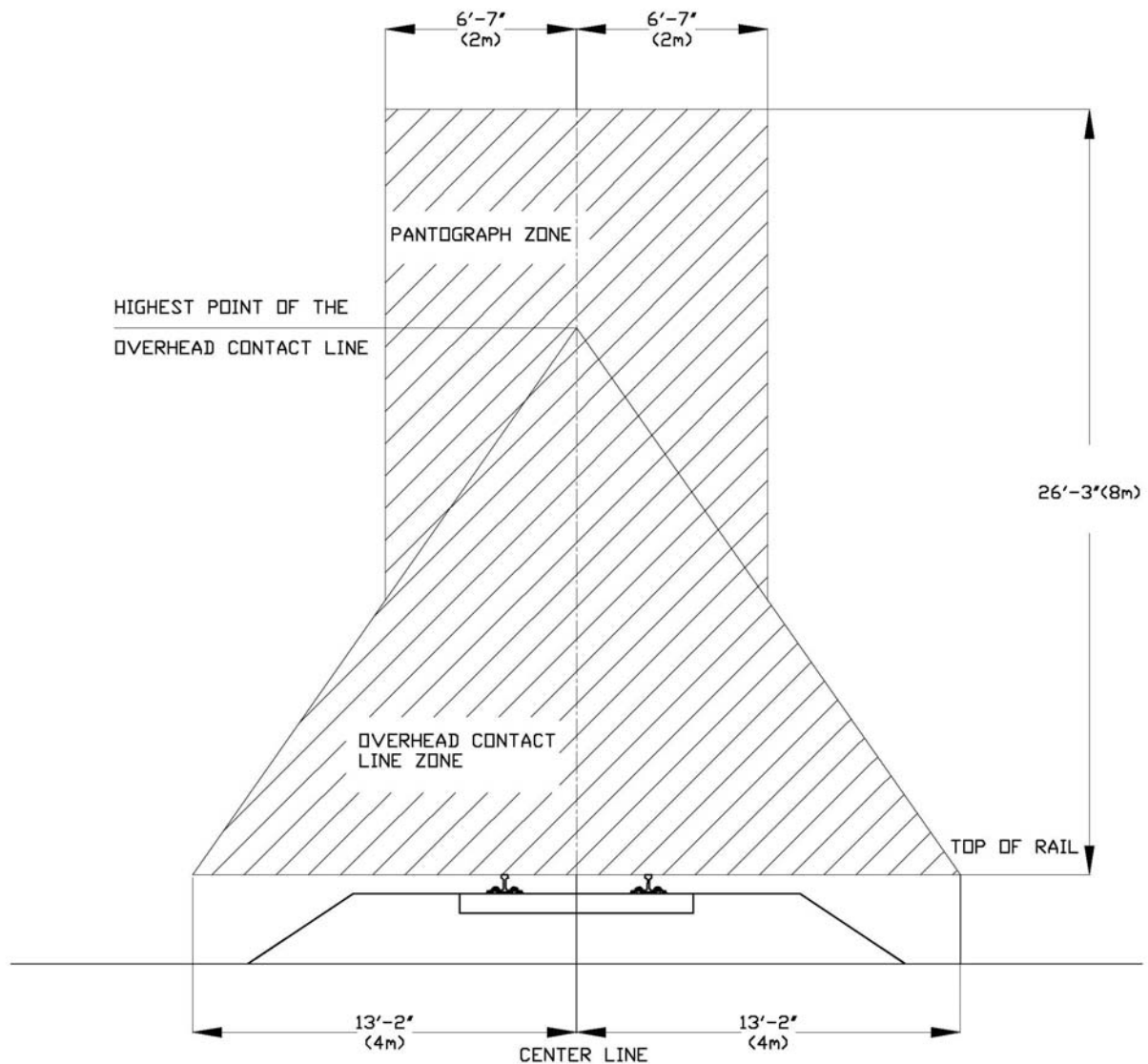
Refer to Section 22.7 for additional requirements.

A live broken contact line, or live parts of a broken or de-wired pantograph or energized fragments, may accidentally come into contact with wayside structures and equipment. Figure 22-1 defines the zone inside which such contact is considered probable and which limits are unlikely to be exceeded, in general, by a broken overhead contact line or damaged energized pantograph, or energized fragments.

The limits of the overhead contact line zone below top of rail extend vertically down to the earth surface, except where the tracks are located on a viaduct where they extend down to the

- 1 viaduct deck. In the case of energized out-of-running OCS conductors, the overhead contact line
- 2 zone shall be extended accordingly.
- 3 Non-current-carrying metallic components that lie within the overhead contact line and
- 4 pantograph zone shall be either directly grounded or bonded to the static wire to provide for
- 5 personnel safety.

6 **Figure 22-1: Overhead Contact Line Zone and Pantograph Zone**



#### 22.6.4 Return System

The ground grid at each traction power facility, and the center tap of the secondary of main power transformers and the center tap of autotransformers, shall be connected to the traction power facility ground bus. The ground bus shall be connected to the rails through impedance bonds, and to both static wires through 2 independent connections. The return cable shall be sized to carry the maximum load current, thereby allowing for the failure of 1 return cable. The static wire shall be connected at regular intervals to the running rails, via impedance bonds at locations approved by the ATC Designer. Additionally the impedance bonds shall be cross-connected at intervals in both 2-track and multi-track areas at a frequency that does not compromise the broken rail detection system.

This aspect of the grounding and bonding system design shall be coordinated between the Traction Power Supply System, OCS and ATC designs.

#### 22.6.5 DC Traction Systems Adjacent to the Authority's Right-of-Way

Where tracks operated by dc traction power systems are located adjacent to the tracks, a considerable degree of coordination will be required with the dc traction system operator to minimize the possibility of creating dc stray current circuit paths through the ac system traction power return circuits. Track insulation between rails and ground shall be enhanced in these areas in compliance with the requirements of the *Trackwork* chapter.

The design of the TES shall minimize the possibility of creating dc stray current circuit paths through the return system. The static wire in the area potentially subject to dc stray currents shall be electrically insulated from the OCS poles by supporting the static wire on insulators. The OCS poles shall be grounded through interconnection of the pole, anchor bolts and steel reinforcement of the concrete foundation so that the ground resistance of individual poles is kept low, and does not exceed 25 ohms. Where the ground resistance of individual OCS poles exceeds 25 ohms, individual ground rods or other grounding solutions shall be applied. Fault conditions shall be evaluated and grounding designs shall be developed such that unsafe touch voltages are not created.

The Designer shall monitor and document any dc stray current leakage from dc system tracks adjacent to the Authority's right-of-way/facilities during the design phase to establish baseline levels. Similarly, the contractor shall monitor and document any dc stray current leakage from the dc system tracks during the field testing and commissioning phase, to evaluate any differences and take any necessary remedial action to ensure the integrity of the system.

Where passenger platforms and emergency walkways are located adjacent to dc system tracks, the Designer shall investigate whether inadmissible touch voltages could occur between the rail and ground, and shall determine whether a voltage-limiting device, such as non-permanent rail to ground connection, should be installed to control touch voltages. If necessary, the designs shall require the contractor to install such devices.

- 1 All of the above measures shall be coordinated with the ATC System Designer.
- 2 Early in the design phase, the Authority will have to coordinate with the dc electrified railroad
- 3 operator to obtain assurance that the operator will maintain a high level of insulation between
- 4 the dc system rails and ground in these sections to minimize the possibility of any dc stray
- 5 currents leaking into the ac traction return system.

## **22.7 Grounding and Bonding Protection Systems for the Automatic Train Control System Equipment and Structures**

### **22.7.1 Automatic Train Control Houses and Automatic Train Control Rooms**

- 6 Ground grid and risers to the ATC rooms in stations shall be included in the facilities design,
- 7 and shall stub out 6 inches above the floor inside the ATC room.
- 8 Ground rods or ground grid and risers to the ATC houses and cases shall be provided by the
- 9 ATC contractor where these units are located at-grade.
- 10 The grounding system for ATC equipment shall be designed as a single-point ground system.
- 11 Equipment safety grounding shall be designed to limit touch voltages to safe levels, as specified
- 12 in Table 22-2, with and without a fault on the ac system.
- 13 Solid copper ground busbars designed for mounting on the framework of open or cabinet-
- 14 enclosed ATC equipment racks shall be provided. Ground bars, within equipment racks, shall
- 15 be bonded together using solid copper splice plates. All busbars and the metal structure of the
- 16 houses and cases shall be bonded to the ground conductor, which in turn shall be bonded to the
- 17 local ground provision (either ground rods or grounding plate integrated into the civil
- 18 structures.
- 19 Bonding conductors shall be continuous and routed in the shortest, straight-line path possible.
- 20 Ac and dc ground detectors shall be provided for each train control room and house, and the
- 21 sensitivity shall be sufficient to detect a ground leakage resistance of 0 to 2,000 ohms for ac
- 22 ground and 0 to 10,000 ohms for dc ground.

### **22.7.2 Trackside ATC Equipment and Structures**

- 23 Grounding of wayside equipment and metallic structures, including houses and wayside cases,
- 24 shall be localized as much as practical with ground rods driven into the earth as close to the
- 25 equipment/structure as possible. Where the structure design prevents the use of ground rods,
- 26 grounding plates and grounding conductor (as detailed above for the aerial structures, retained
- 27 fill, trenches, and tunnels) to which the signal equipment shall be grounded, shall be included
- 28 in the Infrastructure design. Refer to Section 22.5 for further details of facilities, buildings and
- 29 structure grounding requirements.

The ground resistance shall not exceed 15 ohms as measured from equipment to ground where the ground rods are provided by the Systems contractor and 5 ohms from equipment to the grounding plate connection provided by the Contractor.

The base of a ground-mounted signal mast or dwarf signal shall be bonded to the traction return system by direct connection to the neutral leads of the impedance bond adjacent to the signal. There shall be no other electrical connections between the signal mast and other structures or other rails or neutral leads unless specifically called for on the plan as part of an “A” point (see below).

Signal bridges or cantilever structures at a location that is not an “A” point (as defined in Section 22.7.3.1) shall not be electrically connected to any neutral leads or any portion of any track structure that is part of the signal system. These structures shall not be bonded to the static wire of the OCS, but shall be connected to ground rods in at-grade locations, or to grounding plates on aerial structures, in tunnels, or in trench/retaining wall locations.

### 22.7.3 Cross Bonding

In signaling systems using track circuits in which the block lengths are defined by insulated joints in the track, impedance bonds are employed to permit the traction return current in the rails to pass through a relatively low impedance on its way back to the substation, while at the same time presenting a very high impedance to the signal circuit. The connections to impedance bonds are configured in a variety of ways, as discussed below, and the bonds are usually installed at the insulated joints. The bonds are designed so that the center tap can be grounded or connected to a return conductor in the traction power return circuit.

The preferred locations for impedance bonds are at or in close proximity to the traction power facilities where the center tap is connected to the traction facility ground grid. Rail potential rise results from the flow of traction power return current in the rails and additional locations for impedance bonds are dictated by the need to minimize that potential rise within safe limits.

The preferred locations for all impedance bonds will be identified under the TP system design but must then be coordinated and confirmed by ATC Designer who shall undertake the block design. Once the locations have been confirmed, the Trackwork Contractor, who shall supply the joints, will install the insulated joints at the agreed locations. The ATC Contractor shall supply and install the bonds, and the OCS Contractor will supply and install the center tap bond connections and the exothermically welded rail tap connections. The ATC Contractor shall supply and install the track circuit connections.

#### 22.7.3.1 General Requirements

Impedance Bonds at Cross Bonding Locations shall be of 2 different configurations:

- An “A” point is defined as a location where impedance bond neutral leads on all tracks are bonded together, and to 1 or more OCS support structures which are in turn bonded

to the static return wires and, at traction power facilities (such as substations, switching stations, and paralleling stations) directly to the traction power facility return bus.

- A "B" point is defined as a location where the impedance bonds neutral leads connect 2 tracks together and are also connected to ground, but are not connected to the static wire or the traction power return bus.

Impedance Bonds at Other Locations shall be of 2 different configurations:

- A "C" point is defined as a location with impedance bonds that bypass insulated joints on 1 track but with no cross-bonding to adjacent impedance bonds or tracks, or to the OCS static wires.
- A drain bond is an impedance bond installed to connect the rails to traction power facilities, such as substations, switching stations, and paralleling stations, where no insulated joints exist in the tracks in the vicinity of the traction power facility. At such locations, the neutral leads of the drain bonds shall be connected directly to the traction power facility return bus. Drain bonds shall also be used at stations for connecting the platform counterpoise or grounding system to the rails where there is no adjacent "A" or "C" bond to which the platform grounds could be connected.

The purpose of cross-bond locations ("A" and "B" points) is to minimize step voltages on the rails to values that are less than the limits specified in Table 22-2. Cross-bond locations shall include a minimum of 2 track circuits between them and should, if practical, be located not less than 6,000 feet apart but not more than the permissible potentials will permit. The ATC Designer shall coordinate the cross-bonding locations with the TES designers.

Where cross-bonds are more than 6,000 feet apart:

- Distance between cross-bond locations shall be not less than 167 percent of the length of the longest track circuit, any portion of which lies between the cross-bond points.
- Conversely, the total length of any track circuit, any portion of which is between the cross-bond points, shall not exceed 60 percent of the distance between the cross-bond points.
- The ideal arrangement shall be 2 equal length track circuits between cross-bond points, each 50 percent of the total distance between them.

Where the cross-bonds are up to a maximum of 6,000 feet apart:

- There shall be a minimum of 3 track circuits between the "A" and "B" point cross-bond locations.
- Distance between cross-bond locations shall be not less than 250 percent of the length of the longest track circuit, any portion of which lies between the cross-bond points.



- Conversely, the total length of any track circuit, any portion of which is between the cross-bond points, shall not exceed 40 percent of the distance between the cross-bond points.
- The ideal arrangement would be 3 equal length track circuits between cross-bond points, each 33.33 percent of the total distance between them. Total distance shall be as close to 6,000 feet as possible.
- In no case will a distance of less than 3,000 feet between “A” point cross-bond locations be permitted.

The “percent ratio” shall be calculated for any given section between cross-bond locations as  $D(XB)$  divided by  $D(LTC)$ , where  $D(XB)$  is the distance between cross-bond points defining the section and  $D(LTC)$  is the length of the longest track circuit in the section.

Where “drain” bonds constitute an “A” point at other than insulated joint locations, the “ $D(LTC)$ ” shall be the total length of the longest track circuit in the section including any portion of that track circuit outside the limits of the section defined by the cross-bond points.

#### 22.7.3.2 Cross-bonding at Interlockings

Cross-bonding shall be placed as close to interlocking crossovers as practical to reduce the possibility of flashover of insulated joints in crossovers. If possible, an “A” point shall be placed at 1 of the interlocking home signal locations at each interlocking. This shall be done consistent with these design criteria and the need to place an “A” point at each substation, switching station or paralleling station return bus location.

Only 1 impedance bond shall be provided at the fouling insulated joints on the turnout track, located on the side of the joints away from the switch points. The neutral leads on this impedance bond shall be tied to the neutral leads between the impedance bonds located at the adjacent insulated joints on the main or straight track.

If there are no insulated joints on the main or straight track within approximately 20 feet of the fouling insulated joints on the turnout track, then a second impedance bond may be used on the turnout track at the fouling insulated joints, located on the switch point side of these joints and the neutral leads of the 2 impedance bonds at these joints connected in the usual manner. In this case, the neutral leads of the impedance bonds on the turnout track must not be connected to the neutral leads on the main or straight track.

#### 22.7.3.3 Cross-bonding at Single and Twin Single Bore Tunnels

Where an “A” point is required within a single bore tunnel, the neutral leads of the impedance bonds shall be connected to the static wire in that tunnel only. Where an “A” point is required within twin single bore tunnels that incorporate cross passage(s) between bores, cross-bonding between bores shall be accomplished by laying the cables in the cross-passage(s).

#### 22.7.3.4 Connections to Platform Grounding Systems

Counterpoises for at-grade station platforms shall be connected to the rail through the neutral leads of an impedance bond at 1 end of the platform only. The preference is to connect to the neutral leads of an “A” point. If it is not practicable to attach to an “A” point, the counterpoise shall be connected to the neutral leads of a “C” point. If this is not practicable, a drain bond shall be installed. Each platform shall have an independent counterpoise and impedance bond. The interconnection between the impedance bond neutral leads and the counterpoise conductor shall be an exothermic weld which shall be made in a handhole interface box.

Platforms located on aerial structures shall be grounded as identified above and shall be connected to the track through the neutral leads of an impedance bond at 1 end of the platform only, and with the same order of preference as detailed above. Each platform shall have an independent grounding system and impedance bond. If the impedance bonds used for this purpose are either “C” points or drain bonds, the location becomes a “B” point as the platform counterpoises on structures are attached to the structure and therefore both tracks will be connected together through the neutral leads of the impedance bonds.

### 22.8 Grounding and Bonding Protection Systems for the Communications System Equipment and Structures

#### 22.8.1 General Requirements

Communications and electronic systems shall be grounded and bonded in accordance with the requirements specified in NFPA 70E, NFPA 75, ANSI/TIA/EIA-607, CEC, IEEE 1100 and ITU standards. The Communications Designer shall design a communications grounding system to have an impedance from device to ground per IEEE 1100.

The grounding methods for enclosures, chassis, panels, switch boxes, pull boxes, conduits, terminal boxes, and similar enclosures or structures shall be designed to provide proper terminations for equipment and cable shielding (as necessary) and to avoid conducted coupling, low impedance ground loops, noise, surges from adversely affecting system operation, and hazardous operating conditions—refer to the *Electromagnetic Compatibility and Interference* chapter for more details.

The ac grounding electrode system is the fundamental grounding element supporting the communications grounding system. The ac grounding electrode system design must be verified to provide a suitable ground resistance for all communications equipment it serves. The Communications Designer shall use the building structural steel as an additional bonding point for the communications grounding system. The impedance between the structural steel and the ac ground electrode system shall be compliant with IEEE Std. 142.

If the ac grounding electrode system does not supply compliant ground resistance, supplemental grounding electrodes shall be installed to lower ground resistance, and shall be connected to the ac grounding electrode system and the ground grid.

The communications grounding system serves to establish a common reference voltage for communications equipment cabinets, enclosures, equipment, and power supplies, and provide an intentional path for fault current to the ac grounding electrode system.

The components that make up the communications grounding system include the following:

- Supplemental grounding electrodes
- Bonding conductor for communications
- Telecommunications Bonding Backbone (TBB)
- TBB Interconnecting Bonding Conductor (TBBIBC)
- Telecommunications Grounding Busbar (TGB)
- Rack Grounding Busbar (RGB)

Design documents shall clearly articulate details and connectivity of the communications grounding system.

The Communications Designer shall not design a separate communications-only or isolated ground system.

Bonding conductors shall be sized according to applicable standards and codes.

Bonding conductors run for distances less than 100 feet shall be minimum 6 AWG. For distances greater than 100 feet, the Communications Designer shall size bonding conductors using the more conservative requirements of either the NEC or the CEC.

#### **22.8.2 Communication Equipment within Rooms at Stations and Facilities (located within a Shared Automatic Train Control and Communications Room and/or Dedicated Communications Room)**

Refer to Section 22.5 for ground grid requirements for buildings.

When communications equipment and ATC equipment occupy a shared room, the Designer shall coordinate the equipment and shield grounding between the 2 disciplines.

##### **22.8.2.1 Telecommunications Bonding Backbone within Stations and Facilities**

Communications room grounding busbars shall be connected together by means of the telecommunications bonding backbone, which shall provide for the interconnection of the grounding busbars, located in each communications room or closet throughout the building, to the telecommunications main grounding busbar.

The telecommunications bonding backbone route and cable size shall be planned to minimize length and eliminate splices.

Wherever 2 or more vertical telecommunications bonding backbones are used in a building, the grounding busbars shall be interconnected at the top of each riser and at every third floor with a telecommunications bonding backbone interconnecting bonding conductor in accordance with ANSI/TIA/EIA-607 and the CEC and NEC.

#### **22.8.2.2 Grounding Busbars within Communications Rooms within Stations and Facilities**

A grounding busbar shall be provided in every communications room and within every shared ATC and communications room. An entrance facility shall be identified for each building where communications conduit and cable penetrates. The entrance facility shall be provided with a grounding busbar. The grounding busbar shall be located close to the ac ground electrode system. Where a panel board for telecommunications is located in the same room or space as the grounding busbar, that panel's ground bus or the enclosure shall be bonded to the grounding busbar.

In steel building structures, the telecommunications grounding busbar and telecommunications bonding backbone shall be bonded to the structural steel. The ground resistance between the structural steel and the ac grounding electrode system shall be compliant with IEEE Std. 142.

The communications grounding system in each room shall be bonded to the busbar in each communications room and communications closet. In communications rooms and closets, the busbar shall be bonded to the ac grounding electrode and the nearest structural steel member.

#### **22.8.3 Communications Equipment and Structures**

Provide a grounding busbar within all communications interface cabinets (CICs). Within CICs, the equipment grounding system shall bond equipment, rack rails, cabinets and cabinet doors to the telecommunications grounding busbar which shall be bonded to the incoming ac grounding electrode.

### **22.9 Grounding and Bonding Requirements for Facility Power Systems and Lighting Systems**

Refer to Section 22.5 for ground grid requirements.

A bare grounding electrode conductor shall be provided between the HV switchgear/transformer ground bus and the ground grid.

The secondary neutral of pad mounted medium and/or high voltage transformers shall be grounded. Additionally the pad design shall include the following features:

- Ground grid in accordance the CEC, or power utility service requirements as applicable.
- The concrete support pad reinforcement steel shall be bonded to the ground grid.

Exterior transformers supplying interior service equipment shall have the neutral grounded at the transformer secondary and a grounding electrode shall be provided at the transformer.

In the case of separately derived systems (i.e., transformers downstream from service equipment), ground the secondary neutral at the transformer to the nearest component of the grounding grid.

Lightning arresters on medium and high voltage equipment shall be connected to the equipment ground bus or ground rods as applicable.

For secondary switchgear, switchboards, and motor control centers, the following requirements shall apply:

- The equipment grounding conductors shall be connected to the ground bus in the enclosure with suitable pressure connectors.
- Metallic conduits, which terminate without mechanical connection to the housing, shall include grounding bushings and grounding conductor to the equipment ground bus.
- For service entrance equipment, the grounding electrode conductor, carried in the power supply conduit, shall be connected to the ground bus.

Ground the frames of motors larger than 25 hp by a ground conductor carried in power conduit.

Fixed electrical appliances and equipment shall be provided with a ground lug for termination of the equipment grounding conductor. Ground lugs shall be provided in each box and enclosure for equipment grounding conductor termination.

Panel boards shall contain a ground bus, bolted to the housing, with sufficient lugs to terminate the equipment grounding conductors.

Ground light fixtures to the equipment grounding conductor of the wiring system.

Receptacles shall not be grounded through their mounting screws. Ground with a jumper from the receptacle green ground terminal to the device box ground screw and the branch circuit equipment grounding conductor.

Feeder and branch ac power and lighting circuits shall have a separate insulated equipment grounding conductor.

Bond the equipment grounding conductor to each pullbox, junction box, outlet box, device box, cabinets, and other enclosures through which the conductor passes.

## 22.10 Grounding Requirements for Raceway, Cable Tray, Underground Ductbanks, and Structures

- 1 Metallic raceway and cable trays systems shall be bonded together to provide a continuous
- 2 electrical ground path.
- 3 Metallic raceways shall be bonded to other raceway components using insulated grounding
- 4 bushings. Grounding bushings shall be connected to the grounding system using conductors
- 5 sized in compliance with the applicable code.
- 6 Connect each isolated metallic cable tray system or the entire cable tray system to the building
- 7 grounding systems with a bare copper conductor in accordance with the CEC, NEC, and NEMA
- 8 VE 1.
- 9 Provide an equipment ground conductor, sized in accordance with the CEC and NEC (but not
- 10 less than 2 AWG for medium voltage power circuits) in each conduit of an underground
- 11 ductbank that contains power cables.
- 12 Raceways for lighting and power feeders to motor, lighting, and receptacle loads shall contain a
- 13 separate green insulated safety grounding conductor.
- 14 All normally-non-current-carrying conductive parts of manholes, handholes, pull boxes, splice
- 15 boxes, metallic raceway, and/or cable tray systems shall be bonded and grounded. Provide at
- 16 least 1 driven ground rod at each underground structure.

## 22.11 Cables

### 22.11.1 General

- 17 The metallic sheaths, armor or shields of power cables shall be electrically continuous through
- 18 manholes, pull boxes, and splice boxes. The metallic sheaths, armor or shields of power cables
- 19 shall also be designed and routed in accordance with the requirements detailed in the
- 20 *Electromagnetic Compatibility and Interference* chapter.
- 21 Conductor splice case grounding and bonding requirements shall comply with the
- 22 manufacturer's recommendations and CEC and NEC. It shall be ensured that the touch voltages
- 23 at the non-grounded end of the metallic sheaths, armor or shields of cables do not exceed the
- 24 maximum permissible touch voltages specified in Table 22-2.

### 22.11.2 Automatic Train Control System

- 25 Signal control or lighting cables and switch cables shall not have metallic shielding. Metallic
- 26 messenger or duct shall not be used in any way that could cause an electrical interconnection
- 27 between signals or signal structures and signal equipment housings.

### 22.11.3 Communications System

Bonding of shielded twisted-pair (STP) cables is necessary to mitigate the effects of unwanted noise signals (antenna effect) on communications cables and to avoid interference with overall network performance.

The shield of STP cables shall be bonded to the connecting hardware in accordance with the manufacturer's instructions. As appropriate, the connecting hardware at the cross-connect shall be bonded to the ground busbar in the ATC room or house, communications equipment shelter or termination room, or communications room. Grounding at the work area is usually accomplished through the equipment power connection. Shield connections at the work area are accomplished through an STP patch cord. At the work area end of the horizontal cabling, the voltage measured between the shield and the ground wire of the electrical outlet used to supply power to the work station shall not exceed 1.0 V rms. Telephone and public address cables at ATC rooms or houses, communications shelters or termination rooms, and communications rooms that originate from the field devices, shall require surge protection.

Bond telephone protector units to the grounding system with at least a No. 6 AWG ground conductor.

### 22.11.4 Facility Power System and Lighting System

The shields of medium voltage ac power cables shall be grounded at the facility power electrical rooms and/or yards in accordance with the requirements detailed in the *Electromagnetic Compatibility and Interference* chapter. The shields shall be electrically continuous through manholes, pull boxes, and splice boxes.

The safety grounding conductors for feeder circuits shall each be bonded at 1 end to the electrical room or yard ground bus and at the other end to the ground bus of a panelboard or a motor control center ground bus. Each branch circuit shall have a safety-insulated grounding conductor extended from the ground bus of the panelboard or motor control center to the device it is serving.

### 22.11.5 Cable Trough and Outside Plant

Provide an equipment ground conductor, sized in accordance with the CEC (but not less than 2 AWG for medium voltage power circuits), in each conduit of an underground ductbank that contains power cables.

All normally non-current-carrying conductive parts of manholes, handholes, pull boxes, splice boxes, metallic raceway, and/or cable tray systems shall be bonded and grounded.

The Communications Designer shall coordinate with other disciplines and submit code compliant Outside Plant Cable (OSP) infrastructure bonding system.

## 22.12 Grounding and Bonding Requirements for Utilities

1 Non-railway pipes or cable shields should have no connection to the traction return and  
2 grounding systems. Metallic utility lines entering or passing through the Authority's right-of-  
3 way shall be fitted with insulated joints to separate the external services and isolate them from  
4 the traction return and grounding systems.

5 Pipes or shielded cables to or from non-railway installations may transfer potentials that could  
6 be bridged by persons as step or touch voltages. Additionally, corrosion may be caused by  
7 potential differences, if different grounding systems are connected together. For these reasons  
8 non-railway grounding systems shall not be connected to railway systems.

9 Metallic sleeves or casings, installed to permit utility lines to cross the tracks (refer to the  
10 *Utilities* chapter) shall be grounded at 1 end only, with the grounding electrode having a  
11 resistance of 25 ohms or less.

12 Grounding and bonding for the electrical service shall be provided in accordance with the  
13 electric utility company's requirements.

14 Unless formally approved by the utility owner there shall be no connection between the  
15 grounding system and any utility (including water) outside the dielectric coupling which is  
16 used to isolate facilities from utilities outside the building line.

## 22.13 Lightning Protection

17 Each facility and exposed structure shall be provided with appropriate lightning protection  
18 measures, based on the incidence of strikes in the area local to each facility, which shall be  
19 grounded in accordance with the recommendations of the equipment manufacturer, CEC, NEC,  
20 NESC, GO 95, and NFPA 780 – Standard for the Installation of Lightning Protection Systems, as  
21 applicable.

22 Insulated cables carrying feeds to the OCS shall be protected with surge arresters.

23 The OCS Designer shall investigate the incidence of lightning storms on a project section-by-  
24 section basis and shall determine appropriate lightning protection measures, based upon the  
25 incidence of lightning strikes in each area. Where lightning protection of the OCS is deemed  
26 necessary, a shield wire shall be installed at the top of the OCS poles, which may necessitate an  
27 increase in pole height to achieve the required clearances. If the cone of protection afforded  
28 from this position is insufficient, the shield wire shall be mounted on an outrigger cantilever so  
29 that shield wire is more closely positioned above and affords protection for all of the OCS  
30 conductors. The shield wire shall be insulated from the OCS poles in passenger station areas.  
31 Additional protection/mitigation measures, e.g., additional grounding conductors/grids, shall  
32 be provided as required.



- 1 Lightning arresters and other circuit protection devices shall be provided as necessary to protect  
2 wayside ATC equipment from damage and false operation due to lightning. The lightning  
3 arresters shall comply with AREMA Signal Manual, for lightning protection.
- 4 Trackside antenna towers (e.g., at ATC houses, communication equipment shelters, TPF, etc.)  
5 within the Authority's right-of-way are specified to be 100 feet tall. Appropriate lightning  
6 protection measures shall be provided based on the incidence of strikes in the area local to each  
7 antenna tower and/or roof mounted antenna.
- 8 Reinforced concrete structures may not be able to take direct lightning strikes without damage.  
9 Exposed prestressed concrete structures shall be provided with lightning protection, especially  
10 in lightning prone areas.
- 11 The electrodes—ground rods or ground grids—used to ground lightning protection systems  
12 shall not be the same as those used for grounding of either the traction or facility electrical  
13 systems, but the electrodes from both systems must be bonded together.



## Chapter 23

### Corrosion Control

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	Nov 2013	Rolling Stock Procurement



## Table of Contents

<b>23</b>	<b>CORROSION CONTROL</b>	<b>23-1</b>
23.1	Scope	23-1
23.2	Regulations, Codes, Standards, and Guidelines	23-1
23.3	Soil and Water Corrosion Control	23-2
23.3.1	General	23-2
23.3.2	Coating	23-4
23.3.3	Electrical Insulation of Piping	23-4
23.3.4	Electrical Continuity of Metallic Structures	23-5
23.3.5	Cathodic Protection	23-5
23.3.6	Ferrous Pressure Piping	23-6
23.3.7	Copper Piping	23-6
23.3.8	Gravity Flow Piping (Non-Pressured)	23-7
23.4	Atmospheric Corrosion Control	23-8
23.4.1	Coating Systems	23-8
23.4.2	Metallic-Sacrificial Coatings	23-9
23.4.3	Organic Coatings	23-9
23.4.4	Barrier Coating System	23-9
23.5	Graffiti-Resistant Coatings	23-10
23.6	Stray Current Sources and Protection	23-10
23.6.1	Cast-in-Place Reinforced Concrete Retaining Walls	23-11
23.6.2	Mechanically Stabilized Earth Walls	23-11

## Acronyms

ac	Alternating Current
AWWA	American Water Works Association
dc	Direct Current
MSE	Mechanically Stabilized Earth
PVC	Polyvinyl Chloride
SSPC	Society of Protective Coating

## 23 Corrosion Control

### 23.1 Scope

This chapter describes the requirements for a corrosion control system design that shall prevent premature corrosion failure, and be economical to install, operate, and maintain. Corrosion control provisions shall be required for all facilities, regardless of location or material of construction subject to corrosion, where failure would affect safety or interrupt continuity of operations.

Two major types of corrosion control shall be implemented: soil and water corrosion control, and atmospheric corrosion control.

The design criteria for each of these categories, and their implementation, shall meet the design life of structures and systems listed in the *General* chapter:

- Minimize annual operating and maintenance costs associated with material deterioration
- Provide continuity of operations by reducing or eliminating corrosion related failures

### 23.2 Regulations, Codes, Standards, and Guidelines

Refer to the *General* chapter for requirements pertaining to regulations, codes, standards, and guidelines. Refer to the regulations, codes, standards, and/or guidelines regarding corrosion control published by the following organizations:

- American Concrete Institute (ACI)
- American National Standards Institute (ANSI)
- American Railway Engineering and Maintenance-of-Way Association (AREMA)
- American Society of Mechanical Engineers (ASME)
- American Society for Testing and Materials International (ASTM)
- American Water Works Association (AWWA)
- California Department of Transportation (Caltrans)
- Concrete Reinforcing Steel Institute (CRSI)
- Electronic Industries Association (EIA)
- Environmental Protection Agency (EPA)
- European Standards (EN for European Norms)
- Federal Highway Administration (FHWA)

- 1 • Insulated Cable Engineers Associated (ICEA)
- 2 • International Electrotechnical Commission (IEC)
- 3 • Institute of Electrical and Electronics Engineers (IEEE)
- 4 • International Organization for Standardization (ISO)
- 5 • National Association of Corrosion Engineers International (NACE)
- 6 • National Electrical Manufacturers Association (NEMA)
- 7 • National Fire Protection Association (NFPA)
- 8 • Occupational Safety and Health Administration (OSHA)
- 9 • State of California Codes and Code of Regulations
- 10 • Society for Protective Coatings (SSPC)
- 11 • Underwriters' Laboratories (UL)

## **23.3 Soil and Water Corrosion Control**

### **23.3.1 General**

12 The goal of this design is to prevent corrosion of structures due to soil and water. Soil and  
13 ground water characteristics shall be determined and documented through boring surveys.  
14 Analysis of the data obtained from on-site borings shall be the basis for corrosion control  
15 designs. The soil/water samples shall be analyzed for resistivity (or conductivity), pH, chloride  
16 and sulfate ion concentrations (refer to the *Geotechnical* chapter). Corrosion survey on site,  
17 testing soil resistivity, pH, redox potential, and stray current presence shall supplement  
18 water/soil samples laboratory analysis.

19 During the pre-design and design phases of the project, the corrosion control design shall  
20 identify unique and special design cases such as existing building foundations, abandoned  
21 foundations, paralleling power lines, stray currents, unusual soil conditions and other  
22 conditions that may be revealed during the baseline soil surveys, boring program and stray  
23 current surveys.

24 In these cases, the corrosion control design shall evaluate and recommend special design  
25 measures as appropriate.

26 Normally, affected structures, as in the conditions listed below, shall be protected against the  
27 environment by coating, insulation, electrical continuity and/or cathodic protection, whichever  
28 is applicable. Water treatment shall be considered to mitigate chemical and microbiologically  
29 influenced corrosion inside of pipes and water tanks. Special attention shall be taken to prevent  
30 salt water corrosion on underground metallic structures.



Structures which may be affected by soil and water corrosion typically include, but are not limited to the following:

- Buried and on-grade reinforced concrete structures
- Pier and piling structures
- Concrete tunnel liner
- Ferrous piping (gas, water, sewage, casings, etc.)
- Underground storage tanks
- Electrical conduits
- Copper piping
- Metallic fencing

#### **23.3.1.1 Materials of Construction**

All pressure and non-pressure piping and conduit shall be non-metallic, unless metallic materials are required for specific engineering purposes.

Aluminum and aluminum alloys shall not be used in direct burial applications.

Dissimilar metals shall not be in contact with each other thereby avoiding bi-metallic corrosion.

If non-native fill is to be used for backfilling concrete or ferrous structures, then it shall meet the following criteria:

- pH 6 to 8
- Maximum chloride ion concentration of 250 ppm
- Maximum sulfate ion concentration of 200 ppm

Methods to control corrosion of reinforced concrete structures shall be directed toward preventing or impeding ingress of water and chloride ions into concrete and/or protecting steel rebar after chloride contamination occurs. Based on ACI guidelines, methods in use are the following:

- Control of the concrete mix to obtain low permeable concrete
- Limit chlorides in the original concrete mix ingredients
- Use an inhibitor in the concrete mix
- Increase concrete cover to reinforcement
- Use concrete sealers and coatings
- Use epoxy coated reinforcement
- Apply cathodic protection

### 23.3.1.2 Safety and Continuity of Operations

- 1 Corrosion control protection shall be required for those facilities where failure caused by  
2 corrosion of such facilities may affect the safety, or interrupt the continuity of operations.

### 23.3.1.3 Accessibility of Installations

- 3 Where required, permanent test facilities installed with certain corrosion control provisions  
4 shall be accessible after installation, allowing for periodic maintenance and monitoring.

## 23.3.2 Coating

- 5 Coatings specified for corrosion control of buried metallic or concrete facilities shall satisfy the  
6 following criteria:

- 7 • Minimum volume resistivity of 10,000,000,000 ohm-centimeters (10 billion ohm-centimeters)
- 8 • Minimum thickness as recommended for the specific system, but not less than 15 mils
- 9 • A chemical or mechanical bond to the metal or concrete surface. Pressure-sensitive systems  
10 are not acceptable.
- 11 • Minimum 5-year performance record for the intended service
- 12 • Shop application wherever possible, with field application of a compatible paint or tape  
13 system
- 14 • Mechanical characteristics capable of withstanding coating damage during handling and  
15 earth pressure after installation for the design life of the system

### 23.3.2.1 Generic Coating Systems

- 16 Generic coating systems include but are not limited to the following:

- 17 • Extruded polyethylene/butyl based system
- 18 • Coal-tar epoxies (2 component systems)
- 19 • Polyethylene-backed butyl mastic tapes (cold applied)
- 20 • Bituminous mastics (airless spray)

## 23.3.3 Electrical Insulation of Piping

- 21 Devices used as electrical insulators for corrosion control shall include non-metallic inserts,  
22 insulating flanges, couplings, unions, and/or concentric support spacers. Devices shall meet the  
23 following criteria:

- 24 • A minimum resistance of 10 megohms prior to installation.
- 25 • Sufficient electrical resistance after insertion into the operating piping system such that no  
26 more than 2 percent of a test current applied across the device flows through the insulator,  
27 including flow through conductive fluids if present.

- Mechanical and temperature ratings equivalent to the structure in which they are installed.
- Internal coating (except complete non-metallic units) with a polyamide epoxy for a distance on each side of the insulator equal to 2 times the diameter of the pipe in which they are used. Where conductive fluids with a resistivity of less than 2,000 ohm-centimeters are present, internal coating requirements shall be based on separate evaluation.
- Devices (except non-metallic units) buried in soils shall be encased in a protective coating.
- Devices (except non-metallic units) installed in chambers or otherwise exposed to partial immersion or high humidity shall have a protective coating applied over all components.
- Inaccessible insulating devices, such as buried or elevated insulators, shall be equipped with accessible permanent test facilities.
- A minimum clearance of 12 inches shall be provided between new and existing metallic structures. When conditions do not allow a 12-inch clearance, the design shall include special provisions to prevent electrical contact with existing structure(s).

#### **23.3.4 Electrical Continuity of Metallic Structures**

Electrical continuity shall be provided for all non-welded metallic joints and shall meet the following criteria:

- Use direct burial, insulated, stranded, copper wire with the minimum length necessary to span the joint being bonded.
- Wire size shall be based on the electrical characteristics of the structure and resulting electrical network to minimize attenuation and allow for cathodic protection.
- Use a minimum of 2 wires per joint for redundancy.

#### **23.3.5 Cathodic Protection**

Cathodic protection shall be accomplished by sacrificial galvanic anodes to minimize corrosion interaction with other underground utilities. Impressed current systems shall be used only when the use of sacrificial systems is not technically and/or economically feasible.

Cathodic protection system design shall be based on theoretical calculations that include the following parameters:

- Estimated percentage of bare surface area (minimum 1 percent)
- Cathodic protection current density (minimum of 1.0 mA/sq.ft. of bare surface area)
- Estimated current output per anode
- Estimated total number of anodes, size, and spacing
- Minimum anode life of 25 years (minimum 50 percent efficiency)
- Estimated anode groundbed resistance

1 Impressed current rectifier systems shall be designed using variable voltage and current output  
2 rectifiers. Rectifiers shall be rated at a minimum of 50 percent above calculated operating levels  
3 to overcome a higher-than-anticipated anode groundbed resistance, lower-than-anticipated  
4 coating resistance, or presence of interference mitigation bonds. Other conditions which may  
5 result in increased voltage and current requirements shall be considered.

6 Test facilities consisting of a minimum of 2 structure connections, 1 reference electrode  
7 connection, conduits and termination boxes shall be designed to permit initial and periodic  
8 testing of cathodic protection levels, interference currents, and system components (anodes,  
9 insulating devices, and continuity bonds). By request of the particular utility owner/operator,  
10 remotely monitored test facilities should be designed. The Designer shall specify the locations  
11 and types of test facilities for each cathodic protection system.

### **23.3.6 Ferrous Pressure Piping**

12 All new buried cast iron, ductile iron, and steel pressure piping shall be cathodically protected.  
13 System design shall satisfy the following minimum criteria:

- 14 • Application of a protective coating to the external surface of the pipe (refer to Section 23.3.2).
- 15 • Electrical insulation of pipe from interconnecting pipe, other structures and segregation into  
16 discrete electrically isolated sections depending upon the total length of piping (refer to  
17 Section 23.3.3).
- 18 • Electrical continuity through the installation of copper wires across all mechanical pipe  
19 joints other than intended insulators (refer to Section 23.3.4).
- 20 • Permanent test/access facilities to allow for verification of electrical continuity, electrical  
21 effectiveness of insulators and coating, and evaluation of cathodic protection levels,  
22 installed at all insulated connections. Additional test/access facilities shall be installed at  
23 intermediate locations, either at intervals not greater than 200 feet, or at greater intervals  
24 determined on an individual structure basis.
- 25 • Number and location of anodes and size of rectifier (if required) shall be determined on an  
26 individual structure basis.

### **23.3.7 Copper Piping**

27 Buried copper pipe shall be electrically isolated from non-buried piping, such as that contained  
28 in a station structure, through use of an accessible insulating union installed where the piping  
29 enters through a wall or floor. Pipe penetrations through the walls and floors shall be  
30 electrically isolated from building structural elements. The insulator shall be located inside the  
31 structure and not buried.

### 23.3.8 Gravity Flow Piping (Non-Pressured)

#### 23.3.8.1 Corrugated Steel Piping

- 1 Corrugated steel piping shall be internally and externally coated with a sacrificial metallic  
2 coating and a protective organic coating.

#### 23.3.8.2 Cast or Ductile Iron Piping

- 3 Cast or ductile iron piping shall be designed and fabricated to include the following provisions:
- 4 • An internal mortar lining with a bituminous coating on ductile iron pipe only (not required  
5 for cast iron soil pipe)
  - 6 • A bonded protective coating or unbonded dielectric encasement on the external surfaces in  
7 contact with soils (AWWA Standard C105)
  - 8 • A bituminous mastic coating on the external surfaces of pipe 6 inches on each side of a  
9 concrete/soil interface

#### 23.3.8.3 Casings

- 10 Pipeline casings, if required, shall be installed bare, unless coating and cathodic protection is  
11 required by the owner. Casing isolators and spacers shall be installed on the carrier pipe to  
12 avoid electrical contact between the casing and the carrier pipe. Test leads are required for the  
13 carrier pipe and at each end of the casing.

#### 23.3.8.4 Reinforced Concrete Non-Pressure Piping

- 14 Reinforced concrete non-pressure piping shall include the following provisions:
- 15 • Water/cement ratios meeting the minimum provisions of AWWA
  - 16 • Maximum 250 ppm chloride ion concentration in the total concrete mix (mixing water,  
17 cement, admixture, and aggregates)

#### 23.3.8.5 Electrical Conduits

- 18 Buried metallic conduits shall include the following provisions:
- 19 • Galvanized steel with Polyvinyl chloride (PVC) or other coating acceptable for direct burial,  
20 including couplings and fittings. The PVC coating is not required when conduits are  
21 installed in concrete.
  - 22 • Electrical continuity through use of standard threaded joints or bond wires installed across  
23 non-threaded joints.

#### 23.3.8.6 Utility Structures

- 24 All piping and conduit shall be non-metallic, unless metallic facilities are required for specific  
25 engineering purposes. There are no special provisions required if non-metallic materials are  
26 used.

### 23.3.8.7 Metallic Facilities (System-wide)

- 1 Pressure or non-pressure piping exposed within tunnels or crawl spaces or embedded in
- 2 concrete inverts shall not require special provisions.
- 3 Pressure piping that penetrates tunnel foundation, or tunnel walls shall be electrically insulated
- 4 from the external piping to which it connects and from watertight wall sleeves. Electrical
- 5 insulation of interior piping from external piping shall be made on the inside of the tunnel.
- 6 Pressure piping running on the top of the bridge or aerial structure shall be electrically isolated
- 7 from the underground portion of the piping.
- 8 All buried pressurized ferrous piping shall meet the criteria specified in Section 23.3.6.

## 23.4 Atmospheric Corrosion Control

- 9 Alternating wet and dry weather together with industrial and chemical pollutants contributes
- 10 to increased corrosion rates of exposed metal structures and hardware. The atmospheric
- 11 corrosion conditions shall be derived from different existing sources (environmental reports, air
- 12 pollution reports, climatological reports). Designs and associated coatings shall be based on
- 13 recommendations of the reports and shall be used to significantly decrease atmospheric
- 14 corrosion rates.
- 15 The purpose of these criteria is to insure the function, preservation and appearance of project
- 16 structures exposed to the environment in the most cost effective manner. Criteria include the
- 17 following:
- 18 • **Materials selection** – acceptable materials shall have proven past performance records for
- 19 the service application.
- 20 • **Protective coatings** – barrier or sacrificial coatings shall be used on steel.
- 21 • **Design** – recess moisture traps and dissimilar metals shall be avoided.
- 22 • **Sealants** – accumulation of moisture in crevices shall be prevented by use of sealants.

### 23.4.1 Coating Systems

- 23 Exposed metal surfaces on aerial and tunnel structures affected by atmospheric corrosion shall
- 24 be coated. Marine atmosphere shall be considered along the track alignment to develop coating
- 25 systems. Coating for the catenary poles shall be developed based on local atmospheric
- 26 conditions.
- 27 Coatings shall have established performance records for the intended service and be compatible
- 28 with the base metal to which they are applied.
- 29 Coatings shall be able to demonstrate satisfactory gloss retention, color retention, and resistance
- 30 to chalking over their minimum life expectancies.

Coatings shall have minimum life expectancies, defined as the time prior to major maintenance or reapplication, of 15 to 20 years.

#### 23.4.2 Metallic-Sacrificial Coatings

Acceptable coatings for carbon and alloy steels for use in tunnels, crawlspace, vaults, or above grade are as follows:

- Zinc (hot-dip galvanizing [2 ounces per square feet] or flame sprayed)
- Aluminum (hot-dip galvanizing [2 mil thickness] or flame sprayed)
- Aluminum-zinc
- Inorganic or organic zinc (as a primer)

#### 23.4.3 Organic Coatings

Organic coating systems shall consist of a wash primer (for galvanized and aluminum substrates only), a primer, intermediate coat(s), and a finish coat. Acceptable organic coatings, for exposure to the atmosphere, are as follows:

- Aliphatic polyurethanes
- Vinyl copolymers
- Fusion-bonded epoxy polyesters, polyethylenes, and nylons
- Acrylics, where not exposed to direct sunlight
- Alkyds, where not exposed to direct sunlight
- Epoxy as a primer, where exposed to the atmosphere, or as the complete system, where sheltered from sunlight

#### 23.4.4 Barrier Coating System

Use 1 of the following barrier coating systems where corrosion protection is needed but appearance is not a primary concern:

- Near white blast surface according to SSPC-SP 10. Follow with a 3-coat epoxy system.
- Commercial blast surface according to SSPC-SP 6. Follow with a 2-coat inorganic zinc and high build epoxy system.
- Near white blast surface according to SSPC-SP 10. Follow with a 3-coat epoxy zinc, high build epoxy system.
- Apply all coatings according to manufacturer's specifications.

Use 1 of the following barrier coating systems where corrosion protection and good appearance are needed:

- 1 • Near white blast surface according to SSPC-SP 10. Follow with a 3-coat inorganic zinc, high  
2 build epoxy, polyester urethane system.
- 3 • Near white blast surface according to SSPC-SP 10. Follow with a 3-coat vinyl system.
- 4 • Commercial blast surface according to SSPC-SP 6. Follow with a 3-coat epoxy zinc, high  
5 build epoxy and polyester urethane system.
- 6 • Commercial blast surface according to SSPC-SP 6. Follow with a 3-coat epoxy zinc, high  
7 build epoxy and acrylic urethane system.
- 8 • Apply all coatings according to manufacturer's specifications.

### **23.5 Graffiti-Resistant Coatings**

9 Surfaces that are accessible to graffiti shall be protected with a graffiti-resistant coating. This  
10 includes concrete and painted steel surfaces such as walls, columns, and equipment enclosures.  
11 All such areas shall be protected up to a height of 10 feet. The coating shall be a urethane-type  
12 coating and shall be applied in accordance with the manufacturer's latest published  
13 instructions.

### **23.6 Stray Current Sources and Protection**

14 Direct Current (dc)-powered transit systems like BART, LA Metro, VTA, and LRT in San Diego  
15 could be a source of stray currents for metallic and concrete underground facilities. Welding  
16 operations in the industrial areas could be another source of stray currents. Stray current  
17 corrosion survey in the vicinity of dc-powered transit lines and in industrial portions of project  
18 proposed track alignment shall identify areas with stray current activity. Local utility  
19 companies shall be contacted to obtain information regarding their practice of stray current  
20 protection in areas where dc stray currents are present. Alternating Current (ac) influence from  
21 a 2x25 kV traction power system can cause excessive voltage (over 15 volts) on neighboring  
22 utilities. Induced ac stray current can also cause corrosion of metallic underground structures.  
23 Personal electrical safety and corrosion control measures shall be considered in areas where  
24 metallic utility structures run parallel to or approach the project tracks. Refer to the *Grounding*  
25 *and Bonding Requirements* chapter for safety requirements. Test stations to periodically monitor  
26 ac voltage on these utilities shall be designed and installed in accordance with NACE SP 0177.

27 Corrosion control requirements for buried utilities installed by the utility owner/operator as  
28 part of high-speed train construction shall be the responsibility of the individual utility  
29 owner/operator. Relocated or replaced utilities, installed by high-speed train contractors as part  
30 of a contractual agreement between the Authority and the utility, shall be installed in  
31 accordance with the utility owner specifications and shall include the following minimum  
32 provisions:



- 1 • Electrical continuity through the installation of insulated copper wires across all mechanical
- 2 joints for which electrical continuity cannot be assured
- 3 • Electrical access to the utility structure via test facilities installed at nominal 200 feet
- 4 intervals

5 These provisions are applicable to ferrous and reinforced concrete cylinder piping. Other

6 materials and structures will require individual review.

7 The need for additional measures, such as electrical isolation, application of a protective coating

8 system, installation of cathodic protection, or any combination of the preceding, shall be based

9 on the characteristics of the specific structure and shall not adversely affect the existing

10 performance within the environment.

### 23.6.1 Cast-in-Place Reinforced Concrete Retaining Walls

11 Where cast-in place retaining walls are parallel or adjacent to a dc-powered transit system at a

12 separation of less than 100 feet, corrosion control measures against stray current corrosion to the

13 retaining wall shall be applied. These measures include the following:

- 14 • All longitudinal bar overlaps in both faces of the wall, including the top and bottom bars of
- 15 the footing, shall be tack welded to ensure electrical continuity.
- 16 • Longitudinal bars in the footing shall be made electrically continuous to the longitudinal
- 17 bars of the walls.
- 18 • Test facilities to monitor stray current level shall be installed at each end of the wall and at
- 19 spacing not to exceed 300 feet.

### 23.6.2 Mechanically Stabilized Earth Walls

20 Designer shall determine Mechanically Stabilized Earth (MSE) wall reinforcement material. If

21 structurally permissible, non-metallic reinforcing material can be used for MSE walls instead of

22 metallic reinforcing material.

23 Electrical continuity is not required between the embedded steel reinforcement of the face

24 panels.

25 Steel reinforcing strips of adjacent face panels shall not be electrically interconnected. Steel

26 reinforcing strips shall be coated for corrosion protection. Coating is not required for non-

27 metallic soil reinforcing material.

28 Tie-strips shall be coated for corrosion protection prior to face panel construction.

29 Designer shall provide the materials and coating thickness to meet the design life of the

30 structure as specified in the *General* chapter.

When MSE walls are parallel or adjacent to dc-powered transit system at a separation of less than 100 feet, AASHTO recommends that a corrosion expert evaluate the hazard and possible mitigating features on a project-specific basis. Furthermore, it is recommended that a long-term corrosion monitoring program be integrated into the design.

Structural backfill for retaining walls using steel reinforcing strips shall be a granular (well-drained) and non-corrosive select backfill material. Laboratory analysis shall demonstrate that the backfill meets the following requirements:

1. Chemical constituents Chlorides - 100 ppm (maximum)
  - Sulfates - 200 ppm (maximum)
2. Saturated Resistivity of Backfill
  - Not less than 3,000 ohm-cm.
3. pH acceptable range 6 to 8
  - Design and construction of MSE walls shall meet the requirements of the FHWA Publication No. FHWA-NHI-09-087

## Chapter 24

### Automatic Train Control

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	Oct 2013	Rolling Stock Procurement
2	Feb 2014	Revision 2



## Table of Contents

<b>24</b>	<b>AUTOMATIC TRAIN CONTROL.....</b>	<b>24-1</b>
24.1	Scope .....	24-1
24.2	Regulations, Codes, Standards, and Guidelines .....	24-1
24.3	General Design Requirements .....	24-3
24.3.1	Design Speeds .....	24-5
24.3.2	Design Life .....	24-5
24.3.3	Headway .....	24-5
24.3.4	Product Selection .....	24-5
24.3.5	Uniformity .....	24-5
24.3.6	Environmental Conditions .....	24-6
24.3.7	Electromagnetic Compatibility .....	24-6
24.3.8	Reliability, Availability, and Maintainability .....	24-6
24.3.9	Safety Critical System Requirements.....	24-8
24.3.10	Safe Braking.....	24-9
24.3.11	Speed Limits.....	24-11
24.3.12	Hardware Requirements .....	24-11
24.3.13	Software Requirements.....	24-13
24.4	Related Documentation .....	24-14
24.5	Automatic Train Control Functional Requirements .....	24-14
24.5.1	On-board Functions .....	24-14
24.5.2	Wayside Automatic Train Control Functions .....	24-17
24.5.3	Automatic Train Supervision Functions .....	24-18
24.6	Grounding, Bonding, and Lightning Protection System .....	24-20
24.7	On-board Equipment.....	24-20
24.7.1	Display Unit .....	24-20
24.7.2	Data Radio Display.....	24-20
24.7.3	Speed and Location Determination.....	24-20
24.7.4	Antennas .....	24-21
24.7.5	Electronics.....	24-21
24.7.6	Interfaces with Rolling Stock Subsystems .....	24-21
24.7.7	Event Recording.....	24-23
24.8	Wayside Equipment .....	24-23
24.8.1	Signals.....	24-23
24.8.2	Wayside Signs.....	24-24
24.8.3	Track Circuits .....	24-24
24.8.4	Interlockings .....	24-25
24.8.5	Local Control Panels .....	24-26
24.8.6	Impedance Bonds .....	24-26
24.8.7	Switch Machines .....	24-26
24.8.8	Equipment Enclosures .....	24-26
24.8.9	Insulated Joints .....	24-27
24.8.10	Cables, Cable Trough, and Conduits .....	24-27
24.8.11	Radio Block Control Centers.....	24-28
24.8.12	Transponders .....	24-29
24.8.13	Signal Power .....	24-29

24.8.14	Event Recording.....	24-29
24.8.15	Wayside Interfaces.....	24-29
24.8.16	Highway Grade Crossings .....	24-30
24.9	System Time.....	24-30
24.10	Maintenance and Diagnostic Functions .....	24-30
24.11	Automatic Train Supervision Subsystem .....	24-31
24.11.1	Automatic Train Supervision Functions .....	24-31
24.11.2	Automatic Train Supervision Hardware (Office) Requirements .....	24-32
24.11.3	Automatic Train Supervision Interfaces with Other Systems.....	24-32
24.11.4	Automatic Train Supervision Power .....	24-32

## Figures

Figure 24-1:	Safe Braking for Safe Train Separation Principles.....	24-10
Figure 24-2:	Safe Braking for Curve Speed Enforcement Principles.....	24-11

## Acronyms

AREMA C&S	American Railway Engineering and Maintenance-of-Way Association Communications & Signals
ATC	Automatic Train Control
ATO	Automatic Train Operation
ATP	Automatic Train Protection
ATS	Automatic Train Supervision
CFR	Code of Federal Regulations
EMU	Electric Multiple Unit
ERA	European Railway Agency
ERTMS	European Rail Traffic Management System
FFFIS	Form Fit Function Interface Specification
FMEA	Failure Mode and Effect Analysis
HMI	Human Machine Interface
ID	Identification
IEEE	Institute of Electrical and Electronics Engineers
MA	Movement Authority
MOI	Maintenance of Infrastructure
MTBF	Mean Time Between Failures
MTTR	Mean Time to Restore
MTTT	Mean Time To Travel
OCC	Operation Control Center
OCS	Overhead Contact System
OEM	Original Equipment Manufacturer
POSIX	Portable Operating System Interface
PTC	Positive Train Control
RBC	Radio Block Centers
RAM	Reliability, Availability, Maintainability
RAMS	Reliability, Availability, Maintainability, and Safety
RCC	Regional Control Center
RS	Rolling Stock
SBD	Safe Braking Design
SCADA	Supervisory Control and Data Acquisition
SIL	Safety Integrity Level
UNISIG	Union Industry of Signalling (UNISIG)

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015





## 24 Automatic Train Control

### 24.1 Scope

The Automatic Train Control (ATC) system and its subsystems shall meet the requirements of the standard specifications and preliminary engineering drawings and all applicable regulatory and interoperability requirements. The ATC system shall meet all of the applicable portions of the Code of Federal Regulations (CFR): 49 CFR 236 except as modified by the Guidance Document of Required Safety Elements Necessary for FRA Regulatory Approval. The ATC system shall meet all applicable requirements of all modifications to the CFR. The ATC system shall provide all specified functions and shall consist of the equipment required to provide safety-critical Automatic Train Protection (ATP), Automatic Train Operation, (ATO), and Automatic Train Supervision (ATS) functions.

### 24.2 Regulations, Codes, Standards, and Guidelines

Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.

- Code of Federal Regulations (CFR); Title 49 CFR, Parts 229, 233 and 236
- American Railway Engineering and Maintenance-of-Way Association Communications & Signals (AREMA C&S) Manual of Recommended Practices
  - Part 17 – Recommended
- U.S. Department of Defense (USDOD) Standards
  - MIL-HDBK-217F – Military handbook – Reliability Prediction of Electronic Equipment
  - MIL-STD-785B – Reliability Program for System and Equipment Development and Production
  - MIL-STD-756B – Reliability Modeling and Prediction
  - MIL-STD-1629A – Procedures for Performing a Failure Mode, Effects and Criticality Analysis
- European Committee for Electrotechnical Standardization (CENELEC) Standards
  - EN 50121-4 – Railway applications – Electromagnetic compatibility Part 4: Emission and immunity of the signalling and telecommunications apparatus
  - EN 50125-1 – Railway applications – Environmental conditions for equipment – Part 1: Equipment onboard rolling stock
  - EN 50125-3 – Railway applications – Environmental conditions for equipment – Part 3: Equipment for signalling and telecommunications

- 1       – EN 50126-1 – Railway applications – The specification and demonstration of Reliability,  
2       Availability, Maintainability and Safety – Part 1: Basic requirements and generic process  
3       (1999)
- 4       – CLC/TR 50126-2 Railway applications – The specification and demonstration of  
5       Reliability, Availability, Maintainability and Safety (RAMS) – Part 2: Guide to the  
6       application of EN 50126-1 for safety (2007)
- 7       – EN 50128 – Railway applications – Communications, signalling and processing systems  
8       – Software for railway control and protection systems (2011)
- 9       – EN 50129 – Railway applications – Communications, signalling and processing systems  
10      – Safety related electronic systems for signalling (2003)
- 11      – EN 50159 – Railway applications – Communications, signalling and processing systems :  
12      Safety-related communication in transmission systems (2010)
- 13      • International Electrotechnical Commission (IEC) Standards
- 14      – IEC 60812 – Analysis techniques for system reliability – Procedures for failure mode and  
15      effect analysis (FMEA)
- 16      • Institute of Electrical and Electronics Engineers (IEEE) Standards
- 17      – IEEE 828-2005 - IEEE Standard for Software Configuration Management Plans.
- 18      – IEEE 1558 – IEEE Standard for Software Documentation for Rail Equipment and Systems
- 19      – ANSI/IEEE 1028 – IEEE Standard for Software Reviews – The Institute of Electrical and  
20      Electronics Engineers
- 21      – IEEE 1003 – IEEE Standard for Information Technology - Portable Operating System  
22      Interface (POSIX)
- 23      – IEEE 1008 – IEEE Standard for Software Unit Testing
- 24      – IEEE 1228 – IEEE Standard for Software Safety Plans
- 25      – IEEE 1698 – IEEE Guide for the Calculation of Braking Distances for Rail Transit  
26      Vehicles (2009)
- 27      • European Railway Agency (ERA)/ Union Industry of Signalling (UNISIG) Specifications
- 28      - ERA/UNISIG Subset 036 – FFFIS for Eurobalise
- 29      - ERA/UNISIG SUBSET 036 – Form Fit Function Interface Specification (FFFIS) for  
30      Eurobalise
- 31      - ERA/UNISIG Subset 037 – Euro Radio Functional Interface Specification (FIS)
- 32      - ERA/UNISIG SUBSET 039 – Functional Interface Specification for the RBC/RBC  
33      Handover
- 34      - ERA/UNISIG SUBSET 040 – Dimensioning and Engineering Rules

- ERA/UNISIG Subset 047 – Trackside-Train borne FIS
- ERA/UNISIG Subset 085 – Test specification for Eurobalise FFFIS
- ERA/UNISIG Subset 092-1 – ERTMS Euro Radio Conformance Requirements
- ERA/UNISIG Subset 092-2 – ERTMS Euro Radio Test cases Safety Layer
- ERA/UNISIG SUBSET 097 – Requirements for RBC-RBC Safe Communications Interface
- ERA/UNISIG SUBSET 098 – RBC-RBC Safe Communication Interface
- ERA/UNISIG SUBSET 099 – Test Specification for Safe Communication Interface
- ERA/UNISIG SUBSET A11T6001-12 – MORANE Radio Transmission FFFIS for Euro Radio
- EEIG 96S126 – ERTMS/ETCS RAMS Requirements Specification
- International Union of Railways (UIC) Codes
  - UIC 612 – 0 - Driver Machine Interfaces for EMU/DMU, Locomotives, and Driving Coaches – Functional and system requirements associated with harmonized Driver Machine Interfaces

### 24.3 General Design Requirements

The signaling and train control system shall be collectively known as the ATC system. ATC includes all the safety critical and non-safety critical functions of a train control system, and includes all Positive Train Control (PTC) functions.

Functionality of the ATC system shall include the following:

- Route setting and locking by means of interlocking functions
  - PTC functions enforcing speed limits, prevention of collisions between trains, compliance with route limits, and prevention of intrusion into work zones
  - Integrating with track-side signaling equipment including interlockings, wayside signals, and, track circuits
  - Interfacing to the Overhead Contact System (OCS) for the provision of electrical power to interlocking and other ATC facilities and equipment
  - Interfacing to the communications systems including radio and network to communicate between the ATC subsystems including wayside, central, and on-board
  - Integrating to Rolling Stock (RS) defect detectors (where provided) mounted at the wayside including hot axle bearings, dragging equipment, and high/wide load
  - Interfacing to on-board defect, event, and incident detectors including hot axle bearings

- Interfacing to wayside defect, event, and incident detectors (where provided) including, landslides, seismic activity, intrusion, excessive wind speed, flood, excessive rain, etc.
  - Communication with customer information systems including passenger information and public address systems providing real time train identification (ID) and schedule adherence data
  - Locating trains with sufficient accuracy and reliability so as to meet safety, reliability and operational performance criteria
  - Onboard – Enforcing Movement Authorities (MAs) including track description, speed limits (permanent and temporary), and implementing other commands assigned to specific trains
  - Providing secure and efficient communications paths between the ATC wayside and on-board subsystems for the following information:
    - Wayside Transponder (Eurobalise or similar) transmission to the train
    - Communicating safety critical information to trains from the wayside ATC subsystem, by radio or by coded track circuit, including MAs and temporary and permanent database updates
    - Communicating non-safety critical information to trains from the wayside ATC zone controller, by radio
    - Communicating from trains to the ATS subsystem to facilitate route setting and other supervision functions, alarm and event notification, schedule regulation, and subsystem maintenance functions
- Refer to the *Communications* chapter for the data communications design requirements including network and radio.
- The design of the ATC system shall provide for ease of maintenance including easy access to all equipment and effective diagnostics provided for maintainers at the local equipment and at remote locations.
- All enclosures and the equipment mounted within them, equipment mounted in rooms provided by others, and equipment mounted along the Authority's right-of-way shall be designed in accordance with the seismic considerations as defined in the *Seismic* chapter.
- Refer to the *Seismic* and *Structures* chapters for additional slipstream and seismic requirements for ATC equipment mounting.
- The secure mounting of enclosures and equipment along the right-of-way shall be accomplished in accordance with the *Structures* chapter.
- Final site preparation shall be designed to be compatible with the requirements for placing foundations for enclosures including houses and cases and for using the compound as a working site and in accordance with the *Civil* chapter.

These criteria allow either a radio-based or a coded track circuit-based ATC system.

If the Designer proposes an ERTMS Level 2 system, the air gap interfaces shall meet the following requirements:

- ERA/UNISIG Subset 037 – Euro Radio Functional Interface Specification (FIS)
- ERA/UNISIG Subset 047 – Trackside-Train borne FIS
- ERA/UNISIG Subset 092-1 – ERTMS Euro Radio Conformance Requirements
- ERA/UNISIG Subset 092-2 – ERTMS Euro Radio Test cases Safety Layer
- ERA/UNISIG SUBSET A11T6001-12 – MORANE Radio Transmission FFFIS for Euro Radio

Where Eurobalises are deployed, the air gap interfaces shall meet the following requirements:

- ERA/UNISIG Subset 036 – FFFIS for Eurobalise
- ERA/UNISIG Subset 085 – Test specification for Eurobalise FFFIS

Spare capacity – provision for spare capacity (scalability) shall be designed into all enclosure spaces, cable conductors (copper and fiber), and network and data communication channels.

#### **24.3.1 Design Speeds**

Refer to the *General* chapter for the maximum design and operating speeds.

#### **24.3.2 Design Life**

The ATC subsystems shall have useful lives as defined in the *General* chapter.

#### **24.3.3 Headway**

The design headway shall be 3 minutes between trains at maximum civil speed limits (based on passenger comfort speed limits).

#### **24.3.4 Product Selection**

Equipment and products (including software) shall have been proven in high-speed passenger rail service (at operating speeds consistent with the design objectives) for 5 years and shall be demonstrated to meet the Reliability, Availability, Maintainability, and Safety (RAMS) requirements in a similar high-speed rail application and in the range of climates similar to the California environment.

#### **24.3.5 Uniformity**

Equipment and products including firmware and software shall be uniform throughout the project subject to procurement strategies and contract limits so as to simplify maintenance

1 procedures and to minimize the spare and replacement parts holdings across the project to the  
2 greatest extent practical.

#### 24.3.6 Environmental Conditions

3 The climatic and physical environmental specifications and performance of ATC equipment  
4 installed at the wayside shall comply with AREMA C&S Manual of Recommended Practices  
5 and the relevant portions of EN 50125-3 – Railway applications – Environmental conditions for  
6 equipment – Part 3: Equipment for signaling and telecommunications. In the event that these  
7 requirements are in conflict, the more stringent and restrictive conditions shall apply.

8 The climatic and physical environmental specifications and performance of ATC equipment  
9 installed on-board passenger trains shall comply with the criteria defined in the Infrastructure  
10 Interface Specification section of the Next Generation Amtrak/CHSRA Trainsets Performance  
11 Specification..

12 ATC equipment installed on work trains shall comply with EN 50125-1 – Railway applications –  
13 Environmental conditions for equipment – Part 1: Equipment onboard rolling stock.

14 For other climatic conditions refer to the *General* chapter.

#### 24.3.7 Electromagnetic Compatibility

15 ATC equipment shall not interfere with or be interfered with by other equipment or equipment  
16 external to the Project. The ATC system design shall comply with the requirements of the  
17 *Electromagnetic Compatibility and Interference* chapter.

#### 24.3.8 Reliability, Availability, and Maintainability

18 The reliability of the ATC and its subsystems shall be consistent with the overall Reliability,  
19 Availability, and Maintainability (RAM) requirements. RAMS requirements shall be in  
20 accordance with EN 50126-1 Railway Applications – The specification and Demonstration of  
21 Reliability, Availability, Maintainability and Safety (RAMS). Other standards will be considered  
22 on the condition that the Designer provides a comprehensive equivalency report that maps the  
23 correlation between EN 50126-1 and their proposed standard(s).

24 ATC availability shall also be consistent with the availability requirements of the overall project  
25 high level thresholds. ATC system availability shall be higher than 99.99 percent. Failures shall  
26 be identified as Immobilizing, Service, and Minor; these terms are defined as follows:

- 27 • **Immobilizing Failure** – An Immobilizing Failure is one which occurs in the ATC system  
28 and fulfils any of the following:
  - 29 – It causes or could cause more than one train to be delayed longer than the threshold  
30 delay time.

- It causes more than one train to run in a more restrictive operating mode than would have otherwise been the case in its absence.
- **Service Failure** — A Service Failure is one which occurs in the ATC system, is caused by the ATC system, and fulfils any of the following:
  - It causes or could cause one train to be delayed longer than the threshold delay time.
  - It causes one train to run in a more restrictive operating mode than would have otherwise been the case in its absence.
- **Minor Failure** — A Minor Failure is one which occurs in the ATC System and is neither an Immobilizing Failure nor a Service Failure.

For assessment of the RAM quantified specifications, the target system shall be defined as one ATC equipped and supervised train plus all of the ATC system wayside and office equipment of the most complex track section encountered during a 1 hour trip at the speed of 220 miles per hour (mph).

The most complex track section is that part of the line which has the highest density of the ATC system elements.

The following conditions may be taken as a reference for the most complex (the worst case) application on the target system:

- |  |         |
|--|---------|
| • Trip duration                        | 1 hour  |
| • Train speed                          | 220 mph |
| • Transponder messages                 | 366     |
| • Radio messages                       | 850     |
| • Number of Radio Block Centers (RBCs) | 5       |
| • Number of switches                   | 72      |
| • Number of interlockings              | 20      |
| • Number of transponder locations      | 367     |
| • Number of transponders per location  | 2       |

The ATC target system shall achieve the following Mean Time Between Failures (MTBF) quantitative requirements:

- |                                   |                                |
|-----------------------------------|--------------------------------|
| • MTBF for Immobilizing Failures: | $MTBF_{Im} \geq 120000$ hours  |
| • MTBF for Service Failures:      | $MTBF_{Se} \geq 14000$ h hours |
| • MTBF for Minor Failures:        | $MTBF_{Mi} \geq 480$ hours     |

1 The Designer shall extrapolate all reliability calculations for the system under contract and  
2 demonstrate that it shall achieve in service achieve the extrapolated reliability requirements.

3 Human factors contributions to RAM shall be considered as follows:

- 4 • Mean Time To Travel (MTTT) will be a fixed value selected by the Authority, based on  
5 experience from other high-speed rail operators, reflecting the location of Maintenance of  
6 Infrastructure (MOI) facilities, and highway access to ATC wayside locations.
- 7 • The contractor shall demonstrate that a Mean Time to Restore (MTTR) of 30 minutes is  
8 achievable. MTTR is a mean value for all ATC Line Replaceable Units; it includes diagnosis  
9 of faults measured from the actual start of work on site, replacing of faulty components,  
10 testing, and placing the system back into service in the affected area. MTTR does not include  
11 MTTT.

#### **24.3.9 Safety Critical System Requirements**

12 The ATC system shall meet all safety requirements of 49 CFR 236, except as modified by the  
13 FRA Guideline documents.

14 Safety shall be demonstrated in accordance with EN 50126-1 – Railway Applications – The  
15 specification and Demonstration of Reliability, Availability, Maintainability and Safety (RAMS);  
16 Part 1: Basic requirements and generic process.

17 Other standards will be considered on the condition that the Designer provides a  
18 comprehensive equivalency report that maps between EN 50126-1 and other specified  
19 standards and their proposed standard(s).

20 The related application guide CLC/TR 50126-2 Railway applications – The specification and  
21 demonstration of Reliability, Availability, Maintainability and Safety (RAMS) – Part 2: Guide to  
22 the application of EN 50126-1 for safety (2007) shall also apply.

23 The design, construction, installation, acceptance, operation, maintenance, and  
24 modification/extension of the ATC system, including the development of hazard mitigation  
25 procedures and other means, shall provide a quantitative level of safety such that any single,  
26 independent hardware, software or communication failure, or any combination of such failures  
27 with the potential of causing death or severe injury to passengers or staff, shall not occur with a  
28 frequency greater than once per  $10^9$  system operating hours. This shall be documented and  
29 certified to Safety Integrity Level (SIL) 4 in accordance with the following:

- 30 • EN 50126 – Part 1, Railway applications – The specification and demonstration of Reliability,  
31 Availability, Maintainability and Safety (RAMS) – Part 1: Basic requirements and generic  
32 process (1999)
- 33 • EN 50128 – Railway applications – Communications, signalling and processing systems.  
34 Software for railway control and protection system (2011)



- EN 50129 – Railway applications – Communication, signalling and processing systems. Safety related electronic systems for signaling (2003)
  - EN 50159 – Railway applications – Communication, signalling and processing systems – Safety-related communication in transmission systems (2010)
- Tolerable Hazard Rates attributed to operations and maintenance actions shall be considered to be zero for the purpose of the ATC safety calculations.

#### 24.3.10 Safe Braking

The ATC system shall implement a Safe Braking Design (SBD) for each type of train that can operate on the system that is equipped with an on-board ATC subsystem and is subject to ATC enforcement. The SBD shall be the set of design provisions and procedures which together ensure that a train's ATP safe stopping distance is safe in normal situations and in all likely combinations of adverse factors and failure conditions. The ATC SBD shall cover passenger trains consisting of 1 or 2 coupled Electric Multiple Unit (EMU) trainsets, and work trains of variable consists that are equipped with ATC. Non-ATC-equipped work trains and failed-equipped trains shall operate under rules and procedures and in compliance with wayside signal aspects to mitigate hazards. The wayside signal design shall be compatible with the SBD for manually operated unequipped and failed trains.

The ATC system requires guaranteed train braking performance that includes an Assured Emergency Brake Rate (AEBR) with a specified level of worst case wheel-rail adhesion. The Braking section of the Next Generation Amtrak/CHSRA Trainsets Performance Specification defines the minimum braking performance levels. Coordination is required with RS engineering to confirm the braking performance of passenger vehicles including brake rates, build up times, variation in rate application versus time and train speed, spin-slide control parameters, and an agreed level of degradation of braking effort due to failure(s) within the braking system.

MOI Engineering shall confirm the braking performance of maintenance (non-passenger) rolling stock including the same values for passenger rolling stock as described earlier in this section.

Guaranteed braking performance shall be determined for both passenger trains and maintenance vehicles that are equipped with the on-board ATC subsystem. Acceleration and other functional elements of the rolling stock shall be taken into consideration in the SBD which shall include the following characteristics:

- Train operates at just below the intervention speed until it experiences an acceleration failure (at maximum acceleration) at the same time as the start of the service braking curve should begin for the obstruction ahead.
- Acceleration is cut off after a defined time following train speed exceeding the intervention speed limit; the train coasts at a constant speed for a period defined by the reaction time to

1 apply the emergency brake. Allowances shall also be made for the build-up of full  
2 emergency braking effort.

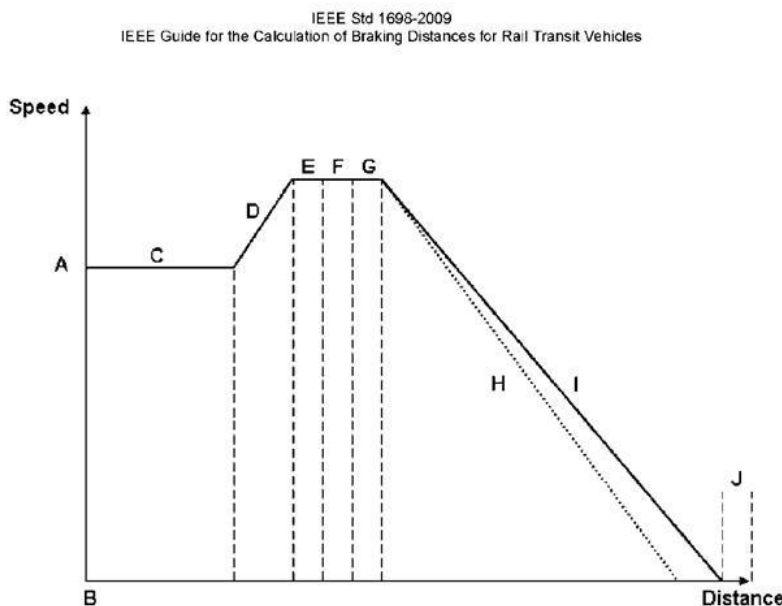
- 3 • In the event that ATC determines that the train is not braking within the calculated  
4 parameters, an emergency brake application shall be commanded. The train shall apply  
5 braking at the emergency rate including friction and electric braking, and wheel slide  
6 protection shall be active; the ATC design shall assume that only friction braking and wheel  
7 slide protection is applied.

8 Allowances shall be made for gradient, overhang (distance between outer end and the first  
9 axle), location error of the on-board ATC subsystem including sensors, speed measurement  
10 error of the on-board ATC subsystem including sensors, and all latencies and reaction delay  
11 times within the relevant ATC and RS subsystems.

12 Management of the safety critical interface between ATC and RS is defined in detail by the  
13 Safety and Security Management Plan (SSMP). The SBD shall be maintained throughout the  
14 design process for both ATC and RS.

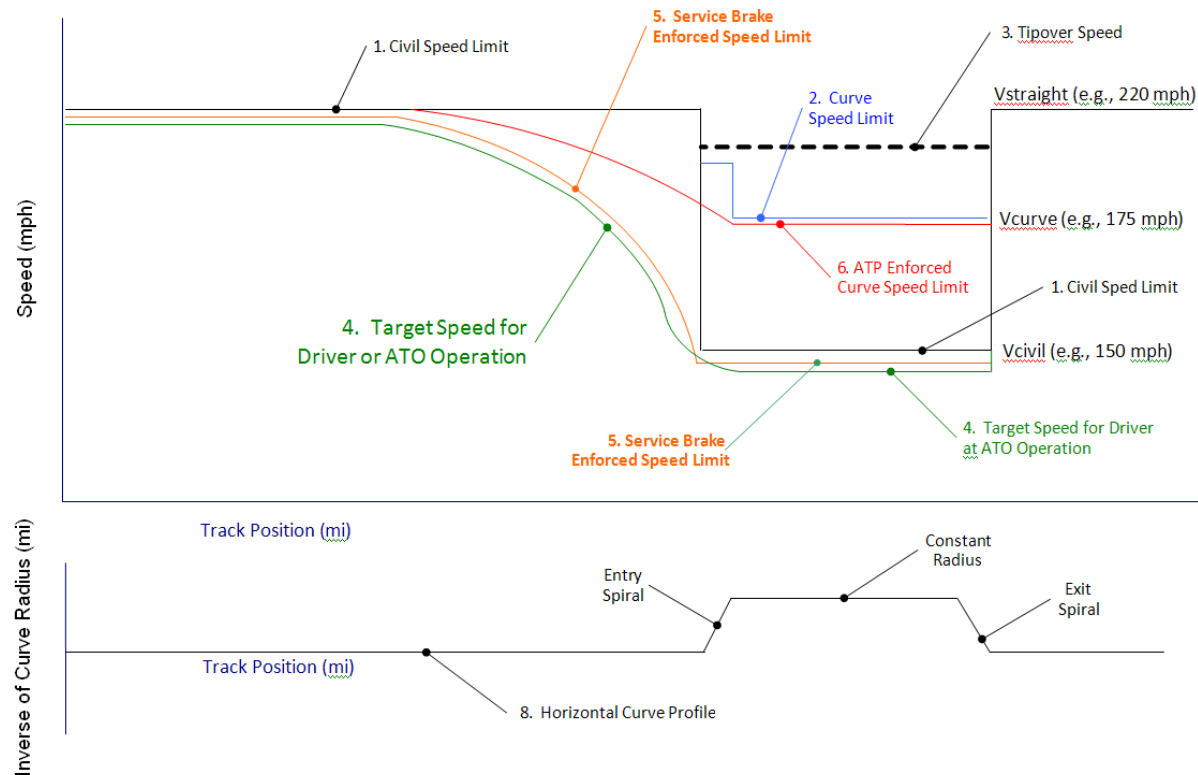
15 The safe braking principles of train separation shall be consistent with IEEE 1698 – IEEE Guide  
16 for the Calculation of Braking Distances for Rail Transit Vehicles (2009), Figure 24-1, and for  
17 speed limits on curves with Figure 24-2.

#### 18 **Figure 24-1: Safe Braking for Safe Train Separation Principles**



19 **Figure 1—Braking model**  
20

**Figure 24-2: Safe Braking for Curve Speed Enforcement Principles**



### 24.3.11 Speed Limits

Speed limits on horizontal curves will be provided to the ATC Designer in the following formats:

- Comfort Limits: speed calculated at 3 inches superelevation imbalance
- Not-to-exceed Limits: speeds calculated at 6 inches superelevation imbalance

Speed limits on other track sections will be provided as civil speed limits.

ATC shall enforce the civil speed limits and not-to-exceed limits in a vital manner for protecting against obstructions including trains ahead, a switch left in the wrong position or an unlocked switch, and established work zone limits. ATC may enforce comfort speed limits in a non-vital manner when enforcing speeds for curves and turnouts. Where curves and obstructions coincide in the limits of a train's MA, ATC shall enforce speeds in the more restrictive manner.

### 24.3.12 Hardware Requirements

Hardware shall be developed under a comprehensive program that ensures that safety-related maintainable hardware will be delivered. The program shall be a documented, certified SIL 4 or equivalent system for vital subsystem hardware.

1 The design shall fulfill the specified operational and safety requirements.

2 Hardware shall comply with appropriate sections from the following codes and standards:

- 3 • EN 50129 – Railway applications – Communications, signalling and processing systems –  
4 Safety related electronic systems for signaling (2003)
- 5 • EN 50159 – Railway applications – Communications, signalling and processing systems –  
6 Safety-related communication in transmission systems (2001)

7 ATC equipment shall be located in houses and cases that shall be designed and supplied as part  
8 of this system contract and also at the Operation Control Center (OCC) and Regional Control  
9 Center (RCC) equipment and control rooms, and offices that will be designed and built by  
10 others. Enclosures in the open shall comply with National Electrical Manufacturer's Association  
11 (NEMA) 4 standard.

12 Equipment on the wayside shall be located in houses and wayside cases that shall be located  
13 along the right-of-way. Space provision for multiple houses has been made at all interlocking  
14 sites. Additional space for ATC equipment has been provided for in each station  
15 communications and ATC equipment room. The actual equipment houses, foundations and  
16 connections to utilities, power, and cable trough interface points shall be designed in detail and  
17 provided by the ATC Designer.

18 Equipment on-board high-speed passenger and work trains shall be installed in purpose  
19 provided lockers on the car body, in antenna locations on or close to the train roof, externally  
20 mounted beneath the car body, and externally mounted on the sprung and unsprung  
21 components of the trucks. In addition to the EN standards cited above, on-board hardware shall  
22 also comply with the requirements of the Infrastructure Interface Specification section of the  
23 Next Generation Amtrak/CHSRA Trainsets Performance Specification.

24 Other equipment along the wayside shall be installed on or adjacent to the track and in wayside  
25 cases that comply with the clearance standards for fixed equipment. All foundations for houses  
26 and cases shall be designed and provided by the Designer in compliance with the interface  
27 definition with elevated, at-grade, in trench, and in tunnel rights-of-way.

28 Cables shall be installed in the multi compartment cable trough designed and supplied by  
29 others. Provisions have been made for access of cables in and out of the cable trough on both  
30 sides to connect to track and wayside mounted equipment. Conduits shall be provided to  
31 connect between the provided entry and exit points from the cable trough to the Designer  
32 supplied equipment, houses and cases.

33 Space provisions have been provided in other facilities including stations and maintenance of  
34 equipment and infrastructure yards and shops. Refer to the *Stations* and *Support Facilities*  
35 chapter.

Wayside equipment including cases, houses, cables, signals, switch machines, etc., shall be located with reference to clearance constraints for fixed equipment as described in the *Trackway Clearances* chapter and the associated Standard and Directive Drawings.

### 24.3.13 Software Requirements

Software (including firmware and databases) shall be developed under a comprehensive program that ensures that safety-critical, maintainable software will be delivered. The program shall be a documented, certified SIL 4 or equivalent system for vital subsystem software.

A robust configuration management scheme shall also be used to minimize any risks from placing incompatible and incorrect software versions in the field.

The software requirements specified herein shall apply to all Designers and Original Equipment Manufacturers (OEMs) that deliver software products for any ATC subsystem. All required software documentation shall describe the processes that will be followed by all Designers and the approach to be taken with OEMs.

Software shall be structured to the greatest extent possible such that system extensions and track configuration changes do not require complete testing and validation of the ATC system, testing shall be limited to the data portions.

Software shall comply with appropriate sections from the following codes and standards:

- EN 50128 – Railway applications. Communications, signalling and processing systems. Software for railway control and protection systems (2011)
- IEC 61508 – Functional Safety of Electrical / Electronic / Programmable Electronic Safety-related Systems
- IEEE 1558 – Standard for Software Documentation for Rail Equipment and Systems
- ANSI/IEEE 1028, IEEE Standard for Software Reviews and Audits
- IEEE 1003 – IEEE Standard for Information Technology – Portable Operating System Interface (POSIX)
- IEEE 1008 – IEEE Standard for Software Unit Testing
- IEEE 1228 – IEEE Standard for Software Safety Plans

#### 24.3.13.1 Safety Critical (Vital) Code

Software for safety critical (vital) subsystems shall be developed in accordance with a recognized and approved standard which produces documented, certified SIL 4 software, or equivalent.

### 24.3.13.2 Non Safety Critical (Non-vital)

- 1 Software for non-safety critical (non-vital) subsystems shall be developed under a recognized
- 2 and approved standard for reliable software which produces a minimum SIL 2 or equivalent
- 3 software.

## 24.4 Related Documentation

- 4 Related documentation includes other chapters and the Next Generation Amtrak/CHSRA
- 5 Trainsets Performance Specification.

## 24.5 Automatic Train Control Functional Requirements

### 24.5.1 On-board Functions

- 6 The ATC on-board functionality shall establish and continuously supervise and enforce each
- 7 train movement along a safe path (Movement Authority) established by the wayside ATC and
- 8 transmitted to the on-board ATC subsystem. The on-board ATC subsystem shall perform the
- 9 following functions:
  - 10 • Communicating with ATC trackside and wayside mounted equipment may include the
  - 11 following:
    - 12 – Transponder (Eurobalise or other)
    - 13 – Track circuit conveyed signals, with coded information transmitted in the running rails
    - 14 of a track circuit occupied by a specific train
    - 15 – Radio transmission from wayside and distant antennas
  - 16 • Communicating information and alarms to the Locomotive Engineer via the operating
  - 17 display(s)
  - 18 • Accurately determining the train location and correctly using the location in the generation
  - 19 of speed distance profiles. It shall be possible to determine the location of the entire train
  - 20 taking into account error of odometry and transmit it to the control center.
  - 21 • Indicating actual train speed to the Locomotive Engineer on the display unit. There shall be
  - 22 no discrepancy between the speed shown to the driver and the speed used for supervision
  - 23 of MAs and speed limits functions.
  - 24 • Ensuring the on-board databases are the correct ones, modifying them correctly with
  - 25 temporary limits transmitted from the wayside, and updating the database when
  - 26 permanent updates are transmitted from the wayside.
  - 27 • Accurately calculating the dynamic speed-distance profile, an emergency braking curve,
  - 28 and a service braking curve from the train's present location to its limit of MA and other
  - 29 supervised locations in accordance with the SBD.

- 1 • Supervising train speed against the authorized dynamic speed profile, and enforcing a  
2 speed reduction when needed, including commanding both retrievable and irretrievable  
3 penalty brake applications if the train sustains an overspeed condition, to ensure that the  
4 train does not pass the point being protected or pass any point faster than the not-to-exceed  
5 speed limit at that location.
- 6 • It shall be possible to permit and/or inhibit service brake applications in order to avoid  
7 trains stopping at certain locations such as in tunnels, across switches, or in traction power  
8 phase breaks.
- 9 • Speed restriction enforcement shall ensure enforcement over the complete train length  
10 including allowing for position uncertainty.
- 11 • It shall be possible to define certain locations (e.g., tunnels) where speed increase is related  
12 to the front of the train.
- 13 • In protecting trains from obstructions including other trains and unlocked switches, a brick  
14 wall approach shall be used to calculate the speed-distance profiles which protect the point  
15 beyond which a train must not pass.
- 16 • Correctly selecting and implementing the mode of operation.
- 17 • Providing the necessary intervention actions per the speed supervision function.
- 18 • Enforcing temporary speed limits and work zone commands received from the wayside.
- 19 • Providing intervention actions per wayside ATC commands, in response to the interfaced  
20 condition detectors, including seismic activity, intrusion, excessive wind speed, landslide,  
21 high water, excessive rainfall, etc. The ATC responses shall include stop now commands,  
22 restricted speed and alarm indications to Dispatchers and others. Specifics of the nature of  
23 responses shall be determined during the detail design phase.
- 24 • Supervising the accurate berthing of trains at station platforms and ensuring correct-side  
25 door operation.
- 26 • Correctly establishing the train dynamic and static characteristics.
- 27 • Providing ATO functions including interfacing with the rolling stock propulsion and brake  
28 subsystems to control train movement along the right-of-way in ATC-ATO mode. ATO  
29 movements shall be subject to ATP limitations and to the station stop schedule. ATO  
30 movements will be available within the yard only for movements between the main track  
31 and yard transition tracks.
- 32 • Adjusting acceleration and braking rates, and speeds in response to commands from the  
33 wayside system to meet schedule regulation requirements.
- 34 • Monitoring of ATC and train equipment health status and reporting to the ATC wayside  
35 and central; includes interfacing to on-board non-ATC subsystems to transmit subsystem  
36 health data to the wayside and central.
- 37 • Supporting degraded mode operations include the following:

- 1       – Initializing the on-board ATC functionality
- 2       – Providing degraded mode operations when selected by external input
- 3       – Isolating on-board ATC functionality when selected by external input
- 4       • Recording data for regulatory purposes.
- 5       • Recording data and transmitting it to the wayside for maintenance purposes.
- 6       • Forwarding information and commands to the Locomotive Engineer's displays and
- 7       interfacing to rolling stock subsystems as needed to support other functions including the
- 8       following:
  - 9       – Opening and closing passenger train ventilation flaps
  - 10       – Raising and lowering the pantograph(s)
  - 11       – Opening and closing the main power switch to respect phase break operational
  - 12       requirements
- 13       • Interfacing with defect sensors on the rolling stock and conveying alarms and events to the
- 14       control center(s) and correctly responding with an ATC intervention where defined
- 15       (emergency brake application, service brake application, etc.).
- 16       • Providing and monitoring ATP-enforced limitations on train performance to improve SBD
- 17       performance, including limitation on train acceleration and speed in terminal areas.
- 18       • **Operating Modes** – A minimum of 4 ATC operating modes shall be provided
  - 19       – **ATC Mode** – power and brake commands are made by the Locomotive Engineer but
  - 20       MAs with continuous overspeed supervision are enforced by the on-board ATC
  - 21       subsystem.
  - 22       – **ATC-ATO Mode** – power and brake commands are transmitted to the train
  - 23       subsystems by the on-board ATC equipment and MAs with continuous overspeed
  - 24       supervision are enforced by the on-board ATC subsystem.
  - 25       – **Restricted Manual Mode** – power and brake commands are made by the Locomotive
  - 26       Engineer but with a continuous maximum speed limit (restricted speed) enforced by the
  - 27       on-board ATC subsystem.
  - 28       – **Yard Mode** – power and brake commands are made by the Locomotive Engineer but
  - 29       with a continuous maximum speed limit (yard speed) enforced by the on-board ATC
  - 30       subsystem. For enforcement of wayside signals in the yard refer to the *Yard Signaling*
  - 31       chapter.
- 32       • A Bypass function shall also be provided for when the Locomotive Engineer breaks a seal on
- 33       a cab switch and operates the Bypass switch. Train in Bypass will have the ATC functions
- 34       cut-out and will be operated by the Locomotive Engineer subject to a maximum speed of 59
- 35       mph which will be enforced by the rolling stock subsystem.
- 36       • Transition between ATC and yard modes shall be as follows:



- Trains shall enter a yard transition track from the main track in ATC or ATC-ATO mode. Trains shall be enforced to stop at the signal controlling entry to the yard tracks at the yard end of the transition track unless the train is able to transition to yard mode once the train is completely within transition track limits, yard mode is available, and the Locomotive Engineer acknowledges the mode change. Once in yard mode, the Locomotive Engineer will control train movement in accordance with the signal aspects subject to the enforcement of yard speed limit and enforcement of red signal aspects by the on-board ATC subsystem. If the transition to yard mode does not occur in time, ATC shall ensure the train stops before passing the signal controlling entry to the yard. Once stopped, the Locomotive Engineer may manually select yard mode.
- Trains entering a transition track from the yard shall do so in yard mode – If a valid MA is received by the train at any point once the front of the train is on the transition track, the on-board ATC shall automatically transition to ATC mode. The exit signal at the main track end of the transition track shall be enforced if displaying a red aspect in both ATC and yard modes. If the train does not transition to ATC mode automatically, the Locomotive Engineer shall be able to operate a switch in the cab once the train has been stopped and a cab indication is received that ATC mode is available. Once in ATC mode, the train will be able to depart in either ATC mode or ATC-ATO mode once a MA has been received allowing the train to enter the main tracks and pass the transition track exit signal.

#### 24.5.2 Wayside Automatic Train Control Functions

The ATC wayside shall provide the following functions:

- Supporting safe train operation at the design speeds and headway in both directions on all main tracks and through interlockings
- Assembling data for MAs including track description, speed limits (permanent and temporary), and physical limit of movement allowed, and orders for individual trains and ensuring the secure and reliable transmission of each MA to the correct train
- Route setting and locking by means of interlocking functions
- The prevention of routes being set and cleared for high-speed trains onto non high-speed tracks
- The prevention of routes being set for non high-speed trains (including commuter, Amtrak, and freight) to dedicated high-speed tracks
- Implementation of PTC functions enforcing speed limits, train separation, compliance with route limits, and prevention of intrusion into work zones by the ATC wayside and on-board subsystems
- Wayside home signals shall be provided to give clear to next interlocking aspects for ATC unequipped and failed trains. Approach fixed signs shall be provided to give notice of signal ahead to trains without operative ATC.

- 1 • Integration with track-side signaling equipment including interlockings, wayside signals,  
2 and track circuits
- 3 • Interfacing with wayside detectors (where provided) including hot box, dragging  
4 equipment, landslides, seismic activity, intrusion, excessive wind speed, flood, excessive  
5 rain, track deformation, etc. Wayside detectors shall be located on the right-of-way and also  
6 on adjacent railroads on approach to where tenant train may enter the right-of-way.
- 7 • Transmitting the status information received from wayside signaling equipment to the on-  
8 board ATC subsystem
- 9 • Communicating with the on-board ATC subsystem by means of one or more of the  
10 following media:
  - 11 – Fixed and variable message transponder (Eurobalise or similar)
  - 12 – Radio
  - 13 – Coded track circuit
- 14 • Wayside to train communication by means of inductive links using transposed conductors  
15 laid on the track or at the wayside is not acceptable.
- 16 • Communicating with the ATS to facilitate route setting and other supervision functions,  
17 alarm and event notification, schedule regulation, and subsystem maintenance functions.
- 18 • Communicating with customer information systems including customer information signs  
19 and public address systems providing real time train ID and schedule adherence data.
- 20 • Snow melters including heaters for power operated machines to ensure that ice and snow  
21 does not accumulate and prevent their normal operation. Such provision is required in  
22 interlocking locations where ice and snow is prevalent. Remote control of the snow melters  
23 shall be provided from the OCC and RCCs with various modes of operation including  
24 automatic (local thermostat and humidity detection active), and snow melters on and off.
- 25 • Traction power phase break requirements – Coordinate the ATC wayside and system  
26 design with the traction power and OCS designs to ensure that trains cannot normally be  
27 stopped or slowed by the ATC system in approach to or within a phase break section such  
28 that a train cannot operate through the phase break without stalling.

### **24.5.3 Automatic Train Supervision Functions**

29 ATS shall provide the following centralized control and supervision functions. ATS  
30 functionality, subsystem performance, and hardware requirements shall be part of the ATC  
31 system.

- 32 • ATC shall be controlled, supervised, and monitored by ATS from the OCC or the RCCs.

- 1 • Train routing – The ATS shall provide automatic and remote manual control of  
2 interlockings, including route setting and canceling, and individual switch operation.  
3 Automatic train routing shall provide sub-modes by the following:
  - 4 – Schedule
  - 5 – First come, first served
  - 6 – Train run ID
- 7 • Dispatching shall be achieved by the clearing of signal routes for trains about to depart,  
8 allowing sufficient time for last minute passengers to board and for the doors to close and  
9 lock. Signals or MAs shall be held against trains to prevent them from departing stations  
10 and yards early.
- 11 • Scheduling – The ATS shall store multiple versions of the operating schedule and allow real-  
12 time editing by dispatchers and operating supervisors, to cancel, add, or adjust trip  
13 information for individual and groups of trains. The ATS shall automatically regulate trains  
14 in accordance with the timetable amended or otherwise.
- 15 • Temporary Speed Restrictions (TSRs) and work zones will be entered through the ATS  
16 subsystem and shall be transmitted to and implemented by the respective ATC subsystems  
17 for enforcement by the on-board ATC.
- 18 • Access to the ATS and its functional layers shall be password protected. Management of  
19 functional layers and of passwords shall be performed by a System Administrator, Chief  
20 Dispatcher, or equivalent.
- 21 • ATS facilities shall be provided in the OCC, RCCs, and training facilities to allow for  
22 playback and related analysis of events following incidents and emergencies using ATS  
23 recorded data and for training dispatchers and managers.
- 24 • ATS facilities shall be provided to enable the training of dispatchers and managers. The  
25 simulator functions shall allow the creation of scenarios both from scratch and from  
26 recorded data by a trainer. They shall allow the trainee to manage the scenario through to a  
27 conclusion. A separate set of workstations shall be provided for training purposes. The  
28 training facilities shall also be available for playback of events and incidents.
- 29 • ATS facilities shall be provided to enable construction and evaluation by simulation of new  
30 timetables including supplements to existing timetables and for minor and major editing of  
31 existing timetables.
  - 32 – ATS shall interface to passenger information facilities including customer information  
33 signs and audible public address subsystems with train status information for display at  
34 passenger stations and other public locations.

## 24.6 Grounding, Bonding, and Lightning Protection System

- 1 Grounding, bonding, and lightning protection requirements are defined in the *Grounding and*  
2 *Bonding Requirements* chapter.
- 3 The grounding and bonding design shall meet requirements for safety and function including  
4 safe touch potential limits, immunity from lightning strikes, ensuring the integrity of track  
5 circuit operation and detection of rolling stock, and safe operation in the event of equipment  
6 failures.

## 24.7 On-board Equipment

- 7 All on-board hardware shall meet the requirements of EN 50125-1, Railway applications;  
8 Environmental conditions for equipment; equipment on-board rolling stock, and with the  
9 requirements of the Infrastructure Interface Specification section of the Next Generation  
10 Amtrak/CHSRA Trainsets Performance Specification.
- 11 Preference shall be given in the design to equipment that is mounted on the railcar body as  
12 opposed to the truck.
- 13 On-board equipment including housings, cables, sensors, etc., shall be located with reference to  
14 the rolling stock dynamic envelope (loading gauge) constraints as described in the *Trackway*  
15 *Clearances* chapter and the associated Standard and Directive Drawings.

### 24.7.1 Display Unit

- 16 The on-board ATC display unit shall be a Human Machine Interface (HMI) that complies with  
17 all relevant parts 229 and 236 of 49 CFR. It shall provide ergonomically acceptable visibility of  
18 ATC displayed indications and ATC functions that require train operator interaction with the  
19 display. Reference shall also be made to UIC 612 – 0 - Driver Machine Interfaces for EMU/DMU,  
20 Locomotives, and Driving Coaches – Functional and system requirements associated with  
21 harmonized Driver Machine Interfaces.

### 24.7.2 Data Radio Display

- 22 The data radio display unit for ATC and voice radio functions shall be an HMI that effectively  
23 displays the radio functions and interactive controls. The HMI shall also comply with 49 CFR  
24 Part 236, Appendix E. Reference shall also be made to UIC 612 – 0 - Driver Machine Interfaces  
25 for EMU/DMU, Locomotives, and Driving Coaches – Functional and system requirements  
26 associated with harmonized Driver Machine Interfaces.

### 24.7.3 Speed and Location Determination

- 27 The on-board ATC subsystem shall determine train speed and location using sensors and  
28 subsystem logic in a combination that provides adequate safety-critical accuracy, reliability, and

availability, in the environment in which the equipment is mounted on the RS. Non-truck mounted equipment shall be preferred over truck mounted equipment.

Different combinations of sensors may be used for vital applications than those used for non-vital applications.

#### 24.7.4 Antennas

The design for the location of all antennas on the rolling stock including ATC data radio, ATC balise readers, and voice and other data radio, shall be such that reliable communication is assured at the extremes of the track geometry and route geography that the rolling stock traverses. Rolling stock movement and dynamic and static behavior shall be taken into account.

The location of antennas on or about the vehicle roof and underneath the RS floor shall take into account non-ATC antennas and other equipment and structural elements that might affect radio and transponder detection and communication.

#### 24.7.5 Electronics

The on-board ATC subsystem shall meet the allocated reliability and availability requirements for on-board. It shall also meet the maintainability requirements with easy access for diagnostics and ease of replacement modules.

#### 24.7.6 Interfaces with Rolling Stock Subsystems

Refer also to the *Rolling Stock-Core Systems Interfaces* chapter.

##### 24.7.6.1 Propulsion

The ATC on-board equipment shall interface to the RS propulsion subsystem to provide the following functions:

- Vital propulsion cut-off during overspeed or in approach to MA limit
- Non-vital command of tractive effort in ATC-ATO mode of operation
- Non-vital propulsion and regenerative brake cut-off and restoration when approaching and leaving an OCS/Traction Power Supply System (TPS) phase break, based on ATC location data for phase break start and end
- Vital limit on train acceleration and speed when approaching a bumping post

##### 24.7.6.2 Brakes

The ATC on-board equipment shall interface to the rolling stock braking subsystem to provide the following functions:

- Non-vital command of variable service brake effort in ATC-ATO mode
- Non-vital command of service brake following an overspeed

- Vital emergency brake command as a penalty (to zero speed)

#### **24.7.6.3 Train Doors**

The ATC on-board equipment shall interface to the RS door subsystem to provide the following functions:

- Command of door release on correct side for a train berthed at platform
- Command of door inhibit for end door(s) where trains may be stopped in a station but not be fully berthed due to a short station platform
- Detection of doors closed and locked, the loss of which will result in a penalty enforced (service) brake by ATC

#### **24.7.6.4 Power Switching and Air Flaps**

The ATC on-board equipment shall interface to the RS subsystems to provide the following functions:

- Commanding opening of the main circuit breaker when approaching and running through an OCS/TPS section break and reclosing upon leaving the section break, based on ATC location data for the phase breaks
- Commanding the closure of air pressure flaps or equivalent devices when approaching tunnels and commanding their opening upon leaving, based on ATC location data for tunnel portals

#### **24.7.6.5 Flange Lubrication System**

The ATC on-board equipment shall interface to the appropriate RS subsystem to provide the following function:

- Turning on and off on-board flange lubrication equipment when operating through curves and special trackwork, based on ATC location data for lubrication locations

#### **24.7.6.6 Hot Axle Bearing and Other Sensors**

The ATC on-board equipment shall interface to the RS subsystems to provide the following function:

- Sending event and alarm messages to the OCC and other locations that are generated by on-board diagnostic systems including hot axle bearing detectors

#### **24.7.6.7 Other Interfaces**

The RS shall provide for the mounting locations, cabling between the sensors and the on-board ATC package location, and other environmental considerations as specified.

### 24.7.7 Event Recording

The on-board ATC subsystem shall record data for diagnostic and record keeping purposes and also shall also interface to the on-board data and event recorders so as to be in compliance with the CFR regulations.

## 24.8 Wayside Equipment

All wayside equipment shall be designed to meet the relevant recommendations of the AREMA C&S Manual of Recommended Practice and to accommodate additional forces, shock, and vibration resulting from trains passing at up to 250 mph.

Loading design of houses, cases and other enclosures, signals, and other wayside equipment mounted on elevated structures shall comply with the limits defined in the *Structures* chapter.

### 24.8.1 Signals

All wayside signals installed along the right-of-way including main tracks and transition tracks shall meet the recommendations of the AREMA C&S Manual of Recommended Practice Parts 2.1.1. Use and placement of wayside ATC signals will be further described in the Specifications and Standard and Directive Drawings.

The purpose of wayside signals shall be to support degraded modes of operation in the event of failures of the ATC subsystems and components. Wayside signals will not be required to control the movement of trains under normal ATC operating modes. Wayside signals shall provide indications to the Locomotive Engineer of a train operating in degraded mode, of the status of routes through interlocking, and the status of the block to the next interlocking. Signals shall display a red stop aspect where a route is not cleared or where the block is not clear to the next interlocking (for approaching non-ATC trains). Signals shall be dark when a route is clear for ATC active trains. Approaching ATC trains shall send their ATC status to the wayside to enable the interlocking to suppress the display of any aspect when there is a MA in effect through the interlocking.

Wayside signals shall be interlocked with each other and the track switches. The signal aspects shall provide visual indication to the Locomotive Engineers of non-ATC trains and the status of the route ahead to the next interlocking. When ATC trains are operating in normal ATC modes such as ATC and ATC-ATO, the signals shall be dark, which would be interpreted as a "Proceed on Cab Signal" instruction.

A red aspect shall mean stop to ATC and non-ATC trains.

Dwarf style signals shall be used except where aspect visibility and sighting distances at train speeds in degraded mode operation require high signals.

### 24.8.2 Wayside Signs

Signs installed along the right-of-way as part of the ATC system shall meet the recommendations of the AREMA C&S Manual of Recommended Practices Part 14.6.1, except that retro-reflective signs shall be used rather than enamel or vitreous enamel signs.

Signs shall be similar to signals in that they shall be visible to the Locomotive Engineer to safely allow for an approach speed defined by 49 CFR 236 Subpart I and per AREMA recommended practices. Sign design shall also be consistent with the requirements of the *Civil* chapter.

Signs shall be provided as a minimum at the following locations:

- At track circuit block boundaries between interlockings
- At station approaches at the start of service braking distances at speeds defined by degraded mode operation
- At interlocking approaches at the start of service braking distances at speeds defined by degraded mode operation

### 24.8.3 Track Circuits

The ATC wayside train detection subsystem shall comprise track circuits, which shall be compatible with the RS, and which maximize the ability to detect broken rails. The electrical characteristics of the RS wheelsets including the impedance of the wheel/rail interface shall be consistent with the specified track circuit shunting sensitivity. The track circuits shall be compatible with the conducted and inductive emissions of the RS.

Other train detection systems such as axle counters may be used in addition to, but not instead of, track circuits if needed for specific functions or to support the system safety and performance criteria.

Axle counter equipment shall be of proven design with 5 years or more demonstrable operation in a similar high-speed rail environment.

Broken rail detection shall be provided through the use of track circuits which shall also be the primary means of wayside train detection. Track circuits shall meet the requirements of 49 CFR 236. Shunt fouling is not allowed on diverging routes with speed exceeding 45 mph.

Track circuits shall conform to the AREMA C&S Manual of Recommended Practice, Section 8; Track Circuits, as applicable in electrified territory.

All main, interlocking and transition tracks shall be track circuited. The number of and the placement of the track circuits shall be determined during final design based on the following criteria:

- For headway purposes the track circuits shall be of sufficient length in order to provide the minimum design headway.



1 • Track circuits shall be short enough so as to reliably shunt beneath trains of any  
2 configuration including high-speed, work trains, and single work train locomotives.

3 • The overall number of track circuits shall be minimized to reduce procurement and life cycle  
4 costs.

5 Track circuits shall reliably detect revenue RS, high-speed MOI equipment, and locomotive  
6 hauled work trains. High-speed MOI equipment is defined as equipment that can operate at 60  
7 mph or faster. The preferred shunting sensitivity of track circuits shall be 0.25 ohms. A higher  
8 shunting sensitivity value is allowed. Shunting sensitivity shall be demonstrated to detect a  
9 broken rail with cross bonds between tracks installed at maximum 12,000 feet apart. The  
10 Designer shall design the cross bonding scheme taking into account the requirements for  
11 shunting sensitivity, broken rail detection, and step and touch potential of the rails, working in  
12 coordination with the traction power Designer. Refer to the *Grounding and Bonding Requirements*  
13 chapter.

14 The design shall take into account the specified rail to rail impedance. Refer to the *Trackwork*  
15 chapter.

16 Jointless track circuits are preferred for track circuits between interlockings.

17 Track circuit equipment at the boundaries between adjacent track circuits outside of  
18 interlocking limits shall be installed in enclosures mounted adjacent to the main tracks in line  
19 with the OCS poles.

20 Track circuit occupancy and equipment health status information shall be transmitted from  
21 each track circuit receiver location to the adjacent interlockings in each direction. This  
22 information shall be transmitted as vital information. Connections to interlockings may be  
23 made over copper wire in a cable or in a dedicated ATC fiber cable. If appropriate to the ATC  
24 system, MA information shall be transmitted from the interlockings to the leading occupied  
25 track circuit by similar means. Wireless transmission shall not be used for transmission of track  
26 circuit information between the wayside track circuit equipment and the adjacent interlockings.  
27 The cable used for transmitting track circuit information to the interlockings shall be installed in  
28 the cable troughs.

#### 24.8.4 Interlockings

29 Interlocking functions shall be in accordance with 49 CFR, Part 236, Subparts A through I,  
30 except as modified by the FRA Guideline documents.

31 Microprocessor based interlockings shall be used.

32 Vital communication between adjacent interlockings shall be provided.

33 Vital communication with adjacent yard signaling systems shall be provided.

#### 24.8.5 Local Control Panels

Local control panels for the setting and canceling of routes together with the individual operation of signals and switches shall be provided in each signal house and also in station control rooms. These panels shall only be used in the event of failures and similar emergencies or to facilitate testing during maintenance activities. The capability to take local control at the local control panel shall be protected by a log-in procedure that shall prevent unauthorized local action if the interlocking is in communication with the ATC-ATS.

#### 24.8.6 Impedance Bonds

Air cooled impedance bonds shall meet the current capacity required for the RS operation and shall meet the grounding and bonding requirements as described in the *Grounding and Bonding Requirements* chapter.

#### 24.8.7 Switch Machines

Power operated switch machines shall be provided on switches and moveable point frogs (other than slip frogs) on the main tracks of sufficient power so as to reliably operate each switch type. Where multiple switch machines are required per switch point and frog, a control device to coordinate the effort of multiple machines shall be provided. Correct switch machine operation is critically important for on-time performance; therefore, the switch machines shall be highly reliable and dependable and readily maintainable to ensure that the ATC system meets its RAMS requirements.

Switch detection shall be arranged such that each end of a crossover is detected independently. If detection is lost on one end of a crossover, train movement at restricted speed only shall be allowed across the detected end in the normal position.

AREMA standard compliant machines are preferred. Contractor shall justify the use of non-AREMA standard compliant machines if proposed.

Switch machine layouts, mounting, and connection details shall be coordinated with the Trackwork Designer.

#### 24.8.8 Equipment Enclosures

Equipment enclosures in the form of houses and cases that meet the environmental requirements of the project and provide security from access by non-authorized persons shall be provided. Cases shall provide easy and safe access for inspection and maintenance. All doors and panels shall be located and secured so they cannot be struck by passing trains or blown shut. The enclosures, doors, and panels shall be located so as to minimize risk of maintenance personnel being struck by passing trains when working on the equipment inside the enclosures.

A telephone instrument shall be provided in each house.

- 1 Houses shall be provided with fluorescent lighting.
- 2 Houses and equipment cases shall be provided with an intrusion alarm system that includes a  
3 delay timer allowing a maintainer to deactivate the alarm following authorized access to the  
4 house or case.
- 5 Houses shall be provided with a fire detection and alarm system.
- 6 All alarms shall be transmitted to the OCC through the communications system, refer to the  
7 *Communications* chapter.
- 8 Compounds have been provided by others at interlocking sites and within yards on which to  
9 locate ATC houses and cases. Cases on elevated structures shall be mounted in locations  
10 provided with access points to the cable trough and to the trackway itself by others.
- 11 Where interlockings are located on elevated structures, space shall be provided under or in near  
12 proximity of the structure to locate the ATC houses with provision on the columns supporting  
13 the structure for routing the cables from the houses to the interlocking wayside equipment on  
14 the structure. *Refer to Structures* chapter for provisions for the installation of conduit embedment  
15 on specific columns of aerial structures and site specific walls of earth retaining structures.
- 16 Where interlockings are located on elevated structures, in a cut or a trench, a stairway shall be  
17 provided from a maintenance road parking spot to the right-of way within 250 feet of the  
18 location of the ATC house. *Refer to Structures* and *Civil* chapters for maintenance access and  
19 stairway requirements.
- 20 Grounding of houses and cases shall comply with the requirements of the *Grounding and*  
21 *Bonding Requirements* chapter.
- 22 Refer to Section 24.3 for details of seismic and structural design of equipment enclosures.

#### 24.8.9 Insulated Joints

- 23 Insulated joints shall be kept to a minimum on the main tracks. Jointless track circuits are  
24 preferred. Where used, insulated joints shall be of the glued type and pre-assembled before  
25 being brought to the site and welded in place. The strength and integrity of the joint and its  
26 insulation shall be compatible with the forces imparted by high-speed train operation.
- 27 Insulated joints shall be provided at entrances to interlockings, within interlockings to provide  
28 trailing release functions between crossovers and turnouts, and within special trackwork as  
29 needed.

#### 24.8.10 Cables, Cable Trough, and Conduits

- 30 Cables shall comply with AREMA standards for cables and shall be constructed of materials to  
31 suit the physical environment and installation methods with which they are installed. Exterior

components of the cable shall be zero smoke, zero halogen when installed in underground and enclosed facilities. All cables shall be run in ducts, cable troughs, or conduits. No cable shall be laid exposed directly on ballast, on non-ballasted track, or buried directly underground.

Cable troughs along the main tracks and within yards (cable trough) will be designed and supplied by others. Provisions have been made for the entry and exit of cables to and from the cable trough every 30 feet. Refer to the *Civil* chapter.

Where required to link ATC equipment with the cable trough and other locations and facilities, the cable trough and ducts shall be of sufficient strength and integrity for the environment of high-speed train operation. Where power cables carrying circuits in excess of 120 V are run in ducts or troughs, these cables shall be physically separated from other signal and communications cables using the separation rods in the cable trough or by using separate conduits. See the *Communications* chapter for conductor routing and cabling segregation for core systems equipment. Cable troughs will have lids that protect against access by unauthorized persons.

Connections between cable troughs and other sleeves and ducts provided in the infrastructure including the track slabs shall be made using conduits. Connections between cable troughs, cable vaults including those provided for under-track crossings, ATC houses and cases, and other external equipment such as power transformers, shall also be made in conduit.

Cables that cross under tracks shall be routed through the under-track crossings. For the under-track crossing requirements refer to the *Communications* chapter.

Refer to *Structures* chapter for provisions for the installation of conduit embedment on specific columns of aerial structures and site specific walls of earth retaining structures.

#### **24.8.11 Radio Block Control Centers**

If a radio based ATC system is implemented (ERTMS or similar), the MAs for trains shall be determined by the RBC, part of the wayside control equipment. The functional interfaces between RBCs shall be in accordance with ERA/UNISIG SUBSET 039 – Functional Interface Specification for the RBC/RBC Handover. The technical interfaces between RBCs shall be in accordance with the following:

- ERA/UNISIG SUBSET 097 – Requirements for RBC-RBC Safe Communications Interface
- ERA/UNISIG SUBSET 098 – RBC-RBC Safe Communication Interface
- ERA/UNISIG SUBSET 099 – Test Specification for Safe Communication Interface

In addition, the interface between the wayside part of the radio network and the RBCs shall be in accordance with ERA/UNISIG SUBSET A11T6001-12 – MORANE Radio Transmission FFFIS for Euro Radio.

#### 24.8.12 Transponders

Where transponders are used, standard Eurobalises or other passive and/or active data transponders may be used (including the Amtrak ACSES device as used on the Northeast Corridor). The transponder design, including physical size, transmission power, and data rate, shall be compatible with the rolling stock mounted transponder antenna that will pass over the transponder antenna at a maximum design speed of 250 mph.

If Eurobalises are used, it shall be in accordance with ERA/UNISIG SUBSET 036 – Form Fit Function Interface Specification (FFFIS) for Eurobalise and deployed in accordance with ERA/UNISIG SUBSET 040 – Dimensioning and Engineering Rules and other relevant documents.

#### 24.8.13 Signal Power

Power for signal equipment shall be provided from at least 2 sources with back-up batteries at each location. The preferred primary power source is a feed from the OCS via step-down transformer to provide 480 V single phase supply to the ATC enclosure. Additional transformers, rectifiers, and filtering equipment shall be provided to distribute the necessary regulated voltage supplies to various items of equipment housed in the enclosure or fed from it.

A circuit breaker disconnect shall be provided on the low-voltage side of the transformer, marking the demarcation between OCS and ATC maintenance responsibility. Alternative power sources may consist of one or more of the following — feed from local utility, feed from an adjacent facility that has a direct utility feed, solar panels, and wind turbines.

Battery backup shall be provided in all locations with a guaranteed minimum duration of 8 hours when maintained per the manufacturer's recommendations.

In addition, each ATC facility shall be provided with a plug-in point to receive power from a portable generator.

Loss of primary signal power shall not result in the loss of any ATC function for the duration of the charged life of the batteries. The status of power supplies and batteries shall be alarmed and indicated remotely through the ATS.

#### 24.8.14 Event Recording

The central, wayside, and on-board ATC subsystems shall record data for diagnostic and record keeping purposes. The on-board event recorder shall also interface to the RS on-board data and event recorders so as to be in compliance with the CFR regulations.

#### 24.8.15 Wayside Interfaces

The wayside ATC subsystem shall interface to the following:

- Event and incident detectors (number and type to be determined) to provide indications and ATC system responses as necessary to detections

- 1 • OCS for power supplies at certain locations
- 2 • Utility power supplies at locations where provided
- 3 • Grounding points provided by others

#### 24.8.16 Highway Grade Crossings

- 4 No highway grade crossings shall be permitted on the main tracks of the dedicated high-speed  
5 train sections; therefore, no warning systems or interfaces to the ATC system are required for  
6 the dedicated high-speed train sections.
- 7 There are highway grade crossings on the shared corridor sections of the CHSTS. The ATC  
8 system shall be interfaced and integrated with the signal and train control systems on the  
9 shared corridors including with highway grade crossing devices in order to achieve full  
10 interoperability when CHSTS trains operate on the shared corridors.

### 24.9 System Time

- 11 All pieces of ATC equipment shall derive their time reference from a single GPS Network  
12 Timing System (GNTS). Each time-tagged data element within any ATC equipment item shall  
13 be accurately time-stamped for real time control and system safety needs and also for later  
14 event and incident analysis.

### 24.10 Maintenance and Diagnostic Functions

- 15 All ATC equipment shall be designed with built in diagnostics and health monitoring  
16 functionality. Equipment shall be monitored such that degrading performance can be identified  
17 to the greatest degree possible and indicated as a warning through the ATS component of the  
18 Supervisory Control and Data Acquisition (SCADA) subsystem and locally in the equipment  
19 houses and cases. Warnings shall be given prior to the loss of function of the equipment. Refer  
20 to the *Supervisory Control and Data Acquisition Subsystems* chapter.
- 21 ATC equipment status shall be indicated as healthy or failed through the ATS subsystem and  
22 locally in the equipment houses and cases.
- 23 Indications shall be given in the form of color coded indications and/or in the form of text  
24 readouts in English. Alphanumeric codes shall be avoided as far as is practicable, health status  
25 messages shall be in plain text.
- 26 Design of all equipment shall consider ease of maintenance including easy access to all  
27 replaceable components and equipment that requires inspection and routine testing in place.
- 28 Maintenance and diagnostic functions and equipment shall support the ATC system  
29 maintainability requirements.

## 24.11 Automatic Train Supervision Subsystem

### 24.11.1 Automatic Train Supervision Functions

1 The ATS system shall provide the remote supervision and remote control facilities for the ATC  
2 system. Many of the functions including route setting, junction management, and schedule  
3 regulation shall have automated functional options with the ability for dispatchers and  
4 managers to intervene as necessary.

5 The ATS shall also provide event recording facilities of all data input to the system and  
6 commands sent from the system.

7 All ATS equipment needed for standalone operation shall be installed in the OCC and in at least  
8 2 additional RCCs. Under normal conditions, the OCC shall have overriding command and  
9 control. Staff at the RCCs may supervise and dispatch trains on their respective corridors and  
10 the OCC staff can supervise and dispatch trains in all other sections of the system.

11 Any and all control, supervision, and monitoring functions shall be possible from any  
12 workstation within the OCC or RCCs, once authorized with password protected configuration  
13 setting functions. Servers and other processors shall be distributed such that the loss of any and  
14 all equipment at one of the control centers shall not prevent either of the remaining control  
15 centers from supervising and controlling any part of the system.

16 Remote workstations giving access to indications and limited command sets shall be installed in  
17 station control rooms, equipment and MOI maintenance facilities.

18 ATC shall provide an automatic field-fallback mode for all interlockings when unsupervised by  
19 ATS, except for terminals and major through stations, including Los Angeles. In the event of  
20 loss of ATS control from the OCC or RCCs, routes at interlockings shall automatically revert to  
21 previously programmed through routes and fleet settings, or as otherwise commanded from a  
22 local control panel.

23 A local control panel or panels shall also be provided in station control rooms to allow  
24 operating personnel to set and cancel routes in the event of a failure of ATS communications  
25 between the field sites and the OCC and RCCs, or in other special cases with approval from the  
26 Dispatcher.

27 A local control panel shall also be provided in the interlocking control house for emergencies  
28 and for maintenance testing needs.

29 Regional control of each workstation shall be definable by the System Administrator. Definition  
30 of access to each set of functional levels shall be password protected. System access shall be  
31 structured such that different levels of access can be afforded to individuals based on their  
32 responsibilities and access authorization managed by the System Administrator.

### 24.11.2 Automatic Train Supervision Hardware (Office) Requirements

- 1 ATS hardware shall consist to the maximum extent practical of commercial off-the-shelf (COTS)  
2 devices and components. Hardware shall be chosen based on its suitability to the environment  
3 in which it will operate and also to facilitate future upgrades of hardware as the COTS  
4 equipment becomes obsolete.
- 5 Loading design of ATS hardware and other ATS equipment shall comply with the limits  
6 defined in the *Structures* chapter.
- 7 The ATS subsystem architecture shall provide for high availability and reliability of the ATS in  
8 order to ensure that the ATC system meets the RAMS requirements.

### 24.11.3 Automatic Train Supervision Interfaces with Other Systems

- 9 The ATS system shall interface with other systems including the following:
- 10 • Traction Power SCADA to enable blocking of track sections to approaching electric trains in  
11 the event that power is off in specific sections
  - 12 • Facilities SCADA including fans and pumps to provide remote indications on the dispatcher  
13 ATS workstations
  - 14 • Tunnel facilities SCADA to provide remote indications on the dispatcher ATS workstations
  - 15 • Event and incident detectors to provide remote indications on the dispatcher workstations
  - 16 • Yard ATC for the transition of trains between the yards and main tracks and vice versa
  - 17 • Customer Information and Public Address Systems
- 18 The ATS subsystem may be implemented as part of an integrated control system and the  
19 functions may be implemented on a common hardware platform to SCADA, security, and other  
20 control and command systems.

### 24.11.4 Automatic Train Supervision Power

- 21 Power for the control center ATS subsystems will be provided by others, it will be provided  
22 from redundant sources and backed up by a combination of batteries (4 hours) and diesel  
23 generators (continuous).



## Chapter 25

### Yard Signaling

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	Oct 2013	Rolling Stock Procurement
2	Feb 2014	Revision 2



## Table of Contents

<b>25</b>	<b>YARD SIGNALING .....</b>	<b>25-1</b>
25.1	Scope .....	25-1
25.2	Regulations, Codes, Standards, and Guidelines .....	25-1
25.3	General Design Requirements .....	25-2
25.4	Design Life.....	25-4
25.5	Environmental.....	25-4
25.6	Onboard Equipment.....	25-5
25.7	Wayside Equipment .....	25-5
25.7.1	General.....	25-5
25.7.2	Signals.....	25-6
25.7.3	Wayside Signs.....	25-6
25.7.4	Train Detection .....	25-6
25.7.5	Route Logic Controllers.....	25-7
25.7.6	Workstations and Control Panels.....	25-7
25.7.7	Impedance and Other Bonds .....	25-7
25.7.8	Switch Machines .....	25-7
25.7.9	Equipment Enclosures .....	25-7
25.7.10	Insulated Joints .....	25-8
25.7.11	Cables, Cable Trough, and Conduit.....	25-8
25.7.12	Transponders .....	25-8
25.7.13	Power Operated Derails.....	25-9
25.7.14	Yard Signal System Power .....	25-9
25.8	System Configuration.....	25-9
25.9	Functional Requirements .....	25-10
25.10	Yard Signal Control System.....	25-10
25.11	Yard Signal Control System Hardware Requirements .....	25-11
25.12	Electromagnetic Compatibility .....	25-11
25.13	Reliability, Availability, and Maintainability Requirements .....	25-11
25.14	Software Requirements .....	25-12
25.15	Yard Safety Critical System Requirements .....	25-13
25.16	Maintenance and Diagnostic Functions .....	25-14
25.17	Yard Signal Control System Interfaces with Other Systems .....	25-14
25.18	Transition Tracks between Main Tracks and Yard Tracks .....	25-15
25.19	Maintenance of Infrastructure Yards.....	25-15

## Acronyms

AREMA C&S	American Railway Engineering and Maintenance-of-Way Association Communications and Signals
ATC	Automatic Train Control
ATS	Automatic Train Supervision
CHSTP	California High-Speed Train Project
IIMP	Integrated Information Management Platform
LED	Light-Emitting Diode
MOI	Maintenance of Infrastructure
MTTR	Mean Time to Restore
MTTT	Mean Time To Travel
RAM	Reliability, Availability, and Maintainability
RAMS	Reliability, Availability, Maintainability, and Safety
RSM	Rolling Stock Maintenance
SIL	Safety Integrity Level

## 25 Yard Signaling

### 25.1 Scope

1 The yard signaling system and its subsystems shall meet the requirements of the standard  
2 specifications and preliminary engineering drawings and all applicable regulatory  
3 requirements.

4 The term “yard” with respect to signaling refers to the following:

- 5 • Rolling Stock Maintenance (RSM) facilities including the heavy maintenance facility (HMF)  
6 and all terminal storage and maintenance facilities – The yard signal system consisting of  
7 the signals, switch machines, train detection, and the controlling subsystems shall meet the  
8 requirements of the standard, functional, and performance specifications and preliminary  
9 engineering drawings for the yard signal system and all applicable interoperability and  
10 regulatory requirements extracted from the system requirements.
- 11 • Maintenance of Infrastructure (MOI) facilities – MOI yard tracks shall not be equipped with  
12 a signal system. However, the transition tracks connecting the yard tracks with the main  
13 tracks shall be equipped with main track automatic train control (ATC) (refer to the  
14 *Automatic Train Control* chapter) and with yard signals protecting train movements in both  
15 directions between the transition tracks and the yard tracks.

### 25.2 Regulations, Codes, Standards, and Guidelines

16 Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.

- 17 • American Railway Engineering and Maintenance-of-Way Association Communications and  
18 Signals (AREMA C&S) Manual of Recommended Practices
  - 19 – Part 17 – Recommended
- 20 • European Committee for Electrotechnical Standardization (CENELEC) Standards
  - 21 – EN 50121-4 - Railway applications – Electromagnetic compatibility Part 4: Emission and  
22 immunity of the signaling and telecommunications apparatus (2006)
  - 23 – EN 50125-1 - Railway applications - Environmental conditions for equipment- Part 1:  
24 Equipment on board rolling stock (1999)
  - 25 – EN 50125-3 - Railway applications - Environmental conditions for equipment - Part 3:  
26 Equipment for signalling and telecommunications (2003)

- EN 50126-1 – Railway applications – The specification and demonstration of Reliability, Availability, Maintainability and Safety – Part 1: Basic requirements and generic process (1999)
- CLC/TR 50126-2 - Railway applications – The specification and demonstration of Reliability, Availability, Maintainability and Safety (RAMS) – Part 2: Guide to the application of EN 50126-1 for safety (2007)
- EN 50128 - Railway applications – Communications, signalling and processing systems - Software for railway control and protection systems (2011)
- EN 50129 - Railway applications – Communications, signalling and processing systems - Safety related electronic systems for signalling (2003)
- EN 50159 - Railway applications – Communications, signalling and processing systems - Safety-related communication in transmission systems (2011)
- International Electrotechnical Commission (IEC) Standards
  - IEC 60812: Analysis techniques for system reliability – Procedures for failure mode and effect analysis (FMEA)
- Institute of Electrical and Electronics Engineers (IEEE) Standards
  - IEEE 828 – IEEE standard for Software Configuration Management Plans
  - IEEE Standard 1558, Standard for Software Documentation for Rail Equipment and Systems
  - ANSI/IEEE STD 1028, IEEE Standard for Software Reviews”, The Institute of Electrical and Electronics Engineers
  - IEEE Standard 1003, IEEE Standard for Information Technology - Portable Operating System Interface (POSIX)
  - IEEE Standard 1008, IEEE Standard for Software Unit Testing
  - IEEE Standard 1228, IEEE Standard for Software Safety Plans

### **25.3 General Design Requirements**

The Yard Signal System shall be safe and reliable, as defined by the RAMS requirements of this chapter and designed to a Safety Integrity Level 2 of safety.

The Yard Signal System shall incorporate the following:

- Route setting and locking by means of route logic control functions – Routes shall be protected with approach locking and overswitch (detector) locking. Sectional release shall be provided on specified tracks that will allow short turns when drilling trains between tracks. Signals shall be provided to indicate that trains may safely proceed through switch

1 areas. Signals shall be provided to govern movements into and out of layup tracks, at  
2 approaches to shop entrances, and at entry and exit between transition tracks and the yard.

- 3 • Signal aspects shall be determined by switch position, route locking, and track occupancy  
4 status. Aspects shall also indicate if there is at least 663 feet of train berth space available in a  
5 layup track. Signals shall be provided to protect all power-operated derails, powered  
6 switches, in the midpoint of ladder tracks if a short drill move is permitted, and at the  
7 midpoint of layup tracks where up to 1,326 feet of train layup space is provided.
- 8 • Onboard ATC equipment shall enforce compliance with wayside signal aspects (stop). A  
9 single yard maximum speed limit shall also be enforced by the onboard system; lower  
10 speeds where applicable shall be signposted at the wayside and will be manually observed  
11 and complied with by the Locomotive Engineer.
- 12 • Yard tracks shall be equipped with a signal system that includes a combination of track  
13 circuits and/or axle counters, low level wayside signals, power-operated switch machines,  
14 power operated derails, and route logic controllers that shall control train movements  
15 within yard limits.
- 16 • Blue flag protection for workers in the yards shall be achieved through the use of power  
17 operated switches on inspection tracks being lined and locked away from the tracks on  
18 which inspection is taking place, and by a block being placed on the yard signal control  
19 system. Power-operated derails shall also be provided where necessary and where a  
20 particular track cannot be protected by lining switches away from that track. These derails  
21 shall also be controllable from the yard signal control system.
- 22 • Train location shall be detected by means of track circuits. Other train detection systems  
23 such as axle counters may be used in addition to, or except for Transition Tracks instead of,  
24 track circuits. Where track circuits are used in the yards, they shall be of single rail  
25 configuration except in transition tracks where double rail track circuits shall be employed.
- 26 • Wayside signals shall be connected to and controlled by route logic control equipment so as  
27 to provide visual indication of the status of routes through switches and occupation of  
28 layup tracks to the Locomotive Engineers. Fixed wayside signs shall also be provided for  
29 indications to Locomotive Engineers of key locations within lay-up tracks (e.g., “Limit of 8  
30 Car Unit Stop”, etc.).
- 31 • Yard Tracks shall be separated from main tracks by means of transition tracks. Train  
32 movement between the transition tracks and main tracks shall be under the control of the  
33 ATC system. Train movement between the yard tracks and the transition tracks shall be  
34 under the control of the yard signal system.
- 35 • Transition between yard and ATC modes on the transition tracks shall be automatic,  
36 transition between ATC and yard modes shall be semi automatic. When ATC is able to  
37 transition to yard mode, an indication shall be given to the Locomotive Engineer that yard  
38 mode is available. Acknowledgement by the Locomotive Engineer shall cause the transition  
39 to yard mode to occur. If the Locomotive Engineer does not acknowledge the pending

- 1 change of mode, the train shall remain in ATC mode and a stop before entrance to the yard  
2 tracks shall be enforced. For more details refer to the *Automatic Train Control* chapter.
- 3 • Power switch machines within the yard shall be of the yard (trailable) type. Switch  
4 machines on any part of the transition tracks shall be of the main track type.
  - 5 • Control and supervision of the yard wayside signal system shall be through a local  
6 dedicated yard signal control facility with a combination of control panel(s) and/or  
7 workstation(s).
  - 8 • A train information management system shall be provided that allows for the designation of  
9 trainsets by maintenance personnel to specific train numbers. The train information  
10 management system shall track designated trains with their allocated information around  
11 the yard and shall exchange this information with the yard signal control system and the  
12 ATC-ATS subsystem.
  - 13 • All enclosures and the equipment mounted within them, equipment mounted in rooms  
14 provided by others, and equipment mounted along the Authority's right-of-way shall be  
15 designed in accordance with the seismic considerations as defined in the *Seismic* chapter.
  - 16 • The secure mounting of enclosures and equipment along the Authority's right-of-way shall  
17 also be accomplished in accordance with the *Structures* chapter.

## **25.4 Design Life**

18 The yard signal system shall have the same useful life as the ATC system and is defined in the  
19 *General* chapter.

## **25.5 Environmental**

20 The climatic and physical environmental specifications and performance of the Yard Signal  
21 System equipment installed at the wayside shall comply with AREMA C&S and the relevant  
22 portions of EN 50125-3, "Railway applications - Environmental conditions for equipment - Part  
23 3: Equipment for signalling and telecommunications". In the event that these requirements  
24 conflict, the more stringent and restrictive conditions shall apply.

25 The climatic and physical environmental specifications and performance of ATC equipment  
26 installed onboard passenger trains shall comply with the criteria defined in the Infrastructure  
27 Interface Specification section of the Next Generation Amtrak/CHSRA Trainsets Performance  
28 Specification.

29 ATC equipment installed on work trains shall comply with EN 50125-1, "Railway applications -  
30 Environmental conditions for equipment - Part 1: Equipment onboard rolling stock".

31 For other climatic requirements refer to the *General* chapter.



## 25.6 Onboard Equipment

Onboard equipment shall enforce stop signals and yard speed limits. A single onboard ATC equipment set is preferred for main track ATC functions and yard signal enforcement. In yards, the onboard ATC subsystem shall be in yard operating mode. The details of this mode are described further in the *Automatic Train Control* chapter.

All onboard ATC hardware shall meet the environmental criteria identified in the specifications for the respective location (body, sprung part of trucks, and unsprung part of trucks). Preference shall be given in the design to equipment that is mounted on the railcar body as opposed to the truck.

In yards, the onboard ATC subsystem shall be in yard mode; power and brake commands are made by the Locomotive Engineer but with a continuous maximum speed limit (yard speed) enforced by the onboard ATC subsystem. In yard mode, red signal aspects shall be enforced with a stop command to the train braking subsystem. Enforcement shall ensure trains stop before running past an unlocked switch point or beyond the fouling point.

Onboard equipment including housings, cables, sensors, cables, etc., shall be located with reference to the rolling stock dynamic envelope (loading gauge) constraints as described in the *Trackway Clearances* chapter and the associated Standard and Directive Drawings.

## 25.7 Wayside Equipment

### 25.7.1 General

All enclosures and the equipment mounted within, equipment mounted in rooms provided by others, and equipment mounted along the Authority's right-of-way shall be designed in accordance with the seismic considerations as defined in the *Seismic* chapter.

The secure mounting of enclosures and equipment along the Authority's right-of-way shall also be accomplished in accordance with the *Structures* chapter.

Enclosure (house and case) foundations shall be designed in accordance with the *Civil* chapter. Loading design of houses, cases and other enclosures, signals and other equipment mounted on elevated structures shall comply with the limits defined in the *Structures* chapter.

All wayside equipment except for houses shall be secured with a standard set of padlocks.

Houses shall be secured in compliance with the access control requirements as described in the *Civil* chapter. Preparation of sites is also described in the *Civil* chapter. Final site preparation shall be designed to be compatible with the requirements for placing foundations for enclosures including houses and cases and for using the compound as a working site.

1 All wayside equipment within a yard shall be designed to meet the relevant recommendations  
2 of the AREMA C&S and accommodate additional forces, shock, and vibration resulting from  
3 trains passing at up to 250 miles per hour (mph) on adjacent high-speed main tracks where  
4 applicable.

5 Spare capacity and provision for spare capacity (scalability) shall be designed into all enclosure  
6 spaces, cable conductors (copper and fiber), and network and data communication channels.

7 Enclosures in the open shall comply with the National Electrical Manufacturer's Association  
8 (NEMA) 4 standard.

9 Wayside equipment including houses, cases, cables, signals, switch machines, transponders,  
10 etc., shall be located with reference to clearance constraints for fixed equipment as described in  
11 the *Trackway Clearances* chapter and associated Standard and Directive Drawings.

### **25.7.2 Signals**

12 All signals installed in the yards shall meet the recommendations of the AREMA C&S Parts  
13 2.1.1 and 7.1.5. Light Emitting Diodes (LED) shall be used for display of signal aspects.

14 Wayside signals shall be provided so as to be visible to Locomotive Engineers when operating  
15 within the yard limits. These signals shall be the primary means of indicating routes set and  
16 locked. Stop aspects shall be enforced.

17 Wayside signals shall be interlocked with each other and with the switches as part of the yard  
18 route logic controller functionality.

19 Signal aspects shall indicate either "stop", or that a route is clear to the far side of the switch  
20 combination, or that there is sufficient lay-up distance for a single passenger train unit clear  
21 beyond the signal.

### **25.7.3 Wayside Signs**

22 Signs installed adjacent to yard tracks for train control or lay-up purposes shall meet the  
23 recommendations of the AREMA C&S Part 14.6.1, except that retro reflective signs shall be used  
24 rather than enamel or vitreous enamel signs.

25 Sign design shall also be consistent with the requirements of the *Civil* chapter.

### **25.7.4 Train Detection**

26 Train detection within the yards shall be by means of single rail track circuits. Other train  
27 detection systems such as axle counters may be used in addition to, or except for the transition  
28 track instead of, track circuits.

### 25.7.5 Route Logic Controllers

The yard signal system shall provide route logic control functions using proven and reliable microprocessor based equipment. Safety critical requirements of the yard signal system are provided in Section 25.15.

### 25.7.6 Workstations and Control Panels

Remote control of yard signal system elements shall be achieved by means of a combination of workstations and control panels for the setting and canceling of yard routes. These workstations and control panels together with the individual operation of signals and switches shall be provided in each yard signal control tower or control room and shall be integrated into the overall system Integrated Information Management Platform (IIMP).

Stand-alone (non-IIMP integrated) control panels shall also be provided in each signal house within the yard. The panels inside houses shall be used in the event of failures and similar emergencies and also to facilitate testing during maintenance activities.

### 25.7.7 Impedance and Other Bonds

Impedance bonds, where required, shall meet the current capacity required for the operation of the rolling stock and meet the grounding and bonding requirements of the California High-Speed Train Project (CHSTP); refer to the *Grounding and Bonding Requirements* chapter.

The grounding and bonding design shall meet requirements for safety and function including safe touch potential limits, immunity from lightning strikes, ensuring the integrity of track circuit operation and detection of rolling stock, and safe operation in the event of equipment failures.

### 25.7.8 Switch Machines

Power operated, high voltage yard style switch machines shall be provided on yard switches. The yard switch machines shall be of the trailable type. Switch machines on transition tracks shall be of a main line type.

Switch machine layouts, mounting, and connection details shall be coordinated with the Trackwork Designer.

### 25.7.9 Equipment Enclosures

Equipment enclosures in the form of houses and cases that meet the environmental requirements of the CHSTP and provide security from access by non-authorized persons shall be provided. Cases shall provide easy access for inspection and maintenance; all opening doors and panels shall be located and secured in such a manner that they cannot be struck by passing trains. The enclosures, doors, and panels shall also be located so as to minimize risk of

1 maintenance personnel being struck by passing trains when working on the equipment inside  
2 the enclosure.

3 All enclosures and the equipment mounted within them, equipment mounted in rooms  
4 provided by others, and equipment mounted along the right-of-way shall be designed in  
5 accordance with the seismic considerations as defined in the *Seismic* chapter.

6 The secure mounting of enclosures and equipment within the yards, including signals, signs,  
7 etc., shall also be accomplished in accordance with the *Structures* chapter.

#### **25.7.10 Insulated Joints**

8 Insulated joints shall be of the glued type and pre-assembled before being brought to the site in  
9 accordance with the *Trackwork* chapter. The strength and integrity of the joint and its insulation  
10 shall be compatible with the forces imparted by train operation within the yards and  
11 maintenance facilities.

#### **25.7.11 Cables, Cable Trough, and Conduit**

12 Cables shall be constructed of materials to suit the physical environment and installation  
13 methods used. Cables shall be run in ducts, cable troughs, or conduits. No cable shall be laid  
14 directly onto ballasted or on non-ballasted tracks, nor shall it be directly ploughed into the  
15 ground.

16 Where conduits and cable troughs cross tracks, they shall do so perpendicular to the rails. Cable  
17 troughs will be provided throughout the yards as part of the system-wide cable trough per the  
18 *Communications* chapter. Cable troughs and ducts required to connect yard signal system  
19 equipment with the system-wide cable trough shall be of sufficient strength and integrity for  
20 the yard environment. Where power cables with voltages higher than 120 V are run in ducts or  
21 troughs, these cables shall be physically separated from other signal and communications cables  
22 using the separation rods in the trough or by using separate conduits. Refer to the  
23 *Communications* chapter for conductor routing and cabling segregation for core systems  
24 equipment.

25 Cable troughs shall have lids that prevent access by unauthorized persons.

26 Cable troughs shall be compatible with the requirements of the *Civil* chapter. Refer to the  
27 *Structures* chapter for provisions for the installation of conduit embedment on specific columns  
28 of aerial structures and site specific walls of earth retaining structures.

#### **25.7.12 Transponders**

29 Where transponders are used in yards, standard Eurobalises or other forms of passive and/or  
30 active data transponders shall be used. The transponder design shall be compatible with  
31 transponders used on main tracks provided as part of the ATC system such that a separate  
32 antenna and reader system is not required as part of the on-board equipment.

### 25.7.13 Power Operated Derails

Power operated derails, including switch point and block, shall be provided as needed to protect tracks where work on rolling stock can take place and where switches cannot be set to divert approaching trains away from that track. Where derails are provided, they shall be power operated, interlocked with signals, and controlled by the yard signal control system. Derail provisions shall be consistent with the *Trackwork* chapter.

### 25.7.14 Yard Signal System Power

Power for the yard signal system equipment shall be provided from at least 2 sources with back-up batteries at each location. The preferred primary power source is a feed from the OCS via step-down transformer to provide 480 V single phase supply to each yard signal system enclosure. Additional transformers, rectifiers, and filtering equipment shall be provided to distribute the necessary regulated voltage supplies to various items of equipment housed in the enclosure or fed from it.

A circuit breaker disconnect shall be provided on the low-voltage side of the transformer, marking the demarcation between OCS and yard signal system maintenance responsibility. Alternative power sources may consist of one or more of the following: feed from local utility, feed from an adjacent facility that has a direct utility feed, solar panels, and wind turbines.

Battery backup shall be provided in all locations with a guaranteed minimum duration of 8 hours when maintained per the manufacturer's recommendations.

In addition, each ATC facility shall be provided with a plug-in point to receive power from a portable generator.

Loss of primary signal power shall not result in the loss of any yard signal system function for the duration of the charged life of the batteries. The status of power supplies and batteries shall be alarmed and indicated remotely through the ATS to the Operation Control Center/Regional Control Centers.

Loss of signal power and the status of batteries shall also be alarmed and indicated to the local yard control system.

## 25.8 System Configuration

The wayside yard signaling subsystem shall interface with the following:

- Main track (ATC) signaling at the transition tracks
- The yard remote control and supervision subsystem
- The yard rolling stock maintenance management subsystem
- The traction power subsystem (through the impedance bonds)

## 25.9 Functional Requirements

The yard signaling system shall provide the following functions:

- Setting of routes shall be done from a local control facility within each yard through workstations and a computer based control system that shall be separate from, but interfaced to, the ATC-ATS subsystem.
- Wayside signals and switches shall be interlocked; signals shall display a clear aspect when the route is set and locked and no track occupancy is detected between the signal and the opposing signal. Signals controlling routes into layup tracks shall also require a clear berth track in order to display a clear aspect.
- Where multiple routes are possible from a single wayside signal, an indicator shall be provided displaying the route destination track to the Locomotive Engineer.
- Where trains are routed to lay-up tracks where the track is partially occupied, the midpoint of the track shall be marked with a signal or fixed sign and the entry signal to the track shall show a clear aspect when at least 663 feet of track is unoccupied at the entry end of the track. The length of 663 feet is associated with the required trainset length of 656 feet with the additional 1 percent tolerance.
- Signal stop aspects shall be detected and red signals enforced by an onboard function.
- Signals at switches shall be located such that a train attempting to pass a red signal shall be brought to a stop before the unsafe location, which includes unlocked switch points or the fouling point of a switch not set and locked for the merging move.
- Midpoint signals and bumping posts at the end of tracks shall not be absolutely enforced; the yard signal system shall enforce a reduction of speed (5 mph) prior to the red signal and bumping post. A worst case maximum collision speed of 3 mph with the bumping posts shall be designed for within the yard signal safe braking concept.
- Signals at shop entrances shall be located such that a train attempting to pass a red signal shall be brought to a stop before it passes the door threshold.

## 25.10 Yard Signal Control System

The yard signal control system shall provide the remote supervision and remote control facilities for yard operations. Operation of switches and signals shall be commanded by inputs to the route logic controllers by means of a computer workstation located in a dedicated room or tower within each yard. The control system shall also provide event recording facilities of all data input to the system and commands sent from the system.

A set of train management functions shall be provided that allows for the designation of specific data including unit, train, and crew IDs to be attributed to a detected train within the yard and allows for the tracking of that train and its allotted data around the yard tracks. The yard signal

control system shall also exchange this data with the ATC-ATS subsystem when trains enter the transition tracks in preparation for entry onto the main tracks. Trains that have no data attributed to them will be assumed to be drilling in the transition track and no data exchange shall take place with the ATC-ATS.

## 25.11 Yard Signal Control System Hardware Requirements

Yard signal control system hardware shall consist to the maximum extent practical of commercial off-the-shelf (COTS) devices and components. Hardware shall be selected based on its suitability to the environment in which it will operate and also to facilitate future upgrades of hardware as the COTS equipment becomes obsolete.

## 25.12 Electromagnetic Compatibility

Yard signal system equipment shall not interfere with or be interfered with by other equipment or equipment external to the CHSTP. The yard signal system design shall comply with the requirements of the *Electromagnetic Compatibility and Interference* chapter.

## 25.13 Reliability, Availability, and Maintainability Requirements

The reliability of the yard signal system and its subsystems shall be consistent with the overall RAM requirements. RAM and safety requirements shall be defined, achieved, and maintained in accordance with EN 50126-1. Other standards may be considered on the condition that the Contractor provides a comprehensive equivalency report that maps the correlation between EN 50126-1 and their proposed standard(s).

Yard signal system availability shall also be consistent with the availability requirements of the overall project high level thresholds. Each yard signal system shall have an availability higher than 99.99 percent.

Reliability targets shall be apportioned to the equipment such that the availability targets can be achieved.

Human factors contributions to RAM shall be considered as follows:

- Mean Time To Travel (MTTT) shall be a fixed value as defined in the contract documents reflecting the locations of MOI and RSM facilities and highway access to yard signal system wayside locations.
- The Contractor shall demonstrate that a Mean Time to Restore (MTTR) of 30 minutes is achievable. MTTR is a mean value for all yard signal system line replaceable units; it includes diagnosis of faults measured from the actual start of work on site, replacing of

1 faulty components, testing, and placing the system back into service in the affected area.  
2 MTTR does not include MTTT.

3 Care shall be given throughout the design for the maintainability of all equipment including  
4 access for inspection and the replacement of all line replaceable units, and to provide user  
5 friendly diagnostics. Diagnostics shall primarily be through indicators using a combination of  
6 colored LEDs and alphanumeric displays on the front face of equipment that can be readily seen  
7 and understood without the need for reference manuals to decode the displays. Diagnostics  
8 shall also be provided that allow the use of plug in portable computers and notebooks to access  
9 internal diagnostic routines that shall facilitate the easy and rapid diagnosis of failures of the  
10 equipment. All diagnostic routines shall be protected by secure access functions.

## **25.14 Software Requirements**

11 Software (including firmware and databases) shall be developed under a comprehensive  
12 program that ensures that reliable and maintainable software will be delivered. The program  
13 shall be a documented, certified software SIL-2 per EN 50128 or equivalent system for safety  
14 related subsystem software.

15 A robust configuration management scheme shall also be used to minimize any risks from  
16 placing incompatible and incorrect software versions in the field.

17 The software requirements specified herein shall apply to all Designers and Original Equipment  
18 Manufacturers (OEMs) that deliver software products for any yard signal system and  
19 subsystem. All required software documentation shall describe the processes that will be  
20 followed by the Designer, their subcontractors, and the approach to be taken with OEMs.

21 Software shall be structured to the greatest extent possible such that system extensions and  
22 track configuration changes do not require complete testing and validation of the yard signaling  
23 system; testing shall be limited to the data portions.

24 Software shall comply with appropriate sections from the following codes and standards:

- 25 • EN 50128; "Railway applications – Communications, signalling and processing systems -  
26 Software for railway control and protection systems (2011)"
- 27 • IEEE Standard 1558, "Standard for Software Documentation for Rail Equipment and  
28 Systems"
- 29 • Software Engineering Institute (SEI), "Standard CMMI Appraisal Method for Process  
30 Improvement (SCAMPI<sup>SM</sup> Version 1.1)"
- 31 • ANSI/IEEE STD 1028, "IEEE Standard for Software Reviews"
- 32 • IEEE 828 – IEEE standard for Software Configuration Management Plans



- IEEE Standard 1003, “IEEE Standard for Information Technology - Portable Operating System Interface (POSIX)”
- IEEE Standard 1008, “IEEE Standard for Software Unit Testing”
- IEEE Standard 1228, “IEEE Standard for Software Safety Plans”

## 25.15 Yard Safety Critical System Requirements

Transition tracks shall be designed to a safety level compliant with SIL-4 as per the main line ATC system.

The yard signal system shall be designed to a safety level of at least SIL-2 and as follows:

- Safety shall be specified and assured in accordance with EN 50126-1 and EN 50129.
- Other standards will be considered on the condition that the Designer provides a comprehensive equivalency report that maps between EN 50126-1 and EN 50129, and other specified standards and their proposed standard(s).
- The related application guide CLC/TR 50126-2 Railway applications – The specification and demonstration of Reliability, Availability, Maintainability and Safety (RAMS) – Part 2: Guide to the application of EN 50126-1 for safety (2007) shall also apply.

The design, construction, installation, acceptance, operation, maintenance, and modification/extension of the ATC system, including the development of hazard mitigation procedures and other means, shall provide a quantitative level of safety such that any single, independent hardware, software, or communication failure, or any combination of such failures, with the potential of causing death or severe injury to passengers or staff, shall not occur with a frequency greater than once per  $10^4$  system operating hours. This shall be documented and certified to SIL-2 in accordance with the following:

- EN 50126-1 - Railway applications – The specification and demonstration of Reliability, Availability, Maintainability and Safety (RAMS) – Part 1: Basic requirements and generic process (1999).
- EN 50128 - Railway applications – Communications, signalling and processing systems. Software for railway control and protection systems (2011).
- EN 50129 - Railway applications – Communication, signalling and processing systems. Safety related electronic systems for signaling (2003).
- EN 50159 - Railway applications – Communications, signalling and processing systems - Safety-related communication in transmission systems (2011).

Tolerable hazard rates attributed to operations and maintenance actions shall be considered to be zero for the purpose of the yard signal system calculations.

## 25.16 Maintenance and Diagnostic Functions

1 All yard signal system equipment shall be designed with built in diagnostics and health  
2 monitoring functionality. Equipment shall be monitored such that degrading performance can  
3 be identified to the greatest degree possible and indicated as a warning through the yard signal  
4 control system. Warnings shall be given to the maximum extent possible prior to the loss of  
5 function of the equipment.

6 Yard signal equipment status shall be indicated as either healthy or failed, both remotely  
7 through the yard signal control system, and locally in the equipment houses and cases.

8 Indications shall be given in the form of color coded indications and/or in the form of text  
9 readouts in English. Alphanumeric codes shall be avoided as far as is practicable; health status  
10 indications and messages shall be in plain text.

11 Design of all equipment shall consider ease of maintenance including easy access to all  
12 replaceable components and equipment that requires inspection and routine testing in place.

13 Maintenance and diagnostic functions and equipment shall support the yard signal system  
14 maintainability requirements.

## 25.17 Yard Signal Control System Interfaces with Other Systems

15 The yard signal control system shall interface with other systems including the following:

- 16 • IIMP to provide a common Human Machine Interface for yard signaling and other control  
17 systems in the yard control room
- 18 • SCADA to facilitate the blocking of track sections to approaching trains in the event that  
19 power is off in yard tracks and to provide overview indications of the status of traction  
20 power sections within the yard
- 21 • Building and other yard facilities to prevent trains from colliding with shop entry doors and  
22 other barriers and to provide remote indications on the dispatcher workstations including  
23 shop entry door status
- 24 • Event and incident detectors to provide remote indications of yard signal system status on  
25 the yard controller workstations and/or panels
- 26 • The ATC-Automatic Train Supervision (ATC-ATS) subsystem for the transition of trains  
27 and associated information between the yards and main tracks and vice versa
- 28 • Rolling stock maintenance management system to allow the transfer of train ID data to train  
29 locations in the yard signal control system tracking functions

## 25.18 Transition Tracks between Main Tracks and Yard Tracks

The tracks where trains transition between ATC mode of operation (main tracks) and yard mode shall be called transition tracks. Control of train movements between the transition tracks and the yard tracks shall be by means of the yard signal control system. Control of train movements between the main tracks and the transition tracks shall be by the main track ATC and ATC-ATS system.

The yard route logic controllers shall interface with the ATC interlocking equipment to ensure continuity of route locking and prevention of head to head movements for train movements between the yards and main tracks in both directions.

Train mode transitions between ATC and yard modes are described in the *Automatic Train Control* chapter.

## 25.19 Maintenance of Infrastructure Yards

MOI yards will be divided into two sections:

- Transition tracks that connect the main tracks with the yard tracks
- Yard tracks

MOI yard transition tracks shall be equipped in a similar fashion to yard transition tracks. Main track standard equipment shall be installed on the transition tracks and signals shall be provided at both ends of the track(s). Routes between the main track and the transition tracks shall be equipped, protected, and controlled to main track standards. Routes between the yard tracks and the transition tracks shall be inhibited when MOI equipment is operating from a yard track to the transition track(s); conflicting train movements from the main track to the transition track(s), and from the MOI tracks to the transition tracks, shall be prevented.

MOI transition tracks shall be supervised and controlled from the Operation Control Center and/or Regional Control Centers.

ATC equipped trains shall transition modes in the same manner on transition tracks for MOI yards as for other yards. Refer to the *Automatic Train Control* chapter.

Switches within the MOI yard will be hand operated and equipped with mechanical or electrical indicators of switch position. MOI tracks, with the exception of transition tracks, need not be track circuited or provided with axle counters.

Signals are required only to indicate safe movement between the transition tracks, the main tracks, and yard tracks in each direction. Signals are not required within the MOI yard tracks.



## Chapter 26

# Electromagnetic Compatibility and Interference

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	07 Mar 12	Initial Release, R0
1	Oct 2013	Rolling Stock Procurement
2	Feb 2014	Revision 2



## Table of Contents

<b>26</b>	<b>ELECTROMAGNETIC COMPATIBILITY AND INTERFERENCE</b>	<b>26-1</b>
26.1	Scope	26-1
26.2	Regulations, Codes, Standards, and Guidelines	26-2
26.3	Description of Electromagnetic Compatibility Program and Electromagnetic Interference	26-4
26.4	General Design Requirements	26-5
26.5	Overview of EMC Design Criteria Categories	26-5
26.6	Communications EMC Design Criteria	26-7
26.6.1	Equipment and EMC Considerations	26-7
26.6.2	Cable	26-8
26.6.3	Grounding	26-12
26.6.4	Equipment Design	26-15
26.6.5	Facility Power	26-17
26.6.6	Motors and Controllers	26-18
26.6.7	Equipment Rooms and Locations	26-19
26.6.8	Equipment Emission and Immunity Limits	26-20
26.6.9	FCC Type Accepted Radio Equipment	26-21
26.6.10	Human Exposure to Electromagnetic Fields	26-22
26.7	Automatic Train Control EMC Design Criteria	26-24
26.7.1	Equipment and EMC Considerations	26-24
26.7.2	Cable	26-26
26.7.3	Grounding	26-30
26.7.4	Equipment Design	26-33
26.7.5	Facility Power	26-36
26.7.6	Motors and Controllers	26-37
26.7.7	Equipment Rooms and Locations	26-38
26.7.8	Equipment Emission and Immunity Limits	26-39
26.7.9	FCC Type Accepted Radio Equipment for ATC	26-41
26.7.10	Human Exposure to Electromagnetic Fields	26-42
26.8	Traction Electrification System EMC Design Criteria	26-43
26.8.1	Equipment and EMC Considerations	26-43
26.8.2	Cable	26-45
26.8.3	Grounding	26-49
26.8.4	Equipment Design	26-52
26.8.5	Facility Power	26-55
26.8.6	Motors and Controllers	26-57
26.8.7	Equipment Rooms and Locations	26-58
26.8.8	Equipment Emission and Immunity Limits	26-59
26.8.9	FCC Type Accepted Radio Equipment	26-60
26.8.10	Human Exposure to Electromagnetic Fields	26-60
26.8.11	Animal Exposure to Electromagnetic Fields	26-62
26.8.12	Effects on Parallel Adjacent Metal Structures	26-62
26.9	Rolling Stock EMC Design Criteria	26-63
26.9.1	Equipment and EMC Considerations	26-63
26.9.2	Cable	26-64

26.9.3	Grounding.....	26-67
26.9.4	Equipment Design.....	26-70
26.9.5	Facility Power.....	26-73
26.9.6	Motors and Controllers.....	26-73
26.9.7	Equipment Locations.....	26-73
26.9.8	Equipment Emission and Immunity Limits .....	26-73
26.9.9	FCC Type Accepted Radio Equipment .....	26-75
26.9.10	Human Exposure to Electromagnetic Fields.....	26-76
26.10	Station and Facility Equipment EMC Design Criteria .....	26-78
26.10.1	Equipment and Considerations.....	26-78
26.10.2	Cable .....	26-80
26.10.3	Grounding.....	26-84
26.10.4	Equipment Design.....	26-87
26.10.5	Facility Power .....	26-89
26.10.6	Motors and Controllers.....	26-90
26.10.7	Equipment Rooms and Locations .....	26-91
26.10.8	Equipment Emission and Immunity Limits .....	26-92
26.10.9	FCC Type Accepted Radio Equipment for Stations and Facilities.....	26-93
26.10.10	Human Exposure to Electromagnetic Fields.....	26-94

## Tables

Table 26-1:	Electromagnetic Compatibility Cable Categories .....	26-8
Table 26-2:	Grounding Categories .....	26-13
Table 26-3:	Electromagnetic Compatibility Cable Categories .....	26-27
Table 26-4:	Grounding Categories .....	26-31
Table 26-5:	Electromagnetic Compatibility Cable Categories .....	26-46
Table 26-6:	Grounding Categories .....	26-50
Table 26-7:	Electromagnetic Compatibility Cable Categories .....	26-64
Table 26-8:	Grounding Categories .....	26-68
Table 26-9:	Electromagnetic Compatibility Cable Categories .....	26-80
Table 26-10:	Grounding Categories .....	26-85

## Figures

Figure 26-1:	Worst-Case Magnetic Field due to a 930 A train, close to the track.....	26-96
Figure 26-2:	Worst-Case Magnetic Field due to a 930 A train, farther from the track.....	26-97



## Acronyms

ac	Alternating Current
ATC	Automatic Train Control
CCTV	Closed Circuit Television
CEC	California Electric Code
CENELEC	European Committee for Electrotechnical Standardization
CFR	Code of Federal Regulations
CHSTP	California High-Speed Train Project
COTS	Commercial Off-The-Shelf
CPUC	California Public Utilities Commission
dc	Direct Current
EMC	Electromagnetic Compatibility
EMI	Electromagnetic Interference
EMI/EMF	Electromagnetic Interference/Electromagnetic Field
FCC	Federal Communications Commission
FRA	Federal Railroad Administration
GSM-R	Global System for Mobile Communications – Railway
HVAC	Heating, Ventilation, and Air Conditioning
ICD	Implantable Cardioverter Defibrillator
IEEE	Institute of Electrical and Electronics Engineers
IG	Insulated Ground
MOD	Motor Operated Disconnect
MPE	Maximum Permissible Exposure
NEC	National Electric Code
NFPA	National Fire Protection Association
OCS	Overhead Contact System
RF	Radio Frequency
rms	root mean square
RS	Rolling Stock
SCADA	Supervisory Control and Data Acquisition Subsystems
SRS	Signal Reference Structure
TES	Traction Electrification System
TPF	Traction Power Facility
UL	Underwriters Laboratories
UPS	Uninterruptible Power Supplies

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015



## 26 Electromagnetic Compatibility and Interference

### 26.1 Scope

The Electromagnetic Compatibility (EMC) Program scope includes the following:

- Electromagnetic interactions and related design characteristics
- CHSTS equipment and facilities
- Equipment and facilities of CHSTS neighbors

CHSTS equipment and facilities shall work with and not interfere with other CHSTS equipment and facilities and shall provide mitigations as needed to work with neighbor equipment and facilities.

The EMC design scope includes the following:

- Design aspects of the equipment and facilities which can electromagnetically interact with themselves, with other equipment and facilities, and with the equipment and facilities of system neighbors
- CHSTS Communications, Automatic Train Control (ATC), Traction Electrification System (TES), Rolling Stock (RS), and Station and Facility Equipment

Generally, if a change is required in existing neighbor equipment and facilities to resolve an EMC issue, the CHSTP must make a specific agreement with the neighbor.

CHSTP EMC interaction scope includes the following:

- Design scope equipment and facilities
- Passengers, staff, and neighbors
- Neighbor equipment and facilities
- Electronic devices carried or used by CHSTS passengers and staff in CHSTS facilities and trains

Neighbor equipment and facilities include the following:

- Electronic devices of neighbors and of California public safety, government, utility, and industrial staff
- Implanted medical devices in CHSTS passengers, staff, and neighbors
- Commercial, residential, and industrial buildings and the industrial and commercial equipment used by the buildings and their occupants
- Freight, passenger, commuter, and transit railroads, in shared corridors and adjacent land

- 1 • Airports
- 2 • Parallel fences, pipelines, metal structures
- 3 • Farms and feedlots and their animals

## 26.2 Regulations, Codes, Standards, and Guidelines

4 Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.

- 5 • Federal Communications Commission (FCC)
  - 6 – FCC, Part 15 Part 15 of Title 47 of the Code of Federal Regulations (CFR)
- 7 • California Building Standards Commission: California Electrical Code (CEC)
- 8 • California Public Utilities Commission (CPUC) General Orders (GOs) and Decisions
  - 9 – CPUC GO No. 95 – Overhead Electric Line Construction
  - 10 – CPUC GO No. 131 – Rules Relating to the Planning and Construction of Electric
  - 11 Generation, Transmission/Power/Distribution Line Facilities and Substations Located in
  - 12 California
  - 13 – CPUC D93-11-013 – interim EMF Policy
  - 14 – CPUC D06-01-042 – updated EMF Policy
- 15 • National Fire Protection Association (NFPA) Codes and Standards
  - 16 – NFPA 70 – National Electric Code (NEC)
  - 17 – NFPA 780 – Standard for the Installation of Lightning Protection Systems
- 18 • European Committee for Electrotechnical Standardization (CENELEC) Standards
  - 19 – EN 45502-2-1, Active implantable medical devices - Part 2-1: Particular requirements for
  - 20 active implantable medical devices intended to treat bradyarrhythmia (cardiac
  - 21 pacemakers).
  - 22 – EN 45502-2-2, Active implantable medical devices - Part 2-2: Particular requirements for
  - 23 active implantable medical devices intended to treat tachyarrhythmia (includes
  - 24 implantable defibrillators).
  - 25 – EN 45502-2-3, Active implantable medical devices - Part 2-3: Particular requirements for
  - 26 cochlear and auditory brainstem implant systems.
  - 27 – EN 50121-1, Railway applications – Electromagnetic Compatibility – Part 1: General
  - 28 – EN 50121-2, Railway applications – Electromagnetic Compatibility – Part 2: Emission of
  - 29 the whole railway system to the outside world
  - 30 – EN 50121-3-1, Railway applications – Electromagnetic compatibility – Part 3-1: Rolling
  - 31 stock – Train and complete vehicle

- EN 50121-3-2, Railway applications – Electromagnetic compatibility – Part 3-2: Rolling stock – Apparatus
- EN 50121-4, Railway applications – Electromagnetic Compatibility – Part 4: Emission and immunity of signaling and telecommunications apparatus
- EN 50121-5, Railway applications - Electromagnetic compatibility – Part 5: emission and immunity of fixed power supply installations and apparatus
- EN 50155, Railway applications – Electronic equipment used on rolling stock
- EN 50238-1, Railway applications – Compatibility between rolling stock and train detection systems
- EN 50238-2, Railway applications – Compatibility between rolling stock and train detection systems – Railway applications – Part 2: Compatibility with track circuits
- EN 50238-3, Railway applications – Compatibility between rolling stock and train detection systems – Railway applications – Part 3: Compatibility with axle counters
- EN 50388, Railway applications – Power supply and rolling stock – Technical criteria for the coordination between power supply (substation) and rolling stock to achieve interoperability
- EN 50500, Measurement procedures of magnetic field levels generated by electronic and electrical apparatus in the railway environment with respect to human exposure
- American Public Transportation Association (APTA) Standards, SS-E-10-98, Standard for the Development of an Electromagnetic Compatibility Program Plan
- American Railway Engineering and Maintenance-of-Way Association (AREMA) Communications and Signals Manual of Recommended Practice
- Federal Railroad Administration (FRA)
  - 49CFR 229, Railroad Locomotive Safety Standards
  - 49CFR 236, Rules, Standards, And Instructions Governing The Installation, Inspection, Maintenance, And Repair Of Signal And Train Control Systems, Devices, And Appliances
  - 49CFR 238, Passenger Equipment Safety Standards
- Federal Transit Administration (FTA)
  - UMTA-MA-06-0153-85-11, Radiated Interference in Rapid Transit Signaling Systems - Volume II: Suggested Test Procedures
  - UMTA-MA-06-0153-85-6 (also identified as UMTA-MA-06-0153-87-2), Conductive Interference in Rapid Transit Signaling Systems Volume II: Suggested Test Procedures
  - UMTA-MA-06-0153-85-8, Inductive Interference in Rapid Transit Signaling Systems - Volume II: Suggested Test Procedures

- 1 • Institute of Electrical and Electronics Engineers (IEEE) Standards
- 2     – IEEE Std C95.1, IEEE Standard for Safety Levels with Respect to Human Exposure to
- 3       Electromagnetic Fields, 3 kHz - 300 GHz
- 4     – IEEE Std C95.6, IEEE Standard for Safety Levels with Respect to Human Exposure to
- 5       Electromagnetic Fields, 0 - 3 kHz
- 6     – IEEE Std 1100, Recommended Practice for Powering and Grounding Electronic
- 7       Equipment
- 8     – IEEE Std 1143, IEEE Guide on Shielding Practice for Low Voltage Cables
- 9     – IEEE Std 142, Recommended Practice for Grounding of Industrial and Commercial
- 10       Power Systems
- 11     – IEEE Std 518, IEEE Guide for the Installation of Electrical Equipment to Minimize
- 12       Electrical Noise Inputs to Controllers from External Sources
- 13     – IEEE Std 519, Recommended Practice and Requirements for Harmonic Control in
- 14       Electrical Power Systems
- 15     – IEEE Std 525, IEEE Guide for the Design and Installation of Cable Systems in Substations
- 16     – IEEE Std C2, National Electrical Safety Code
- 17 • Architectural Electromagnetic Shielding Handbook, Leland H. Hemming, IEEE Press, 1992
- 18     and any revisions
- 19 • U.S. Department of Defense (USDOD) Standards: MIL-HDBK-419A, Grounding Bonding
- 20     and Shielding for Electronic Equipment and Facilities (Volume I, Basic Theory, 1987)
- 21 • Pacific Gas & Electric Company Transmission Line EMF Design Guidelines, May 1994
- 22 • Southern California Edison EMF Design Guidelines For Electrical Facilities, September 2004
- 23 • CPUC Summary, San Diego Gas and Electric, summary of EMF related activities of the
- 24     CPUC, <http://sdge.com/safety/electric/emf/emfActivities.shtml>
- 25 • Federal Communications Commission (FCC) Reports
- 26     – FCC OET-65, Evaluating Compliance with FCC Guidelines for Human Exposure to
- 27       Radio frequency Electromagnetic Fields, FCC Office of Engineering and Technology
- 28       Bulletin 65, Edition 97-01, August 1997
- 29     – FCC OET-65c, Evaluating Compliance with FCC Guidelines for Human Exposure to
- 30       Radiofrequency Electromagnetic Fields, FCC Office of Engineering and Technology
- 31       Supplement C (Edition 01-01) to OET Bulletin 65 (Edition 97-01)

### **26.3 Description of Electromagnetic Compatibility Program and Electromagnetic Interference**

32 The EMC Program objective is to achieve EMC between all EMC design scope and interaction  
33 scope equipment and facilities.

The EMC program shall ensure that Electromagnetic Interference (EMI) does not adversely affect the following:

- The health of passengers, staff, and neighbors
- The safety or dependability of the system and service
- The safety or dependability of neighbor equipment and facilities

To meet the EMC Program objective, the EMC Program specifies the following:

- Activities and deliverables at each project stage and phase for the Authority, equipment suppliers, construction contractors, and subcontractors
- EMC design requirements to be included in the procurement specifications of each affected system or piece of equipment and each affected construction contract
- EMC analyses and tests to demonstrate compliance with EMC requirements
- Design guidelines, criteria, and methods, including in this chapter

## 26.4 General Design Requirements

CHSTS equipment and facilities shall be electromagnetically compatible with one another and shall provide mitigations as needed to be compatible with the equipment and facilities of neighbors.

The Contractor shall apply the EMC design criteria to all equipment, systems, and facilities, specifically including the following:

- Communications
- Automatic Train Control
- Traction Electrification System
- Rolling Stock
- Station and Facility Equipment

## 26.5 Overview of EMC Design Criteria Categories

The EMC design criteria cover equipment, cable, grounding, facility and high power, motors and controllers, equipment rooms and location, emission and immunity limits, FCC type-accepted radio equipment, and human exposure. The following is an overview of EMC design criteria:

- **Cable** – Cables shall be designed with proper shielding, shield grounding, entry protection, and termination. Each cable may be grouped only with others with similar signal type and

1 energy level. Each cable or group shall be segregated appropriately from other cable groups.  
2 Cable runs shall be placed in conduit, raceway, or duct as needed to provide segregation  
3 and prevent magnetic or electric coupling from high-energy sources. The Contractor shall  
4 use fiber optic cable where practical for EMC. Power cables shall be treated according to  
5 required practice for their voltage class.

- 6 • **Grounding** – Grounding shall conform to the listed standards, provide a suitable safety  
7 ground, and signal reference structure ground connections. Long adjacent fences and  
8 pipelines shall be regularly grounded or if not grounded, divided into insulated sections to  
9 prevent electric shock.

- 10 • **Equipment** – Equipment designs shall control emissions and enhance immunity. Design  
11 considerations shall include placement, enclosures, filters, modulation methods,  
12 interconnect design, and component characteristics.

- 13 • **Facility Power** – Ac power for equipment shall be properly taken from separate feeder and  
14 branch circuits, isolated, regulated, backed up, and protected as required. Ac power for  
15 remote trackside locations can be taken from the negative feeder by a dedicated transformer  
16 and disconnect, and filtered and voltage stabilized to protect supplied equipment. High-  
17 current power supply ac cables shall be run twisted together, in metal conduit where  
18 possible, to minimize magnetic coupling. Traction power cables shall be run with the  
19 smallest feasible separation of supply feed and return cables. Layout shall minimize the  
20 loop area of high-current cables. Utility power distribution lines shall be routed and carried  
21 to system facilities according to applicable electromagnetic interference/electromagnetic  
22 field (EMI/EMF) regulations and guidelines.

- 23 • **Motors and Controllers** – Motor starter or inverters shall be provided with suitable  
24 protection and line and load filtering to minimize harmonics, transients, and surges at start  
25 and stop. Wiring shall be by twisted and/or shielded cables in conduit as appropriate.

- 26 • **Equipment Rooms and Location** – Within physical constraints of planned facilities,  
27 equipment shall be located so that high power sources are physically separated as far as  
28 practical from the most susceptible equipment. Shielding shall be provided as needed.

- 29 • **Emission and Immunity Limits** – Equipment shall be designed and tested to conform to the  
30 selected emission and immunity limits. Commercial off-the-shelf (COTS) equipment shall  
31 meet the specified standards. Custom equipment shall meet the selected standards, which  
32 are FCC Part 15, EN 50121 series, and applicable standards. In cooperation with the adjacent  
33 railroads, the Contractor shall mitigate coupling of system 60 Hz power into track circuits of  
34 adjacent railroads. The Contractor shall coordinate with the operator of any airport adjacent  
35 the alignment to ensure EMC.

- 36 • **FCC Type-Accepted Radio Equipment** – Radio equipment shall be FCC-type approved.  
37 Frequencies for licensed radio equipment shall be coordinated within the system and with  
38 other California users. Equipment that transmits or receives on a specific frequency shall be  
39 coordinated with the established list of frequencies used by other equipment. Industrial,  
40 scientific, and medical device frequency band (ISM) equipment shall be FCC Type Accepted



in the 2.4 or 5.8 GHz band. ISM design applications shall operate adequately with interference from other ISM band users.

- **Human Exposure** – Placement of radio transmit antennas shall result in human exposure to fields below limits. Traction electrification facilities shall be posted with signs alerting staff with implanted medical devices such as pacemakers of potentially hazardous electromagnetic field (EMF) levels.
- **Animal Exposure** - Placement of radio transmit antennas shall result in animal exposure to EMF levels below the same limits that apply to humans.

## 26.6 Communications EMC Design Criteria

### 26.6.1 Equipment and EMC Considerations

Communications systems equipment is described in the *Communications* chapter. The equipment comprises the following:

- Wired wayside communication systems will be connected by copper wires and by fiber optic cable. Examples include Supervisory Control and Data Acquisition Subsystems (SCADA), Closed Circuit Television (CCTV), data network, intercom and public address, telephone, security and access control, traction power control, and fire and life safety subsystems and functions.
- Radio systems will be on the wayside and onboard trains. Examples include ATC data and voice radio, operations and maintenance radio, voice and data; police, fire, and emergency public safety radio; Wifi and other unlicensed radio; and ATC radio such as 160 MHz, 220 MHz, and Global System for Mobile Communications - Railway.
- Equipment includes cabinets and contents, control panels, displays, cables, conduits, ducts, raceways, enclosures, and antennas.

Cell phone service and passenger information and entertainment radio will be provided separately. They are not in the present scope.

Communication equipment EMC provisions shall ensure the following:

- Safe and dependable operation
- No interference with or from neighbors
- Compliance with human exposure limits

The following design criteria shall apply to communications systems and to the communication system interfaces with ATC, trains, traction power, and station systems and equipment:

- Cables and cable segregation

- 1 • Grounding
- 2 • Equipment design
- 3 • Facility power
- 4 • Motors and controllers
- 5 • Equipment rooms, cabinets, and locations
- 6 • Equipment emission and immunity limits
- 7 • Requirements for radios
- 8 • Human exposure limits
- 9 The communication systems EMC design criteria are described in the following subsections.

## **26.6.2 Cable**

### **26.6.2.1 General**

10 The Contractor shall use optical interconnection cables wherever it is technically feasible and  
 11 cost-effective. Optical connections between equipment and equipment locations are immune to  
 12 EMI.

13 The Contractor shall select, install, and connect electrical cables with proper shielding, shield  
 14 grounding, conduit or duct protection, entry protection, routing, and termination to control  
 15 EMI in copper electrical cables as described in this section.

16 Electrical cable provisions shall comply with the National Electric Code (NEC) Chapter 2  
 17 (Wiring and Protection), Chapter 3 (Wiring Methods and Materials), Chapter 4 (Equipment for  
 18 General Use), Chapter 7 (Special Conditions) and Chapter 8 (Communication Systems),  
 19 wherever the NEC applies, except to the extent other Design Criteria provisions specify  
 20 otherwise.

### **26.6.2.2 Electrical Cable Categories**

21 For EMC purposes, each cable shall be assigned to 1 of the cable categories in Table 26-1.

**Table 26-1: Electromagnetic Compatibility Cable Categories**

ID	Name	Examples, References, Comments
C1	Power at 600 V ac RMS or higher	115 kV feeder
C2	Traction Power	25 kV ac traction power, negative feeder, and return cables
C3	Power at less than 600 V ac RMS	480 V ac, 120 V ac; ATC cables including track switch motors, signal lights, and related circuits; OCS Motor Operated Disconnects; lighting circuits, heating, ventilation and air conditioning (HVAC) motors, uninterruptible power supplies (UPS), etc.

ID	Name	Examples, References, Comments
C4	NEC Class 1 circuits	NEC Class 1 power limited circuits per NEC 725.41 (A), limited to less than 30 V and 1000 VA. NEC Class 1 remote-control and signaling circuits per NEC 725.21 (B), less than 600 V. Includes safety circuits such as fire alarm
C5	NEC Class 2 and 3 circuits	Power sources for NEC Class 2 and Class 3 circuits per NEC 725.121, shall be limited per Chapter 9 Table 11(a) and Table 11(b)
C6	Unshielded twisted pair signal circuits	Telephone, speaker level audio output, Ethernet, track circuit transmitter and receiver cables
C7	Shielded signal circuits including shielded twisted pair, coax, and multi-axial cables	CCTV from camera to converter box, microphone and line level audio input, line level audio output
C8	Leaky coax cable	Distributed communication antenna
C9	Optical cable	Backbone Network, Local Area Network, CCTV camera feeds from converter box to communications room

### 26.6.2.3 Cable Separation Criteria

Cables in the same cable category may be run together in the same steel conduit, raceway, duct, trough, manhole, or cable tray, if all cables that are grouped and run together are insulated to the highest voltage level present in the group.

Cables in different cable categories shall not be run together in the same cable assembly, conduit, raceway, duct, or cable tray, without engineering justification documenting the specific cables, signals, and protections against EMI for the proposed combination. However, optical cables (C9) may be run together with any other cable.

Wayside cables running parallel to and crossing the track shall be placed in cable troughs and duct banks. These will include communication cables, train control cables, OCS motor operated disconnects (MODs), 480 VAC, 125 VDC and 48 VDC. The cable trough shall not contain C1, power at greater than 600 V, or C2, traction power cable. Cables in the cable trough, manholes, and duct banks shall be separated by cable category, so that a suitable separation is maintained between, for example 480 VAC cables and ethernet cables. If EMI impact occurs between cables in the cable trough or duct banks, the Contractor shall apply suitable shielding techniques to reduce the emission from the EMI source, to increase the immunity of the EMI receiver, or both. Shielding techniques include cable shields and use of metal conduit.

Traction return cables shall be run in troughs, manholes, and ducts separate from other wayside cables. Traction return cables may share a duct bank but not a duct with other cable categories.

Wires carrying circuits belonging to a single cable category but which use different signal levels shall not be run together in the same cable, without engineering justification documenting the specific cables, signals, and protections against EMI for the proposed combination.

1 Where cables of dissimilar type, service, or signal level cross, the crossing of the cables and their  
2 conduits and raceways shall be perpendicular where practical.

3 Cable separation design shall conform to IEEE Standard 1100-2005, Recommended Practice for  
4 Powering and Grounding Electronic Equipment, Section 9.9.

#### **26.6.2.4 Cable Shielding**

5 The Contractor shall use shielded cables and steel conduits and ducts to protect components,  
6 circuits, and systems against the effects of undesirable external disturbing EMI sources. A cable  
7 shield can:

- 8 • Increase immunity and provide additional protection against interference to the enclosed  
9 circuits; and
- 10 • Reduce emissions and provide additional reduction of the level of interference emitted by  
11 an enclosed circuit.

12 Wire and cable shielding and shield grounding design for electronic signals shall conform to  
13 IEEE Std 1100-2005, Sections 3.3, 4.9, 9.9, 10.2, and 10.4; and to IEEE Std 1143-1994, Guide on  
14 Shielding Practice, Section 7.

#### **26.6.2.5 Cable Shield Grounding**

15 The connections of cable shields and the connection of the shields to the grounding system  
16 determine the effectiveness of the shielding in mitigating EMI.

17 The Contractor shall specify and use a coordinated and consistent method for terminating and  
18 connecting cable shields to equipment. The method shall conform to these criteria. The  
19 Contractor shall document the specific provisions for cable shield grounding at the system and  
20 detail level as part of a Cable EMC Schedule. The Contractor shall ensure that the Cable EMC  
21 Schedule is fully implemented across the systems.

22 The general topology for shield connection to power supply ground shall conform to IEEE Std  
23 1100-2005, Sections 8.5, 9.9, 10.2, and 10.4.

24 The Contractor shall properly terminate cable conductors, shield, and drain wire, if used, on the  
25 connector to prevent the inadvertent grounding of the shield or conductor connection via the  
26 connector shell to the equipment enclosure or to another unintended potential.

#### **26.6.2.6 Conduit and Duct**

27 The Contractor shall make the following provisions for conduit and duct:

- 28 • Run specified cables in cable troughs and in connecting ducts.
- 29 • Metal conduits or ducts for cables provide mechanical protection and electrical safety as  
30 well as additional electromagnetic isolation compared to physical separation of cables.

1 Metal conduits and ducts increase the immunity of and decrease the emissions from  
2 enclosed circuits.

- 3 • Use steel conduit and ducts; do not use aluminum.
- 4 • Run cables in steel conduit or steel duct on any run that is outside of a single enclosure for  
5 any part of its route, except for cables in the cable troughs and cables for which engineering  
6 justification documents the basis for the cables to be run out of conduit.

#### 26.6.2.7 Conduit and Duct Bonding and Grounding

7 Metal conduit and duct can serve as an overall shield for cables carrying sensitive signals, and  
8 for cables carrying signals, which tend to emit EMI. To provide a shielding function, metal  
9 conduit and duct shall be grounded and bonded per the NEC Article 250 where not in conflict  
10 with the *Grounding and Bonding Requirements* chapter.

11 Only hardware listed by a nationally recognized testing laboratory such as Underwriters  
12 Laboratories (UL) shall be used to join and bond conduits and ducts to enclosures, including  
13 conduit fittings and EMI gaskets.

14 Fully coordinate insulated ground (IG) ac circuit conduit routing and connections with the  
15 grounding of the systems powered by insulated ground circuits. Refer to Section 26.6.3.6 for IG  
16 details.

#### 26.6.2.8 Entry Protection

17 The Contractor shall provide the following:

- 18 • Room entry EMI impulse protection for wires entering an equipment room
- 19 • Enclosure entry EMI impulse protection for wires entering an equipment enclosure

20 The protection shall consider and be consistent with the cable type, the cable source, and the  
21 potential EMI hazards to be protected against. As noted above, cables entering an equipment  
22 room shall be enclosed in steel conduit unless the system designer provides specific technical  
23 justification for another entry protection method consistent with the above criteria.

24 Room entry and line entry impulse protection shall consist of line-to-line and/or line-to-ground  
25 protection, as appropriate, for each circuit type. Circuit protections on wires include  
26 combinations of lightning arrestors, transient suppressors, fuses, spark gaps, filters, ferrite  
27 beads and clamshells, etc.

#### 26.6.2.9 Cable Termination

28 The Contractor shall terminate all impedance controlled cables, such as coax, with the proper  
29 impedance.

#### 26.6.2.10 Circuit Loops

30 The Contractor shall make the following provisions for circuit loops:

- 1 • Minimize the area of circuit loops in which magnetic fields can couple.
- 2 • For high-current circuits, run the feed and return conductors as closely together as feasible
- 3 and minimize the length over which the feed and return cables are run separated from one
- 4 another, to reduce the flux generating loop. To improve the immunity of signal circuits, use
- 5 twisted pair cables.
- 6 • To minimize coupling between the flux generating loop and the signal circuit, maximize the
- 7 distance between power cables and signal cables, and minimize the distance over which the
- 8 signal cable runs near and parallel to the power cable.
- 9 • Ensure that design layout drawings have sufficient detail to show the physical routing of all
- 10 conduits and cables, so that EMC compatibility can be designed and documented.

### **26.6.3 Grounding**

#### **26.6.3.1 General**

11 The Contractor shall make the following grounding provisions:

- 12 • Provide each equipment item with a suitable safety ground and suitable EMI-controlling
- 13 signal ground and return connections.
- 14 • Provide a Grounding EMC Schedule and drawings detailing the specific design provisions
- 15 for each equipment item and its grounding connections.
- 16 • Prepare an overall grounding diagram that defines the grounding scheme for each system
- 17 and subsystem, the ac power (both non-insulated ground and insulated ground) circuits
- 18 serving these systems, and the interconnection means and interconnection points of the
- 19 grounding systems.

20 For safety grounding requirements, refer to the *Grounding and Bonding Requirements* chapter.

#### **26.6.3.2 Grounding Categories**

21 For EMC purposes, the Contractor shall use the grounding categories in Table 26-2 to categorize

22 each ground connection. The Contractor shall apply these grounding categories to achieve the

23 following:

- 24 • Minimize the effects of electrical noise generated by the system and by neighboring
- 25 electrical equipment.
- 26 • Minimize disturbing effects within directly affected control equipment and propagating
- 27 effects on associated equipment, which might result from ground currents, fault currents, or
- 28 nearby lightning strikes.
- 29 • Minimize the shock exposure potential which might appear on non-current-carrying
- 30 equipment or conducting structures, in case an insulation failure energizes the enclosing
- 31 structure with a dangerous voltage. Proper grounding of equipment and conducting
- 32 structures will minimize the shock hazard due to a power line insulation failure.

- 1 • Provide a discharge path for stray radio frequency (RF) energy.

**Table 26-2: Grounding Categories**

ID	Name	Examples, References, Comments
G1	Lightning protection ground	The building lightning protection ground shall connect to the power safety ground, at a single point per NFPA 780-2004: Standard for the Installation of Lightning Protection Systems.
G2	Power safety ground	Governed by the NEC Article 250
G3	Sensitive electronic equipment ground	The sensitive electronic equipment ground should connect by bonding to the power safety ground at only a single point.
G4	Signal reference structure (SRS) ground	G4 SRS ground configurations include grids and planes which provide a high-frequency common ground reference to which control and communications signals are referenced. A matrix ground mat in or under a sensitive equipment room is a typical SRS implementation. The SRS provides a better reference than a single G3 ground as it provides a lower impedance at high frequencies. If the connection from a G3 sensitive electronic equipment ground to the G2 power safety ground is long, higher impedance in the ground structure results in more electrical noise. A G4 SRS can resolve the problem.
G5	Traction power return	The traction power return and running rails are grounded at each Traction Power Facility (TPF) via a connection from the TPF return bus to the TPF ground grid. The running rails are also connected to the aerial ground wire and to the earth at impedance bond locations at intervals along the track.

2

3 The Contractor shall make the following provisions:

- 4 • Provide a direct, dedicated ground connection when connecting signal and communication  
5 ground circuits to a TPF ground grid.
- 6 • Identify, coordinate, keep separate, insulate, and appropriately connect the grounds in each  
7 of the ground categories in Table 26-2. Where necessary, identify special provisions for  
8 grounds in tunnels, trenches, at-grade ballasted or non-ballasted track, and aerial structures  
9 or bridges.
- 10 • Integrate and coordinate the lightning protection grounding system with the other  
11 grounding system designs.

12 Optical or other galvanic isolation of signals may be needed to create and maintain the  
13 separation of grounds.

### 26.6.3.3 Traction Power Return

The traction power return circuit shall be connected to the TPF ground grid at each TPF. In addition, the running rails shall be connected via impedance bonds to the earth and to the aerial ground wire at intervals between TPF locations, to control step and touch voltage hazards.

For safety grounding requirements, refer to the *Grounding and Bonding Requirements* chapter.

### 26.6.3.4 Industry Standard for Powering and Grounding Electronic Equipment

The designs for powering and grounding electronic equipment shall conform to the applicable portions of the National Electric Code of IEEE Std 1100 and IEEE Std 142, as adapted for the system environment.

For equipment designs and connections, the Contractor shall use the specified techniques for powering and grounding electronic equipment.

Note that in the recommended power and grounding schemes, the "grounded conductor" refers to that leg of the circuit (usually the neutral) that is intentionally held at ground potential. The "grounded conductor" is part of the current-carrying circuit. The "grounding conductor" refers to the conductor(s) that connect(s) exposed metal parts of a device to ground; primarily for safety, secondarily for performance. The "grounding conductor" is not part of the current-carrying circuit.

### 26.6.3.5 Grounding Design Criteria

The Contractor shall:

- Use a safety grounding technique for equipment in racks per IEEE Std 1100-2005, Sections 3.3, 4.9, 8.5, 9.9, 10.2, and 10.4; and
- Use a high frequency or RF grounding technique for high frequency signals per IEEE Std 1100-2005, Sections 3.3, 4.6, 8.5, 9.9, 10.2, and 10.4.

The grounding system shall provide that when electronic equipment has a conducting connection to other devices within a structure that all interconnected devices be referenced at the same lightning protection ground (G1) to minimize possible lightning damage.

### 26.6.3.6 Insulated Ground Design Criteria

Sensitive equipment may use an NEC-compliant isolated ground to provide basic fault/personnel protection and to protect the equipment. These Design Criteria refer to the NEC-compliant isolated ground using the IEEE Std 1100 nomenclature, which is "insulated ground" (IG).

For IG connections for equipment in racks, the Contractor shall modify the safety grounding configuration from that shown in IEEE Std 1100-2005, Figures 8-17 and 8-18, by placing a listed nonmetallic raceway fitting between the equipment rack and the conduit system, as shown in



1 NEC Handbook Exhibit 250.45. The configuration shall ensure that equipment and enclosures  
2 served by an IG power circuit are:

- 3 • Correctly and safety grounded at the IG location; and
- 4 • Not inadvertently bonded to unintended non-insulated grounding paths.

#### 26.6.4 Equipment Design

##### 26.6.4.1 General

5 Equipment and system designs shall control emissions and increase EMI immunity. Design  
6 considerations include placement, enclosures, filters, interconnect design, component  
7 characteristics, and modulation methods.

8 The Contractor shall:

- 9 • Specify and implement design provisions for equipment placement, enclosures, filters,  
10 interconnect design, component characteristics, and modulation methods, for each  
11 equipment item, per the following subsections; and
- 12 • Ensure that plan and elevation drawings show sufficient detail of the layout of equipment in  
13 each rack, the layout of racks within a room, and ancillary equipment within a room, so that  
14 EMC compatibility can be designed and documented.

##### 26.6.4.2 Placement

15 The Contractor shall place high-power potential sources as far as practical from sensitive  
16 potential susceptible equipment, to minimize magnetic and electric field coupling between  
17 nearby pieces of equipment.

##### 26.6.4.3 Enclosures

18 The Contractor shall make the following provisions for enclosures:

- 19 • Install electronic equipment in a suitable EMI-Shielded Equipment Rack Cabinet.
- 20 • Protect signal and power lines entering enclosures from EMI by using appropriate cable  
21 termination techniques as well as proper conduit fittings, gaskets, etc.
- 22 • Enclose cables entering or leaving electronic enclosures in steel conduit, unless otherwise  
23 specifically justified by engineering documentation.
- 24 • Use a double-wall-shielded equipment enclosure when necessary for sensitive equipment  
25 located near a powerful emissions source. In such cases, the inner enclosure shall be  
26 connected to the signal reference structure ground or IG, while the outer enclosure shall be  
27 connected to the power safety ground.
- 28 • Use shielded enclosures, metal ducts, and metal conduits to control the electromagnetic  
29 coupling between and from equipment and wiring.

1 Within enclosures, the Contractor shall make the following provisions:

- 2 • Carefully bundle, twist, shield, protect, route, and properly terminate high-current cables  
3 such as motor cables, to minimize uncontrolled emissions. The Contractor shall ensure that  
4 the cable installation does not exceed the minimum bend radius for the cable.
- 5 • Connect low voltage cable shields to the appropriate low voltage ground at the source and,  
6 per IEEE Std 1100-2005, Sections 3.3 and 8.5 and IEEE Std 1143-1994, Section 7 to the  
7 receiver-end equipment.
- 8 • Use twisted pair and shielded cables, coax, or multiwire for sensitive signals.
- 9 • Place inductive components perpendicularly to nearby susceptible components to minimize  
10 the exposure to stray flux.

#### **26.6.4.4 Filters**

11 The Contractor shall make the following provisions for filters:

- 12 • Use appropriate low-pass filters on wired connections. Filters minimize conducted  
13 emissions from a piece of equipment into other pieces of equipment and improve the  
14 immunity of the equipment to conducted emissions on the connecting wires.
- 15 • Provide filters on power inputs and outputs to and from ac inverters and converters,  
16 transmitters, receivers, and signal circuits.
- 17 • Use supply suitable transient suppressors for relay contacts switching inductive loads,  
18 including relay and contactor coils.

#### **26.6.4.5 Interconnect Design**

19 The Contractor shall use appropriate techniques to minimize emission and maximize immunity  
20 of power and signal conductors, including the following:

- 21 • Arrange wires, cables, and conduits to physically separate signals with different signal type,  
22 voltage and energy levels.
- 23 • Separate and shield circuits carrying power, high level signals, and low level signals.
- 24 • Use balanced circuits with coordinated current return paths.
- 25 • Minimize the loop area between source and return conductors.
- 26 • Use photocouplers or optical fiber to galvanically isolate circuit potentials when needed.
- 27 • Shield all sensitive signal conductors in suitable electric and magnetic shielding structures  
28 using appropriate combinations of steel conduit and shielded cable.
- 29 • Connect shields to appropriate system ground at the high energy end.

30 The Contractor designs shall include the following:

- 1 • Matching networks to match transmission line and antenna systems to maximize  
2 performance and minimize loss and reduce EMI for controlled impedance circuits such as  
3 radio frequency signals.
- 4 • Shielded cables that have shield coverage of at least 90 percent.
- 5 • Guard shields to improve immunity as required for sensitive electronic circuits.
- 6 • Optical cables, optically-isolated signals, or other galvanic isolation for connection between  
7 equipment enclosures, whenever practical.
- 8 • Isolation transformers, chokes and/or isolators for sensitive audio, video, and data circuits  
9 as required.

#### 26.6.4.6 Component Characteristics

10 The Contractor shall select appropriate components to reduce EMI. These components include  
11 the following:

- 12 • For power converters, such as UPS, dc-to-ac inverters, and variable speed motor drives, use  
13 inductor-capacitor line EMI filters on each power input and output, to reduce power  
14 frequency and switching harmonic currents and radiated emissions.
- 15 • Use ferrite beads and clamshells to reduce emissions from signal cables and mitigate line-to-  
16 ground (common mode) and line-to-line (differential mode) noise.
- 17 • Use LCD displays for computers. Do not use CRT displays.

#### 26.6.4.7 Modulation Methods

18 The Contractor shall:

- 19 • Use a modulation method, that controls and minimizes the conducted, inductive, and  
20 radiated emissions of the equipment for all equipment that converts power from  
21 1 frequency, voltage, or current level to another; and
- 22 • For each power converter, provide a report, manufacturer's data sheet, or other technical  
23 data to describe the modulation methods used as well as the mitigation methods used to  
24 minimize EMI.

### 26.6.5 Facility Power

#### 26.6.5.1 General Criteria

25 The Contractor shall make the following provisions for facility power:

- 26 • Take power for communication electronic circuits, heating, ventilation and air conditioning  
27 (HVAC) equipment, lighting, elevators and escalators, and other systems from separate  
28 branch circuits which are isolated, regulated, backed up, and protected as required. Facility  
29 power shall comply with the requirements of the NEC Chapter 2 (Wiring and Protection),  
30 Chapter 3 (Wiring Methods and Materials), and Chapter 4 (Equipment for General Use).

- 1 • Provide a Communications Power EMC Schedule and drawings detailing the specific
- 2 design provisions for each equipment item and its power and UPS connections.
- 3 • Prepare overall power diagrams that define the power distribution scheme for each system
- 4 and subsystem and show the ac power with both non-insulated ground and insulated
- 5 ground circuits serving these systems.

#### **26.6.5.2 High Power**

6 The Contractor shall run high-current power ac cables, including phase, ground, and neutral  
7 conductors, with minimum loop size and conductor separation, physically separated from  
8 sensitive circuits, or if near sensitive circuits, twisted or in steel conduit.

#### **26.6.5.3 Utility Power**

9 The Contractor shall make the following provisions for utility power:

- 10 • Provide a power distribution design that controls potential effects of EMI generated by high
- 11 power equipment on sensitive electronic equipment. Use separate transformers and
- 12 distribution buses to ensure high power loads are separated from sensitive loads.
- 13 • Use the physical placement of power distribution equipment to increase the immunity of
- 14 sensitive electronic equipment to EMI generated by high power equipment.
- 15 • Route utility power distribution lines to facilities according to applicable EMI/EMF
- 16 regulations and guidelines.

#### **26.6.5.4 Grounding of Uninterruptible Power Supply Circuits**

17 The Contractor shall follow IEEE Std 142-2007, Recommended Practice for Grounding of  
18 Industrial and Commercial Power Systems, Section 1.9.1, for UPS grounding arrangements.

#### **26.6.5.5 Remote Powered Locations**

19 Remote communication locations where utility power is not available may use power supplied  
20 from the Overhead Contact System (OCS) negative feeder, from a solar storage system or from  
21 another source.

22 Power at a remote location fed from the negative feeder shall be via a transformer, disconnect,  
23 and suitable filtering and power conditioning to protect connected equipment from the  
24 transients and harmonics present in the negative feeder.

25 The selected remote power supply shall make appropriate design provisions for cable, ground,  
26 shielding, filtering, and other EMC constraints.

#### **26.6.6 Motors and Controllers**

27 For motors and motor controllers used within the Communication Systems, such as for  
28 ventilation equipment, the following criteria shall apply:

- 1 • The Contractor shall provide a Motor and Controller EMC Schedule and drawings detailing  
2 the specific design provisions for each motor and controller of more than 1 hp.
- 3 • The Contractor shall provide each motor starter, controller, or inverter with suitable  
4 protection and line and load filtering to minimize transients and surges at start and stop.  
5 Provide twisted and/or shielded cables in conduit as appropriate.
- 6 • The Contractor shall run input power wires and cables to motors in steel conduit, locate  
7 input conductors as far as practical from sensitive equipment, and run wires from the  
8 output of the any UPS that powers motors in steel conduit.
- 9 • The Contractor shall use soft-start motor controllers or inverters to minimize conducted line  
10 transients caused by motor starts. When multiple motors are in a room, coordinate and  
11 delay the starting of the motors in a sequence to avoid the intense transient and line dip  
12 effects of all motors starting at once.

## 26.6.7 Equipment Rooms and Locations

### 26.6.7.1 Location

13 The Contractor shall make the following provisions for equipment locations:

- 14 • Within the physical constraints of the planned facilities, locate equipment so that  
15 high-power sources are physically separated as far as practical from susceptible equipment  
16 and cables.
- 17 • Where communication equipment, radio, ATC, and similar rooms are near traction power  
18 transformers, HVAC motors, or other high-current systems, locate the susceptible  
19 equipment as far as practical from the stray magnetic fields generated by the transformers,  
20 motors, and power cables.
- 21 • Preferably, locate high-current equipment and cables in rooms that are not adjacent (either  
22 vertically or horizontally) to rooms with sensitive and susceptible equipment. The  
23 Contractor shall run all input power cables to high-current equipment in steel conduit.
- 24 • Maximize distance between EMI emitter rooms and EMI susceptible receiver rooms. The  
25 Contractor shall lay out the equipment rooms with high-current equipment at 1 end of a  
26 suite of rooms and susceptible equipment at the other (far) end of the suite of rooms. To the  
27 extent practical, for example, the Contractor shall locate rooms so that they start from the  
28 left in a hypothetical plan:
  - 29 – Traction power, HVAC, or other high-current rooms would be at the leftmost position  
30 (room 1).
  - 31 – The electrical distribution room (room 2) would be adjacent to and to the right of  
32 room 1.
  - 33 – The UPS room (room 3) would be adjacent to and to the right of room 2.
  - 34 – The communications rooms would be adjacent to and to the right of room 3.

### 26.6.7.2 Equipment Room Shielding

When architectural or structural constraints do not allow room locations and cable and equipment placement per Section 26.6.7.1 and when required by the characteristics of sensitive equipment installed in a room, the Contractor shall use architectural shielding in the sensitive room to mitigate the electric and magnetic fields from the high-current equipment. Architectural shielding materials include steel sheet and steel mesh wall linings; other shielding wall linings such as sheet copper, copper mesh, or conductive polyamide mesh; EMI-shielded doors, windows, access panels, and air ventilation panels; EMI-conductive systems; shielding components; shielding laminates; and EMI-shielded equipment racks.

### 26.6.8 Equipment Emission and Immunity Limits

Typical rail and general-purpose communication systems make broad use of commercial-off-the-shelf (COTS) equipment. COTS equipment shall meet FCC regulations and other EMC standards governing emissions and immunity. Manufacturers of COTS equipment shall guarantee that their equipment meets required regulations and selected published standards, although they will generally not test to confirm or modify their equipment to meet standards other than the required and selected published ones.

All electrical and electronic equipment shall conform to the electromagnetic emission and immunity limits and related requirements in the following subsections.

Each item of electrical or electronic equipment shall be certified and documented to conform to the applicable limits. For equipment that is non-COTS, this documentation shall include a test report, manufacturer's performance data, and acceptance certificates from a qualified test laboratory. Each equipment supplier shall demonstrate and document compliance of each item.

#### 26.6.8.1 Commercial-Off-The-Shelf Communications Equipment Emission Limits

COTS equipment that is not radio transmission equipment shall meet the following conducted and radiated emission requirements of Part 15 of Title 46 of the Code of Federal Regulation (FCC Part 15), sub-part B:

- Section 15.107(b), Conducted Limits, Class A digital devices
- Section 15.109(b), Radiated Limits, Class A digital devices

FCC Part 15.3(h) defines Class A devices as digital devices that are marketed for commercial, industrial, or business use, not for use in general public or home applications.

FCC Part 15 does not provide immunity limits.

COTS non-radio equipment shall be proven in similar service and shall have a Certification or a Declaration of Conformity per FCC Part 15, sub-part B, Class A devices. For such equipment, the Contractor shall provide documentation of the suitability or test results. Suitability considerations are the following:

- The equipment is COTS.
- The equipment is suited for the planned application.
- The equipment has Certification or a Declaration of Conformity per FCC Part 15, sub-part B, Class A device regulations.

A set of COTS equipment, when installed in a compliant and correct manner in an adequate shielded equipment rack, can be treated as a single item of COTS equipment for the purposes of tests and documentation. However, treatment of a shielded rack of equipment as COTS does not relieve the supplier of responsibility to achieve and document electromagnetic compatibility for the equipment rack and all the equipment in it.

#### **26.6.8.2 Non-COTS Communications Equipment Emissions and Immunity**

Non-COTS communications equipment radiated and conducted immunity limits shall conform to EN 50121-4, Railway applications - Electromagnetic Compatibility, Part 4: Emission and immunity of signaling and telecommunications apparatus.

Per EN 50121-4, the non-COTS communications equipment radiated and conducted emission limits shall conform to EN 61000-6-4, Electromagnetic Compatibility, Part 6-4: Generic Standards - Emission Standard for industrial environments

Each communications equipment supplier shall provide equipment that conforms to the applicable limits, as well as test certification or documentation demonstrating compliance.

In addition, FCC Part 15 governs both COTS and non-COTS communications equipment radiated and conducted emissions, other than for vehicle equipment. FCC limits are more restrictive than EN 50121-4 limits.

Non-COTS communication equipment immunity limits for electrostatic discharge, fast transients, and surges shall conform to EN 50121-4, Railway applications - Electromagnetic Compatibility, Part 4: Emission and immunity of signaling and telecommunications apparatus.

#### **26.6.9 FCC Type Accepted Radio Equipment**

Radio equipment shall be FCC Type Accepted. Frequencies for licensed radio equipment shall be coordinated with the project needs and as part of the coordination effort with other nearby users.

For FCC Type Accepted radio equipment, the Contractor shall provide a Radio EMC Schedule and drawings detailing the specific design provisions for each FCC Type Accepted radio.

For radio transmission equipment which has been proven in similar service and which has received Type Acceptance per applicable FCC Part 15 regulations, the suppliers shall document its suitability, demonstrating the following:

- The radio transmission equipment is suited for the planned application.

- The radio transmission equipment has received FCC Part 15 Type Acceptance.

#### **26.6.9.1 Radio Equipment Design Considerations**

The Contractor shall develop a detailed frequency and intermodulation analysis and report for each radio system, as part of the Radio EMC Schedule. The report shall cover modulation methods and the following:

- **Transmission Lines** – Use transmission lines, with proper terminating impedances, and matched circuits in RF applications.
- **Antenna Placement** – Place antennas to consider radio propagation factors for reliable operation, and also human exposure levels for any antennas which share an environment with patrons, staff, and neighbors.
- **Intermodulation Suppression** – Perform a frequency and interference analysis during the design phase for sites with more than 1 transmitter and receiver. As appropriate, apply the following:
  - Use cavity filters to attenuate interfering signals from co-located transmission equipment.
  - Use isolators to further suppress intermodulation.
  - Properly select antenna combiners and multi-couplers and segregation of high and low power transmission equipment.

#### **26.6.9.2 ISM Equipment**

Industrial, scientific and medical device frequency band (ISM) equipment shall be FCC type-accepted ISM band equipment in either the 2.4 GHz or 5.8 GHz ISM band. ISM design applications shall provide acceptable performance when interference from other ISM band users is present from many other mobile and fixed sources.

#### **26.6.9.3 Radio System Frequency Coordination**

The Contractor shall make the following provisions for radio system frequency coordination:

- Use frequency coordination to minimize EMI. Frequency coordination shall consist of coordinating the transmission or emission frequencies and the reception or susceptible frequencies of all equipment with frequencies already in use in the system and by neighbors.
- Coordinate equipment that transmits or receives on a specific frequency with the list of frequency bands used by other equipment.

#### **26.6.10 Human Exposure to Electromagnetic Fields**

The Contractor shall place radio transmit antennas and design the connected radio systems so that human exposure is below the applicable specification limits for EMF in and around the CHSTP.



The CPUC EMF Policy in Decision 06-01-042 states:

- Health hazards from exposures to EMF have not been established;
- State and federal public health regulatory agencies have determined that setting numeric exposure limits is not appropriate; and
- Existing no-cost and low-cost precautionary-based EMF policy should be continued.

The IEEE developed a set of internationally recognized standards to achieve the safe use of electromagnetic energy in the range of 0 Hz to 300 GHz relative to the potential hazards from exposure of such energy to man, volatile materials, and explosive devices. In compliance with the CPUC decision and international practice, the CHSTP established EMF exposure limits in and around the system per the IEEE standards:

- IEEE Standard C95.6-2002 – IEEE Standard for Safety Levels with Respect to Human Exposure to Electromagnetic Fields, 0-3 kHz [IEEE Std C95.6]
- IEEE Standard C95.1-2005 – IEEE Standard for Safety Levels with Respect to Human Exposure to Radio Frequency Electromagnetic Fields, 3 kHz to 300 GHz [IEEE Std C95.1]

The IEEE standards and these criteria establish Maximum Permissible Exposure (MPE) limits for the general population and authorized people in controlled environments. Only workers, contractors, and other authorized personnel may be in the controlled environments, such as in equipment rooms, or along the Authority's right-of-way near the tracks. The MPE limits recognize that workers and others in these controlled environments have the knowledge to minimize the duration or extent of exposure to the higher levels that may be present.

#### **26.6.10.1 Maximum Permissible Exposure Limits**

The magnetic field MPE limits for general public and controlled environments shall conform to IEEE Std C95.6 Table 2 and IEEE Std C95.1 Tables 2, 8, and 9. The limit for the general public at 60 Hz is 9.0 G.

The static electric field MPE limits for general public and controlled environments shall conform to the IEEE Std C95.1 Tables 4, 8, and 9 and IEEE Std C95.6 Table 4.

The propagating EMF MPE limits for general public and controlled environments shall conform to the IEEE Std C95.1 Tables 8 and 9.

The MPE limits in the cited revisions of the standards shall apply unless the Contractor establishes that a newer revision of the Standard is applicable and that the newer revision of the Standard provides adequate protection to people near the tracks.

#### **26.6.10.2 Animal Exposure to Electromagnetic Fields**

Farms and ranches may be near CHSTP radio transmit antennas. The Contractor shall place radio transmit antennas and design the connected radio systems so that farm and ranch animal exposures conform with the same MPE limits as apply to humans.

## 26.7 Automatic Train Control EMC Design Criteria

### 26.7.1 Equipment and EMC Considerations

#### 26.7.1.1 Automatic Train Control Equipment

ATC equipment are described in the *Automatic Train Control* chapter. The equipment comprises the following:

- Wayside ATC systems include wayside control units; ATC radio units including using P25, TETRA, NXDN, WiFi, 160 MHz, and/or 220 MHz frequencies and protocols; trackside signals; switches, and hazard detectors; track circuits for train detection, train communication, and/or broken rail detection; transponders; OCC/RCC facilities, ATC SCADA, ATC workstations and display units; ATC data network; and support subsystems and functions.
- Trainborne systems include onboard ATC unit, ATC radio unit, Driver's display unit, tachometers and transponder sensors, ATC onboard data radio, and support subsystems and functions.
- Equipment includes cabinets and contents, control panels, displays, cables, conduits, ducts, raceways, enclosures, and antennas.

Depending on the selected ATC technology, data radio communication between onboard and wayside control units may provide ATC data and functions, or coded track circuits may provide the equivalent function. In either case, the ATC equipment will include track circuits to provide broken rail detection. The ATC equipment may include track-mounted transponders, train-mounted transponder interrogators, and track and train auxiliary data radio equipment.

#### 26.7.1.2 EMC Considerations

ATC equipment shall conform to EMC design criteria. ATC equipment EMC provisions shall ensure the following:

- Safe and dependable operation
- No interference with or from neighbors
- Compliance with human exposure limits to magnetic and electric fields

Track circuits shall have specified and demonstrated immunity to train-conducted and inductive emissions. Train-conducted emissions will include 60 Hz power frequency harmonics with significant levels at power harmonics across the audio frequency band. Train propulsion design criteria developed in coordination with the ATC system supplier shall control the amplitude of higher order, higher amplitude 60 Hz power frequency harmonics, and their potential impact on track circuits.

ATC equipment shall conform to electrical safety requirements.

ATC equipment shall provide safety-critical functions with the documented assurance that unsafe failures are controlled to an acceptable limit.

Design and maintenance provisions shall ensure that EMI cannot compromise the safety level of operations achieved by the ATC system.

ATC equipment generally does not use COTS equipment for safety-critical functions. ATC suppliers shall document the achieved level of compliance of their equipment to designated EMC standards.

The following design criteria apply to the ATC system and its interfaces with other systems:

- Cables and cable segregation
- Grounding
- Equipment design
- Facility power
- Motors and controllers
- Equipment rooms and locations
- Equipment emission and immunity limits
- Requirements for radios
- Human exposure limits

The ATC system EMC design criteria are described in following subsections.

#### **26.7.1.3 EMC with Adjacent Railroads**

Freight and commuter railroads including the Union Pacific Railroad and the BNSF Railway and electric railroads including Bay Area Rapid Transit and Santa Clara Valley Transportation Authority run adjacent to the alignment for long sections. On the tracks, 60 Hz traction power for high-speed trains will flow in the OCS, running rails, and in the earth, at 750 A or more per train. The EMF of the traction power system will couple some 60 Hz traction power current into the tracks of adjacent railways. High frequency currents may also couple into adjacent railroad communications equipment.

Where an adjacent railroad parallels the alignment for many miles, the induced voltage and current in the adjacent railroad tracks may interfere with the normal operation of the adjacent railroad signaling system, depending on the specific track circuit equipment in service on the adjacent railroad.

The Contractor shall work with the engineering department of adjacent railroads that parallel the alignment, to assess the specific track signal and communication equipment in use on nearby sections, determine potential impacts of EMFs and radio frequency interference on the adjacent railroad equipment, and make suitable design provisions to prevent interference.

1 Design provisions may include replacement of specific track circuit types with other types  
2 developed for operation on or near electric railways, providing filters for sensitive  
3 communication equipment, coordination of radio communication frequencies with public safety  
4 and other users, relocating or reorientating radio antennas, and grounding or isolating of  
5 parallel metal structures.

## **26.7.2 Cable**

### **26.7.2.1 General**

6 The Contractor shall use optical interconnection cables wherever it is technically feasible and  
7 cost-effective. Optical connections between equipment and equipment locations are immune to  
8 EMI.

9 The Contractor shall select, install, and connect electrical cables with proper shielding, shield  
10 grounding, conduit or duct protection, entry protection, routing, and termination to control  
11 EMI in copper electrical cables as described in this section.

12 Electrical cable provisions shall comply with the NEC Chapter 2 (Wiring and Protection),  
13 Chapter 3 (Wiring Methods and Materials), Chapter 4 (Equipment for General Use), Chapter 7  
14 (Special Conditions), and Chapter 8 (Communication Systems), wherever the NEC applies,  
15 except to the extent other Design Criteria provisions specify otherwise.

### **26.7.2.2 ATC System Cables**

16 Equipment-specific EMC design criteria shall apply for specialized ATC system cables,  
17 including for the following:

- 18 • Cables from equipment rooms to wayside track circuit equipment, track switches and  
19 signals, transponders or balises
- 20 • Cables for ATC equipment onboard RS, including for power, RS interfaces, antennas,  
21 trainlines, and networks

### **26.7.2.3 Electrical Cable Categories**

22 For EMC purposes, each cable shall be assigned to 1 of the cable categories in Table 26-3.

**Table 26-3: Electromagnetic Compatibility Cable Categories**

ID	Name	Examples, References, Comments
C1	Power at 600 V ac RMS or higher	115 kV feeder
C2	Traction Power	25 kV ac traction power, negative feeder and return cables
C3	Power at less than 600 V ac RMS	480 V ac, 120 V ac; ATC cables including track switch motors, signal lights, and related circuits; OCS Motor Operated Disconnects; lighting circuits, HVAC motors, UPS, etc.
C4	NEC Class 1 circuits	NEC Class 1 power limited circuits per NEC 725.41 (A), limited to less than 30 V and 1000 VA. NEC Class 1 remote-control and signaling circuits per NEC 725.21 (B), less than 600 V. Includes safety-critical signaling circuits such as fire alarm.
C5	NEC Class 2 and 3 circuits	Power sources for NEC Class 2 and Class 3 circuits per NEC 725.121, shall be limited per Chapter 9 Table 11(a) and Table 11(b)
C6	Unshielded twisted pair signal circuits	Telephone, speaker level audio output, Ethernet, track circuit transmitter and receiver cables
C7	Shielded signal circuits including shielded twisted pair, coax, and multi-axial cables	CCTV from camera to converter box, microphone and line level audio input, line level audio output
C8	Leaky coax cable	Distributed communication antenna
C9	Optical cable	Backbone network, local area network

#### 26.7.2.4 Cable Separation Criteria

Cables in the same cable category may be run together in the same steel conduit, raceway, duct, trough, manhole, or cable tray, if all cables that are grouped and run together are insulated to the highest voltage level present in the group.

Cables in different cable categories shall not be run together in the same cable assembly, conduit, raceway, duct, or cable tray, without engineering justification documenting the specific cables, signals, and protections against EMI for the proposed combination. However, optical cables (C9) may be run together with any other cable.

Wayside cables running parallel to and crossing the track shall be placed in cable troughs and duct banks. These will include communication cables, train control cables, OCS motor operated disconnects (MODs), 480 VAC, 125 VDC and 48 VDC. The cable trough shall not contain C1, power at greater than 600 V, or C2, traction power cable. Cables in the cable trough, manholes, and duct banks shall be separated by cable category, so that a suitable separation is maintained between, for example 480 VAC cables and ethernet cables. If EMI impact occurs between cables

1 in the cable trough or duct banks, the Contractor shall apply suitable shielding techniques to  
2 reduce the emission from the EMI source, to increase the immunity of the EMI receiver, or both.  
3 Shielding techniques include cable shields and use of metal conduit.

4 Traction return cables shall be run in troughs, manholes, and ducts separate from other wayside  
5 cables. Traction return cables may share a duct bank but not a duct with other cable categories.

6 Wires carrying circuits belonging to a single cable category, but which use different signal  
7 levels, shall not be run together in the same cable without engineering justification  
8 documenting the specific cables, signals, and protections against EMI for the proposed  
9 combination.

10 Where cables of dissimilar type, service, or signal level cross, the crossing of the cables and their  
11 conduits and raceways shall be perpendicular where practical.

12 Cable separation design shall conform to IEEE Std 1100-2005, Recommended Practice for  
13 Powering and Grounding Electronic Equipment, Section 9.9.

#### **26.7.2.5 Cable Shielding**

14 The Contractor shall use shielded cables and steel conduits and ducts to protect components,  
15 circuits, and systems against the effects of undesirable external disturbing EMI sources. A cable  
16 shield can:

- 17 • Increase immunity and provide additional protection against interference to the enclosed  
18 circuits; and
- 19 • Reduce emissions and provide additional reduction of the level of interference emitted by  
20 an enclosed circuit.

21 Wire and cable shielding and shield grounding design shall conform to IEEE Std 1100-2005,  
22 Sections 3.3, 4.9, 9.9, 10.2, and 10.4; and to IEEE Std 1143-1994, Guide on Shielding Practice,  
23 Section 7.

24 Special considerations apply to signal control and lighting cables and switch control cables  
25 which connect to track signals and switches and run parallel to the track. Shielding and conduit  
26 non-continuity for track circuit signal cables shall comply with the guidelines provided in the  
27 *Grounding and Bonding Requirements* chapter, Signal Cables section: "Signal control or lighting  
28 cables and switch cables shall not have metallic shielding. Metallic messenger or duct shall not  
29 be used in any way that could cause an electrical interconnection between signals or signal  
30 structures and signal equipment housings." This criterion does not apply when otherwise  
31 directed by equipment-specific design criteria such as noted in Section 26.7.2.2.

#### **26.7.2.6 Cable Shield Grounding**

32 The connections of cable shields and the connection of the shields to the grounding system  
33 determine the effectiveness of the shielding in mitigating EMI.

Specify and use a coordinated and consistent method for terminating and connecting cable shields to equipment. The method shall conform to these criteria. The Contractor shall document the specific provisions for cable shield grounding at the system and detail level as part of a Cable EMC Schedule. The Contractor shall ensure that the Cable EMC Schedule is fully implemented across all systems.

The general topology for shield connection to power supply ground shall conform to IEEE Std 1100-2005, Sections 8.5, 9.9, 10.2, and 10.4.

The Contractor shall properly terminate cable conductors, shield, and drain wire, if used, on the connector to prevent the inadvertent grounding of the shield or conductor connection via the connector shell to the equipment enclosure or to another unintended potential.

#### **26.7.2.7 Conduit and Duct**

The Contractor shall make the following provisions for conduit and duct:

- Run specified cables in cable troughs and in connecting ducts. Metal conduits or ducts for cables provide mechanical protection and electrical safety as well as additional electromagnetic isolation compared to physical separation of cables. Metal conduits and ducts increase the immunity of and decrease the emissions from enclosed circuits.
- Use steel conduit and ducts; do not use aluminum.
- Run cables in steel conduit or steel duct on any run that is outside of a single enclosure for any part of its route, except for cables in the cable troughs, for wayside signal cables as otherwise specifically required, and for other cables for which engineering justification documents the basis for the cables to be run out of conduit.

#### **26.7.2.8 Conduit and Duct Bonding and Grounding**

Metal conduit and duct can serve as an overall shield for cables carrying sensitive signals, and for cables carrying signals which tend to emit EMI. To provide a shielding function, conduit and duct shall be grounded and bonded per the NEC Article 250 where not in conflict with the *Grounding and Bonding Requirements* chapter.

Only hardware listed by a nationally recognized testing laboratory such as UL shall be used to join and bond conduits and ducts to enclosures, including conduit fittings and EMI gaskets.

Fully coordinate IG ac circuit conduit routing and connections with the grounding of the systems powered by insulated ground circuits. Refer to Section 26.7.3.6 for IG details.

#### **26.7.2.9 Entry Protection**

The Contractor shall provide:

- Room entry EMI impulse protection for wires entering an equipment room; and
- Enclosure entry EMI impulse protection for wires entering an equipment enclosure.

1 The protection shall consider and be consistent with the cable type, the cable source, and the  
2 potential EMI hazards to be protected against. As noted above, cables entering an equipment  
3 room shall be enclosed in steel conduit unless the system designer provides specific technical  
4 justification for another entry protection method consistent with the above criteria.

5 Room entry and line entry impulse protection shall consist of line-to-line and/or line-to-ground  
6 protection, as appropriate, for each circuit type. Circuit protections on wires include  
7 combinations of lightning arrestors, transient suppressors, fuses, spark gaps, filters, ferrite  
8 beads and clamshells, etc.

#### **26.7.2.10 Cable Termination**

9 The Contractor shall terminate all impedance controlled cables, such as coax, with the proper  
10 impedance.

#### **26.7.2.11 Circuit Loops**

11 The Contractor shall make the following provisions for circuit loops:

- 12 • Minimize the area of circuit loops in which magnetic fields can couple.
- 13 • For high-current circuits, run the feed and return conductors as closely together as feasible  
14 and minimize the length over which the feed and return cables are run separated from one  
15 another, to reduce the flux generating loop. To improve the immunity of signal circuits, the  
16 Contractor shall use twisted pair cables.
- 17 • To minimize coupling between the flux generating loop and the signal circuit, maximize the  
18 distance between power cables and signal cables, and minimize the distance over which the  
19 signal cable runs near and parallel to the power cable.
- 20 • Ensure that design layout drawings have sufficient detail to show the physical routing of all  
21 conduits and cables, so that EMC compatibility can be designed and documented.

### **26.7.3 Grounding**

#### **26.7.3.1 General**

22 The Contractor shall make the following provisions for circuit loops:

- 23 • Provide each ATC equipment item with a suitable safety ground and suitable EMI-  
24 controlling ATC ground and return connections.
- 25 • Provide an ATC Grounding EMC Schedule and drawings, which detail the specific design  
26 provisions for each equipment item and its grounding connections.
- 27 • Prepare an overall grounding diagram that defines the grounding scheme for each system  
28 and subsystem, the ac power (both non-insulated ground and insulated ground) circuits  
29 serving these systems, and the interconnection means and interconnection points of the  
30 grounding systems.



The *Grounding and Bonding Requirements* chapter contains specific criteria for the following:

- Grounding ATC equipment
- Grounding track circuits and other ATC equipment. The track circuit type will determine the grounding provisions for running rails.

Special considerations apply to signal control and lighting cables and switch cables which run parallel to the track. Shielding and conduit non-continuity for signal control and lighting cables and switch cables shall comply with the guidance in the *Grounding and Bonding Requirements* chapter, Signal Cables section: “Signal control or lighting cables and switch cables shall not have metallic shielding. Metallic messenger or duct shall not be used in any way that could cause an electrical interconnection between signals or signal structures and signal equipment housings.”

### 26.7.3.2 Grounding Categories

For EMC purposes, the Contractor shall use the grounding categories in Table 26-4 to categorize each ground connection and apply these grounding categories to achieve the following:

- Minimize the effects of electrical noise generated by all equipment and by neighboring electrical equipment
- Minimize disturbing effects within directly affected control equipment and propagating effects on associated equipment, which might result from ground currents, fault currents, or nearby lightning strikes
- Minimize the shock exposure potential which might appear on non-current-carrying equipment or conducting structures, in case an insulation failure energizes the enclosing structure with a dangerous voltage. Proper grounding of equipment and conducting structures will minimize the shock hazard due to a power line insulation failure.
- Provide a discharge path for stray RF energy.

**Table 26-4: Grounding Categories**

ID	Name	Examples, References, Comments
G1	Lightning protection ground	The building lightning protection ground shall connect to the power safety ground, at a single point per NFPA 780-2004: Standard for the Installation of Lightning Protection Systems.
G2	Power safety ground	Governed by the NEC Article 250
G3	Sensitive electronic equipment ground	The sensitive electronic equipment ground should connect by bonding the to the power safety ground at only a single point.

G4	Signal reference structure (SRS) ground	<p>G4 SRS ground configurations include grids and planes which provide a high-frequency common ground reference to which control and communications signals are referenced.</p> <p>A matrix ground mat in or under a sensitive equipment room is a typical SRS implementation. The SRS provides a better reference than a single G3 ground as it provides a lower impedance at high frequencies.</p> <p>If the connection from the G3 sensitive electronic equipment ground to the G2 power safety ground is long, higher impedance in the ground structure results in more electrical noise. A G4 SRS can resolve the problem.</p>
G5	Traction power return	<p>The traction power return and running rails are grounded at each TPF via a connection from to the TPF return bus to the TPF ground grid. The running rails are also connected to the aerial ground wire and to the earth at impedance bond locations at intervals along the track.</p>

The Contractor shall make the following provisions:

- Provide a direct, dedicated ground connection when connecting ATC and communication ground circuits to a TPF ground grid.
- Identify, coordinate, keep separate, insulate, and appropriately connect the grounds in each of the ground categories in Table 26-4. Where necessary, identify special provisions for grounds in tunnels, trenches, at-grade ballasted or non-ballasted track, and aerial structures or bridges.
- Integrate and coordinate the lightning protection grounding system with the other grounding system designs.

Optical or other galvanic isolation of signals may be needed to create and maintain the separation of grounds.

### 26.7.3.3 Traction Power Return

As noted in the following section on traction power, the traction power return circuit shall be connected to the TPF ground grid at each TPF. In addition, the running rails shall be connected via impedance bonds to the earth and to the aerial ground wire at intervals between supply station locations, to control step and touch voltage hazards.

Refer to the *Grounding and Bonding Requirements* chapter.

### 26.7.3.4 Industry Standard for Powering and Grounding Electronic Equipment

The designs for powering and grounding ATC equipment shall conform to the applicable portions of the National Electric Code, of IEEE Std 1100, and of IEEE Std 142, as adapted for the environment.

For equipment designs and connections, the Contractor shall use the specified techniques for powering and grounding electronic equipment.

Note that in the recommended power and grounding schemes, the "grounded conductor" refers to that leg of the circuit (usually the neutral) that is intentionally held at ground potential. The "grounded conductor" is part of the current-carrying circuit. The "grounding conductor" refers to the conductor(s) that connect(s) exposed metal parts of a device to ground; primarily for safety, secondarily for performance. The "grounding conductor" is not part of the current-carrying circuit.

#### **26.7.3.5 Grounding Design Criteria**

The Contractor shall use the following:

- A safety grounding technique for equipment in racks per IEEE Std 1100-2005, Sections 3.3, 4.9, 8.5, 9.9, 10.2, and 10.4
- A high frequency or RF grounding technique signals per IEEE Std 1100-2005, 3.3, 4.6, 8.5, 9.9, 10.2, and 10.4

The grounding system shall provide that when electronic equipment has a conducting connection to other devices within a structure that all interconnected devices be referenced at the same lightning protection ground (G1) to minimize possible lightning damage.

#### **26.7.3.6 Insulated Ground Design Criteria**

Sensitive ATC equipment may use an NEC-compliant isolated ground to provide basic fault/personnel protection and to protect the equipment. These Design Criteria refer to the NEC-compliant isolated ground using the IEEE Std 1100 nomenclature, which is "insulated ground" (IG).

For IG connections for equipment in racks, the Contractor shall modify the safety grounding configuration from that shown in IEEE Std 1100-2005, Figures 8-17 and 8-18, by placing a listed nonmetallic raceway fitting between the equipment rack and the conduit system, as shown in NEC Handbook Exhibit 250.45. The configuration shall ensure that equipment and enclosures served by an IG power circuit are:

- Correctly and safety grounded at the IG location; and
- Not inadvertently bonded to unintended non-insulated grounding paths.

### **26.7.4 Equipment Design**

#### **26.7.4.1 General**

Equipment and system designs shall control emissions and increase EMI immunity. Design considerations include placement, enclosures, filters, interconnect design, component characteristics, and modulation methods.

The Contractor shall:

- Specify and implement design provisions for equipment placement, enclosures, filters, interconnect design, component characteristics, and modulation methods, for each equipment item, per the following subsections; and
- Shall ensure that plan and elevation drawings show sufficient detail of the layout of equipment in each rack, the layout of racks within a room, and ancillary equipment within a room, so that EMC compatibility can be designed and documented.

#### **26.7.4.2 Power Frequency Track Circuits**

Because the system will use 60 Hz traction power, the use of power frequency track circuits at 60 Hz or below is not permitted.

#### **26.7.4.3 Placement**

The Contractor shall place high power potential sources as far as practical from sensitive potential susceptible equipment, to minimize magnetic and electric field coupling between nearby pieces of equipment.

#### **26.7.4.4 Enclosures**

The Contractor shall make the following provisions for enclosures:

- Install ATC electronic equipment in a suitable EMI-Shielded Equipment Rack Cabinet.
- Protect signal and power lines entering enclosures from EMI by using appropriate cable termination techniques as well as proper conduit fittings, gaskets, etc.
- Enclose cables entering or leaving electronic enclosures in steel conduit, unless otherwise specifically justified by engineering documentation.
- A double wall shielded equipment enclosure may be necessary for sensitive equipment located near a powerful emissions source. In such cases, the inner enclosure shall be connected to the signal reference structure ground or IG, while the outer enclosure shall be connected to the power safety ground.
- Use shielded enclosures, metal ducts, and metal conduits to control the electromagnetic coupling between and from equipment and wiring.

Within enclosures, the Contractor shall make the following provisions:

- Carefully bundle, twist, shield, protect, route, and properly terminate high-current cables such as motor cables, to minimize uncontrolled emissions. Ensure that the cable installation does not exceed the minimum bend radius for the cable.
- Connect low voltage cable shields to the appropriate low voltage ground at the source and, per IEEE Std 1100-2005, Sections 3.3 and 8.5 and IEEE Std 1143-1994, Section 7, to the receiver-end equipment.
- Use twisted pair and shielded cables, coax, or multiwire for sensitive signals.

- Place inductive components perpendicularly to nearby susceptible components to minimize the exposure to stray flux.

#### 26.7.4.5 Filters

The Contractor shall make the following provisions for filters:

- Use appropriate low-pass filters on wired connections. Filters minimize conducted emissions from a piece of equipment into other pieces of equipment and improve the immunity of the equipment to conducted emissions on the connecting wires.
- Provide filters on power inputs and outputs to and from ac inverters and converters, transmitters, receivers, and signal circuits.
- Use supply suitable transient suppressors for relay contacts switching inductive loads, including relay and contactor coils.

#### 26.7.4.6 Interconnect Design

The Contractor shall use appropriate techniques to minimize emission and maximize immunity of power and signal conductors, including the following:

- Arrange wires, cables, and conduits to physically separate signals with different signal type, voltage and energy levels.
- Separate and shield circuits carrying power, high level signals, and low level signals.
- Use balanced circuits with coordinated current return paths.
- Minimize the loop area between source and return conductors.
- Use photocouplers or optical fibers to galvanically isolate circuit potentials when needed.
- Shield all sensitive signal conductors in suitable electric and magnetic shielding structures using appropriate combinations of steel conduit and shielded cable.
- Connect shields to appropriate system ground at the high energy end

The Contractor designs shall include the following:

- Matching networks to match transmission line and antenna systems to maximize performance and minimize loss and reduce EMI for controlled impedance circuits such as radio frequency signals.
- Shielded cables that have shield coverage of at least 90 percent
- Guard shields to improve immunity as required for sensitive electronic circuits
- Optical cables, optically-isolated signals, or other galvanic isolation for connection between equipment enclosures, whenever practical
- Isolation transformers, chokes and/or isolators for sensitive audio, video, and data circuits as required

#### 26.7.4.7 Component Characteristics

The Contractor shall select appropriate components to reduce EMI. These components include the following:

- For power converters, such as UPS, dc-to-ac inverters, and variable speed motor drives, use inductor-capacitor line EMI filters on each power input and output, to reduce power frequency and switching harmonic currents and radiated emissions.
- Use ferrite beads and clamshells to reduce emissions from signal cables and mitigate line-to-ground (common mode) and line-to-line (differential mode) noise.
- Use LCD displays for computers. Do not use CRT displays.

#### 26.7.4.8 Modulation Methods

The Contractor shall:

- Use a modulation method that controls and minimizes the conducted, inductive, and radiated emissions of the equipment for all equipment which converts power from 1 frequency, voltage, or current level to another; and
- For each power converter, provide a report, manufacturer's data sheet, or other technical data to describe the modulation methods used as well as the mitigation methods used to minimize EMI.

### 26.7.5 Facility Power

#### 26.7.5.1 General Criteria

The Contractor shall make the following provisions for facility power:

- Take power for ATC electronic circuits, HVAC equipment, lighting, elevators and escalators, and other systems from separate branch circuits which are isolated, regulated, backed up, and protected as required. Facility power shall comply with the requirements of the NEC Chapter 2 (Wiring and Protection), Chapter 3 (Wiring Methods and Materials), and Chapter 4 (Equipment for General Use).
- Provide an ATC Power EMC Schedule and drawings detailing the specific design provisions for each equipment item and its power and UPS connections.
- Prepare overall power diagrams that define the power distribution scheme for each system and subsystem and show the ac power with both non-insulated ground and insulated ground circuits serving these systems.

#### 26.7.5.2 High Power

The Contractor shall run high-current power ac cables including phase, ground and neutral conductors, with minimum loop size and conductor separation, physically separated from sensitive circuits, or if near sensitive circuits, twisted or in steel conduit.

### 26.7.5.3 Utility Power

The Contractor shall make the following provisions for utility power:

- Provide a power distribution design that controls potential effects of EMI generated by high power equipment on sensitive electronic equipment. Use separate transformers and distribution buses to ensure high power loads are separated from sensitive loads.
- Use the physical placement of power distribution equipment to increase the immunity of sensitive electronic equipment to EMI generated by high power equipment.
- Route utility power distribution lines to facilities according to applicable EMI/EMF regulations and guidelines.

### 26.7.5.4 Grounding of Uninterruptible Power Supply Circuits

The Contractor shall follow IEEE Std 142-2007, Recommended Practice for Grounding of Industrial and Commercial Power Systems, Section 1.9.1, for UPS grounding arrangements.

### 26.7.5.5 Remote Powered Locations

Remote ATC locations where utility power is not available may use power supplied from the OCS negative feeder, from a solar storage system, or from another source.

Power at a remote location fed from the negative feeder shall be via a transformer, disconnect, and suitable filtering and power conditioning to protect connected equipment from the transients and harmonics present in the negative feeder.

The selected remote power supply shall make appropriate design provisions for cable, ground, shielding, filtering, and other EMC constraints.

### 26.7.6 Motors and Controllers

For general-purpose motors and motor controllers used within the ATC system, such as for ventilation equipment, the following criteria shall apply:

- The Contractor shall provide an ATC Motor and Controller EMC Schedule and drawings detailing the specific design provisions for each motor and controller of more than 1 hp.
- The Contractor shall provide each motor starter, controller, or inverter with suitable protection and line and load filtering to minimize transients and surges at start and stop. Provide twisted and/or shielded cables in conduit as appropriate.
- The Contractor shall run input power wires and cables to motors in steel conduit, locate input conductors as far as practical from sensitive equipment, and run wires from the output of the any UPS that powers motors in steel conduit.
- The Contractor shall use soft-start motor controllers or inverters to minimize conducted line transients caused by motor starts. When multiple motors are in a room, coordinate and delay the starting of the motors in a sequence to avoid the intense transient and line dip effects of all motors starting at once.

1 Service-proven track switch motors and controllers are governed by signal system design  
2 practice.

### **26.7.7 Equipment Rooms and Locations**

#### **26.7.7.1 Location**

3 The Contractor shall make the following provisions for equipment locations:

- 4 • Within the physical constraints of the planned facilities, locate equipment so that high  
5 power sources are physically separated as far as practical from susceptible equipment and  
6 cables.
- 7 • Where communication equipment, radio, ATC, and similar rooms are near traction power  
8 transformers, HVAC motors, or other high-current systems, locate the susceptible  
9 equipment as far as practical from the stray magnetic fields generated by the transformers,  
10 motors, and power cables.
- 11 • Preferably, locate high-current equipment and cables in rooms that are not adjacent (either  
12 vertically or horizontally) to rooms with sensitive and susceptible equipment. The  
13 Contractor shall run all input power cables to high-current equipment in steel conduit.
- 14 • Maximize distance between EMI emitter rooms and EMI susceptible receiver rooms. The  
15 Contractor shall lay out the equipment rooms with high-current equipment at 1 end of a  
16 suite of rooms and susceptible equipment at the other (far) end of the suite of rooms. To the  
17 extent practical, for example, the Contractor shall locate rooms so that they start from the  
18 left in a hypothetical plan:
  - 19 – Traction power, HVAC, or other high-current rooms would be at the leftmost position  
20 (room 1)
  - 21 – The electrical distribution room (room 2) would be adjacent to and to the right of room  
22 1.
  - 23 – The UPS room (room 3) would be adjacent to and to the right of room 2.
  - 24 – The ATC room would be adjacent to and to the right of room 3.

#### **26.7.7.2 Equipment Room Shielding**

25 When architectural or structural constraints do not allow room locations and cable and  
26 equipment placement per Section 26.7.7.1 and when required by the characteristics of sensitive  
27 equipment installed in a room, the Contractor shall use architectural shielding in the sensitive  
28 room to mitigate the electric and magnetic fields from the high-current equipment.  
29 Architectural shielding materials include steel sheet and steel mesh wall linings; other shielding  
30 wall linings such as sheet copper, copper mesh, or conductive polyamide mesh; EMI shielded  
31 doors, windows, access panels, and air ventilation panels; EMI conductive systems; shielding  
32 components; shielding laminates; and EMI shielded equipment racks.



### 26.7.7.3 ATC Equipment Locations on Rolling Stock

Within the physical constraints of the railcar, the Contractor shall locate equipment so that railcar high power sources and cables are physically separated as far as practical from susceptible ATC equipment and cables.

## 26.7.8 Equipment Emission and Immunity Limits

### 26.7.8.1 General

While train control equipment is industrial equipment with similarities to electrical equipment used in other applications, much of it is not COTS equipment. ATC equipment suppliers shall document the achieved level of compliance of their equipment to designated standards.

### 26.7.8.2 Track Circuit EMC

The ATC and RS suppliers shall jointly establish specific immunity limits to protect ATC equipment, particularly track circuits and track-based data communication circuits, from train-generated interference. These limits shall be implemented by coordinated action by the RS and ATC suppliers during ATC and RS design, implementation, and qualification testing.

For EMC between RS and ATC track circuits, EN 50238-1, Railway applications - Compatibility between RS and train detection Systems, shall govern the relationship. The ATC supplier shall use EN 50238 procedures to document and convert the electromagnetic immunity characteristics of planned wayside equipment into corresponding emission limits which shall apply to RS. The RS supplier shall ensure that the RS emissions comply with the limits developed by the ATC supplier. If necessary, the suppliers shall modify ATC and RS designs to ensure compatibility.

U.S. Department of Transportation Federal Transit Administration test procedures shall govern testing of the compatibility of trains and track circuits. The RS supplier shall perform field qualification tests to demonstrate that the RS conforms to the emission limits using the following procedures:

- UMTA-MA-06-0153-85-6, Conductive Interference in Rapid Transit Signaling Systems Volume II: Suggested Test Procedures
- UMTA-MA-06-0153-85-8, Inductive Interference in Rapid Transit Signaling Systems - Volume II: Suggested Test Procedures

The ATC supplier shall perform field qualification tests to demonstrate that the susceptibility limits of the track circuits are as documented.

Track circuits and related equipment shall be immune to the effects of traction power currents in the OCS, running rails, and ground, and to the effects of utility power transmission and distribution lines which run parallel to or which cross the Authority's right-of-way, whether inside or outside the fence line.

### **26.7.8.3 ATC Radio EMC**

For radio transmission equipment which has been proven in similar service and which has received Type Acceptance per applicable FCC Part 15 regulations, the supplier shall document its suitability, demonstrating the following:

- The radio transmission equipment is suited for the planned application.
- The radio transmission equipment has received FCC Part 15 type acceptance.

### **26.7.8.4 ATC EMC Criteria**

Specific ATC system EMC design criteria shall apply for ATC equipment. Euronorm standards govern emissions and immunity for wayside and onboard equipment:

- EN 50121-1, Railway applications - Electromagnetic compatibility. General.
- EN 50121-2, Railway applications - Electromagnetic compatibility. Emissions of the whole railway system to the outside world.
- EN 50121-3-2, Railway applications - Electromagnetic Compatibility. Rolling stock – Apparatus
- EN 50121-4, Railway applications - Electromagnetic Compatibility, Part 4: Emission and immunity of signaling and telecommunications apparatus

Equipment immunity limits for electrostatic discharge, fast transients, and surges shall conform to EN 50121-3-2 and EN 50121-4.

As noted above, track circuit immunity shall be established per EN 50238-1, Railway applications - Compatibility between RS and train detection Systems; EN 50238-2, Railway applications - Compatibility between rolling stock and train detection systems - Part 3: Compatibility with track circuits; and EN 50238-3, Railway applications - Compatibility between rolling stock and train detection systems - Part 3: Compatibility with axle counters.

Each item of electrical or electronic equipment shall be certified and documented to conform to the applicable limits. For equipment that is non-COTS equipment, this documentation shall include a test report, manufacturer's performance data, and acceptance certificates from a qualified test laboratory. Each equipment supplier shall demonstrate and document compliance of each item.

### **26.7.8.5 ATC Equipment in Equipment Racks**

A set of COTS equipment, when installed in a compliant and correct manner in an adequate shielded equipment rack, can be treated as a single item of equipment for the purposes of tests and documentation. However, treatment of a shielded rack of equipment as COTS does not relieve the supplier of responsibility to achieve and document electromagnetic compatibility for the equipment rack and all the equipment in it.

#### 26.7.8.6 ATC Equipment Emissions and Immunity

EN 50121-3-2 governs onboard ATC equipment radiated and conductive emissions and immunity. EN 50121-4 governs wayside ATC equipment radiated and conductive emissions and immunity. Each California High-Speed Train ATC equipment supplier shall provide equipment which conforms to these limits, as well as test certification or documentation demonstrating compliance.

The ATC equipment immunity limits for electrostatic discharge, fast transients, and surges shall conform to EN 50121-4, Railway applications - Electromagnetic Compatibility, Part 4: Emission and immunity of signaling and telecommunications apparatus.

#### 26.7.9 FCC Type Accepted Radio Equipment for ATC

Radio equipment for ATC shall be FCC Type Accepted. Frequencies for licensed radio equipment shall be coordinated with project needs and as part of the coordination effort with other nearby users.

For FCC Type Accepted radio equipment, provide a Radio EMC Schedule and drawings detailing the specific design provisions for each FCC Type Accepted radio.

##### 26.7.9.1 Radio Equipment Design Considerations

Develop a detailed frequency and intermodulation analysis and report for each radio system, as part of the Radio EMC Schedule. The report shall cover modulation methods and the following:

- **Transmission Lines** – Use transmission lines, with proper terminating impedances, and matched circuits in RF applications.
- **Antenna Placement** – Place antennas to consider radio propagation factors for reliable operation, and also human exposure levels for any antennas which share an environment with patrons, staff, and neighbors.
- **Intermodulation Suppression** – Perform a frequency and interference analysis during the design phase for sites with more than 1 transmitter and receiver. As appropriate, apply:
  - Use cavity filters to attenuate interfering signals from co-located transmission equipment.
  - Use isolators to further suppress intermodulation.
  - Properly select antenna combiners and multi-couplers and segregation of high and low power transmission equipment.

##### 26.7.9.2 ISM Equipment

Industrial, scientific and medical device frequency band (ISM) equipment shall be FCC type-accepted ISM band equipment in either the 2.4 GHz or 5.8 GHz ISM band. ISM design applications shall provide acceptable performance when interference from other ISM band users is present from many other mobile and fixed sources.

### 26.7.9.3 Radio System Frequency Coordination

The Contractor shall make the following provisions for radio system frequency coordination:

- Use frequency coordination to minimize EMI. Frequency coordination shall consist of coordinating the transmission or emission frequencies and the reception or susceptible frequencies of all equipment with frequencies already in use in the system and by neighbors.
- Coordinate equipment that transmits or receives on a specific frequency with the list of frequency bands used by other equipment.

### 26.7.10 Human Exposure to Electromagnetic Fields

The Contractor shall place ATC radio transmit antennas and design the connected radio systems so that human exposure is below the applicable specification limits for EMF in and around the system.

The CPUC EMF Policy in Decision 06-01-042 states:

- Health hazards from exposures to EMF have not been established;
- State and federal public health regulatory agencies have determined that setting numeric exposure limits is not appropriate; and
- Existing no-cost and low-cost precautionary-based EMF policy should be continued.

The IEEE developed a set of internationally recognized standards to achieve the safe use of electromagnetic energy in the range of 0 Hz to 300 GHz relative to the potential hazards from exposure of such energy to man, volatile materials, and explosive devices. In compliance with the CPUC decision and international practice, the CHSTP established EMF exposure limits in and around the system per the IEEE standards:

- IEEE Standard C95.6-2002 – IEEE Standard for Safety Levels with Respect to Human Exposure to Electromagnetic Fields, 0-3 kHz [IEEE Std C95.6]
- IEEE Standard C95.1-2005 – IEEE Standard for Safety Levels with Respect to Human Exposure to Radio Frequency Electromagnetic Fields, 3 kHz to 300 GHz [IEEE Std C95.1]

The IEEE standards and these criteria establish MPE limits both for the general population and for authorized people in controlled environments. Only workers, contractors, and other authorized personnel may be in the controlled environments, such as in equipment rooms, or along the Authority's right-of-way near the tracks. The MPE limits recognize that workers and others in these controlled environments have the knowledge to minimize the duration or extent of exposure to the higher levels that may be present.

#### 26.7.10.1 Maximum Permissible Exposure Limits

The magnetic field MPE limits for general public and controlled environments shall conform to IEEE Std C95.6 Table 2 and IEEE Std C95.1 Tables 2, 8, and 9. The limit for the general public at 60 Hz is 9.0 G.

The static electric field MPE limits for general public and controlled environments shall conform to the IEEE Std C95.1 Tables 4, 8, and 9 and IEEE Std C95.6 Table 4.

The propagating EMF MPE limits for general public and controlled environments shall conform to the IEEE Std C95.1 Tables 8 and 9.

The MPE limits in the cited revisions of the standards shall apply unless the Contractor establishes that a newer revision of the Standard is applicable and that the newer revision of the Standard provides adequate protection to people near the tracks.

#### 26.7.10.2 Animal Exposure to Electromagnetic Fields

Farms and ranches may be near CHSTP ATC radio transmit antennas and the connected radio systems. The Contractor shall place ATC radio transmit antennas and design the connected radio systems so that farm and ranch animal exposures conform with the same MPE limits as apply to humans.

### 26.8 Traction Electrification System EMC Design Criteria

#### 26.8.1 Equipment and EMC Considerations

The Traction Electrification System (TES) is described in the *Traction Power Supply System* chapter and the *Overhead Contact System and Traction Power Return System* chapter. The TES equipment comprises the following:

- Ac power distribution from 2 independent high voltage (HV) utility supply circuits to traction power substations
- 2x25 kV traction power substations, switching stations, and paralleling stations
- OCS
- Aerial Negative Feeders
- Traction power return system
- Aerial Static/Ground Conductor
- Traction power SCADA, instrumentation, and data network connections
- Equipment includes transformers, cooling equipment, switchgear, overhead contact system hardware, grounding hardware, impedance bonds, cabinets and contents, control panels, monitoring equipment, displays, cables, conduits, ducts, raceways, bus bars, and enclosures.

1 The 2x25kV configuration reduces propagation of electric and magnetic fields from traction  
2 power currents because it minimizes the physical size of the loop in which supply and return  
3 currents flow.

4 Train conducted emissions will include 60 Hz power frequency harmonics with significant  
5 levels across the audio frequency band. Train propulsion design criteria developed in  
6 coordination with the ATC system supplier shall control the amplitude of higher 60 Hz power  
7 frequency harmonics and the susceptibility of ATC equipment to traction power harmonics.

8 TES equipment EMC provisions shall ensure the following:

- 9 • Safe and dependable operation
- 10 • No interference with or from neighbors
- 11 • Compliance with human exposure limits
  - 12 – Magnetic and electric fields
  - 13 – Step and touch potentials

14 Traction power equipment shall conform to electrical safety requirements.

15 Design and maintenance provisions shall also ensure that EMI cannot compromise the safety  
16 level of operations provided by the traction power system.

17 The following design criteria apply to the traction power system and its interfaces with other  
18 systems:

- 19 • Cables and cable segregation
- 20 • Grounding
- 21 • Equipment design
- 22 • Facility Power
- 23 • Equipment rooms and locations
- 24 • Equipment emission and immunity limits
- 25 • Requirements for radios
- 26 • Human exposure limits
- 27 • Effects on adjacent metallic fences and pipelines

28 The traction power system EMC design criteria are described in following subsections.

## 26.8.2 Cable

### 26.8.2.1 General

1 The Contractor shall select, install, and connect electrical cables with proper shielding, shield  
2 grounding, conduit or duct protection, entry protection, routing, and termination to control  
3 EMI in copper electrical cables as described in this section. Treat power cables according to  
4 required practice for their voltage class.

5 Electrical cable provisions shall comply with the NEC Chapter 2 (Wiring and Protection),  
6 Chapter 3 (Wiring Methods and Materials), Chapter 4 (Equipment for General Use), Chapter 7  
7 (Special Conditions) and Chapter 8 (Communication Systems), wherever the NEC applies,  
8 except to the extent other Design Criteria provisions specify otherwise.

9 Electrical cable provisions shall comply with Section VII of CPUC General Order No. 95, 2006.

### 26.8.2.2 Utility Supply and Overhead Contact System Traction Power Cables

10 The CPUC EMF Policy in Decision 06-01-042 states:

- 11 • Health hazards from exposures to EMF have not been established;
- 12 • State and federal public health regulatory agencies have determined that setting numeric  
13 exposure limits is not appropriate; and
- 14 • Existing no-cost and low-cost precautionary-based EMF policy should be continued.

15 Utility Supply cables shall conform to the applicable CPUC requirements.

16 OCS cables (Contact Wire, Messenger Wire, plus aerial Negative Feeders, and Static cables)  
17 shall conform to CPUC requirements, if any are put into effect.

18 Utility Supply and OCS traction power cables shall be laid out to minimize the size and  
19 asymmetry of the loop formed between power feeders, the aerial traction power feeder,  
20 overhead contact wire, return, aerial and buried ground cables. These cables carry high currents  
21 and will generate correspondingly high magnetic field levels.

### 26.8.2.3 Electrical Cable Categories

22 For EMC purposes, each cable shall be assigned to 1 of the cable categories in Table 26-5.

**Table 26-5: Electromagnetic Compatibility Cable Categories**

ID	Name	Examples, References, Comments
C1	Power at 600 V ac RMS or higher	115 kV feeder
C2	Traction Power	25 kV ac traction power, negative feeder, and return cables
C3	Power at less than 600 V ac RMS	480 V ac, 120 Vac; ATC cables including track switch motors, signal lights, and related circuits; OCS Motor Operated Disconnects; lighting circuits, HVAC motors, UPS, etc.
C4	NEC Class 1 circuits	NEC Class 1 power limited circuits per NEC 725.41 (A), limited to less than 30 V and 1000 VA. NEC Class 1 remote-control and signaling circuits per NEC 725.21 (B), less than 600 V. Includes safety-critical signaling circuits such as fire alarm.
C5	NEC Class 2 and 3 circuits	Power sources for NEC Class 2 and Class 3 circuits per NEC 725.121, shall be limited per Chapter 9 Table 11(a) and Table 11(b)
C6	Unshielded twisted pair signal circuits	Telephone, speaker level audio output, Ethernet, track circuit transmitter and receiver cables
C7	Shielded signal circuits including shielded twisted pair, coax, and multi-axial cables	CCTV from camera to converter box, microphone and line level audio input, line level audio output
C8	Leaky coax cable	Distributed communication antenna
C9	Optical cable	Backbone network, local area network, CCTV camera feeds from converter box to communications room

#### 26.8.2.4 Cable Separation Criteria

Cables in the same cable category may be run together in the same steel conduit, raceway, duct, trough, manhole, or cable tray, if all cables that are grouped and run together are insulated to the highest voltage level present in the group.

Cables in different cable categories shall not be run together in the same cable assembly, conduit, raceway, duct, or cable tray, without engineering justification documenting the specific cables, signals, and protections against EMI for the proposed combination. However, optical cables, C9, may be run together with any other cable.

Wayside cables running parallel to and crossing the track shall be placed in cable troughs and duct banks. These will include communication cables, train control cables, OCS motor operated disconnects (MODs), 480 VAC, 125 VDC and 48 VDC. The cable trough shall not contain C1, power at greater than 600 V, or C2, traction power cable. Cables in the cable trough, manholes, and duct banks shall be separated by cable category, so that a suitable separation is maintained between, for example 480 VAC cables and ethernet cables. If EMI impact occurs between cables



in the cable trough or duct banks, the Contractor shall apply suitable shielding techniques to reduce the emission from the EMI source, to increase the immunity of the EMI receiver, or both. Shielding techniques include cable shields and use of metal conduit.

Traction return cables shall be run in troughs, manholes, and ducts separate from other wayside cables. Traction return cables may share a duct bank but not a duct with other cable categories.

Wires carrying circuits belonging to a single cable category but which use different signal levels shall not be run together in the same cable, without engineering justification documenting the specific cables, signals, and protections against EMI for the proposed combination.

Where cables of dissimilar type, service, or signal level cross, the crossing of the cables and their conduits and raceways shall be perpendicular where practical.

Cable separation design for cables carrying signals shall conform to IEEE Std 1100-2005, Recommended Practice for Powering and Grounding Electronic Equipment, Section 9.9.

#### **26.8.2.5 Cable Shielding**

The Contractor shall use shielded cables and steel conduits and ducts to protect components, low voltage circuits, and electronic systems against the effects of undesirable external disturbing EMI sources. A cable shield can:

- Increase immunity and provide additional protection against interference to the enclosed circuits; and
- Reduce emissions and provide additional reduction of the level of interference emitted by an enclosed circuit.

Wire and cable shielding and shield grounding design for electronic signals shall conform to IEEE Std 1100-2005, Sections 3.3, 4.9, 9.9, 10.2, and 10.4; and to IEEE Std 1143-1994, Guide on Shielding Practice, Section 7.

#### **26.8.2.6 Cable Shield Grounding**

The connections of cable shields and the connection of the shields to the grounding system determine the effectiveness of the shielding in mitigating EMI for low-level signals.

The Contractor shall specify and use a coordinated and consistent method for terminating and connecting cable shields to equipment. The method shall conform to these criteria. The Contractor shall document the specific provisions for cable shield grounding at the system and detail level as part of a Cable EMC Schedule. The Contractor shall ensure that the Cable EMC Schedule is fully implemented across all systems.

The general topology for shield connection to power supply ground shall conform to IEEE Std 1100-2005, Sections 8.5, 9.9, 10.2, and 10.4.

1 The Contractor shall properly terminate cable conductors, shield, and drain wire, if used, on the  
2 connector to prevent the inadvertent grounding of the shield or conductor connection via the  
3 connector shell to the equipment enclosure or to another unintended potential.

#### **26.8.2.7 Conduit and Duct**

4 The Contractor shall provide metal or non-metallic conduits or ducts for traction power cables,  
5 per TES design criteria.

#### **26.8.2.8 Conduit and Duct Bonding and Grounding**

6 Metal conduit and duct can serve as an overall shield for cables carrying sensitive signals, and  
7 for cables carrying signals which tend to emit EMI. To provide a shielding function, metal  
8 conduit and duct shall be grounded and bonded per the NEC Article 250 where not in conflict  
9 with the *Grounding and Bonding Requirements* chapter.

10 Only hardware listed by a nationally recognized testing laboratory such as UL shall be used to  
11 join and bond conduits and ducts to enclosures, including conduit fittings and EMI gaskets.

12 Fully coordinate IG ac circuit conduit routing and connections with the grounding of the  
13 systems powered by insulated ground circuits. Refer to Section 26.8.3.6 for IG details.

#### **26.8.2.9 Entry Protection**

14 The Contractor shall provide the following:

- 15 • Room entry EMI impulse protection for signal wires entering an equipment room
- 16 • Enclosure entry EMI impulse protection for signal wires entering an equipment enclosure

17 The protection shall consider and be consistent with the cable type, the cable source, and the  
18 potential EMI hazards to be protected against. Signal and low voltage power cables entering an  
19 equipment room shall be enclosed in steel conduit unless the system designer provides specific  
20 technical justification for another entry protection method consistent with the above criteria.

21 Room entry and line entry impulse protection shall consist of line-to-line and/or line-to-ground  
22 protection, as appropriate, for each circuit type. Circuit protections on wires include  
23 combinations of lightning arrestors, transient suppressors, fuses, spark gaps, filters, ferrite  
24 beads and clamshells, etc.

#### **26.8.2.10 Cable Termination**

25 The Contractor shall terminate all impedance controlled cables, such as coax, with the proper  
26 impedance.

#### **26.8.2.11 Circuit Loops**

27 The Contractor shall make the following provisions for circuit loops:

- 28 • Minimize the area of circuit loops in which magnetic fields can couple.

- For high-current circuits, run the feed and return conductors as closely together as feasible and minimize the length over which the feed and return cables are run separated from one another, to reduce the flux generating loop. To improve the immunity of signal circuits, use twisted pair cables.
- To minimize coupling between the flux generating loop and the signal circuit, maximize the distance between power cables and signal cables, and minimize the distance over which the signal cable runs near and parallel to the power cable.
- Ensure that design layout drawings have sufficient detail to show the physical routing of all conduits and cables, so that EMC compatibility can be designed and documented.

### 26.8.3 Grounding

#### 26.8.3.1 General

The Contractor shall make the following grounding provisions:

- Provide each TES equipment item with a suitable safety ground and suitable EMI-controlling TES ground and return connections.
- Provide a TES Grounding EMC Schedule and drawings detailing the specific design provisions for each equipment item and its grounding connections.
- Prepare an overall grounding diagram that defines the grounding scheme for each system and subsystem, the ac power (both non-insulated ground and insulated ground) circuits serving these systems, and the interconnection means and interconnection points of the grounding systems.

For safety grounding requirements, refer to the *Grounding and Bonding Requirements* chapter.

#### 26.8.3.2 Grounding Categories

For EMC purposes, the Contractor shall use the grounding categories in Table 26-6 to categorize each ground connection. The Contractor shall apply these grounding categories to achieve the following:

- Minimize the effects of electrical noise generated by the system and by neighboring electrical equipment.
- Minimize disturbing effects within directly affected control equipment and propagating effects on associated equipment which might result from ground currents, fault currents, or nearby lightning strikes.
- Minimize the shock exposure potential which might appear on non-current-carrying equipment or conducting structures, in case an insulation failure energizes the enclosing structure with a dangerous voltage. Proper grounding of equipment and conducting structures will minimize the shock hazard due to a power line insulation failure.
- Provide a discharge path for stray RF energy.

**Table 26-6: Grounding Categories**

ID	Name	Examples, References, Comments
G1	Lightning protection ground	The building lightning protection ground shall connect to the power safety ground, at a single point per NFPA 780-2004: Standard for the Installation of Lightning Protection Systems.
G2	Power safety ground	Governed by the NEC Article 250
G3	Sensitive electronic equipment ground	The sensitive electronic equipment ground should connect by bonding the to the power safety ground only at a single point.
G4	Signal reference structure (SRS) ground	G4 SRS ground configurations include grids and planes which provide a high-frequency common ground reference to which control and communications signals are referenced. A matrix ground mat in or under a sensitive equipment room is a typical SRS implementation. The SRS provides a better reference than a single G3 ground as it provides a lower impedance at high frequencies. If the connection from the G3 sensitive electronic equipment ground to the G2 power safety ground is long, higher impedance in the ground structure results in more electrical noise. A G4 SRS can resolve the problem.
G5	Traction power return	The traction power return and running rails are grounded at each TPF via a connection from the TPF return bus to the TPF ground grid. The running rails are also connected to the aerial ground wire and to the earth at impedance bond locations at intervals along the track.

The Contractor shall make the following provisions:

- Provide a direct, dedicated ground connection when connecting signal and communication ground circuits to a TPF ground grid.
- Identify, coordinate, keep separate, insulate, and appropriately connect the grounds in each of the ground categories in Table 26-6. Where necessary, identify special provisions for grounds in tunnels, trenches, at-grade ballasted or non-ballasted track, and aerial structures or bridges.
- Integrate and coordinate the lightning protection grounding system with the other grounding system designs.

Optical or other galvanic isolation of signals may be needed to create and maintain the separation of grounds.

### **26.8.3.3 Running Rails**

The running rails shall be connected to earth ground via impedance bonds at all TPF locations, including traction power substations, switching stations, and paralleling stations. The running rails shall be also be connected to earth ground via impedance bonds at other wayside locations as necessary to adequately control step and touch potentials.

OCS poles, the static wire, and all other metallic components shall be solidly connected to each other at each location, and the OCS poles shall be grounded through the available path at the OCS pole foundation. However, as noted in the *Grounding and Bonding Requirements* chapter, where an OCS pole can be touched by a person at a passenger station, the OCS pole shall be grounded to the earth but insulated from the static wire.

Coordinate the grounding configuration of the running rails with the requirements of the ATC system because the running rails will be used for ATC circuits and for broken rail protection. Provide platform touch and step voltage protection, including a counterpoise arrangement to control voltage between train and platform.

Refer to the *Grounding and Bonding Requirements* chapter for specific criteria for TES grounding and bonding.

#### **26.8.3.4 Traction Power Return**

The rail return system connects the running rails to ground via the TPF transformer or autotransformer neutral bushing. The transformer neutral bushing is connected to the TPF return bus, which is directly connected to the TPF ground grid at each TPF.

Refer to the *Traction Power Supply System* chapter for specific criteria.

#### **26.8.3.5 Traction Power Facility Ground and Equipment Safety Ground**

Safety ground connection types G1 - G5 in Table 26-6, when connected to a TPF ground grid, shall use a direct, dedicated ground connection.

Specific low-resistance ground resistance criteria may apply to the resistance of the ground grid provided for traction power substations, traction power paralleling stations, and traction power switching stations. Such provisions may be needed to control touch potential along the running rails.

Refer to the *Grounding and Bonding Requirements* chapter for specific criteria for TES grounding and bonding.

#### **26.8.3.6 Industry Standard for Powering and Grounding Electronic Equipment**

The designs for powering and grounding TES electronic equipment shall conform to the applicable portions of the National Electric Code, of IEEE Std 1100, and of IEEE Std 142, as adapted for the environment.

For equipment designs and connections, the Contractor shall use the specified techniques for powering and grounding electronic equipment.

Note that in the recommended power and grounding schemes, the "grounded conductor" refers to that leg of the circuit (usually the neutral) that is intentionally held at ground potential. The "grounded conductor" is part of the current-carrying circuit. The "grounding conductor" refers to the conductor(s) that connect(s) exposed metal parts of a device to ground; primarily for

1 safety, secondarily for performance. The "grounding conductor" is not part of the current-  
2 carrying circuit.

### **26.8.3.7 Electronic Equipment Grounding Design Criteria**

3 The Contractor shall:

- 4 • Use a safety grounding technique for electronic equipment in racks per IEEE Std 1100-2005,  
5 Sections 3.3, 4.9, 8.5, 9.9, 10.2, and 10.4; and
- 6 • Use a high frequency or RF grounding technique for high frequency signals per IEEE Std  
7 1100-2005, Sections 3.3, 4.6, 8.5, 9.9, 10.2, and 10.4.

8 The grounding system shall provide that when electronic equipment has a conducting  
9 connection to other devices within a structure that all interconnected devices be referenced at  
10 the same lightning protection ground (G1) to minimize possible lightning damage. This  
11 grounding provision does not apply to power cables.

### **26.8.3.8 Insulated Ground Design Criteria**

12 Sensitive TES equipment may use an NEC-compliant isolated ground to provide basic  
13 fault/personnel protection and to protect the equipment. These Design Criteria refer to the  
14 NEC-compliant isolated ground using the IEEE Std 1100 nomenclature, which is "insulated  
15 ground" (IG).

16 For IG connections for equipment in racks, the Contractor shall modify the safety grounding  
17 configuration from that shown in IEEE Std 1100-2005, Figures 8-17 and 8-18, by placing a listed  
18 nonmetallic raceway fitting between the equipment rack and the conduit system, as shown in  
19 NEC Handbook Exhibit 250.45. The configuration shall ensure that equipment and enclosures  
20 served by an IG power circuit are:

- 21 • Correctly and safety grounded at the IG location; and
- 22 • Not inadvertently bonded to unintended non-insulated grounding paths.

## **26.8.4 Equipment Design**

### **26.8.4.1 General**

23 Equipment and system designs shall control emissions and increase EMI immunity. Design  
24 considerations include placement, enclosures, filters, interconnect design, component  
25 characteristics, and modulation methods.

26 The Contractor shall:

- 27 • Specify and implement design provisions for equipment placement, enclosures, filters,  
28 interconnect design, component characteristics, and modulation methods, for each  
29 equipment item, per the following subsections; and

- Ensure that plan and elevation drawings show sufficient detail of the layout of equipment in each rack, the layout of racks within a room, and ancillary equipment within a room, so that EMC compatibility can be designed and documented.

#### 26.8.4.2 Placement

The Contractor shall place high power potential sources as far as practical from sensitive potential susceptible equipment, to minimize magnetic and electric field coupling between nearby pieces of equipment.

#### 26.8.4.3 Enclosures

The Contractor shall make the following provisions for enclosures:

- Install TES electronic equipment in a suitable EMI-Shielded Equipment Rack Cabinet.
- Protect signal and power lines entering enclosures from EMI by using appropriate cable termination techniques as well as proper conduit fittings, gaskets, etc.
- Enclose cables entering or leaving electronic enclosures in steel conduit, unless otherwise specifically justified by engineering documentation.
- A double wall shielded equipment enclosure may be necessary for sensitive equipment located near a powerful emissions source. In such cases, the inner enclosure shall be connected to the signal reference structure ground or IG, while the outer enclosure shall be connected to the power safety ground.
- Use shielded enclosures, metal ducts, and metal conduits to control the electromagnetic coupling between and from equipment and wiring.

Within enclosures, the Contractor shall make the following provisions:

- Carefully bundle, twist, shield, protect, route, and properly terminate high-current cables such as motor cables, to minimize uncontrolled emissions. The Contractor shall ensure that the cable installation does not exceed the minimum bend radius for the cable.
- Connect low voltage cable shields to the appropriate low voltage ground at the source and, per IEEE Std 1100-2005, Sections 3.3 and 8.5 and IEEE Std 1143-1994, Section 7, to the receiver-end equipment.
- Use twisted pair and shielded cables, coax, or multiwire for sensitive signals.
- Place inductive components perpendicularly to nearby susceptible components to minimize the exposure to stray flux.

#### 26.8.4.4 Filters

The Contractor shall make the following provisions for filters:

- Use appropriate low-pass filters on wired connections. Filters minimize conducted emissions from a piece of equipment into other pieces of equipment and improve the immunity of the equipment to conducted emissions on the connecting wires.
- Provide filters on power inputs and outputs to and from ac inverters and converters, transmitters, receivers, and signal circuits.
- Use supply suitable transient suppressors for relay contacts switching inductive loads, including relay and contactor coils.

#### **26.8.4.5 Interconnect Design**

The Contractor shall use appropriate techniques to minimize emission and maximize immunity of power and signal conductors, including the following:

- Arrange wires, cables, and conduits to physically separate signals with different signal type, voltage and energy levels
- Separate and shield circuits carrying power, high level signals, and low level signals
- Use balanced circuits with coordinated current return paths
- Minimize the loop area between source and return conductors
- Use photocouplers or optical fiber to galvanically isolate circuit potentials when needed.
- Shield all sensitive signal conductors in suitable electric and magnetic shielding structures using appropriate combinations of steel conduit and shielded cable.
- Connect shields to appropriate system ground at the high energy end.

The Contractor designs shall include the following:

- Matching networks to match transmission line and antenna systems to maximize performance and minimize loss and reduce EMI for controlled impedance circuits such as radio frequency signals.
- Shielded cables that have shield coverage of at least 90 percent.
- Guard shields to improve immunity as required for sensitive electronic circuits.
- Optical cables, optically-isolated signals, or other galvanic isolation for connection between equipment enclosures, whenever practical.
- Isolation transformers, chokes and/or isolators for sensitive audio, video, and data circuits as required.

#### **26.8.4.6 Component Characteristics**

The Contractor shall select appropriate components to reduce EMI. These components include the following:



- For power converters, such as UPS, dc-to-ac inverters, and variable speed motor drives, use inductor-capacitor line EMI filters on each power input and output, to reduce power frequency and switching harmonic currents and radiated emissions.
- Use ferrite beads and clamshells to reduce emissions from signal cables and mitigate line-to-ground (common mode) and line-to-line (differential mode) noise.
- Use LCD displays for computers. Do not use CRT displays.

#### **26.8.4.7 Modulation Methods**

The Contractor shall:

- Use a modulation method that controls and minimizes the conducted, inductive, and radiated emissions of the equipment for all equipment which converts power from 1 frequency, voltage, or current level to another; and
- For each power converter, provide a report, manufacturer's data sheet, or other technical data to describe the modulation methods used as well as the mitigation methods used to minimize EMI.

### **26.8.5 Facility Power**

#### **26.8.5.1 General Criteria**

The Contractor shall make the following provisions for facility power:

- Take power for TES electronic circuits, HVAC equipment, lighting, elevators and escalators, and other systems from separate branch circuits which are isolated, regulated, backed up, and protected as required. Facility power shall comply with the requirements of the NEC Chapter 2 (Wiring and Protection), Chapter 3 (Wiring Methods and Materials), and Chapter 4 (Equipment for General Use).
- Provide a TES Power EMC Schedule and drawings detailing the specific design provisions for each equipment item and its power and UPS connections.
- Prepare overall power diagrams that define the power distribution scheme for each system and subsystem and show the ac power with both non-insulated ground and insulated ground circuits serving these systems.

#### **26.8.5.2 High Power**

The Contractor shall run high-current power ac cables including phase, ground and neutral conductors, with minimum loop size and conductor separation. The Contractor shall physically separate sensitive circuits from high power cables.

#### **26.8.5.3 Utility Power**

Utility power distribution lines shall be routed and carried to facilities according to applicable EMI/EMF regulations and guidelines:

- 1 • CPUC General Order 95, Overhead Electric Line Construction
- 2 • Southern California Edison, EMF Design Guidelines For Electrical Facilities
- 3 • Pacific Gas & Electric Company, Transmission Line EMF Design Guidelines
- 4 CPUC decisions 93-11-013 and 06-01-042 require investor owned electric utility companies to
- 5 implement no-cost and low-cost steps to reduce EMF levels. These include the following:
- 6 • Increasing the distance from the lines to occupied facilities
- 7     – Increasing pole (structure) height,
- 8     – Increasing the width of right-of-way, and/or
- 9     – Locating power lines closer to the centerline of the corridor.
- 10 • Reducing conductor (phase) spacing
- 11 • Optimizing phasing in a multi-circuit corridor
- 12 • Converting single-phase to split-phase circuits
- 13 • Placing facilities underground
- 14 • Reducing line current by:
- 15     – Reducing load
- 16     – Improving power factor
- 17     – For a given load, providing power at a higher voltage
- 18 CPUC decision 06-01-042 defines “low cost” mitigation of EMF levels to be approximately
- 19 4 percent of total project costs, and prioritizes the allocation of mitigation costs according to the
- 20 following land use types:
- 21 • Schools and licensed day care
- 22 • Residential
- 23 • Commercial/industrial
- 24 • Recreational
- 25 • Agricultural
- 26 • Undeveloped land
- 27 Construction, relocation, or upgrading of a transmission line requires the utility to make a filing
- 28 under CPUC GO 131, Rules Relating to the Planning and Construction of Electric Generation,
- 29 Transmission/Power/Distribution Line Facilities and Substations Located in California.
- 30 The California Department of Education established setback limits for locating a school site
- 31 property line near the edge of easements for any electrical power lines rated 50 kV and above:

- 100 feet for 50 to 133 kV power lines
- 150 feet for 220 to 230 kV power lines
- 350 feet for 500 to 550 kV power lines

The 25 kV OCS lines are not covered by this criterion; however, utility distribution lines to TES power stations are likely to be covered by this criterion.

#### **26.8.5.4 Grounding of Uninterruptible Power Supply Circuits**

The Contractor shall follow IEEE Std 142-2007, Recommended Practice for Grounding of Industrial and Commercial Power Systems, Section 1.9.1, for UPS grounding arrangements.

#### **26.8.5.5 Remote Powered Locations**

Remote locations where utility power is not available may use power supplied from the OCS negative feeder, from a solar storage system, or from another source.

Power at a remote location fed from the negative feeder shall be via a transformer, disconnect, and suitable filtering and power conditioning to protect connected equipment from the transients and harmonics present in the negative feeder.

The selected remote power supply shall make appropriate design provisions for cable, ground, shielding, filtering, and other EMC constraints.

#### **26.8.6 Motors and Controllers**

If motors and motor controllers are used within the TES, such as for ventilation equipment, the following criteria shall apply:

- The Contractor shall provide a TES Motor and Controller EMC Schedule and drawings detailing the specific design provisions for each motor and controller of more than 1 hp.
- The Contractor shall provide each motor starter, controller, or inverter with suitable protection and line and load filtering to minimize transients and surges at start and stop. Provide twisted and/or shielded cables in conduit as appropriate.
- The Contractor shall run input power wires and cables to motors in steel conduit, locate input conductors as far as practical from sensitive equipment, and run wires from the output of the any UPS that powers motors in steel conduit.
- The Contractor shall use soft-start motor controllers or inverters to minimize conducted line transients caused by motor starts. When multiple motors are in a room, coordinate and delay the starting of the motors in a sequence to avoid the intense transient and line dip effects of all motors starting at once.

## **26.8.7 Equipment Rooms and Locations**

### **26.8.7.1 Location**

The Contractor shall make the following provisions for equipment locations:

- Within the physical constraints of the planned facilities, locate equipment so that high power sources are physically separated as far as practical from susceptible equipment and cables.
- Where communication equipment, radio, ATC, and similar rooms are near traction power transformers, HVAC motors, or other high-current systems, locate the susceptible equipment as far as practical from the stray magnetic fields generated by the transformers, motors, and power cables.
- Preferably, locate high-current equipment and cables in rooms that are not adjacent (either vertically or horizontally) to rooms with sensitive and susceptible equipment. Run high-current input power cables, other than traction power cables, in steel conduit.
- Maximize distance between EMI emitter rooms and EMI susceptible receiver rooms. Lay out the equipment rooms with high-current equipment at 1 end of a suite of rooms and susceptible equipment at the other (far) end of the suite of rooms. To the extent practical, locate rooms so that, for example, starting from the left in a hypothetical plan:
  - Traction power, HVAC, or other high-current rooms would be at the leftmost position (room 1).
  - The electrical distribution room (room 2) would be adjacent to and to the right of room 1.
  - The UPS room (room 3) would be adjacent to and to the right of room 2.
  - The communications rooms would be adjacent to and to the right of room 3.

### **26.8.7.2 Equipment Room Shielding**

When architectural or structural constraints do not allow room locations and cable and equipment placement per Section 26.8.7.1 and when required by the characteristics of sensitive equipment installed in a room, the Contractor shall use architectural shielding in the sensitive room to mitigate the electric and magnetic fields from the high-current equipment. Architectural shielding materials include steel sheet and steel mesh wall linings; other shielding wall linings such as sheet copper, copper mesh, or conductive polyamide mesh; EMI shielded doors, windows, access panels, and air ventilation panels; EMI conductive systems; shielding components; shielding laminates; and EMI shielded equipment racks.

## 26.8.8 Equipment Emission and Immunity Limits

### 26.8.8.1 General

While traction power equipment is industrial equipment with similarities to electrical equipment broadly used in other applications, it is non-COTS equipment. TES equipment suppliers shall document the achieved level of compliance of their equipment to designated standards.

Other specific criteria below also apply.

### 26.8.8.2 Current and Voltage Distortion Limits

Harmonic currents or voltages generated by the traction power system and rolling stock can affect the performance of power, communication, and other systems. These effects can be minimized by appropriate placement of feeder, return, and ground conductors on the overhead and track structures, by controlling the phase angle of train power converters to minimize harmonics, and by the appropriate use of onboard filters. Current and voltage distortion limits are provided in IEEE Std 519, Recommended Practices and Requirements for Harmonic Control in Electrical Power Systems. Traction power system current and voltage distortion, including the effects of rolling stock in regular operation, shall conform to the IEEE Std 519 limits in tables 10-3, 10-4 and 11-1, unless the limits imposed by the connected power utility are more strict, in which case the stricter limits shall apply.

### 26.8.8.3 Traction Power EMC Design and Emission Standards

Traction power equipment shall conform to the following EMC emissions and immunity specifications:

- EN 50121-1, Railway applications - Electromagnetic compatibility. General
- EN 50121-2, Railway applications - Electromagnetic compatibility. Emissions of the whole railway system to the outside world
- EN 50121-5, Railway applications - Electromagnetic compatibility. Emission and immunity of fixed power supply installations and apparatus
- EN 61000-6-4, Electromagnetic Compatibility (EMC) – Part 6-4: Generic Standards – Emission standard for industrial environments

IEEE Standards specify design, safety, and EMC aspects of traction power system equipment:

- Standard 80, Guide for Safety in AC Substation Grounding
- Standard 519, Recommended Practices and Requirements for Harmonic Control in Electrical Power Systems
- Standard 525, Guide for the Design and Installation of Cable Systems in Substations

1 Because traction power equipment is non-COTS equipment, the suppliers shall certify that their  
2 equipment conforms to the limits and standards specified in the following sections:

- 3 • EN 50121-2 governs radiated emissions of the system into the environment during train  
4 operation.
  - 5 – For the radiated magnetic field and electric field emission limits for the system including  
6 Rolling Stock, track, and OCS during train operation, the “A” curve applies.
- 7 • EN 50121-2 shows radiated magnetic and electric field emission limits for substations.
- 8 • EN 50121-5 governs TES fixed power supply equipment radiated and conductive emissions  
9 and immunity. EN 50121-2 limits in the previous section apply to higher voltage equipment.  
10 Each TES equipment supplier shall provide equipment which conforms to these limits, as  
11 well as test certification or documentation demonstrating compliance.

12 The non-COTS TES equipment immunity limits for electrostatic discharge, fast transients, and  
13 surges shall conform to EN 50121-5, Railway applications - Electromagnetic Compatibility,  
14 Part 5: Emission and immunity of fixed power supply installations and apparatus.

#### **26.8.8.4 Compatibility with Adjacent Airports and Avionic Equipment**

15 The Contractor shall assess whether radiated electric field emissions from the TES or RS could  
16 interfere with aviation equipment or operations at commercial and general airports adjacent to  
17 the alignment. The assessment shall consider the ambient electromagnetic environment,  
18 radiated emissions and other electromagnetic characteristics of the TES, OCS, and RS, as well as  
19 potentially sensitive aviation equipment.

#### **26.8.9 FCC Type Accepted Radio Equipment**

20 Radio equipment, if any, shall be FCC Type Accepted. Refer to Section 26.6.9.

#### **26.8.10 Human Exposure to Electromagnetic Fields**

21 The Contractor shall design the TES and OCS so that human exposure is below the applicable  
22 specification limits for EMF in and around the system.

23 The CPUC EMF Policy in Decision 06-01-042 states:

- 24 • Health hazards from exposures to EMF have not been established;
- 25 • State and federal public health regulatory agencies have determined that setting numeric  
26 exposure limits is not appropriate; and
- 27 • Existing no-cost and low-cost precautionary-based EMF policy should be continued.

28 The IEEE developed a set of internationally recognized standards to achieve the safe use of  
29 electromagnetic energy in the range of 0 Hz to 300 GHz relative to the potential hazards from  
30 exposure of such energy to man, volatile materials, and explosive devices. In compliance with

the CPUC decision and international practice, the CHSTP established EMF exposure limits in and around the system per the IEEE standards:

- IEEE Standard C95.6-2002 - IEEE Standard for Safety Levels with Respect to Human Exposure to Electromagnetic Fields, 0-3 kHz [IEEE Std C95.6]
- IEEE Standard C95.1-2005 - IEEE Standard for Safety Levels with Respect to Human Exposure to Radio Frequency Electromagnetic Fields, 3 kHz to 300 GHz [IEEE Std C95.1]

The IEEE standards and these criteria establish MPE limits both for the general population and for authorized people in controlled environments. Only workers, contractors, and other authorized personnel may be in the controlled environments, such as in equipment rooms, or along the Authority's right of way near the tracks. The MPE limits recognize that workers and others in these controlled environments have the knowledge to minimize the duration or extent of exposure to the higher levels that may be present.

#### **26.8.10.1 Maximum Permissible Exposure Limits**

The magnetic field MPE for general public and controlled environments shall conform to IEEE Std C95.6 Table 2 and IEEE Std C95.1 Tables 2, 8, and 9. At 60 Hz (the frequency of the traction power system), the IEEE Std 95.6 Table 2 provides a MPE limit of 9.0 G for the general public. This applies in all areas open to the public near and around TES equipment including the OCS.

The IEEE Standard C95.6 MPE for controlled environments in which employees work is 27.1 G. This limit applies to areas not open to the public, such as inside TPFs.

The static electric field MPE limits for general public and controlled environments shall conform to the IEEE Std C95.1 Tables 4, 8, and 9 and IEEE Std C95.6 Table 4.

The propagating EMF MPE limits for general public and controlled environments shall conform to the IEEE Std C95.1 Tables 8 and 9. The MPE limits in the cited revisions of the standards shall apply unless the Contractor establishes that a newer revision of the Standard is applicable and that the newer revision of the Standard provides adequate protection to people near the tracks.

#### **26.8.10.2 Implanted Medical Devices**

Euronorm standards establish exposure limits for implanted medical devices such as pacemakers, implantable cardioverter defibrillators (ICDs), cochlear implants and brainstem stimulators.

The applicable EN standards follow:

- EN 45502-2-1:2004, Active implantable medical devices - Part 2-1: Particular requirements for active implantable medical devices intended to treat bradyarrhythmia (cardiac pacemakers)

1 • EN 45502-2-2:2008, Active implantable medical devices - Part 2-2: Particular requirements  
2 for active implantable medical devices intended to treat tachyarrhythmia (includes  
3 implantable defibrillators)

4 • EN 45502-2-3:2010, Active implantable medical devices - Part 2-3: Particular requirements  
5 for cochlear and auditory brainstem implant systems

6 EN 45502-2-2 recommends the exposure for an Implantable Pulse Generator to be less than  
7 10.0 G for a static magnetic field. Consistent with both the EN limits for medical devices and the  
8 IEEE limits for the general public, the magnetic field MPE for all publicly accessible areas of the  
9 CHSTP shall be 9.0 G, from DC to 800 Hz.

10 EMF levels above the recommended 9.0 G limit may exist inside traction power facilities. TPF  
11 sites will be normally unmanned; workers will enter only periodically, for example, to perform  
12 routine maintenance. The safety program will disclose health risks to employees who have  
13 implanted medical devices and will preclude workers with implanted medical devices from  
14 entering any TPF with levels above the recommended limits.

#### **26.8.11 Animal Exposure to Electromagnetic Fields**

15 Passengers may travel with service or pet animals. Farms and ranches are near the TES and  
16 OCS. The EMF exposure limit for animals shall conform to the same limits as for humans.

#### **26.8.12 Effects on Parallel Adjacent Metal Structures**

17 If the right-of-way runs parallel to ungrounded or partially grounded metal irrigation systems,  
18 metal fences, pipelines, or other metal structures for significant distances, a neighbor could  
19 experience a nuisance shock when touching the metal structure and a grounded metal object.

20 The system shall include normal project design features to preclude nuisance shocks, which can  
21 consist of either: fully grounding the metal structure, fully insulating the metal structure, or  
22 segmenting the structure so that the conductive path parallel to the track is interrupted. The  
23 project shall protect metal structures within a to-be-determined specified lateral distance from  
24 the alignment.

25 Fences with metal fence posts, buried metal pipelines, and movable irrigation pipes on metal  
26 wheels are examples of structures which are already adequately grounded. A metal wire fence  
27 with wood fence posts is an example of a structure that may not be adequately grounded. A  
28 movable irrigation pipe on rubber wheels is an example of a structure that is probably  
29 adequately isolated from ground.



## 26.9 Rolling Stock EMC Design Criteria

### 26.9.1 Equipment and EMC Considerations

RS equipment comprises the following:

- Electrically powered passenger trains with steel wheels on steel rails operating at up to 220 mph on a fully grade-separated alignment. Trains will consist of 1 or 2 trainsets of electric multiple units (EMUs).
- Passenger train electrical systems will include propulsion; ATC; onboard communications; radio, commercial radio and cell phone; Wifi and other unlicensed radio; passenger entertainment and communications; trainline network and data systems; HVAC, auxiliary electric, and lighting equipment; and doors.
- Work trains and cars will operate on the CHSTS tracks.

The performance requirements of rolling stock equipment shall be as described in the Next Generation Amtrak/CHSRA Trainsets Performance Specification.

RS equipment EMC provisions shall ensure the following:

- Safe and dependable operation
- No interference
  - Within the train and with passenger equipment on the train
  - With the ATC system
  - With or from radio communications
  - With or from neighbors
- Compliance with human exposure limits, including for the following:
  - Magnetic and electric fields
  - Step and touch potentials

RS equipment shall conform to electrical safety requirements. Design and maintenance provisions shall also ensure that EMI cannot compromise the safety level of operations achieved by the train system.

Rolling stock equipment shall be HSR proven. Train equipment suppliers shall document the achieved level of compliance of their equipment to designated standards.

The following design criteria apply to RS equipment and their interfaces with other systems:

- Cables and cable segregation
- Grounding

- 1 • Equipment design
- 2 • Motors and controllers
- 3 • Equipment cabinets, control spaces and locations
- 4 • Equipment emission and immunity limits
- 5 • Requirements for radios
- 6 • Human exposure limits
- 7 The RS equipment EMC design criteria are described in following subsections.

## 26.9.2 Cable

### 26.9.2.1 General

8 The Contractor shall use optical interconnection cables wherever it is technically feasible and  
9 cost-effective. Optical connections between equipment and equipment locations are immune to  
10 EMI.

11 The Contractor shall select, install, and connect electrical cables with proper shielding, shield  
12 grounding, conduit or duct protection, entry protection, routing, and termination to control  
13 EMI in copper electrical cables as described in this section.

14 Electrical cable provisions shall comply with the NEC Chapter 2 (Wiring and Protection),  
15 Chapter 3 (Wiring Methods and Materials), Chapter 4 (Equipment for General Use), Chapter 7  
16 (Special Conditions) and Chapter 8 (Communication Systems), wherever the NEC applies,  
17 except to the extent other Design Criteria provisions specify otherwise.

### 26.9.2.2 Electrical Cable Categories

18 For EMC purposes, each cable shall be assigned to 1 of the cable categories in Table 26-7.

**Table 26-7: Electromagnetic Compatibility Cable Categories**

ID	Name	Examples, References, Comments
C1	Power at 600 V ac RMS or higher	If used
C2	Traction Power	25 kV ac traction power and return cables
C3	Power at less than 600 V ac RMS	480 V ac, 120 V ac, Lighting Circuits, HVAC motors, UPS, etc.
C4	NEC Class 1 circuits	NEC Class 1 power limited circuits per NEC 725.41 (A), limited to less than 30 V and 1000 VA. NEC Class 1 remote-control and signaling circuits per NEC 725.21 (B), less than 600 V. Includes safety-critical signaling circuits such as fire alarm. Not used in RS.

C5	NEC Class 2 and 3 circuits	Power sources for NEC Class 2 and Class 3 circuits per NEC 725.121, shall be limited per Chapter 9 Table 11(a) and Table 11(b). Not used in RS.
C6	Unshielded twisted pair signal circuits	Telephone, speaker level audio output, Ethernet
C7	Shielded signal circuits including shielded twisted pair, coax, and multi-axial cables	CCTV from camera to converter box, microphone and line level audio input, line level audio output
C8	Leaky coax cable	Distributed communication antenna
C9	Optical cable	Onboard fiber optic communication

### 26.9.2.3 Cable Separation Criteria

Cables in the same cable category may be run together in the same steel conduit, raceway, duct, or cable tray.

Cables in different cable categories shall not be run together in the same cable assembly, conduit, raceway, duct, or cable tray, without engineering justification documenting the specific cables, signals, and protections against EMI for the proposed combination. However, optical cables (C9) may be run together with any other cable.

Wires carrying circuits belonging to a single cable category but which use different signal levels shall not be run together in the same cable, without engineering justification documenting the specific cables, signals, and protections against EMI for the proposed combination.

Where cables of dissimilar type, service, or signal level cross, the crossing of the cables and their conduits and raceways shall be perpendicular where practical.

Cable separation design shall conform to IEEE Std 1100-2005, Recommended Practice for Powering and Grounding Electronic Equipment, Section 9.9.

### 26.9.2.4 Cable Shielding

The Contractor shall use shielded cables and rigid metal conduits and ducts to protect components, circuits, and systems against the effects of undesirable external disturbing EMI sources. A cable shield can:

- Increase immunity and provide additional protection against interference to the enclosed circuits; and
- Reduce emissions and provide additional reduction of the level of interference emitted by an enclosed circuit.

1 Wire and cable shielding and shield grounding design for electronic signals shall conform to  
2 IEEE Std 1100-2005, Sections 3.3, 4.9, 9.9, 10.2, and 10.4; and to IEEE Std 142-2007, Guide on  
3 Shielding Practice, Section 7.

#### **26.9.2.5 Cable Shield Grounding**

4 The connections of cable shields and the connection of the shields to the grounding system  
5 determine the effectiveness of the shielding in mitigating EMI.

6 The Contractor shall specify and use a coordinated and consistent method for terminating and  
7 connecting cable shields to equipment. The method shall conform to these criteria. The  
8 Contractor shall document the specific provisions for cable shield grounding at the system and  
9 detail level as part of a Cable EMC Schedule. The Contractor shall ensure that the Cable EMC  
10 Schedule is fully implemented across all systems.

11 The general topology for shield connection to power supply ground shall conform to IEEE Std  
12 1100-2005, Sections 8.5, 9.9, 10.2, and 10.4.

13 The Contractor shall properly terminate cable conductors, shield, and drain wire, if used, on the  
14 connector to prevent the inadvertent grounding of the shield or conductor connection via the  
15 connector shell to the equipment enclosure or to another unintended potential.

#### **26.9.2.6 Conduit and Duct**

16 Metal conduits or ducts for cables provide mechanical protection and electrical safety as well as  
17 additional electromagnetic isolation compared to physical separation of cables. Metal conduits  
18 and ducts increase the immunity of and decrease the emissions from enclosed circuits.

19 The Contractor shall:

- 20 • Use rigid galvanized steel or rigid aluminum alloy steel conduit and ducts; and
- 21 • Run cables in conduit or rigid metal duct on any run that is outside of a single enclosure for  
22 any part of its route, except for cables for which engineering justification documents the  
23 basis for the cables to be run out of conduit.

#### **26.9.2.7 Conduit and Duct Bonding and Grounding**

24 Metal conduit and duct can serve as an overall shield for cables carrying sensitive signals, and  
25 for cables carrying signals which tend to emit EMI. To provide a shielding function, metal  
26 conduit and duct shall be grounded and bonded per the NEC Article 250.

27 Only hardware listed by a nationally recognized testing laboratory such as UL shall be used to  
28 join and bond conduits and ducts to enclosures, including conduit fittings and EMI gaskets.

29 Fully coordinate IG ac circuit conduit routing and connections with the grounding of the  
30 systems powered by insulated ground circuits. Refer to Section 26.9.3.6 for IG details.

### 26.9.2.8 Entry Protection

The Contractor shall provide enclosure entry EMI impulse protection for wires entering an equipment enclosure.

The protection shall consider and be consistent with the cable type, the cable source, and the potential EMI hazards to be protected against. As noted above, cables entering an equipment room shall be enclosed in rigid metal conduit unless the system designer provides specific technical justification for another entry protection method consistent with the above criteria.

Line entry impulse protection shall consist of line-to-line and/or line-to-ground protection, as appropriate, for each circuit type. Circuit protections on wires include combinations of lightning arrestors, transient suppressors, fuses, spark gaps, filters, ferrite beads and clamshells, etc.

### 26.9.2.9 Cable Termination

The Contractor shall terminate all impedance controlled cables, such as coax, with the proper impedance.

### 26.9.2.10 Circuit Loops

The Contractor shall make the following provisions for circuit loops:

- Minimize area of circuit loops in which magnetic fields can couple.
- For high-current circuits, run the feed and return conductors as close together as feasible and minimize the length over which the feed and return cables are run separated from one another, to reduce the flux generating loop. To improve the immunity of signal circuits, use twisted pair cables.
- To minimize coupling between the flux generating loop and the signal circuit, maximize the distance between power cables and signal cables, and minimize the distance over which the signal cable runs near and parallel to the power cable.
- Ensure that design layout drawings have sufficient detail to show the physical routing of all conduits and cables, so that EMC compatibility can be designed and documented.

## 26.9.3 Grounding

### 26.9.3.1 General

The Contractor shall make the following grounding provisions:

- Provide each onboard equipment item with a suitable safety ground and suitable EMI-controlling signal ground and return connections.
- Provide a Grounding EMC Schedule and drawings detailing the specific design provisions for each onboard equipment item and its grounding connections.
- Prepare an overall grounding diagram that defines the grounding scheme for each system and subsystem, the ac power (both non-insulated ground and insulated ground) circuits

serving these systems, and the interconnection means and interconnection points of the grounding systems.

For safety grounding requirements, refer to the *Grounding and Bonding Requirements* chapter.

### 26.9.3.2 Grounding Categories

For EMC purposes, the Contractor shall use the grounding categories in Table 26-8 to categorize each ground connection. The Contractor shall apply these grounding categories to achieve the following:

- Minimize the effects of electrical noise generated by the system's equipment and neighboring electrical equipment.
- Minimize disturbing effects within directly affected control equipment and propagating effects on associated equipment, which might result from ground currents, fault currents, or nearby lightning strikes.
- Minimize the shock exposure potential which might appear on non-current-carrying equipment or conducting structures in case an insulation failure energizes the enclosing structure with a dangerous voltage. Proper grounding of equipment and conducting structures will minimize the shock hazard due to a power line insulation failure.
- Provide a discharge path for stray RF energy.

**Table 26-8: Grounding Categories**

ID	Name	Examples, References, Comments
G1	Lightning protection ground	Lightning protection ground for RS is typically the same as G2 Chassis Ground.
G2	Power safety ground/Chassis ground	Chassis Ground shall have separate, normally non-conducting ground buses and ground brush connections to running rails.
G3	Sensitive electronic equipment ground	The sensitive electronic equipment ground should connect by bonding the to the power safety ground only at a single point. The connection of G3 to G2 ground may be by a controlled impedance or low impedance.
G4	SRS ground	A G4 SRS ground is typically not used in RS.
G5	Traction power return	G5 Traction Power Return shall be on ground buses and ground brushes separate from the G2 Chassis Ground.

The Contractor shall make the following provisions:

- Identify, coordinate, keep separate, insulate, and appropriately connect the grounds in each of the ground categories in Table 26-8.

- Integrate and coordinate the lightning protection grounding system with the other grounding system designs.

Optical or other galvanic isolation of signals may be needed to create and maintain the separation of grounds.

The grounds for all low voltage systems, including ground connection types G2 to G5 in Table 26-8, shall be isolated from the railcar chassis. All low voltage system grounds shall be tied to a single low voltage reference point per railcar. The single low voltage reference point shall be connected via a suitable non-zero impedance to the chassis and through separate redundant ground brushes via axles and wheels to the running rails.

The railcar chassis and other safety equipment shall be connected via separate redundant ground brushes via axles and wheels to the running rails.

#### **26.9.3.3 Traction Power Return**

The G5 train traction power return shall not be connected to the G2 railcar chassis. The train traction power return shall be connected via separate redundant ground brushes via axles and wheels to the running rails.

As noted in Section 26.8 on Traction Electrification System, the traction power return circuit shall be connected to the TPF ground grid at each TPF. In addition, connections shall be made between the running rails and the Aerial Static/Ground wire via impedance bonds at appropriate intervals between supply station locations. Refer to the *Grounding and Bonding Requirements* chapter.

#### **26.9.3.4 Industry Standard for Powering and Grounding Electronic Equipment**

The designs for powering and grounding electronic equipment shall conform to the applicable portions of the National Electric Code, of IEEE Std 1100, and of IEEE Std 142, as adapted for the RS environment.

For equipment designs and connections, the Contractor shall use the specified techniques for powering and grounding electronic equipment.

Note that in the recommended power and grounding schemes, the "grounded conductor" refers to that leg of the circuit (usually the neutral) that is intentionally connected to ground. The "grounded conductor" is part of the current-carrying circuit. The "grounding conductor" refers to the conductor(s) that connect(s) exposed metal parts of a device to ground; primarily for safety, secondarily for performance. The "grounding conductor" is not part of the current-carrying circuit.

#### **26.9.3.5 Grounding Design Criteria**

The Contractor shall:

- Use a safety grounding technique for equipment in racks per IEEE Std 1100-2005, Sections 3.3, 4.9, 8.5, 9.9, 10.2, and 10.4; and
- Use a high frequency or RF grounding technique for high frequency signals per IEEE Std 1100-2005, Section 3.3, 4.6, 8.5, 9.9, 10.2, and 10.4.

Chassis grounding shall be through dedicated ground brushes. Traction power return shall be by ground brushes separate from the chassis ground brushes. Normally no current flows through the chassis ground brushes.

#### **26.9.3.6 Insulated Ground Design Criteria**

IG connections are not generally used in RS. If used, IG connections shall be NEC-compliant isolated ground using the IEEE Std 1100 criteria.

### **26.9.4 Equipment Design**

#### **26.9.4.1 General**

Equipment and system designs shall control emissions and increase EMI immunity. Design considerations include placement, enclosures, filters, interconnect design, component characteristics, and modulation methods.

The Contractor shall:

- Specify and implement design provisions for equipment placement, enclosures, filters, interconnect design, component characteristics, and modulation methods, for each equipment item, per the following subsections;
- Ensure that plan and elevation drawings show sufficient detail of the layout of equipment in each rack and the layout of equipment, boxes, and racks within the cab, passenger compartment, underfloor, or rooftop spaces so that EMC compatibility can be designed and documented;
- Provide RS propulsion system converters of the 'unity power factor' type, which shall generate a synthesized sinusoidal current waveform with near unity power factor, to control line harmonics in compliance with IEEE 519; and
- Provide RS propulsion system converters controlled so that propulsion converter harmonics are synchronized and coordinated, and so that audio frequency conducted interference on the OCS and running rails is minimized. The converters shall provide phase shifting to maximize audio frequency harmonic cancellation in all train operating configurations, including with normal and failed converters. The converter shall include suitable failure detection so that the phase shifting of the converters can be rearranged when 1 or more converters in the train fails.



#### 26.9.4.2 Placement

To the extent practical in the RS limited equipment spaces, the Contractor shall separate high power and high voltage equipment and cables from low voltage equipment and cables, to minimize magnetic and electric field coupling between nearby pieces of equipment.

#### 26.9.4.3 Enclosures

The Contractor shall make the following provisions for enclosures:

- Install electronic equipment in a suitable EMI-Shielded Equipment Rack Cabinet.
- Protect signal and power lines entering enclosures from EMI by using appropriate cable termination techniques as well as proper conduit fittings, gaskets, etc.
- Enclose cables entering or leaving electronic enclosures in rigid metal conduit, unless otherwise specifically justified by engineering documentation.
- A double wall shielded equipment enclosure may be necessary for sensitive equipment located near a powerful emissions source. In such cases, the inner enclosure shall be connected to the signal reference structure ground or IG, while the outer enclosure shall be connected to the power safety ground.
- Use shielded enclosures, metal ducts, and metal conduits to control the electromagnetic coupling between and from equipment and wiring.

Within enclosures, the Contractor shall make the following provisions:

- Carefully bundle, twist, shield, protect, route, and properly terminate high-current cables such as motor cables, to minimize uncontrolled emissions. The Contractor shall ensure that the cable installation does not exceed the minimum bend radius for the cable.
- Connect low voltage cable shields to the appropriate low voltage ground at the source and, per IEEE Std 1100-2005, Sections 3.3 and 8.5 and IEEE Std 1143-1994, Section 7, to the receiver-end equipment.
- Use twisted pair and shielded cables, coax, or multiwire for sensitive signals.
- Place inductive components perpendicularly to nearby susceptible components to minimize the exposure to stray flux.

#### 26.9.4.4 Filters

The Contractor shall make the following provisions for filters:

- Use appropriate low-pass filters on wired connections. Filters minimize conducted emissions from a piece of equipment into other pieces of equipment and improve the immunity of the equipment to conducted emissions on the connecting wires.
- Provide filters on power inputs and outputs to and from ac inverters and converters, transmitters, receivers, and signal circuits.

- Use supply suitable transient suppressors for relay contacts switching inductive loads, including relay and contactor coils.

#### **26.9.4.5 Interconnect Design**

The Contractor shall use appropriate techniques to minimize emission and maximize immunity of power and signal conductors, including the following:

- Arrange wires, cables, and conduits to physically separate signals with different signal type, voltage and energy levels.
- Separate and shield circuits carrying power, high level signals, and low level signals.
- Use balanced circuits with coordinated current return paths.
- Minimize the loop area between source and return conductors.
- Use photocouplers or optical fiber to galvanically isolate circuit potentials when needed.
- Shield all sensitive signal conductors in suitable electric and magnetic shielding structures using appropriate combinations of rigid metal conduit and shielded cable.
- Connect shields to appropriate system ground at the high energy end.

The Contractor designs shall include the following:

- Matching networks to match transmission line and antenna systems to maximize performance and minimize loss and reduce EMI for controlled impedance circuits such as radio frequency signals.
- Shielded cables that have a shield coverage of at least 90 percent.
- Guard shields to improve immunity as required for sensitive electronic circuits.
- Optical cables, optically-isolated signals, or other galvanic isolation for connection between equipment enclosures, whenever practical.
- Isolation transformers, chokes and/or isolators for sensitive audio, video, and data circuits as required.

#### **26.9.4.6 Component Characteristics**

The Contractor shall select appropriate components to reduce EMI. These components include the following:

- For power converters, such as UPS, dc-to-ac inverters, and variable speed motor drives, use inductor-capacitor line EMI filters on each power input and output, to reduce power frequency and switching harmonic currents and radiated emissions.
- Use ferrite beads and clamshells to reduce emissions from signal cables and mitigate line-to-ground (common mode) and line-to-line (differential mode) noise.
- Use LCD displays for computers. Do not use CRT displays.

#### 26.9.4.7 Modulation Methods

The Contractor shall:

- Use a modulation method that controls and minimizes the conducted, inductive, and radiated emissions of the equipment for all equipment which converts power from 1 frequency, voltage, or current level to another; and
- For each power converter, provide a report, manufacturer's data sheet, or other technical data to describe the modulation methods used as well as the mitigation methods used to minimize EMI.

#### 26.9.5 Facility Power

This section does not apply to RS equipment.

#### 26.9.6 Motors and Controllers

For RS motors and motor controllers including for propulsion and ventilation, the following criteria shall apply:

- The Contractor shall provide a Motor and Controller EMC Schedule and drawings detailing the specific design provisions for each motor and controller of more than 1 hp.
- The Contractor shall provide each motor starter, controller, or inverter with suitable protection and line and load filtering to minimize transients and surges at start and stop. Provide twisted and/or shielded cables in conduit as appropriate.
- To the extent practical, the Contractor shall run input power wires and cables to motors and controllers in rigid metal conduit, or tightly bundle wires, and locate high-current conductors as far as practical from sensitive equipment.

Additional design criteria apply to the train propulsion system, auxiliary power system, and other power converters, as necessary for the train to conform to the applicable equipment emission and immunity limits.

#### 26.9.7 Equipment Locations

To the extent practical in the RS limited equipment spaces, the Contractor shall separate high power and high voltage equipment and cables from low voltage equipment and cables.

#### 26.9.8 Equipment Emission and Immunity Limits

The following EMC standards shall apply to the RS and equipment:

- EN 50121-1, Railway applications - Electromagnetic compatibility. General
- EN 50121-2, Railway applications - Electromagnetic compatibility. Emissions of the whole railway system to the outside world

- EN 50121-3-1, Railway applications - Electromagnetic compatibility, Part 3-1: Rolling stock - Train and complete vehicle. The standard governs total railcar radiated emissions within its intended operating environment.
- EN 50121-3-2, Railway applications - Electromagnetic compatibility, Part 3-2: Rolling stock – Apparatus. The standard governs emissions and susceptibility of railcar equipment.

#### **26.9.8.1 Track Circuit EMC**

The ATC and RS suppliers shall jointly establish specific immunity limits to protect ATC equipment, particularly track circuits and track-based data communication circuits, from train-generated interference. These limits shall be implemented by coordinated action by the RS and ATC suppliers during ATC and RS design, implementation, and qualification testing.

For EMC between RS and ATC track circuits, EN 50238, Railway applications - Compatibility between RS and train detection Systems, shall govern the relationship. The ATC supplier shall use EN 50238 procedures to document and convert the electromagnetic immunity characteristics of planned wayside equipment into corresponding emission limits which shall apply to RS. The RS supplier shall ensure that the RS emissions comply with the limits developed by the ATC supplier. If necessary, the suppliers shall modify ATC and RS designs to ensure compatibility.

U.S. Department of Transportation Federal Transit Administration test procedures shall govern testing of the compatibility of trains and track circuits. The RS supplier shall perform field qualification tests to demonstrate that the RS conforms to the emission limits using the following procedures:

- UMTA-MA-06-0153-85-6, Conductive Interference in Rapid Transit Signaling Systems Volume II: Suggested Test Procedures
- UMTA-MA-06-0153-85-8, Inductive Interference in Rapid Transit Signaling Systems - Volume II: Suggested Test Procedures

#### **26.9.8.2 Current and Voltage Distortion Limits**

Harmonic currents or voltages generated by the rolling stock into the traction power system can affect the performance of power, communication, and other systems. These effects can be minimized by controlling the modulation and phase angle of train power converters to minimize harmonics, and by the appropriate use of onboard filters. Current and voltage distortion limits are provided in IEEE Std 519, Recommended Practices and Requirements for Harmonic Control in Electrical Power Systems. RS current and voltage distortion, when operated in the CHSTS traction electrification system, shall conform to the IEEE Std 519 limits in tables 10-3, 10-4 and 11-1, unless the limits imposed by the connected power utility are more strict, in which case the stricter limits shall apply.

The Contractor shall perform a compatibility assessment on the Trainset as a whole, including all power units which can generate harmonics into the traction power system, per

1 EN 50388:2005, Railway Applications - Power Supply and Rolling Stock - Technical Criteria for  
2 the Coordination between Power Supply (Substation) and Rolling Stock to Achieve  
3 Interoperability, Section 10, and shall demonstrate that the Trainset does not generate  
4 harmonics beyond the defined limits.

### 26.9.8.3 Railcar Emissions

5 EN 50121-3-1 governs total railcar radiated emissions into the environment. Figures 3-1 and 3-2  
6 show railcar radiated magnetic field and electric field emission limits, for a stopped and for a  
7 slowly moving train.

8 EN 50121-3-1 Section 5 states that immunity for railcars is achieved by assembling the railcar  
9 from equipment which complies with a suitable standard such as EN 50121-3-2.

10 EN 50121-3-1 Figure 1 shows the radiated emission limits for a stopped train. The black solid  
11 line applies to RS.

12 EN 50121-3-1 Figure 2 shows the radiated emission limits for a slowly moving train. The slow  
13 moving test is designed to minimize the effects of the interface between the RS pantograph and  
14 the OCS. The “A” curve applies to the RS.

15 EN 50121-3-2 governs railcar equipment radiated and conductive emissions and immunity.  
16 Each RS equipment supplier shall provide equipment which conforms to these limits, as well as  
17 test certification or documentation demonstrating compliance.

18 The RS equipment immunity limits for electrostatic discharge, fast transients, and surges shall  
19 conform to EN 50121-3-2, Railway applications - Electromagnetic Compatibility, Part 3-2:  
20 Rolling stock – Apparatus.

### 26.9.8.4 Compatibility With Adjacent Airports and Avionic Equipment

21 The Contractor shall assess whether radiated electric field emissions from the TES, OCS, or RS  
22 could interfere with aviation equipment or operations at commercial and general airports  
23 adjacent to the line. The assessment shall consider the ambient electromagnetic environment,  
24 radiated emissions and other electromagnetic characteristics of the TES, OCS, and RS, as well as  
25 potentially sensitive aviation equipment.

### 26.9.9 FCC Type Accepted Radio Equipment

26 Radio equipment shall be FCC Type Accepted.

27 Frequencies for licensed radio equipment shall be coordinated with the system needs and as  
28 part of the coordination work with other nearby users.

29 For FCC Type Accepted radio equipment, the Contractor shall provide a Radio EMC Schedule  
30 and drawings detailing the specific design provisions for each FCC Type Accepted radio.

### 26.9.9.1 Radio Equipment Design Considerations

The Contractor shall develop a detailed frequency and intermodulation analysis and report for each radio system, as part of the Radio EMC Schedule. The report shall cover modulation methods and the following:

- **Transmission Lines** – Use transmission lines, with proper terminating impedances, and matched circuits in RF applications.
- **Antenna Placement** – Place antennas to consider radio propagation factors for reliable operation, and also human exposure levels for any antennas which share an environment with patrons, staff, and neighbors.
- **Intermodulation Suppression** – Perform a frequency and interference analysis during the design phase for sites with more than 1 transmitter and receiver. As appropriate, apply the following:
  - Use cavity filters to attenuate interfering signals from co-located transmission equipment.
  - Use isolators to further suppress intermodulation.
  - Properly select antenna combiners and multi-couplers and segregation of high and low power transmission equipment.

### 26.9.9.2 ISM Equipment

Industrial, scientific and medical device frequency band (ISM) equipment shall be FCC type-accepted ISM band equipment in either the 2.4 GHz or 5.8 GHz ISM band. ISM design applications shall provide acceptable performance when interference from other ISM band users is present from many other mobile and fixed sources.

### 26.9.9.3 Radio System Frequency Coordination

The Contractor shall make the following provisions for radio system frequency coordination:

- Use frequency coordination to minimize EMI in the project. Frequency coordination shall consist of coordinating the transmission or emission frequencies and the reception or susceptible frequencies of all equipment with frequencies already in use in the system and by neighbors.
- Coordinate equipment that transmits or receives on a specific frequency with the list of frequency bands used by other equipment.

### 26.9.10 Human Exposure to Electromagnetic Fields

The Contractor shall place radio transmit antennas and design the connected radio systems so that human exposure is below the applicable specification limits for EMF in and around the system.

The CPUC EMF Policy in Decision 06-01-042 states:

- Health hazards from exposures to EMF have not been established;
- State and federal public health regulatory agencies have determined that setting numeric exposure limits is not appropriate; and
- Existing no-cost and low-cost precautionary-based EMF policy should be continued.

The IEEE developed a set of internationally recognized standards to achieve the safe use of electromagnetic energy in the range of 0 Hz to 300 GHz relative to the potential hazards from exposure of such energy to man, volatile materials, and explosive devices. In compliance with the CPUC decision and international practice, the CHSTP established EMF exposure limits in and around the system per the IEEE standards:

- IEEE Standard C95.6-2002 - IEEE Standard for Safety Levels with Respect to Human Exposure to Electromagnetic Fields, 0-3 kHz [IEEE Std C95.6]
- IEEE Standard C95.1-2005 - IEEE Standard for Safety Levels with Respect to Human Exposure to Radio Frequency Electromagnetic Fields, 3 kHz to 300 GHz [IEEE Std C95.1]

The IEEE standards and these criteria establish MPE limits both for the general population and for authorized people in controlled environments. Only workers, contractors, and other authorized personnel may be in the controlled environments, such as in equipment rooms, or along the right of way near the tracks. The MPE limits recognize that workers and others in these controlled environments have the knowledge to minimize the duration or extent of exposure to the higher levels that may be present.

#### **26.9.10.1 Maximum Permissible Exposure Limits**

The magnetic field MPE limits for general public and controlled environments shall conform to IEEE Std C95.6 Table 2 and IEEE Std C95.1 Tables 2, 8, and 9. The limit for the general public at 60 Hz is 9.0 G.

The static electric field MPE limits for general public and controlled environments shall conform to the IEEE Std C95.1 Tables 4, 8, and 9 and IEEE Std C95.6 Table 4.

The propagating EMF MPE limits for general public and controlled environments shall conform to the IEEE Std C95.1 Tables 8 and 9. The MPE limits in the cited revisions of the standards shall apply unless the Contractor establishes that a newer revision of the Standard is applicable and that the newer revision of the Standard provides adequate protection to people near the tracks.

#### **26.9.10.2 Implanted Medical Devices**

Euronorm standards establish exposure limits for implanted medical devices such as pacemakers, ICDs, cochlear implants and brainstem stimulators.

The applicable EN standards are the following:

- 1 • EN 45502-2-1:2004, Active implantable medical devices - Part 2-1: Particular requirements  
2 for active implantable medical devices intended to treat bradyarrhythmia (cardiac  
3 pacemakers)
- 4 • EN 45502-2-2:2008, Active implantable medical devices - Part 2-2: Particular requirements  
5 for active implantable medical devices intended to treat tachyarrhythmia (includes  
6 implantable defibrillators)
- 7 • EN 45502-2-3:2010, Active implantable medical devices - Part 2-3: Particular requirements  
8 for cochlear and auditory brainstem implant systems

9 EN 45502-2-2 recommends the exposure for an Implantable Pulse Generator to be less than  
10 10.0 G for a static magnetic field. Consistent with both the EN limits for medical devices and the  
11 IEEE limits for the general public, the magnetic field MPE for all publicly accessible areas of the  
12 CHSTS shall be 9.0 G, from DC to 800 Hz.

13 The project shall determine whether an EMF level above the recommended 9.0 G limit can occur  
14 inside the rolling stock passenger compartments. During rolling stock design and construction,  
15 the Contractor shall perform a detailed design assessment to determine worst case level EMF  
16 levels inside the passenger compartment, considering the worst case EMF due to OCS and rail  
17 currents and the shielding effect of the railcar body. After construction, the Contractor shall test  
18 the rolling stock to determine the actual worst case EMF level inside the passenger  
19 compartment. The measurements shall be made following procedures in EN 50500,  
20 Measurement procedures of magnetic field levels generated by electronic and electrical  
21 apparatus in the railway environment with respect to human exposure.

22 If the actual worst case EMF level exceeds the established limit, the safety program shall ensure  
23 that passengers or employees with implanted medical devices are aware of the potential  
24 exposure to a magnetic field above the established limit by placing suitable warning signs in the  
25 passenger compartments or on platforms.

### **26.9.10.3 Animal Exposure to Electromagnetic Fields**

26 Passengers may travel with service or pet animals. The EMF exposure limit for animals shall  
27 conform to the same limits as for humans.

## **26.10 Station and Facility Equipment EMC Design Criteria**

### **26.10.1 Equipment and Considerations**

28 Station and facility equipment comprises the following:

- 29 • Elevators and escalators; HVAC; security equipment; and electrical, lighting, and power  
30 equipment
- 31 • Communications equipment, and optical, wired, and wireless data networks



- Passenger communications and entertainment equipment including closed circuit television, dynamic message signs and displays, public address systems, intercoms, telephones, wireless networks, and traction power cut stations
- Fare collection
- Vending machines
- Radio
- Equipment includes transformers, inverters, converters, motors and controllers, cabinets and contents, control panels, displays, cables, conduits, ducts, raceways, and enclosures.

The stations and facility equipment shall be designed as described in the applicable Design Criteria section for stations and equipment.

Stations and facilities and their equipment EMC provisions shall ensure the following:

- Safe and dependable operation
- No interference with or from the ATC system, with or from radio communications, and with or from neighbors
- Compliance with human exposure limits
  - Magnetic and electric fields
  - Step and touch potentials
- Stations and equipment shall conform to electrical safety requirements.

Design and maintenance provisions shall also ensure that EMI does not compromise the safety level of operations achieved by other systems.

Station and facility equipment includes general purpose industrial equipment, specialized transportation equipment, and COTS equipment. Station and facility equipment suppliers shall document the achieved level of compliance of their equipment to designated standards.

The following design criteria shall apply to stations and facilities, to their equipment systems, and to their interfaces with other systems:

- Cables and cable segregation
- Grounding
- Equipment design
- Facility Power
- Motors and controllers
- Equipment rooms and locations

- 1 • Equipment emission and immunity limits
- 2 • Requirements for radios
- 3 • Human exposure limits
- 4 The stations and equipment EMC design criteria are described in following subsections.

## **26.10.2 Cable**

### **26.10.2.1 General**

- 5 The Contractor shall use optical interconnection cables wherever it is technically feasible and
- 6 cost-effective. Optical connections between equipment and equipment locations are immune to
- 7 EMI.
- 8 The Contractor shall select, install, and connect electrical cables with proper shielding, shield
- 9 grounding, conduit or duct protection, entry protection, routing, and termination to control
- 10 EMI in copper electrical cables as described in this section.
- 11 Electrical cable provisions shall comply with the NEC Chapter 2 (Wiring and Protection),
- 12 Chapter 3 (Wiring Methods and Materials), Chapter 4 (Equipment for General Use), Chapter 7
- 13 (Special Conditions) and Chapter 8 (Communication Systems), wherever the NEC applies,
- 14 except to the extent other Design Criteria provisions specify otherwise.

### **26.10.2.2 Electrical Cable Categories**

- 15 For EMC purposes, each cable shall be assigned to 1 of the cable categories in Table 26-9.

**Table 26-9: Electromagnetic Compatibility Cable Categories**

ID	Name	Examples, References, Comments
C1	Power at 600 V ac RMS or higher	115 kV feeder
C2	Traction Power	25 kV ac traction power, negative feeder, and return cables
C3	Power at less than 600 V ac RMS	480 V ac, 120 V ac; ATC cables including track switch motors, signal lights, and related circuits; OCS Motor Operated Disconnects; lighting circuits, HVAC motors, UPS, etc.
C4	NEC Class 1 circuits	NEC Class 1 power limited circuits per NEC 725.41 (A), limited to less than 30 V and 1000 VA. NEC Class 1 remote-control and signaling circuits per NEC 725.21 (B), less than 600 V. Includes safety-critical signaling circuits such as fire alarm.

C5	NEC Class 2 and 3 circuits	Power sources for NEC Class 2 and Class 3 circuits per NEC 725.121, shall be limited per Chapter 9 Table 11(a) and Table 11(b).
C6	Unshielded twisted pair signal circuits	Telephone, speaker level audio output, Ethernet, track circuit transmitter and receiver cables
C7	Shielded signal circuits including shielded twisted pair, coax, and multi-axial cables	CCTV from camera to converter box, microphone and line level audio input, line level audio output
C8	Leaky coax cable	Distributed communication antenna
C9	Optical cable	Backbone network, local area network, CCTV camera feeds from converter box to communications room

### 26.10.2.3 Cable Separation Criteria

Cables in the same cable category may be run together in the same steel conduit, raceway, duct, trough, manhole, or cable tray, if all cables that are grouped and run together are insulated to the highest voltage level present in the group.

Cables in different cable categories shall not be run together in the same cable assembly, conduit, raceway, duct, or cable tray, without engineering justification documenting the specific cables, signals, and protections against EMI for the proposed combination. However, optical cables (C9) may be run together with any other cable.

Wayside cables running parallel to and crossing the track shall be placed in cable troughs and duct banks. These will include communication cables, train control cables, OCS motor operated disconnects (MODs), 480 VAC, 125 VDC and 48 VDC. The cable trough shall not contain C1, power at greater than 600 V, or C2, traction power cable. Cables in the cable trough, manholes, and duct banks shall be separated by cable category, so that a suitable separation is maintained between, for example 480 VAC cables and ethernet cables. If EMI impact occurs between cables in the cable trough or duct banks, the Contractor shall apply suitable shielding techniques to reduce the emission from the EMI source, to increase the immunity of the EMI receiver, or both. Shielding techniques include cable shields and use of metal conduit.

Traction return cables shall be run in troughs, manholes, and ducts separate from other wayside cables. Traction return cables may share a duct bank but not a duct with other cable categories.

Wires carrying circuits belonging to a single cable category but which use different signal levels shall not be run together in the same cable, without engineering justification documenting the specific cables, signals, and protections against EMI for the proposed combination.

Where cables of dissimilar type, service, or signal level cross, the crossing of the cables and their conduits and raceways shall be perpendicular where practical.

1 Cable separation design shall conform to IEEE Std 1100-2005, Recommended Practice for  
2 Powering and Grounding Electronic Equipment, Section 9.9.

#### **26.10.2.4 Cable Shielding**

3 The Contractor shall use shielded cables and steel conduits and ducts to protect components,  
4 circuits, and systems against the effects of undesirable external disturbing EMI sources. A cable  
5 shield can:

- 6 • Increase immunity and provide additional protection against interference to the enclosed  
7 circuits; and
- 8 • Reduce emissions and provide additional reduction of the level of interference emitted by  
9 an enclosed circuit.

10 Wire and cable shielding and shield grounding design shall conform to IEEE Std 1100-2005,  
11 Sections 3.3, 4.9, 9.9, 10.2, and 10.4; and to IEEE Std 1143-1994, Guide on Shielding Practice,  
12 Section 7.

#### **26.10.2.5 Cable Shield Grounding**

13 The connections of cable shields and the connection of the shields to the grounding system  
14 determine the effectiveness of the shielding in mitigating EMI.

15 The Contractor shall specify and use a coordinated and consistent method for terminating and  
16 connecting cable shields to equipment. The method shall conform to these criteria. Document  
17 the specific provisions for cable shield grounding at the system and detail level as part of a  
18 Cable EMC Schedule. Ensure that the Cable EMC Schedule is fully implemented across all  
19 systems.

20 The general topology for shield connection to power supply ground shall conform to IEEE Std  
21 1100-2005, Sections 8.5, 9.9, 10.2, and 10.4.

22 The Contractor shall properly terminate cable conductors, shield, and drain wire, if used, on the  
23 connector to prevent the inadvertent grounding of the shield or conductor connection via the  
24 connector shell to the equipment enclosure or to another unintended potential.

#### **26.10.2.6 Conduit and Duct**

25 The Contractor shall make the following provisions for conduit and duct:

- 26 • Run specified cables in cable troughs and connecting ducts.
- 27 • Metal conduits or ducts for cables provide mechanical protection and electrical safety as  
28 well as additional electromagnetic isolation compared to physical separation of cables.  
29 Metal conduits and ducts increase the immunity of and decrease the emissions from  
30 enclosed circuits.
- 31 • Use steel conduit and ducts; do not use aluminum.

- Run cables in steel conduit or steel duct on any run that is outside of a single enclosure for any part of its route, except for cables in the cable troughs and cables for which engineering justification documents the basis for the cables which are run out of conduit.

#### 26.10.2.7 Conduit and Duct Bonding and Grounding

Metal conduit and duct can serve as an overall shield for cables carrying sensitive signals, and for cables carrying signals which tend to emit EMI. To provide a shielding function, metal conduit and duct shall be grounded and bonded per the NEC Article 250 where not in conflict with the *Grounding and Bonding Requirements* chapter.

Only hardware listed by a nationally recognized testing laboratory such as UL shall be used to join and bond conduits and ducts to enclosures, including conduit fittings and EMI gaskets.

Fully coordinate IG ac circuit conduit routing and connections with the grounding of the systems powered by insulated ground circuits. Refer to Section 26.10.3.6 for IG details.

#### 26.10.2.8 Entry Protection

The Contractor shall provide the following:

- Room entry EMI impulse protection for wires entering an equipment room
- Enclosure entry EMI impulse protection for wires entering an equipment enclosure

The protection shall consider and be consistent with the cable type, the cable source, and the potential EMI hazards to be protected against. As noted above, cables entering an equipment room shall be enclosed in steel conduit unless the system designer provides specific technical justification for another entry protection method consistent with the above criteria.

Room entry and line entry impulse protection shall consist of line-to-line and/or line-to-ground protection, as appropriate, for each circuit type. Circuit protections on wires include combinations of lightning arrestors, transient suppressors, fuses, spark gaps, filters, ferrite beads and clamshells, etc.

#### 26.10.2.9 Cable Termination

The Contractor shall terminate all impedance controlled cables, such as coax, with the proper impedance.

#### 26.10.2.10 Circuit Loops

The Contractor shall make the following provisions for circuit loops:

- Minimize the area of circuit loops in which magnetic fields can couple.
- For high-current circuits, run the feed and return conductors as closely together as feasible and minimize the length over which the feed and return cables are run separated from one another, to reduce the flux generating loop. To improve the immunity of signal circuits, use twisted pair cables.

- To minimize coupling between the flux generating loop and the signal circuit, maximize the distance between power cables and signal cables, and minimize the distance over which the signal cable runs near and parallel to the power cable.
- Ensure that design layout drawings have sufficient detail to show the physical routing of all conduits and cables, so that EMC compatibility can be designed and documented.

### **26.10.3 Grounding**

#### **26.10.3.1 General**

The Contractor shall make the following grounding provisions:

- Provide each equipment item with a suitable safety ground and suitable EMI-controlling signal ground and return connections.
- Provide a Grounding EMC Schedule and drawings detailing the specific design provisions for each equipment item and its grounding connections.
- Prepare an overall grounding diagram that defines the grounding scheme for each system and subsystem, the ac power (both non-insulated ground and insulated ground) circuits serving these systems, and the interconnection means and interconnection points of the grounding systems.

For safety grounding requirements, refer to the *Grounding and Bonding Requirements* chapter.

#### **26.10.3.2 Grounding Categories**

For EMC purposes, use the grounding categories in Table 26-10 to categorize each ground connection. Apply these grounding categories to achieve the following:

- Minimize the effects of electrical noise generated by the system and by neighboring electrical equipment.
- Minimize disturbing effects within directly affected control equipment and propagating effects on associated equipment which might result from ground currents, fault currents, or nearby lightning strikes.
- Minimize the shock exposure potential which might appear on non-current-carrying equipment or conducting structures, in case an insulation failure energizes the enclosing structure with a dangerous voltage. Proper grounding of equipment and grounding structures will minimize the shock hazard due to a power line insulation failure.
- Provide a discharge path for stray RF energy.

**Table 26-10: Grounding Categories**

ID	Name	Examples, References, Comments
G1	Lightning protection ground	The building lightning protection ground shall connect to the power safety ground, at a single point per NFPA 780-2004: Standard for the Installation of Lightning Protection Systems.
G2	Power safety ground	Governed by the NEC Article 250
G3	Sensitive electronic equipment ground	The sensitive electronic equipment ground should connect by bonding the to the power safety ground only at a single point.
G4	SRS ground	G4 SRS ground configurations include grids and planes which provide a high-frequency common ground reference to which control and communications signals are referenced. A matrix ground mat in or under a sensitive equipment room is a typical SRS implementation. It provides a better reference than a single G3 ground as it provides a lower impedance at high frequencies. If the connection from the G3 sensitive electronic equipment ground to the G2 power safety ground is long, higher impedance in the ground structure results in more electrical noise. A G4 SRS can resolve the problem.
G5	Traction power return	The traction power return and running rails are grounded at each TPF via a connection from the TPF return bus to the TPF ground grid. The running rails are also connected to the aerial ground wire and to the earth at impedance bond locations at intervals along the track.

The Contractor shall make the following provisions:

- Provide a direct, dedicated ground connection when connecting signal and communication ground circuits to a TPF ground grid.
- Identify, coordinate, keep separate, insulate, and appropriately connect the grounds in each of the ground categories in Table 26-10. Where necessary, identify special provisions for grounds in tunnels, trenches, at-grade ballasted or non-ballasted track, and aerial structures or bridges.
- Integrate and coordinate the lightning protection grounding system with the other grounding system designs.

Optical or other galvanic isolation of signals may be needed to create and maintain the separation of grounds.

### 26.10.3.3 Traction Power Return

As noted in a previous section on traction power, the traction power return circuit shall be connected to the TPF ground grid at each TPF. In addition, the running rails shall be connected via impedance bonds to the earth and to the aerial ground wire at intervals between TPF locations, to control step and touch voltage hazards.

1 For safety grounding requirements, refer to the *Grounding and Bonding Requirements* chapter.

#### **26.10.3.4 Industry Standard for Powering and Grounding Electronic Equipment**

2 The designs for powering and grounding electronic equipment shall conform to the applicable  
3 portions of the National Electric Code, of IEEE Std 1100, and of IEEE Std 142, as adapted for the  
4 system environment.

5 For equipment designs and connections, use the specified techniques for powering and  
6 grounding electronic equipment.

7 Note that in the recommended power and grounding schemes, the "grounded conductor" refers  
8 to that leg of the circuit (usually the neutral) that is intentionally held at ground potential. The  
9 "grounded conductor" is part of the current-carrying circuit. The "grounding conductor" refers  
10 to the conductor(s) that connect(s) exposed metal parts of a device to ground; primarily for  
11 safety, secondarily for performance. The "grounding conductor" is not part of the current-  
12 carrying circuit.

#### **26.10.3.5 Grounding Design Criteria**

13 The Contractor shall:

- 14 • Use a safety grounding technique for equipment in racks per IEEE Std 1100-2005, Sections  
15 3.3, 4.9, 8.5, 9.9, 10.2, and 10.4; and
- 16 • Use a high frequency or RF grounding technique for high frequency signals per IEEE Std  
17 1100-2005, Sections 3.3, 4.6, 8.5, 9.9, 10.2, and 10.4.

18 The grounding system shall provide that when electronic equipment has a conducting  
19 connection to other devices within a structure that all interconnected devices be referenced at  
20 the same lightning protection ground (G1) to minimize possible lightning damage.

#### **26.10.3.6 Insulated Ground Design Criteria**

21 Sensitive equipment may use an NEC-compliant isolated ground to provide basic  
22 fault/personnel protection and to protect the equipment. These criteria refer to the NEC-  
23 compliant isolated ground using the IEEE Std 1100 nomenclature, which is insulated ground  
24 (IG).

25 For IG connections for equipment in racks, the Contractor shall modify the safety grounding  
26 configuration from that shown in IEEE Std 1100-2005, Figures 8-17 and 8-18, by placing a listed  
27 nonmetallic raceway fitting between the equipment rack and the conduit system, as shown in  
28 NEC Handbook Exhibit 250.45. The configuration shall ensure that equipment and enclosures  
29 served by an IG power circuit are correctly and safety grounded at the IG location, and not  
30 inadvertently bonded to unintended non-insulated grounding paths.



## 26.10.4 Equipment Design

### 26.10.4.1 General

Equipment and system designs shall control emissions and increase EMI immunity. Design considerations include placement, enclosures, filters, interconnect design, component characteristics, and modulation methods.

The Contractor shall:

- Specify and implement design provisions for equipment placement, enclosures, filters, interconnect design, component characteristics, and modulation methods, for each equipment item, per the following subsections; and
- Ensure that plan and elevation drawings show sufficient detail of the layout of equipment in each rack, the layout of racks within a room, and ancillary equipment within a room, so that EMC compatibility can be designed and documented.

### 26.10.4.2 Placement

The Contractor shall place high power potential sources as far as practical from sensitive potential susceptible equipment, to minimize magnetic and electric field coupling between nearby pieces of equipment.

### 26.10.4.3 Enclosures

The Contractor shall make the following provisions for enclosures:

- Install electronic equipment in a suitable EMI-Shielded Equipment Rack Cabinet.
- Protect signal and power lines entering enclosures shall be protected from EMI by using appropriate cable termination techniques as well as proper conduit fittings, gaskets, etc.
- Enclose cables entering or leaving electronic enclosures in steel conduit, unless otherwise specifically justified by engineering documentation.
- Use a double wall shielded equipment enclosure when necessary for sensitive equipment located near a powerful emissions source. In such cases, the inner enclosure shall be connected to the signal reference structure ground or IG, while the outer enclosure shall be connected to the power safety ground.
- Use shielded enclosures, metal ducts, and metal conduits to control the electromagnetic coupling between and from equipment and wiring.

Within enclosures, the Contractor shall make the following provisions:

- Carefully bundle, twist, shield, protect, route, and properly terminate high-current cables such as motor cables, to minimize uncontrolled emissions. The Contractor shall ensure that the cable installation does not exceed the minimum bend radius for the cable.

- 1 • Connect low voltage cable shields to the appropriate low voltage ground at the source and,  
2 per IEEE Std 1100-2005, Sections 3.3 and 8.5 and IEEE Std 1143-1994, Section 7, to the  
3 receiver-end equipment.
- 4 • Use twisted pair and shielded cables, coax, or multiwire for sensitive signals.
- 5 • Place inductive components perpendicularly to nearby susceptible components to minimize  
6 the exposure to stray flux.

#### **26.10.4.4 Filters**

7 The Contractor shall make the following provisions for filters:

- 8 • Use appropriate low-pass filters on wired connections. Filters minimize conducted  
9 emissions from a piece of equipment into other pieces of equipment and improve the  
10 immunity of the equipment to conducted emissions on the connecting wires.
- 11 • Provide filters on power inputs and outputs to and from ac inverters and converters,  
12 transmitters, receivers, and signal circuits.
- 13 • Use supply suitable transient suppressors for relay contacts switching inductive loads,  
14 including relay and contactor coils.

#### **26.10.4.5 Interconnect Design**

15 The Contractor shall use appropriate techniques to minimize emission and maximize immunity  
16 of power and signal conductors, including the following:

- 17 • Arrange wires, cables, and conduits to physically separate signals with different signal type,  
18 voltage and energy levels.
- 19 • Separate and shield circuits carrying power, high level signals, and low level signals.
- 20 • Use balanced circuits with coordinated current return paths.
- 21 • Minimize the loop area between source and return conductors.
- 22 • Use photocouplers or optical fiber to galvanically isolate circuit potentials when needed.
- 23 • Shield all sensitive signal conductors in suitable electric and magnetic shielding structures  
24 using appropriate combinations of steel conduit and shielded cable.
- 25 • Connect shields to appropriate system ground at the high energy end.

26 The Contractor designs shall include the following:

- 27 • Matching networks to match transmission line and antenna systems to maximize  
28 performance and minimize loss and reduce EMI for controlled impedance circuits such as  
29 radio frequency signals.
- 30 • Shielded cables that have a shield coverage of at least 90 percent
- 31 • Guard shields to improve immunity as required for sensitive electronic circuits.

- Optical cables, optically-isolated signals, or other galvanic isolation for connection between equipment enclosures, whenever practical.
- Isolation transformers, chokes and/or isolators for sensitive audio, video, and data circuits as required.

#### **26.10.4.6 Component Characteristics**

The Contractor shall select appropriate components to reduce EMI. These components include the following:

- For power converters, such as UPS, dc-to-ac inverters, and variable speed motor drives, use inductor-capacitor line EMI filters on each power input and output, to reduce power frequency and switching harmonic currents and radiated emissions.
- Use ferrite beads and clamshells to reduce emissions from signal cables and mitigate line-to-ground (common mode) and line-to-line (differential mode) noise.
- Use LCD displays for computers. Do not use CRT displays.

#### **26.10.4.7 Modulation Methods**

The Contractor shall:

- Use a modulation method that controls and minimizes the conducted, inductive, and radiated emissions of the equipment for all equipment which converts power from 1 frequency, voltage, or current level to another; and
- For each power converter, provide a report, manufacturer's data sheet, or other technical data to describe the modulation methods used as well as the mitigation methods used to minimize EMI.

### **26.10.5 Facility Power**

#### **26.10.5.1 General Criteria**

The Contractor shall make the following provisions for facility power:

- Take power for communication electronic circuits, HVAC equipment, lighting, elevators and escalators, and other systems from separate branch circuits which are isolated, regulated, backed up, and protected as required. Facility power shall comply with the requirements of the NEC Chapter 2 (Wiring and Protection), Chapter 3 (Wiring Methods and Materials), and Chapter 4 (Equipment for General Use).
- Provide a Communications Power EMC Schedule and drawings detailing the specific design provisions for each equipment item and its power and UPS connections.
- Prepare overall power diagrams that define the power distribution scheme for each system and subsystem and show the ac power with both non-insulated ground and insulated ground circuits serving these systems.

#### **26.10.5.2 High Power**

- 1 The Contractor shall run high-current power ac cables including phase, ground and neutral  
2 conductors, with minimum loop size and conductor separation, physically separated from  
3 sensitive circuits, or if near sensitive circuits, twisted or in steel conduit.

#### **26.10.5.3 Utility Power**

- 4 The Contractor shall make the following provisions for utility power:
- 5 • Provide a power distribution design that controls potential effects of EMI generated by high  
6 power equipment on sensitive electronic equipment. Use separate transformers and  
7 distribution buses to ensure high power loads are separated from sensitive loads.
  - 8 • Use the physical placement of power distribution equipment to increase the immunity of  
9 sensitive electronic equipment to EMI generated by high power equipment.
  - 10 • Route utility power distribution lines to facilities according to applicable EMI/EMF  
11 regulations and guidelines.

#### **26.10.5.4 Grounding of Uninterruptible Power Supply Circuits**

- 12 The Contractor shall follow IEEE Std 142-2007, Recommended Practice for Grounding of  
13 Industrial and Commercial Power Systems, Section 1.9.1, for UPS grounding arrangements.

#### **26.10.5.5 Remote Powered Locations**

- 14 Remote communication locations where utility power is not available may use power supplied  
15 from the OCS negative feeder, from a solar storage system, or from another source.
- 16 Power at a remote location fed from the negative feeder shall be via a transformer, disconnect,  
17 and suitable filtering and power conditioning to protect connected equipment from the  
18 transients and harmonics present in the negative feeder.
- 19 The selected remote power supply shall make appropriate design provisions for cable, ground,  
20 shielding, filtering, and other EMC constraints.

#### **26.10.5.6 Current and Voltage Distortion Limits**

- 21 Harmonic currents or voltages generated by facility and station loads can affect the  
22 performance of power, communication, and other systems. Current and voltage distortion  
23 limits are provided in IEEE Std 519, Recommended Practices and Requirements for Harmonic  
24 Control in Electrical Power Systems. Facility power current and voltage distortion shall conform  
25 to the IEEE Std 519 limits in tables 10-3, 10-4 and 11-1, unless the limits imposed by the  
26 connected power utility are more strict, in which case the stricter limits shall apply.

#### **26.10.6 Motors and Controllers**

- 27 If motors and motor controllers are used within the stations and facilities, such as for ventilation  
28 equipment, the following criteria shall apply:

- 1 • The Contractor shall provide a Motor and Controller EMC Schedule and drawings detailing  
2 the specific design provisions for each motor and controller of more than 1 hp.
- 3 • The Contractor shall provide each motor starter, controller, or inverter with suitable  
4 protection and line and load filtering to minimize transients and surges at start and stop.  
5 Provide twisted and/or shielded cables in conduit as appropriate.
- 6 • The Contractor shall run input power wires and cables to motors in steel conduit, locate  
7 input conductors as far as practical from sensitive equipment, and run wires from the  
8 output of the any UPS that powers motors in steel conduit.
- 9 • The Contractor shall use soft-start motor controllers or inverters to minimize conducted line  
10 transients caused by motor starts. When multiple motors are in a room, coordinate and  
11 delay the starting of the motors in a sequence to avoid the intense transient and line dip  
12 effects of all motors starting at once.

## 26.10.7 Equipment Rooms and Locations

### 26.10.7.1 Location

13 The Contractor shall make the following provisions for equipment locations:

- 14 • Within the physical constraints of the planned stations and facilities, locate equipment so  
15 that high power sources are physically separated as far as practical from susceptible  
16 equipment and cables.
- 17 • Where communication equipment, radio, ATC, and similar rooms are near traction power  
18 transformers, HVAC motors, or other high-current systems, locate the susceptible  
19 equipment as far as practical from the stray magnetic fields generated by the transformers,  
20 motors, and power cables.
- 21 • Preferably, locate high-current equipment and cables in rooms that are not adjacent (either  
22 vertically or horizontally) to rooms with sensitive and susceptible equipment. Run all input  
23 power cables to high-current equipment in steel conduit.
- 24 • Maximize distance between EMI emitter rooms and EMI susceptible receiver rooms. Lay out  
25 the equipment rooms with high-current equipment at 1 end of a suite of rooms and  
26 susceptible equipment at the other (far) end of the suite of rooms. To the extent practical,  
27 locate rooms so that, for example, starting from the left in a hypothetical plan:
  - 28 – Traction power, HVAC, or other high-current rooms would be at the leftmost position  
29 (room 1).
  - 30 – The electrical distribution room (room 2) would be adjacent to and to the right of  
31 room 1.
  - 32 – The UPS room (room 3) would be adjacent to and to the right of room 2.
  - 33 – The communications rooms would be adjacent to and to the right of room 3.

### 26.10.7.2 Equipment Room Shielding

When architectural or structural constraints do not allow room locations and cable and equipment placement per Section 26.10.7.1 and when required by the characteristics of sensitive equipment installed in a room, the Contractor shall use architectural shielding in the sensitive room to mitigate the electric and magnetic fields from the high-current equipment. Architectural shielding materials include steel sheet and steel mesh wall linings; other shielding wall linings such as sheet copper, copper mesh, or conductive polyamide mesh; EMI shielded doors, windows, access panels, and air ventilation panels; EMI conductive systems; shielding components; shielding laminates; and EMI shielded equipment racks.

### 26.10.8 Equipment Emission and Immunity Limits

Typical rail station and facility equipment makes broad use of COTS equipment. COTS equipment shall meet U.S. FCC regulations and other EMC standards governing emissions and immunity. Manufacturers of COTS equipment shall guarantee their equipment meets required regulations and selected published standards, although they will generally not test to or modify their equipment to meet standards other than the required and selected published ones.

All electrical and electronic equipment shall conform to the following electromagnetic emission and immunity limits and related requirements.

Each item of electrical or electronic equipment shall be certified and documented to conform to the applicable limits. For equipment that is non-COTS equipment, this documentation shall include a test report, manufacturer's performance data, and acceptance certificates from a qualified test laboratory. Each equipment supplier shall demonstrate and document compliance of each item.

#### 26.10.8.1 COTS Station and Facility Equipment Emission Limits

COTS equipment that is not radio transmission equipment shall meet the following conducted and radiated emission requirements of Part 15 of Title 46 of the Code of Federal Regulation (FCC Part 15), sub-part B:

- Section 15.107(b), Conducted Limits, Class A digital devices
- Section 15.109(b), Radiated Limits, Class A digital devices

FCC Part 15.3(h) defines Class A devices as digital devices which are marketed for commercial, industrial, or business use; not for use in general public or home applications.

FCC Part 15 does not provide immunity limits.

COTS non-radio equipment shall be proven in similar service and shall have a Certification or a Declaration of Conformity per FCC Part 15, sub-part B, Class A devices. For such equipment, provide documentation of the suitability or test results. Suitability considerations include the following:

- The equipment is COTS.
- The equipment is suited for the planned application.
- The equipment has Certification or a Declaration of Conformity per FCC Part 15, sub-part B, Class A device regulations.

A set of COTS equipment, when installed in a compliant and correct manner in an adequate shielded equipment rack, can be treated as a single item of COTS equipment for the purposes of tests and documentation. However, treatment of a shielded rack of equipment as COTS does not relieve the supplier of responsibility to achieve and document electromagnetic compatibility for the equipment rack and all the equipment in it.

#### **26.10.8.2 Non-COTS Station and Facility Equipment Emissions and Immunity**

Non-COTS station and facility equipment, if any, shall conform to the radiated and conducted immunity limits of EN 50121-4, Railway applications - Electromagnetic Compatibility, Part 4: Emission and immunity of signaling and telecommunications apparatus.

Per EN 50121-4, the non-COTS communications equipment radiated and conducted emission limits shall conform to EN 61000-6-4, Electromagnetic Compatibility, Part 6-4: Generic Standards - Emission Standard for industrial environments. Each station and facility equipment supplier shall provide equipment which conforms to the applicable limits, as well as test certification or documentation demonstrating compliance.

In addition, FCC Part 15 governs both COTS and non-COTS communications equipment radiated and conducted emissions, other than for vehicle equipment. FCC limits are more restrictive than EN 50121-4 limits.

The non-COTS station and facility equipment immunity limits for electrostatic discharge, fast transients, and surges shall conform to EN 50121-4, Railway applications - Electromagnetic Compatibility, Part 4: Emission and immunity of signaling and telecommunications apparatus.

#### **26.10.9 FCC Type Accepted Radio Equipment for Stations and Facilities**

Station and facility radio equipment shall be FCC Type Accepted. Frequencies for licensed radio equipment shall be coordinated with the project needs and as part of the coordination effort with other nearby users.

For FCC Type Accepted radio equipment, the Contractor shall provide a Radio EMC Schedule and drawings detailing the specific design provisions for each FCC Type Accepted radio.

For COTS radio transmission equipment that has been proven in similar service and that has received Type Acceptance per applicable FCC Part 15 regulations, the supplier shall document its suitability, demonstrating the following:

- The radio transmission equipment is suited for the planned application.

- The radio transmission equipment has received FCC Part 15 type acceptance.

#### **26.10.9.1 Radio Equipment Design Considerations**

The Contractor shall develop a detailed frequency and intermodulation analysis and report for each radio system, as part of the Radio EMC Schedule. The report shall cover modulation methods and the following:

- **Transmission Lines** – Use transmission lines, with proper terminating impedances, and matched circuits in RF applications.
- **Antenna Placement** – Place antennas to consider radio propagation factors for reliable operation, and also human exposure levels for any antennas which share an environment with patrons, staff, and neighbors.
- **Intermodulation Suppression** – Perform a frequency and interference analysis during the design phase for sites with more than 1 transmitter and receiver. As appropriate, apply the following:
  - Use cavity filters to attenuate interfering signals from co-located transmission equipment.
  - Use isolators to further suppress intermodulation.
  - Properly select antenna combiners and multi-couplers and segregation of high and low power transmission equipment.

#### **26.10.9.2 ISM Equipment**

Industrial, scientific and medical device frequency band (ISM) equipment shall be FCC type-accepted ISM band equipment in either the 2.4 GHz or 5.8 GHz ISM band. ISM design applications shall provide acceptable performance when interference from other ISM band users is present from many other mobile and fixed sources.

#### **26.10.9.3 Radio System Frequency Coordination**

The Contractor shall make the following provisions for radio system frequency coordination:

- Use frequency coordination to minimize EMI. Frequency coordination shall consist of coordinating the transmission or emission frequencies and the reception or susceptible frequencies of all equipment with frequencies already in use in the system and by neighbors.
- Coordinate equipment that transmits or receives on a specific frequency with the list of frequency bands used by other equipment.

#### **26.10.10 Human Exposure to Electromagnetic Fields**

The Contractor shall place station and facility radio transmit antennas and design the connected radio systems so that human exposure is below the applicable specification limits for EMF in and around the system.



The CPUC EMF Policy in Decision 06-01-042 states:

- Health hazards from exposures to EMF have not been established;
- State and federal public health regulatory agencies have determined that setting numeric exposure limits is not appropriate; and
- Existing no-cost and low-cost precautionary-based EMF policy should be continued.

The IEEE developed a set of internationally recognized standards to achieve the safe use of electromagnetic energy in the range of 0 Hz to 300 GHz relative to the potential hazards from exposure of such energy to man, volatile materials, and explosive devices. In compliance with the CPUC decision and international practice, the CHSTP established EMF exposure limits in and around the system per the IEEE standards:

- IEEE Standard C95.6-2002 - IEEE Standard for Safety Levels with Respect to Human Exposure to Electromagnetic Fields, 0-3 kHz [IEEE Std C95.6]
- IEEE Standard C95.1-2005 - IEEE Standard for Safety Levels with Respect to Human Exposure to Radio Frequency Electromagnetic Fields, 3 kHz to 300 GHz [IEEE Std C95.1]

The IEEE standards and these criteria establish MPE limits both for the general population and for authorized people in controlled environments. Only workers, contractors, and other authorized personnel may be in the controlled environments, such as in equipment rooms, or along the Authority's right of way near the tracks. The MPE limits recognize that workers and others in these controlled environments have the knowledge to minimize the duration or extent of exposure to the higher levels that may be present.

#### **26.10.10.1 Maximum Permissible Exposure Limits**

The magnetic field MPE for general public and controlled environments shall conform to IEEE Std C95.6 Table 2 and IEEE Std C95.1 Tables 2, 8, and 9. At 60 Hz (the frequency of the traction power system), the IEEE Std 95.6 Table 2 provides a MPE limit of 9.0 G for the general public. This applies in all areas open to the public near and around TES equipment including the OCS.

The IEEE Standard C95.6 MPE for controlled environments in which employees work is 27.1 G. This limit applies to areas not open to the public, such as inside facility equipment rooms.

The static electric field MPE limits for general public and controlled environments shall conform to the IEEE Std C95.1 Tables 4, 8, and 9 and IEEE Std C95.6 Table 4.

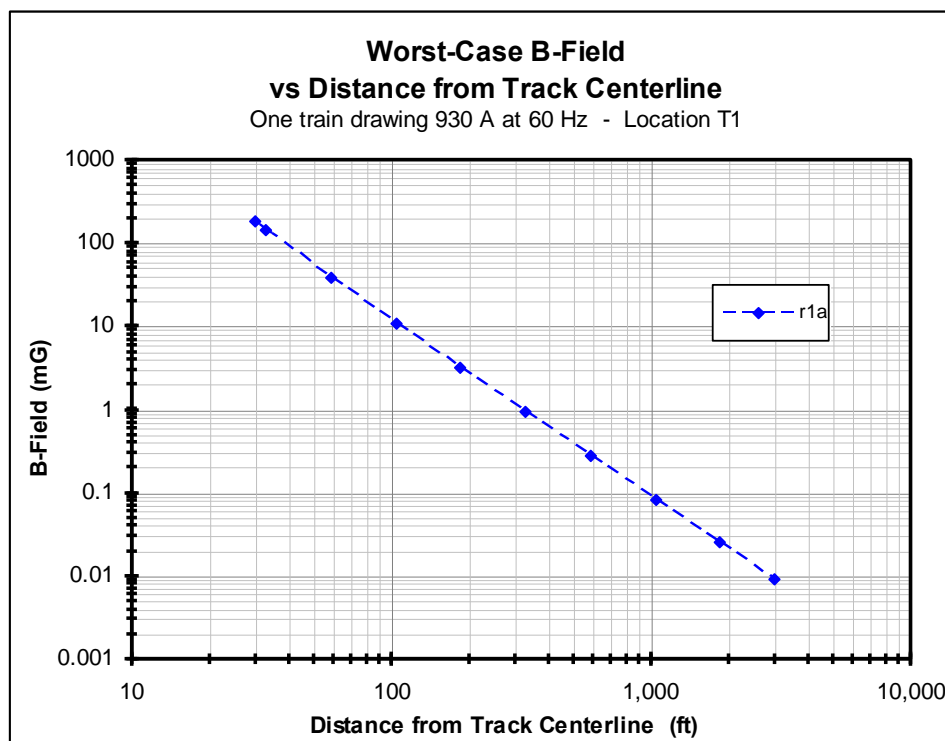
The propagating EMF MPE limits for general public and controlled environments shall conform to the IEEE Std C95.1 Tables 8 and 9. The MPE limits in the cited revisions of the standards shall apply unless the Contractor establishes that a newer revision of the Standard is applicable and that the newer revision of the Standard provides adequate protection to people near the tracks.

Figures 26-1 and 26-2 show the estimated worst-case 60 Hz magnetic field caused by a high-speed train versus perpendicular distance from the center of the right-of-way. For this

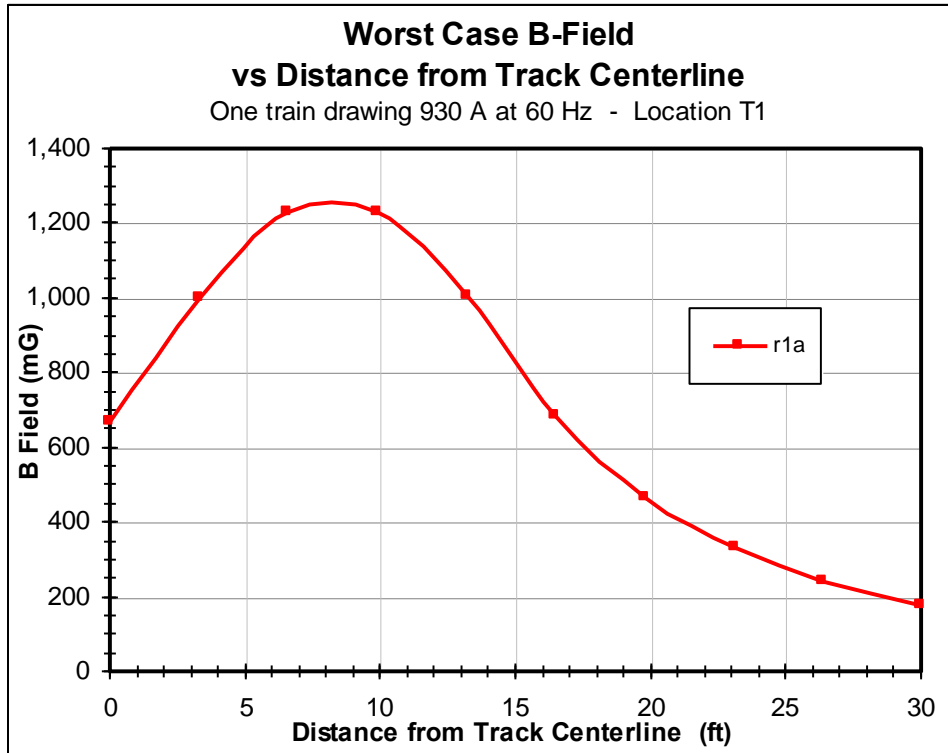
calculation, the train is assumed to continuously draw 930 A at 60 Hz. Figure 26-1 shows the peak magnetic field level at a location close to the traction power substation versus lateral distance from the right-of-way centerline track. The peak in the graph is at the centerline of the track containing the train. Figure 26-2 shows the peak magnetic field level at greater lateral distances from the track, on a log-log scale. In operation, the worst-case peak magnetic field condition is not prolonged. This is because train current does not remain at a peak level under all operating conditions, because currents split between 2 tracks, between running rails and negative feeder, and between current paths to the front and rear front and rear as the train proceeds down the line.

Figure 26-1 shows that a person standing on a station platform, at about 8 feet from the track centerline or 16 feet from the right-of-way centerline, could be exposed to a worst-case magnetic field of about 720 mG. Thus, the worst-case human exposure is about 7.9 percent of the applicable MPE limit and less than the 10 G implanted medical device limit.

**Figure 26-1: Worst-Case Magnetic Field due to a 930 A train, close to the track**



1 **Figure 26-2: Worst-Case Magnetic Field due to a 930 A train, farther from the track**



2  
3 In addition, after construction, EMF levels shall be measured to determine whether actual EMF  
4 levels exist above the recommended 9.0 G limit at or near the passenger station platform edge,  
5 and determine whether passengers or employees with implanted medical devices can be  
6 exposed to magnetic fields above the established limit. The measurements shall be made  
7 following procedures in EN 50500, Measurement procedures of magnetic field levels generated  
8 by electronic and electrical apparatus in the railway environment with respect to human  
9 exposure. If the actual EMF levels exceed the established limit, the safety program shall ensure  
10 that passengers and employees who have implanted medical devices are informed of the  
11 condition by placing suitable warning signs on the passenger station platforms, indicating areas  
12 where passengers and employees with pacemakers can stand or sit without exposure to EMF  
13 levels above the recommended limits.

#### 26.10.10.2 Implanted Medical Devices

14 Euronorm standards establish exposure limits for implanted medical devices such as  
15 pacemakers, ICDs, cochlear implants and brainstem stimulators.

16 The applicable EN standards are the following:

- 17 • EN 45502-2-1:2004, Active implantable medical devices - Part 2-1: Particular requirements  
18 for active implantable medical devices intended to treat bradyarrhythmia (cardiac  
19 pacemakers)

1 • EN 45502-2-2:2008, Active implantable medical devices - Part 2-2: Particular requirements  
2 for active implantable medical devices intended to treat tachyarrhythmia (includes  
3 implantable defibrillators)

4 • EN 45502-2-3:2010, Active implantable medical devices - Part 2-3: Particular requirements  
5 for cochlear and auditory brainstem implant systems

6 EN 45502-2-2 recommends the exposure for an Implantable Pulse Generator to be less than  
7 10.0 G for a static magnetic field. Consistent with both the EN limits for medical devices and the  
8 IEEE limits for the general public, the magnetic field MPE for all publicly accessible areas of the  
9 CHSTP shall be 9.0 G, from DC to 800 Hz.

10 EMF levels above the recommended 9.0 G limit may exist in controlled environments such as  
11 facility equipment rooms. Equipment rooms will be normally unmanned; workers will enter  
12 only periodically, for example, to perform routine maintenance. The safety program will  
13 disclose health risks to employees who have implanted medical devices and will preclude  
14 workers with implanted medical devices from entering any controlled environment with levels  
15 above the recommended limits.

#### **26.10.10.3 Animal Exposure to Electromagnetic Fields**

16 Passengers may travel with service or pet animals. Farms and ranches are near the TES and  
17 OCS. The EMF exposure limit for animals shall conform to the same limits as for humans.

## Chapter 27

### Supervisory Control and Data Acquisition Subsystems

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	May 2013	Revision 1



## Table of Contents

<b>27</b>	<b>SUPERVISORY CONTROL AND DATA ACQUISITIONS SUBSYSTEMS .....</b>	<b>27-1</b>
27.1	Scope .....	27-1
27.2	Regulations, Codes, Standards, and Guidelines .....	27-2
27.3	Related Documentation .....	27-4
27.4	Overview of the SCADA Subsystems and Related Communications Systems .....	27-5
27.4.1	SCADA Overview .....	27-5
27.4.2	Communications Systems for SCADA Data Transport .....	27-6
27.5	SCADA Subsystems .....	27-8
27.5.1	General SCADA Subsystems .....	27-8
27.5.2	Traction Electrification System (TES) SCADA Subsystem .....	27-12
27.5.3	Automatic Train Supervision SCADA Subsystem .....	27-17
27.5.4	Hazard Detection/Early Warning SCADA Subsystem .....	27-20
27.5.5	Infrastructure Mechanical Electrical Plumbing (MEP) SCADA Subsystem .....	27-22
27.5.6	Facilities Mechanical Electrical Plumbing (FAC MEP) SCADA Subsystem .....	27-23
27.5.7	Fixed Remote Telemetry (FRT) SCADA Subsystem .....	27-25

## Tables

Table 27-1:	TES Equipment .....	27-12
Table 27-2:	Hazard Detection/Early Warning System Sensors and Equipment .....	27-21
Table 27-3:	Infrastructure MEP Equipment .....	27-23
Table 27-4:	Facilities MEP Equipment .....	27-24
Table 27-5:	Fixed Remote Telemetry Equipment .....	27-25

## Figures

Figure 27-1:	SCADA and Communications Systems .....	27-6
Figure 27-2:	Typical Wired SCADA Subsystem to Communications Systems Interfaces ..	27-8

## Appendix

Appendix 27.A	SCADA Command, Monitor and Diagnose Matrix
---------------	--

## Acronyms

ac	Alternating Current
ATC	Automatic Train Control
ATS	Automatic Train Supervision
CHST	<i>Not Used; use HST for California High-Speed Train</i>
CHSTP	California High-Speed Train Project
CIC	Communication Interface Cabinet
COTS	Commercial off-the-Shelf
dc	Direct Current
FAC MEP	Facilities Mechanical, Electrical, and Plumbing
FRT	Fixed Remote Telemetry
HDEW	Hazard Detection/Early Warning
HMI	Human Machine Interface
HST	High-Speed Train
HV	High Voltage
HVAC	Heating, Ventilation, and Air Conditioning
IIMP	Integrated Information Management Platform
INF MEP	Infrastructure Mechanical, Electrical, and Plumbing
kV	Kilovolt
LAN	Local Area Network
MEP	Mechanical, Electrical, and Plumbing
MTU	Master Terminal Unit
PLC	Programmable Logic Controller
PS	Parallel Stations
PTC	Positive Train Control
OCC/RCC	Operations / Regional Control Center
OCS	Overhead Contact System
ROW	Right-of-Way
RTU	Remote Terminal Unit
SCADA	Supervisory Control and Data Acquisition
SS	Substation
SWS	Switching Stations
TES	Traction Electrification System
TSR	Temporary Speed Restriction
UPS	Uninterruptible Power Supply
WAN	Wide Area Network
WPC	Wayside Power Control Cubicles



## 27 Supervisory Control and Data Acquisitions Subsystems

### 27.1 Scope

1 The Supervisory Control and Data Acquisition (SCADA) Design Criteria presents the  
2 guidelines, principles and processes that must be followed to successfully design and  
3 implement SCADA subsystems, software, equipment and support systems for the California  
4 High-Speed Train Project (CHSTP). These underlying design criteria govern the design and  
5 implementation of SCADA Systems and shall be applied consistently across the entire  
6 California High-Speed Train (HST) system.

7 The SCADA Design Criteria addresses design criteria for all SCADA systems used to support  
8 the provisioning, safe operation, and cost-effective operations and maintenance of the HST  
9 system. The following SCADA systems shall be designed, built, integrated and tested using  
10 these criteria and guidelines to support the requirements of the CHSTP.

11 This document contains general SCADA guidelines and SCADA subsystem sections that  
12 capture SCADA subsystem design criteria and guidelines.

13 The following SCADA subsystems are the focus of this document:

- 14 • Traction Electrification System (TES) SCADA subsystem
- 15 • Automatic Train Supervision (ATS) SCADA subsystem
- 16 • Hazard Detection/Early Warning (HDEW) SCADA subsystem
- 17 • Infrastructure Mechanical Electrical Plumbing (INF MEP) SCADA subsystem
- 18 • Facilities MEP (FAC MEP) SCADA subsystem
- 19 • Fixed Remote Telemetry (FRT) SCADA subsystem

20 The onboard monitoring and diagnostics system is not included in the scope of this chapter. For  
21 design criteria for interfacing onboard monitoring and diagnostics systems to wayside systems,  
22 refer to the *Rolling Stock-Core Systems Interfaces* chapter.

23 For each system the following classifications of design criteria are defined:

- 24 • **Architecture Design Criteria** – High-level design criteria focused on redundancy, overall  
25 subsystem configuration and similar.
- 26 • **Spare Capacity, Expansion and Obsolescence Design Criteria** – Design criteria focused on  
27 spare capacity, expansion, and obsolescence.

- 1 • **Command and Control Design Criteria** – Design criteria focused on how the SCADA  
2 subsystem is controlled both locally and remotely. A Control, Monitor and Diagnose  
3 matrix is included to show what functionality is controlled by Operations and Maintenance  
4 users at local and remote locations.
- 5 • **Physical, Enclosure and Power Design Criteria** – Low-level design criteria focused on  
6 system components' physical support design criteria including, cabinets, enclosures,  
7 heating, ventilation, and air conditioning (HVAC), power, uninterruptible power supply  
8 and similar.
- 9 • **End-Device Design Criteria** – Low-level design criteria focused on end-device attributes,  
10 placement and characteristics.

## 27.2 Regulations, Codes, Standards, and Guidelines

11 Refer to the *General* chapter for requirements pertaining to regulations, codes, standards, and  
12 guidelines.

- 13 • Code of Federal Regulations (CFR)
  - 14 – Title 47 CFR, Part 15: Class A
  - 15 – Title 49 CFR, Part 236 Appendix E: Human-Machine Interface (HMI) Design
- 16 • Americans with Disabilities Act (ADA)
  - 17 – Title 28, CFR Part 35: Title II Technical Assistance Manual
  - 18 – Title 28, CFR Part 36: Title III Standards of Accessible Design
  - 19 – Title 49, CFR Part 37: Transport Services for Individuals with Disabilities
- 20 • National Fire Protection Association (NFPA) Codes and Standards
  - 21 – NFPA 70: National Electrical Code
  - 22 – NFPA 72: National Fire Alarm Code
  - 23 – NFPA 130: Standard for Fixed Guideway Transit and Passenger Rail Systems
- 24 • American National Standards Institute (ANSI) Standards
  - 25 – ANSI-J-STD-607-A: Commercial Building Grounding (Earthing) and Bonding
  - 26 Requirements for Telecommunications
  - 27 – ANSI/ICEA S-87-640: Standard for Outside Plant Communications Cable
  - 28 – ANSI/ICEA S-80-576: Communications Wire and Cable for Wiring of Premises
  - 29 – ANSI/ICEA S-83-596: Standard for Fiber Optic Premises Distribution Cable
  - 30 – ANSI/HFES 100: Human Factors Engineering Requirements for Visual Display Terminal
  - 31 (VDT) Work Stations

- 1       – ANSI/IEEE C37.90.2: Standard for Tolerance of Radiated Electromagnetic Frequency
- 2       Interference (RFI)
- 3       • Institute of Electrical and Electronics Engineers (IEEE) Standards
- 4       – IEEE 802.1: Bridging and Management
- 5       – IEEE 802.3: Ethernet
- 6       – IEEE 802.11: Wireless LANs
- 7       – IEEE 828: Standard for Software Configuration Management Plans
- 8       • International Organization for Standardization (ISO) Standards
- 9       – ISO/IEC 12207: Systems and software engineering -- Software life cycle processes
- 10      • Electronic Industries Association/Telecommunications Industry Association (EIA/TIA)
- 11      Standards
- 12      – EIA-310: Cabinets, Racks, Panels, and Associated Equipment
- 13      – EIA 472: Generic Specifications for Fiber-optic cable
- 14      – EIA/TIA-598: Optical Fiber Cable Color Coding
- 15      – TIA-475.0000: Generic Specifications for Fiber Optic Connectors
- 16      – TIA/EIA-455: Standard Test Procedures for Optical Fibers, Cables, Transducers, Sensors,
- 17      Connecting and Terminating Devices, and other Fiber Optic Components.
- 18      – TIA/EIA 568: Commercial Building Telecommunications Cabling Standards
- 19      – TIA/EIA 569: Commercial Building Standard for Telecommunications Pathways and
- 20      Spaces
- 21      • Underwriter Laboratories Inc. (UL) Publications
- 22      – UL 6: Rigid Metal Conduit
- 23      – UL 44: Thermoset-Insulated Wires and Cables
- 24      – UL 50: Standard for Enclosures for Electrical Equipment
- 25      – UL 62: Flexible Cords and Cables
- 26      – UL 94: Standard for Tests for Flammability of Plastic Materials for Parts in Devices and
- 27      Appliances
- 28      – UL 467: Grounding and Bonding Equipment
- 29      – UL 486 A-E: Wire Connectors
- 30      – UL 510: Standard for Polyvinyl Chloride, Polyethylene and Rubber Insulating Tape
- 31      – UL 514A: Metallic Outlet Boxes
- 32      – UL 514B: Conduit, Tubing, and Cable Fittings

- 1       – UL 886: Outlet Boxes and Fittings for Use in Hazardous (Classified) Locations
- 2       – UL 1581: Reference Standard for Electrical Wires, Cables and Flexible Cords
- 3       – UL 1778: Uninterruptible Power Supply Equipment
- 4       • Telcordia [Bellcore]
- 5       – Network Equipment-Building System (NEBS) LEVEL 3 requirements
- 6       – GR-20: Generic Requirement for Optical Fiber and Optical Cables
- 7       – GR-63: NEBS Requirements: Physical Protection
- 8       – GR-771: Generic Requirements for Fiber Optic Splice Closures
- 9       • U.S. Department of Defense (USDOD) Standards
- 10      – MIL-STD-1472: Human Engineering
- 11      – MIL-STD-781: Reliability, Test Methods, Plans, and Environments for Engineering,
- 12      Development, Qualification and Production
- 13      – MIL-STD-810: Department of Defense Test Method Standard for Environmental
- 14      Engineering Considerations and Laboratory Tests
- 15      • National Transportation Communications for ITS Protocol (NTCIP) Standards
- 16      • Telecommunication Standardization Sector (ITU-T) Standards

### **27.3      Related Documentation**

- 17      • Design Criteria chapters
- 18      – General
- 19      – Trackway Clearances
- 20      – Utilities
- 21      – Structures
- 22      – Drainage
- 23      – Stations
- 24      – Support Facilities
- 25      – Facility Power and Lighting Systems
- 26      – Rolling Stock and Vehicle Intrusion Protection
- 27      – Building Automation and Management Systems
- 28      – Traction Power Supply System
- 29      – Overhead Contact System and Traction Power Return System

- 1 – Grounding and Bonding Requirements
- 2 – Automatic Train Control
- 3 – Electromagnetic Compatibility and Interference
- 4 – Supervisory Control and Data Acquisition Subsystems
- 5 – Rolling Stock-Core System Interfaces
- 6 – Reliability, Availability, Maintainability and Safety

## 27.4 Overview of the SCADA Subsystems and Related Communications Systems

### 27.4.1 SCADA Overview

7 The SCADA subsystems shall be designed to monitor, acquire, transmit and receive data and  
8 control inputs and outputs to control and monitor systems and equipment. The SCADA  
9 subsystems provide remote control from manned locations including the Operations Control  
10 Center (OCC), Regional Control Center (RCC), Station Control Centers to monitor and control  
11 selected equipment via Programmable Logic Controllers (PLCs) or remote terminal units  
12 (RTUs).

13 SCADA subsystems shall be used to control and monitor physical processes, sensors equipment  
14 and status and have the following elements.

#### 27.4.1.1 SCADA Master Terminal Units

15 A SCADA Master Terminal Unit (MTU) shall include the centrally located SCADA subsystem  
16 servers, processors, databases and software responsible for communicating with the SCADA  
17 subsystem field equipment. (A SCADA MTU is also described as a SCADA headend.) The  
18 designer may ultimately choose to combine the several SCADA subsystem MTUs' functionality  
19 into a single or multiple MTUs. Within this document, each SCADA subsystem will be  
20 described as a stand-alone subsystem with independent MTUs.

#### 27.4.1.2 SCADA Field Equipment

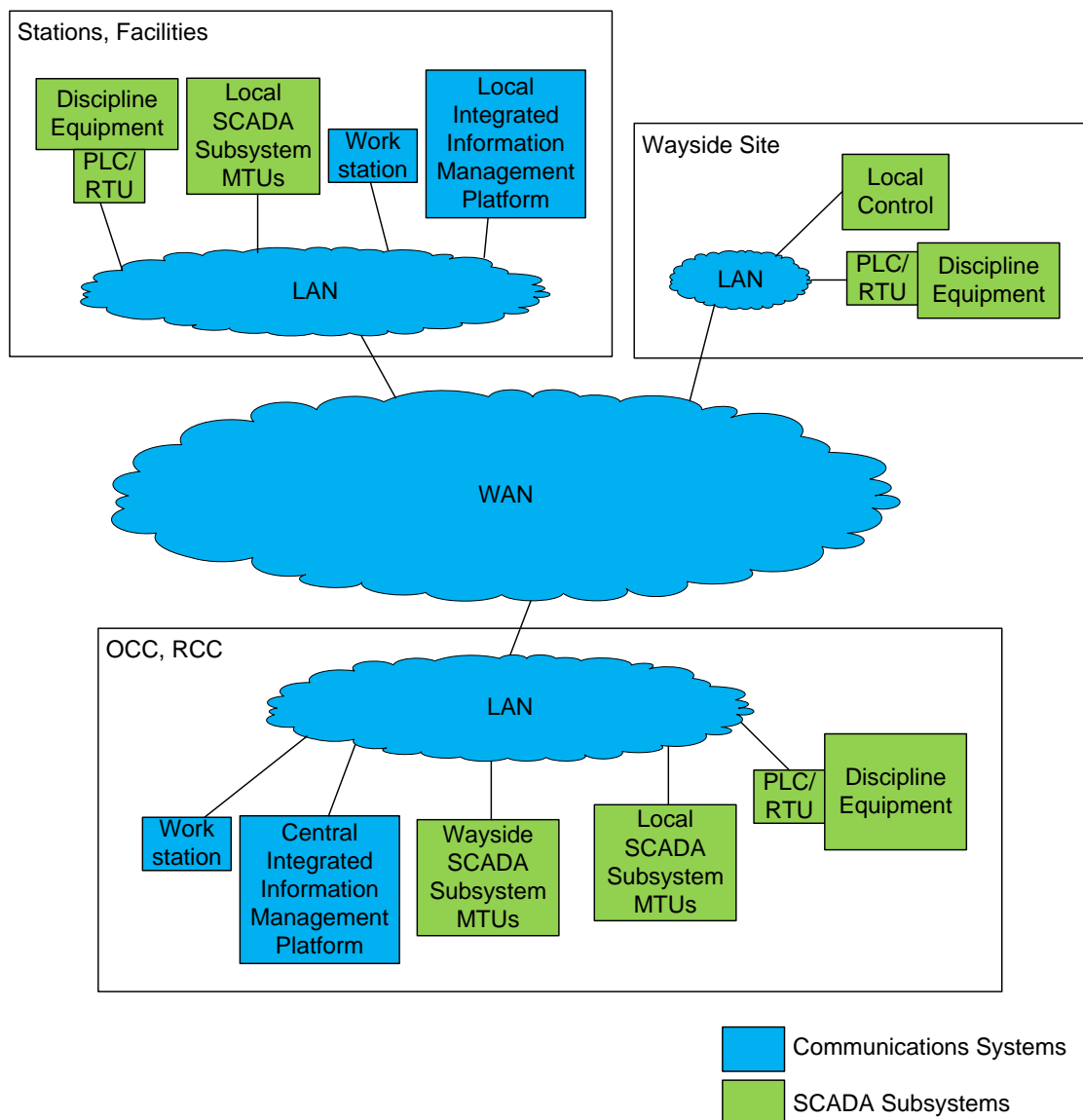
21 RTUs and/or PLCs are the electrical devices which shall interface between physical plant,  
22 equipment or sensors and the communications Wide Area Network (WAN) or Local Area  
23 Network (LAN).

24 SCADA subsystems may also be designed to include local control panels co-located with  
25 physical plant, equipment or sensors being controlled or monitored. These local control panels  
26 may be a workstation with GUI, or control panel and shall be used to provide local control,  
27 monitoring and health diagnosis of the SCADA equipment that is located within the vicinity.

## 27.4.2 Communications Systems for SCADA Data Transport

The SCADA subsystems shall transmit control signals and alarms between remote sites and the OCC/RCC and locally via the interconnection of communications systems (WAN and LAN) or utilize dedicated cable infrastructure. Figure 27-1 shows the relationship between communications systems and SCADA subsystems elements.

**Figure 27-1: SCADA and Communications Systems**



### 27.4.2.1 Wide Area Network and Local Area Networks

Refer to the *Communications* chapter for descriptions of the WAN and the LAN. It is required that all SCADA subsystem devices using communications data transport systems interface electrically and/or optically on the basis of standardized, accepted and interoperable protocols.

#### 27.4.2.2 Integrated Information Management Platform and SCADA Subsystems

1 Refer to the *Communications* chapter for a description of the Integrated Information  
2 Management Platform (IIMP). SCADA data shall be integrated with other data to provide  
3 integrated control over all Project systems via the IIMP.

4 SCADA Human Machine Interface (HMI) functionality shall be provided to give users control  
5 and visibility of SCADA subsystem MTU processes such as feedback, trending, diagnostics, and  
6 management information such as scheduled maintenance procedures, detailed schematics for a  
7 particular sensor or machine, and troubleshooting guides. In general, SCADA data and control  
8 shall be displayed and used by operations personnel at the OCC, RCCs, stations, yards and  
9 facilities via the communications IIMP while HMI functionality shall be provided by the  
10 SCADA subsystem designer. Generally, the IIMP shall only act as the human interface  
11 presentation device for the SCADA subsystems; it performs no SCADA processing. HMI  
12 functionality shall be designed to present SCADA information to the operating personnel  
13 graphically, in the form of a mimic diagram. The IIMP gathers and presents the data provided  
14 by the SCADA subsystems' MTUs to automatically or manually allow the operator, dispatcher,  
15 security officer, station manager or maintenance manager to control, monitor and diagnose  
16 systems. Status reporting, information storage and retrieval, alarm processing, incident and  
17 operation reports shall be provided at the OCC/RCCs and other specified access points  
18 throughout the system. All SCADA points shall be able to be displayed and controlled,  
19 depending on user privileges via the IIMP workstations located throughout the system.

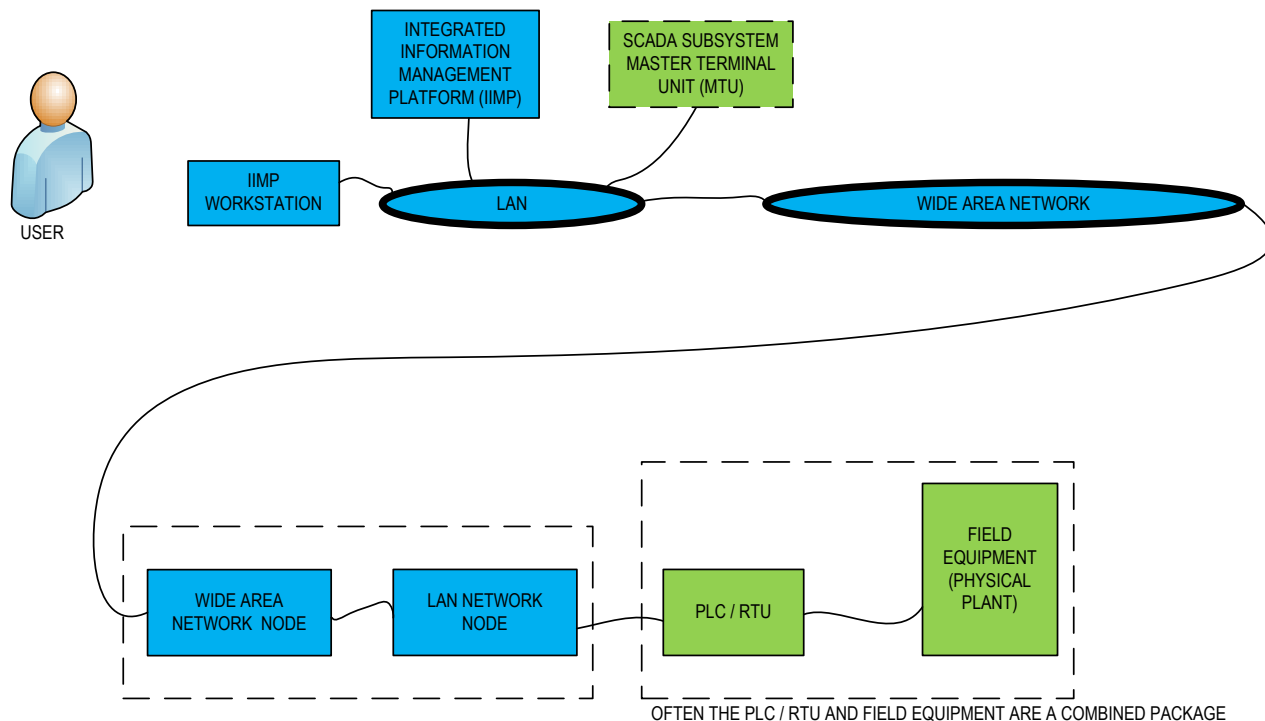
#### 27.4.2.3 SCADA Interfaces

20 The SCADA subsystems shall be designed to interface with communications systems and other  
21 project systems. The designer is to design the SCADA subsystems to interface with the  
22 communications systems. Refer to the *Communications* chapter for details on communications  
23 system design criteria.

##### A. Wired Interface Chain

24 The majority of the SCADA subsystems shall be interfaced to the communications systems via  
25 wired interfaces.

**Figure 27-2: Typical Wired SCADA Subsystem to Communications Systems Interfaces**



## 27.5 SCADA Subsystems

The SCADA subsystem elements shall be designed to be compliant with the protocols and connectivity of the WAN and/or LAN and other communications systems, as appropriate.

### 27.5.1 General SCADA Subsystems

The SCADA subsystems shall be individual systems that deliver remote command, control and monitoring functionality to the system. The following design criteria sections apply to all SCADA subsystems unless more specific or stringent criteria are listed in the individual sections.

#### 27.5.1.1 General SCADA Subsystems Architecture

The core system designer shall design MTUs for SCADA subsystems to be located at OCC and RCCs. For each SCADA subsystem, there shall be redundancy designed into the failover means for SCADA subsystem MTUs installed at each OCC and RCC. The OCC MTU shall be configured to be the master MTU for each SCADA subsystem, in the event of failure the RCC MTUs shall take over from the main OCC MTU.



1 The facility and station designer shall design MTUs for Facility MEP and Fixed Remote  
2 Telemetry SCADA subsystems to be located at main Train Control and Communications (TCC)  
3 rooms at each facility and station.

4 The adherence to open system architecture concepts and non-proprietary standards is required  
5 whenever it is available to ensure interoperability of products from 2 or more manufacturers.

6 Standard commercial off-the-shelf (COTS) devices shall be used when such equipment and  
7 materials meet the functional and performance specifications. Special designs and custom  
8 assemblies shall be confined to those devices that require such features to assure compliance  
9 with the specifications.

#### 27.5.1.2 General SCADA Subsystems Spare Capacity, Expansion and Obsolescence

10 SCADA subsystem design shall minimize the effects of early component or subsystem  
11 obsolescence due to advances in technology.

12 The SCADA subsystems shall be designed to facilitate future integration of additional facilities  
13 without interruption of the already operating system when these additions are implemented.

14 The SCADA subsystems shall be designed to accommodate a full-scale deployment along the  
15 Authority's entire proposed right-of-way. Nothing in the design shall preclude the Authority  
16 from expanding the systems via subsequent, open procurements. The SCADA subsystems shall  
17 be designed to have a scalable, flexible, open, non-proprietary architecture.

18 Over and above the above requirement for accommodating a full-scale deployment, the cable  
19 infrastructure, SCADA subsystems, enclosures, spaces and support systems (e.g., HVAC, UPS)  
20 shall be designed to accommodate the future expansion or installation of 25 percent additional  
21 SCADA subsystems equipment and/or end devices.

#### 27.5.1.3 General Command and Control

22 The SCADA subsystems shall be designed to meet user requirements and command and control  
23 requirements.

24 Refer to *Appendix 27.A SCADA Command, Monitor and Diagnose Matrix* for high-level  
25 functionality to be designed into the SCADA subsystem. This high-level functionality is shown  
26 in column 1. This matrix also defines the level of control to be designed into the SCADA  
27 subsystems for Operations and Maintenance personnel interface. All SCADA subsystem  
28 interface functionality to other systems is defined in the specifications.

29 In general, interfaces and integration between SCADA subsystems and other systems shall be  
30 accomplished via the Integration Logic within the communications IIMP. See the  
31 *Communications* chapter's section on Integrated Information Management Platform.

32 In general, SCADA subsystems shall be designed to have multiple levels of command and  
33 control by users located at OCC, RCC, stations, facilities and yards. Each SCADA subsystem

1 shall be designed to be controlled and accessed via application programming interface (API) by  
2 the appropriate central or local Integrated Information Management Platform (refer to the  
3 *Communications* chapter) whereby the integration, interface and command and control  
4 functionality specified can be controlled via the central and/or local IIMPs.

5 Each SCADA subsystem shall have self-diagnostics built-in to the system whereby end-devices,  
6 inputs and components can be remotely interrogated to determine the health and operational  
7 status of the system and speed fault diagnosis and repair in response to failures.

8 Each SCADA subsystem shall support local initialization and troubleshooting with either a local  
9 control panel or portable test equipment.

10 Each SCADA subsystem shall include specific local and remote command, monitor and  
11 diagnose requirements for users with different functions and locations.

#### 27.5.1.4 General SCADA Subsystems Physical, Enclosure and Power

12 All SCADA Subsystem MTU equipment shall be designed to be enclosed in standardized  
13 equipment racks.

14 Equipment cabinet and enclosure doors shall be equipped with lockable handles. All locks shall  
15 be keyed alike. Doors shall be removable when unlocked.

16 Cabinets within rooms shall be located so that rack mounted equipment can be accessed via the  
17 front and the back of the cabinet. Equipment mounted within wall mounted enclosures or  
18 cabinets shall be mounted such that both front and rear access to the equipment is possible via  
19 swing-out equipment rack mounting.

20 Each cabinet and enclosure shall have a main breaker that can shut off power to all equipment  
21 in the cabinet or enclosure.

22 Equipment cabinets and enclosures shall be as specified in the *Electromagnetic Compatibility and*  
23 *Interference* chapter.

24 Cabinets and enclosures that are not located within environmentally and particulate-controlled  
25 equipment rooms shall be NEMA-4 rated. These cabinets and enclosures shall be enclosed and  
26 lockable and be designed to meet the environmental requirements of the housed equipment.

27 Normal testing shall not require major disassembly of the equipment under test or of any other  
28 equipment.

29 Remote SCADA equipment shall be equipped for protection from electromagnetic interference  
30 levels consistent with location. Bus bars shall be provided for grounding in all SCADA cabinets.

31 SCADA equipment enclosures, housings, conduit pathways and spaces shall not violate the  
32 clearances as specified in the *Trackway Clearances* chapter and shall be spatially coordinated with  
33 the requirements in the *Tunnels* and *Structures* chapters.

Loads of SCADA equipment enclosures, housings, conduit pathways and cabling mounted on structures shall not exceed the load limits specified in the *Structures* chapter.

**A. Power and Uninterruptible Power Supply (UPS)**

Refer to the *Facility Power and Lighting Systems* chapter for Uninterruptible Power Supply (UPS) details. Uninterruptible power shall be provided to all SCADA subsystems equipment including MTU and field devices based on criteria listed in the *Facility Power and Lighting Systems* chapter.

All SCADA subsystem equipment shall be capable of automatic start-up following a power outage of any length without requiring manual re-initialization.

All SCADA subsystem equipment shall retain full status memory and process recall when operating on power from battery or inverter sources.

**27.5.1.5 General SCADA Subsystem End-Devices**

The majority of SCADA subsystem field equipment in tunnel areas shall be located in cross passages.

All end-device equipment shall, wherever practical, be designed to contain a self-diagnostic capability to permit the remote interrogation of the health and operational status of the unit and facilitate speedy repair after failure.

All SCADA subsystem end-device equipment shall be mounted in a serviceable location within proximity to the equipment served.

All SCADA subsystem end-device equipment shall perform self-tests upon power-up and on command from local test equipment and from supervisory stations. Self-tests shall also be performed by input/output equipment.

All SCADA subsystem end-device equipment shall provide for maintenance of input/output circuits (including disabling power to output circuits) and safe replacement of input/output cards while power is applied to the remote SCADA equipment.

All SCADA subsystem end-device equipment shall be capable of continued operation in the electromagnetic environment where located, such as traction power facilities, wayside power control cubicles, signal cases, near Overhead Contact System (OCS), in tunnels and trenches and communications equipment rooms.

Remote SCADA equipment shall utilize a network connection to the WAN or LAN for communication with the MTU. The SCADA network interface shall be compliant with the applicable WAN and/or LAN protocols. The SCADA network interface shall be functionally contiguous among all SCADA remote units and the MTU when linked via the WAN, LAN. Error correction and detection schemes shall utilize an industry standard compatible with the WAN and/or LAN.

- 1 All SCADA subsystem end-device equipment shall operate normally unattended. Remote
- 2 SCADA equipment logic and configuration data shall reside in non-volatile memory.
- 3 Input and output signals shall be electrically isolated from SCADA equipment.
- 4 SCADA end device placement shall not violate the clearances as specified in the *Trackway*
- 5 *Clearances* chapter and shall be spatially coordinated with the requirements in the *Tunnels* and
- 6 *Structures* chapter. SCADA end device locations shall be coordinated with structural and
- 7 architectural elements. Refer to the *Stations*, *Support Facilities*, and *Structures* chapters.
- 8 Loads of SCADA end devices mounted on structures shall not exceed the load limits specified
- 9 in the *Structures* chapter.

## 27.5.2 Traction Electrification System (TES) SCADA Subsystem

### 27.5.2.1 Traction Electrification System and Equipment Introduction

- 10 Refer to the *Traction Power Supply System* chapter for Traction Electrification System description
- 11 and details. The details of OCS design criteria are presented in the *Overhead Contact System and*
- 12 *Traction Power Return System* chapter.
- 13 Table 27-1 is a description of Traction Electrification System (TES) equipment that is controlled
- 14 and monitored by TES SCADA and its general location.

**Table 27-1: TES Equipment**

TES Equipment	Function	General Location
Main transformers	Transform voltage of incoming supply to 2x25 kV	Traction power substations (SS), located roughly 30 miles apart along the ROW
Autotransformers	Boost the OCS voltage	Switching stations (SWS) and paralleling stations (PS) located along the ROW with the configuration SS-PS-PS-SWS-PS-PS-SS- PS and so on.
HV circuit breakers	To disconnect HV supply under load conditions and for protection of the system	HV side of the traction power substations
HV disconnect switches	To disconnect HV supply under no-load conditions	HV side of the traction power substations
25 kV circuit breakers	To disconnect 25 kV supply under load conditions and for protection of the system	At all SS, SWS, PS and at interlockings
Motorized disconnect Switches	To disconnect 25 kV supply under no-load conditions	At all SS, SWS, PS, at railway stations, wayside universal crossovers, maintenance facilities, tunnel portals, and in tunnels longer than 3 miles

15

### 27.5.2.2 Description of the TES SCADA Subsystem

The TES SCADA system monitors and controls remote data input/output units to and from the user control locations and TES equipment. TES equipment is located at traction power facilities: SS, SWS, PS and wayside power control cubicles (WPC). Field TES SCADA equipment shall be installed at these locations. The TES SCADA system shall comprise MTUs located at the OCC and RCCs and remote TES SCADA equipment located in the field at TP facilities. Communications between the MTUs and the remote SCADA equipment shall utilize the communications WAN and LANs and the appropriate communications protocols.

The TES SCADA subsystem shall provide supervisory control of the TES. The TES SCADA system shall transmit in real-time: metering data, indications, alarms, and controls and confirmation thereof between wayside facilities and equipment and the MTUs located at the OCC and RCCs via the WAN and LANs. These SCADA transmissions shall include the following:

- Metering information
- TES alarm and control signals and status indications
- TES related auxiliary and emergency power alarms and signals
- Status reports, information storage and retrieval, alarm processing, incident and operations reports shall be provided by the MTUs

### 27.5.2.3 TES SCADA Subsystem Architecture

The TES SCADA subsystem shall be designed to achieve the following operational objectives:

- Permit complete control of the TES
- Indicate unambiguously the position of every controllable circuit breaker and switch on the TES and the status of equipment and/or subsystems
- Indicate the value of pre-determined analog quantities
- Provide an alarm condition if some preset value is exceeded
- Provide an alarm condition if a circuit breaker is automatically operated due to a fault condition
- Provide an alarm if an unintentional operation takes place
- Indicate geographically the electrical condition (i.e., energized, isolated or grounded) of the TES
- Perform routine tests at any time to verify that the equipment is in correct working order, without interfering with the wiring and without removal of any primary/on-line and/or secondary/"hot-standby" equipment

The TES SCADA subsystem shall be based upon an open modular architecture approach, which is compliant with the Open Software Foundation Distributed Computing environment for

distributed computing functions and totally portable among hardware platforms of different origin.

The TES SCADA subsystem shall be modular to permit expandability for future extensions to the TES and future non-TES applications.

The TES SCADA subsystem shall be designed to do the following:

- Be capable of transmitting digital status and control data, and analog measurement data
- Be self monitoring (i.e., failure of any piece of equipment down to the individual printed circuit boards shall cause an alarm locally and at the MTUs)
- Incorporate hardware and/or software features that prevent access by unauthorized persons
- Employ industry standards for network interfaces (i.e., hardware and software)
- Be capable of archiving and retrieving information over a time span up to 5 years
- Permit the generation of pre-configured and customized reports
- Be compatible with the Train Control System and Hazard Detection/Early Warning SCADA subsystem and permit data exchange in real time (*the information to be exchanged will be defined by CHSTP during the design phase*)
- Be immune to Electromagnetic Interference (EMI) from nearby high current electrical equipment to ensure safe and reliable operations under all loads and faults
- Be designed such that replacement of components can be performed easily in the field

Standard industry-accepted open and non-proprietary network communication protocols compatible with the WAN and LAN design and field communications protocols (between network and field equipment) shall be used.

Operating and application software shall be used that is compliant to industry standards produced by standard organizations, as specified in the *Communications* chapter.

Duplicate redundant communication, processing, power supply equipment, and input/output interface components, necessary as part of the TES SCADA subsystem, shall be provided at each traction power facility, WPC and at the OCC and RCCs.

The hardware or software failure of 1 unit, major or minor, including nodes common to primary/on-line and secondary/"hot-standby" equipment shall not cause complete failure of its associated equipment or any other equipment.

All secondary/"hot-standby" equipment shall be continuously monitored to the same extent as the primary/on-line equipment.

All processing equipment shall be individually addressable.

1 The TES SCADA equipment shall be capable of accommodating variations (without  
2 degradation in communications) in line impedance, delay distortion, etc., which may be  
3 expected on these types of circuits

4 Failure of the SCADA software or hardware shall not create a less safe condition. Indication of  
5 such a loss shall be provided at the OCC/RCC.

#### 27.5.2.4 TES SCADA Subsystem Spare Capacity, Expansion and Obsolescence

6 The SCADA system's hardware and software shall be sized to include 25 percent spare  
7 equipment and points and 100 percent system expandability for future growth and expansion  
8 of the TES.

#### 27.5.2.5 TES SCADA Subsystem Command and Control

9 Refer to *Appendix 27.A SCADA Command, Monitor and Diagnose Matrix* for TES SCADA high-  
10 level functionality and Command, Monitor and Diagnose criteria.

11 At the OCC/RCC, all alarms shall be required to be acknowledged by the central control  
12 operator before they are cleared from the SCADA's alarm summary list at the user interface.

13 At each user interface, a full-featured software program for defining and controlling user access  
14 to the SCADA system shall be provided. The system administrator shall define access  
15 capabilities for each user, user interface and each SCADA application and support function via  
16 access control displays.

17 The user interface displays shall mimic the system single line diagrams and equipment symbols  
18 depicted on design documentation, including equipment and cable designations:

- 19 • Energized and de-energized power equipment and/or conductors (i.e., circuit breakers,  
20 disconnect switches, 25 kV feeder and OCS circuits) shall be depicted by red and green  
21 respectively.
- 22 • Operational and non-operational communication equipment and fiber optic cables shall be  
23 depicted by blue and green respectively.
- 24 • Equipment that is switched to local control shall be depicted by a unique, user-defined  
25 color.
- 26 • Equipment locked and tagged out by the power dispatcher shall be indicated by a unique,  
27 user-defined color.

28 One local control panel user interface shall be provided at each traction power facility control  
29 and relay room equipment enclosure.

30 No permanent user interface is required at WPC. However, the local TES SCADA equipment  
31 shall include the provision to interface a laptop computer to interface and control, monitor and  
32 diagnose local SCADA functionality when necessary.

1 The TES SCADA subsystem shall not monitor and/or control equipment owned, operated and  
2 maintained by the local power utility company but shall monitor the voltage on the HV side of  
3 the main transformers at the traction power substations.

4 The TES SCADA subsystem must provide local and remote SCADA command and control  
5 functionality of TES equipment at the following field locations:

- 6 • Traction Power Substations
- 7 • Traction Power Switching Stations
- 8 • Traction Power Paralleling Stations
- 9 • Wayside Circuit Breakers and Motorized Disconnect Switches
- 10 • Wayside Power Control Cubicles

11 Each piece of similar equipment shall be individually monitored and/or controlled.

12 A failure in the primary and/or secondary SCADA equipment shall annunciate an event alarm.

#### **27.5.2.6 TES SCADA Subsystem Physical, Enclosure and Power**

13 Equipment associated with the TES SCADA subsystem shall be located in secured enclosures  
14 and/or rooms to prevent unauthorized access.

15 TES SCADA equipment at traction power facilities (TPF) and WPC shall be housed in dedicated  
16 enclosures that can be incorporated into an equipment line-up or wall mounted.

17 The TES SCADA equipment at the OCC/RCC shall be powered from normal and UPS. The  
18 SCADA equipment at trackside facilities shall be powered from a battery or from an integral  
19 battery-backed power supply with preferred voltage of 24 V dc; or a 125 V dc control power  
20 source.

21 120 V ac circuits to TES SCADA equipment shall not provide power to other subsystems.  
22 Additionally primary/on-line and secondary/"hot-standby" equipment shall not be fed from the  
23 same 120 V ac circuit.

24 All equipment, including mother boards or back planes shall be so arranged as to be readily  
25 accessible for inspection, maintenance, fault finding, and repair.

26 When the emergency power source is activated due to loss of normal power, an event alarm  
27 shall be annunciated.

28 Where a UPS is utilized the following alarms shall be annunciated:

- 29 • UPS Loss of Incoming Ac Power
- 30 • UPS Low Battery Reserve
- 31 • UPS Rectifier Failure



- UPS Automatic Switch to Bypass Mode
- UPS Manual Switch Set to Bypass Mode
- UPS Switch to Bypass
- UPS Switch to On-line

Metering functions shall be continuous and all other SCADA points shall be binary (two-state).

#### 27.5.2.7 TES SCADA Subsystem End-Devices

There are no specific criteria. Refer to Section 27.5.1.5.

### 27.5.3 Automatic Train Supervision SCADA Subsystem

#### 27.5.3.1 Automatic Train Supervision System and Equipment Introduction

The signaling and train control system shall be collectively known as the Automatic Train Control (ATC) system. ATC includes all the safety critical and non-safety critical functions of a train control system, and includes all Positive Train Control (PTC) functions. Refer to the *Automatic Train Control* chapter for ATC system description and details.

#### 27.5.3.2 Description of the Automatic Train Supervision SCADA Subsystem

The Automatic Train Supervision (ATS) system shall provide the remote supervision and remote control facilities for the ATC system. Many of the functions including route setting, junction management, and schedule regulation shall have automated functional options with the ability for dispatchers and managers to intervene as necessary.

ATS shall provide the following centralized control and supervision functions (ATS functionality, subsystem performance, and hardware requirements shall be part of the ATC system):

- ATC shall be controlled, supervised, and monitored by ATS from the Operations Control Center (OCC) or the Regional Control Centers (RCCs).
- Train routing – The ATS shall provide automatic and remote manual control of interlockings, including route setting and canceling, and individual switch operation. Automatic train routing shall provide sub-modes including the following:
  - By schedule
  - By first come first served
  - By train run ID
- Dispatching shall be achieved by the clearing of signal routes for trains about to depart, allowing sufficient time for last minute passengers to board and the doors to close and lock. Signals or movement authorities shall be held against trains to prevent them from departing stations and yards early.

- 1     • Scheduling – The ATS shall store multiple versions of the operating schedule and allow  
2       real-time editing by dispatchers and operating supervisors, to cancel, add, or adjust trip  
3       information for individual and groups of trains. The ATS shall automatically regulate  
4       trains in accordance with the timetable amended or otherwise.
- 5     • Temporary Speed Restrictions (TSRs) and work zones shall be entered at ATS workstations  
6       and shall be transmitted to and implemented in the field and onboard by the respective  
7       ATC subsystems.
- 8     • Access to the ATS and its functional layers shall be password protected. Management of  
9       functions to layers and of passwords shall be performed by a System Administrator, Chief  
10      Dispatcher or equivalent.
- 11    • ATS facilities shall be provided in the OCC, RCCs, and training facilities to allow for  
12      playback and related analysis of events following incidents and emergencies using ATS  
13      recorded data and for training dispatchers and managers.
- 14    • ATS facilities shall be provided to enable the training of dispatchers and managers. The  
15      simulator functions shall allow the creation of scenarios both from scratch and from  
16      recorded data by a trainer. They shall allow the trainee to manage the scenario through to a  
17      conclusion. A separate set of workstations shall be provided for training purposes. The  
18      training facilities shall also be available for playback of events and incidents.
- 19    • ATS facilities shall be provided to enable construction and evaluation by simulation of new  
20      timetables including supplements to existing timetables and for minor and major editing of  
21      existing timetables.
- 22    • ATS shall interface to passenger information facilities including customer information signs  
23      and audible public address subsystems with train status information for display at  
24      passenger stations and other public locations.

### **27.5.3.3 Automatic Train Supervision SCADA Subsystem Architecture**

25    The ATS shall also provide event recording facilities of all data input to the system and  
26    commands sent from the system.

27    ATC shall provide an automatic field-fallback mode for all interlockings when unsupervised by  
28    ATS, except for terminals and major through stations including Los Angeles. In the event of loss  
29    of ATS control from the OCC or RCCs, routes at interlockings shall automatically revert to  
30    previously programmed through routes and fleet settings, or as otherwise commanded from a  
31    Local Control Panel.

32    All ATS equipment needed for standalone operation shall be installed in the OCC and in at least  
33    2 additional RCCs.

34    The ATS subsystem architecture shall be highly availability and reliable, in order to ensure that  
35    the ATC and CHSTP meet its service dependability requirements.

1 ATS hardware shall consist to the maximum extent possible of COTS devices and components.  
2 Hardware shall be chosen based on its suitability to the environment in which it will operate  
3 and also to facilitate future upgrades of hardware as the COTS equipment becomes obsolete.

4 The ATS shall contain the equivalent of PTC back office server (BOS) functionality so as to  
5 communicate with Interoperable Train Control (ITC) PTC-equipped locomotives and cab cars  
6 operating on shared tracks. The BOS shall send safety- and operation-critical database  
7 information including temporary changes such as speed limits and established work zones  
8 when requested to by an ITC-equipped locomotive approaching the shared track territory.

#### 27.5.3.4 Automatic Train Supervision SCADA Subsystem Spare Capacity, Expansion and Obsolescence

9 There are no specific criteria. Refer to Section 27.5.1.2.

#### 27.5.3.5 Automatic Train Supervision SCADA Subsystem Command and Control

10 Under normal conditions, the OCC shall have overriding command and control. Staff at the  
11 RCCs may supervise and dispatch trains on their respective shared corridors and the OCC staff  
12 can supervise and dispatch trains in all other sections of the system.

13 Any and all control, supervision, and monitoring functions shall be possible from any  
14 workstation within the OCC or RCCs, once authorized with password protected configuration  
15 setting functions. Servers and other processors shall be distributed such that the loss of any and  
16 all equipment at 1 of the Control Centers shall not prevent either of the remaining Control  
17 Centers from supervising and controlling any part of the CHSTP.

18 Remote workstations giving access to indications and limited command sets shall be installed in  
19 station control rooms, equipment and Maintenance of Infrastructure facilities.

20 Regional control of each workstation shall be definable by the System Administrator. Definition  
21 of access to each set of functional levels shall be password protected.

22 A Local Control Panel shall also be provided in the interlocking control house for emergencies  
23 and for maintenance testing needs.

24 A Local Control Panel or panels shall also be provided in station control rooms to allow  
25 operating personnel to set and cancel routes in the event of a failure of ATS communications  
26 between the field sites and the OCC and RCCs or in any other special case by approval from the  
27 main dispatcher.

28 The ATS system shall interface with other systems including the following:

- 29 • Traction Power SCADA to enable blocking of track sections to approaching trains in the  
30 event that power is off in specific sections
- 31 • Facilities SCADA, including fans and pumps to provide remote indications on the  
32 dispatcher ATS workstations

- Tunnel facilities SCADA to provide remote indications on the dispatcher ATS workstations
- Event and incident detectors to provide remote indications on the dispatcher workstations
- Yard ATC for the transition of trains between the yards and main tracks and vice versa
- Customer Information and Public Address Systems

The ATS subsystem may be implemented as part of an integrated control system and the functions may be implemented on a common hardware platform to SCADA, security, and other control and command systems.

Refer to *Appendix 27.A SCADA Command, Monitor and Diagnose Matrix* for ATS SCADA high-level functionality and command, monitor and diagnose criteria.

#### **27.5.3.6 Automatic Train Supervision SCADA Subsystem Physical, Enclosure and Power**

Power for the ATS subsystems shall be redundant and backed up by a combination of batteries (4 hours) and diesel generator (continuous).

#### **27.5.3.7 Automatic Train Supervision SCADA Subsystem End-Devices**

There are no specific criteria. Refer to Section 27.5.1.5.

### **27.5.4 Hazard Detection/Early Warning SCADA Subsystem**

#### **27.5.4.1 Hazard Detection/Early Warning System and Equipment Introduction**

Hazard Detection/Early Warning (HDEW) sensors are distributed throughout and installed to detect conditions caused by man-made or natural events that can negatively impact the safe and timely operation of the HST. In many cases these sensors shall be designed into structures, fences, infrastructure or other physical elements and features of the CHSTP.

The HDEW sensors shall consist of a variety of formats and types, some may be electrical or fiber optic stress and strain gauges, some may be standard electro-mechanical sensors and others may be gas sensors depending upon the type of data gathered as summarized in Table 27-2.

**Table 27-2: Hazard Detection/Early Warning System Sensors and Equipment**

HDEW Sensors	Function	General Location
Earthquake and seismic activity	Detect seismic activity	Seismic sensors are located at all traction power facilities
Excessive winds	Detect wind intensity and speed	At appropriate wayside locations
Flood/Excessive precipitation	Detect high water, snow or flood	At appropriate wayside locations
Structural Hazard	Detect stressed, struck, fatigued or failing aerial track structures	At all aerial structures
Erosion (how is this accomplished?)	Erosion around aerial structures	At all aerial structures
Wildfire	Detect wildfire conditions	At appropriate wayside locations
Right-of-Way Intrusion	Detect when external trains, vehicles, animals, objects and humans encroach on the Authority's right-of-way creating an unknown or unsafe condition	At shared corridor locations and other locations identified by threat / risk analysis
Underground Air monitoring	In underground tunnels or stations (unlikely), sensors shall detect CO and other unsafe gasses	At all underground locations
Rock fall, Mudslide, Landslide	Detect rock, mud, or earth which has encroached on the Authority's right-of-way creating an unknown or unsafe condition	At appropriate wayside locations
Static Asset Monitoring (longitudinal and transversal fiber optic strain and tension sensors, accelerometers, thermocouples)	Continuous aerial health monitoring system: <ul style="list-style-type: none"> <li>Static: <ul style="list-style-type: none"> <li>Observe the maximum strain and stresses</li> <li>Verify cracking, if any</li> <li>Report strain changes, both during construction and during service</li> </ul> </li> <li>Dynamic: <ul style="list-style-type: none"> <li>Evaluate the fundamental frequencies, modes and damping ratios</li> <li>Evaluate the dynamic effects of trains crossing the bridge</li> <li>Evaluate the long-term changes in the bridge's dynamic properties.</li> </ul> </li> </ul>	All aerial sections including bridges.

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

1

#### 27.5.4.2 Description of the Hazard Detection/Early Warning SCADA Subsystem

- 2 The HDEW SCADA Subsystem shall be designed to gather the data outputs of the varied
- 3 HDEW sensors and transmit those indications to the MTUs at various control centers, local

1 locations and IIMPs. The HDEW SCADA subsystem shall provide indication to operations  
2 personnel and other relevant systems of potential impacts.

#### 27.5.4.3 Hazard Detection/Early Warning SCADA Subsystem Architecture

3 There are no specific criteria. Refer to Section 27.5.1.1.

#### 27.5.4.4 Hazard Detection/Early Warning SCADA Subsystem Spare Capacity, Expansion and Obsolescence

4 There are no specific criteria. Refer to Section 27.5.1.2.

#### 27.5.4.5 Hazard Detection/Early Warning SCADA Subsystem Command and Control

5 Refer to *Appendix 27.A SCADA Command, Monitor and Diagnose Matrix* for HDEW SCADA high-  
6 level functionality and Command, Monitor and Diagnose criteria.

#### 27.5.4.6 Hazard Detection/Early Warning SCADA Subsystem Physical, Enclosure and Power

7 HDEW SCADA end devices may be located in harsh and remote environments. SCADA end  
8 devices shall be designed to be operated by UPS power and Power over Ethernet (if  
9 appropriate), which also must be powered by a UPS source. If the HDEW SCADA end devices  
10 are sufficiently remote, the end-device may be powered by solar power and battery backup.

#### 27.5.4.7 Hazard Detection/Early Warning SCADA Subsystem End-Devices

11 There are no specific criteria. Refer to Section 27.5.1.5.

### 27.5.5 Infrastructure Mechanical Electrical Plumbing (MEP) SCADA Subsystem

#### 27.5.5.1 Infrastructure MEP Systems and Equipment Introduction

12 The Infrastructure Mechanical, Electrical, and Plumbing (INF MEP) SCADA subsystem shall  
13 monitor and control all of the non-building/non-facility MEP equipment including all MEP  
14 equipment within tunnels and trenches. There are multiple mechanical, electrical and plumbing  
15 systems supporting the HST in non-facility locations. This equipment is listed in Table 27-3.

**Table 27-3: Infrastructure MEP Equipment**

INF MEP Equipment	Function	General Location
Sump Pumps	Pump water from low lying areas	Low lying areas within tunnels
Standpipe Valves	Control water flow to standpipes	Tunnels
Fire Isolation Valves	Control water flow to fire fighting water feeds	Tunnels
Fire Suppression (not Fire Alarm)	Controls water flow to sprinkles	Tunnels and wayside structures at sites
Tunnel Ventilation Fans	Used for ventilation, fire control and safety	Tunnels
HVAC	Control environmental status within structures and possibly within tunnels via chilled water	Wayside structures and Tunnels
Electrical devices and circuit breakers	Delivers electricity and protects against faults	Tunnels and wayside structures at sites
Plumbing Meters and Valves	Control water flow to miscellaneous plumbing devices and feeds	Tunnels
Lighting Inverters and Breakers	Provides lighting to underground and enclosed areas	Tunnels and wayside structures at sites
Centralized UPS and Generators not integrated with systems equipment.	Provides uninterruptible power to systems and other sensitive equipment	Tunnels and wayside structures at sites

1

## 27.5.6 Facilities Mechanical Electrical Plumbing (FAC MEP) SCADA Subsystem

### 27.5.6.1 Facilities MEP Systems and Equipment Introduction

- 2 The Facility MEP (FAC MEP) SCADA subsystem shall monitor and control the building/facility
- 3 MEP equipment and building status. The Facilities MEP systems provide mechanical, electrical
- 4 and plumbing services for the facilities.

**Table 27-4: Facilities MEP Equipment**

<b>Facility MEP Equipment</b>	<b>Function</b>	<b>General Location</b>
Elevators	Vertical Conveyance	Stations
Escalators	Vertical Conveyance	Stations
Electrical and circuit breakers	Delivers electricity and protects against faults	Stations and Facilities
Hydrogen Detection	Detects unsafe conditions within battery rooms	Battery Rooms
Uninterruptible Power Supplies	Provides uninterrupted power to critical equipment	Stations and Facilities
High/low temperature	Senses extreme room temperatures in critical rooms	Critical communications and other equipment rooms and control rooms
HVAC	Controls climate, within rooms	Stations and Facilities
High humidity	Senses extreme room humidity in critical rooms	Critical communications and other equipment rooms and control rooms
Water detection	Senses water level in critical rooms	Critical communications and other equipment rooms and control rooms

**27.5.6.2 Description of the Facilities MEP SCADA Subsystem**

The FAC MEP SCADA Subsystem shall be designed to gather the data outputs and provide programmed control of the varied facility equipment and plants. Remote data shall be transmitted to the MTUs at various control centers, local locations and IIMPs.

The FAC MEP SCADA subsystem shall provide local and remote control for all Facility MEP plants, equipment and hardware. The FAC MEP SCADA subsystem shall be a component system of the Building Automation System as described in the *Building Automation and Management Systems* chapter.

**27.5.6.3 Facilities MEP SCADA Subsystem Architecture**

There are no specific criteria. Refer to Section 27.5.1.1.

**27.5.6.4 Facilities MEP SCADA Subsystem Spare Capacity, Expansion and Obsolescence**

There are no specific criteria. Refer to Section 27.5.1.2.

**27.5.6.5 Facilities MEP SCADA Subsystem Command and Control**

Refer to *Appendix 27.A SCADA Command, Monitor and Diagnose Matrix* for FAC SCADA high-level functionality and command, monitor and diagnose criteria.



#### 27.5.6.6 Facilities MEP SCADA Subsystem Physical, Enclosure and Power

1 There are no specific criteria. Refer to Section 27.5.1.4.

#### 27.5.6.7 Facilities MEP SCADA Subsystem End-Devices

2 There are no specific criteria. Refer to Section 27.5.1.5.

### 27.5.7 Fixed Remote Telemetry (FRT) SCADA Subsystem

#### 27.5.7.1 Fixed Remote Telemetry Sensors and Equipment Introduction

3 The Fixed Remote Telemetry sensors shall provide detection of faults and control of  
4 miscellaneous electronic equipment.

**Table 27-5: Fixed Remote Telemetry Equipment**

FRT MEP Equipment	Function	General Location
Electronic Equipment without alarms	Miscellaneous	Stations, facilities and wayside
Fare Collection Equipment	Sell fares and manage paid and unpaid borders.	Stations
Power Supplies Dedicated to Equipment (outside of the Electrical System)	Provide dc or ac power to equipment	Generally located within communications rooms, closets, cabinets and CICs
Shelter and Radio Tower Misc. equipment (lightning arrestors, tower lights, and similar)	Provide miscellaneous monitoring and safety functions	Wayside shelters and radio towers

5

#### 27.5.7.2 Description of the Fixed Remote Telemetry SCADA Subsystem

6 The Fixed Remote Telemetry SCADA Subsystem shall provide remote monitoring and control  
7 for miscellaneous equipment deployed throughout the HST system. Monitoring by this SCADA  
8 system may include distributed power supplies and rectifiers and similar equipment that have  
9 no built-in telemetry functions.

#### 27.5.7.3 Fixed Remote Telemetry SCADA Subsystem Architecture

10 There are no specific criteria. Refer to Section 27.5.1.1.

#### 27.5.7.4 Fixed Remote Telemetry SCADA Subsystem Spare Capacity, Expansion and Obsolescence

11 There are no specific criteria. Refer to Section 27.5.1.2.

#### 27.5.7.5 Fixed Remote Telemetry SCADA Subsystem Command and Control

12 Refer to *Appendix 27.A SCADA Command, Monitor and Diagnose Matrix* for FRT SCADA high-  
13 level functionality and Command, Monitor and Diagnose criteria.

**27.5.7.6 Fixed Remote Telemetry SCADA Subsystem Physical, Enclosure and Power**

- 1 There are no specific criteria. Refer to Section 27.5.1.4.

**27.5.7.7 Fixed Remote Telemetry SCADA Subsystem End-Devices**

- 2 There are no specific criteria. Refer to Section 27.5.1.5.

## **Appendix 27.A      SCADA Command, Monitor and Diagnose Matrix**

- 1    The matrix defines the high-level Operation and Maintenance Control, Monitor and Diagnose
- 2    SCADA functionality for each SCADA subsystem.
- 3    Final set of the SCADA Monitoring Points will be defined in the later design phases.
- 4    Remaining Command, Monitor and Diagnose requirements will be defined in the later design
- 5    phases.



**HRFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015**

**RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015**

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015





## Chapter 28

### Communications

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	Sept 2013	Rolling Stock Procurement
2	Feb 2014	Revision 2



## Table of Contents

<b>28</b>	<b>COMMUNICATIONS</b>	<b>28-1</b>
28.1	Scope	28-1
28.2	Regulations, Codes, Standards, and Guidelines	28-2
28.3	Related Documentation	28-5
28.4	Communications Systems	28-6
28.4.1	General Communications Systems	28-6
28.4.2	End-to-End Design Criteria – the Transport Network	28-11
28.4.3	Wide Area Network	28-16
28.4.4	Local Area Networks	28-18
28.4.5	Radio System	28-21
28.4.6	Cable Infrastructure	28-31
28.4.7	Cable Labeling	28-34
28.4.8	Integrated Information Management Platform	28-35
28.4.9	Global Positioning System Network Timing System	28-38
28.4.10	Fire Alarm System	28-40
28.4.11	Public Address and Customer Information Sign System	28-41
28.4.12	Telephone and Intercom System	28-43
28.4.13	Closed Circuit Television System	28-45
28.4.14	Intrusion Detection System	28-53
28.4.15	Electronic Access Control System	28-55
28.5	Communications Physical Support Rooms, Enclosures, Shelters and Equipment	28-56
28.5.1	Communications Rooms supporting Communications Systems and Equipment	28-57
28.5.2	Communications Interface Cabinets	28-61
28.5.3	Communications Shelters	28-61
28.5.4	Standalone Radio Sites	28-62
28.5.5	Radio Towers	28-63
28.5.6	Systems Conduits at Track	28-64
28.5.7	Conductor Routing and Segregation	28-67
		28-69
28.6	External Services	28-69
28.6.1	Telecommunications Services	28-69
28.6.2	External Notification Services	28-70
28.6.3	Infotainment	28-71

## Tables

Table 28-1:	Room Nomenclature	28-57
Table 28-2:	Communications Related Spaces	28-58
Table 28-3:	Quantities of Undertrack and Underground Conduit at At-Grade and Trench Sites and Locations	28-65

Figures

Figure 28-1: Transport Network Partitions ..... 28-12  
Figure 28-2: Layer Networks across the  $ESNI_W$  and  $NNI_{LW}$  Interfaces ..... 28-14  
Figure 28-3: Integrated Information Management Platforms and Workstations..... 28-36  
Figure 28-4: Conceptual Level Cable Routing for Core Systems ..... 28-69

## Acronyms

AC	Alternating Current
ADA	Americans with Disabilities Act
ANSI	American National Standards Institute
ATC	Automatic Train Control
BRS	Broadband Radio System
CAI	Customer Assistance Intercom
CCTV	Closed Circuit Television
CHST	Not Used; use HST for California High-Speed Train
CHSTP	California High-Speed Train Project
CI	Cable Infrastructure
CIC	Communication Interface Cabinet
CIF	Common Intermediate Format
CL-PS	Connection-Less Packet-Switched
DAS	Distributed Antenna System
DC	Direct Current
DOD	Department of Defense
EACS	Electronic Access Control System
EI	Emergency Intercom
EF	Entrance Facility
EIA/TIA	Electronic Industries Association/Telecommunications Industry Association
EMI	Electromagnetic Interference
EMS	Element Management System
ERTMS	European Rail Traffic Management System
ETCS	European Train Control System
EVACS	Emergency Voice Alarm Communication System
FAS	Fire Alarm System
GETS	Government Emergency Telecommunications Service
GNTS	Global Positioning System Network Timing System
GSM-R	Global System for Mobile Communications - Railway
GUI	Graphical User Interface
HMI	Human-Machine Interface
HST	High-Speed Train
HVAC	Heating, Ventilation, and Air Conditioning
ID	Intrusion Detection
IDS	Intrusion Detection System

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

IIMP	Integrated Information Management Platform
ISO	International Organization for Standardization
LAN	Local Area Network
LOS	Line-of-Sight
MOI	Maintenance of Infrastructure
NFPA	National Fire Protection Association
NMS	Network Management System
NS/EP	National Security and Emergency Preparedness
O&M	Operations and Maintenance
OCC	Operations Control Center
OCS	Overhead Contact System
ORS	Operations Radio System
NCS	National Communications System
PACIS	Public Address and Customer Information Sign System
PIN	Personal Identification Number
PoE	Power over Ethernet
PSTTRS	Public Safety Trench and Tunnel Radio System
QoS	Quality-of-Service
RCC	Regional Control Center
RF	Radio Frequency
RSM	Rolling Stock Maintenance
SCADA	Supervisory Control and Data Acquisition
SPT	Sound Power Telephone
TCC	Train Control and Communications
TCCR	Train Control and Communications Room
TIS	Telephone and Intercom System
TPF	Traction Power Facility
TSP	Telecommunications Service Priority
UL	Underwriters Laboratories Inc.
UPS	Uninterruptible Power Supply
VPN	Virtual Private Network
WAN	Wide Area Network
WLAN	Wireless LAN
WPS	Wireless Priority Service

## 28 Communications

### 28.1 Scope

The purpose of the Communications Design Criteria is to present the requirements, guidelines, principles, and processes that must be followed to successfully design communications systems for the California High-Speed Train (HST) system. These underlying design criteria govern the design of communications systems and shall be applied consistently across the entire system.

Communications systems are used to support the provisioning, safe operation, and cost-effective operations and maintenance of the HST system. The Designer shall design the communications systems using these criteria and guidelines to support the requirements of the project.

This chapter is organized into the following sections:

- General design criteria that applies to all communications systems
  - End-to-End design criteria that applies to the design of the Transport Network consisting of the Wide Area Network (WAN), Local Area Networks (LANs), and Radio System
  - Individual Communications System design criteria that applies uniquely to the individual communications system
- Design criteria for communications sites, rooms, spaces, and other field equipment
- Design criteria for external communications services
  - Telecommunications Services
  - Infotainment Services
  - Backup Virtual Private Network (VPN) connections

Each section (excluding sections pertaining to Communications Physical Support Rooms, Enclosures, Shelters and Equipment, and External Services) is divided into 5 subsections of design criteria:

- Architecture Design Criteria – High-level design criteria focused on system architecture, redundancy, overall system configuration, and management of the system (i.e., the administrative tasks associated with monitoring the performance of a system, fault diagnostics, configuring the system and similar)
- Spare Capacity, Expansion and Obsolescence Design Criteria – Design criteria focused on spare capacity, expansion, and obsolescence

- Command and Control Design Criteria – Design criteria focused on how the systems are controlled locally and remotely
- Physical, Enclosure and Power Design Criteria – Low-level design criteria focused on system components' physical support characteristics, including, cabinets, enclosures, heating, ventilation and air conditioning (HVAC), power, uninterruptible power supply, and similar
- End-Device Design Criteria – Low-level design criteria focused on end-device attributes, placement, and characteristics

The following is a list of the communications systems that are the topic of these criteria:

- Cable Infrastructure (CI)
- Wide Area Network (WAN)
- Local Area Network (LAN)
- Integrated Information Management Platform (IIMP)
- Global Positioning System (GPS) Network Timing System (GNTS)
- Radio System
- Fire Alarm System (FAS)
- Telephone and Intercom System (TIS)
- Closed Circuit Television (CCTV) System
- Public Address and Customer Information Sign System (PACIS)
- Intrusion Detection System (IDS)
- Electronic Access Control System (EACS)

Detailed functional and performance requirements of the communications systems are to be provided in separate communications system performance specifications.

## **28.2 Regulations, Codes, Standards, and Guidelines**

Refer to the *General* chapter for requirements pertaining to regulations, codes, standards, and guidelines.

- Code of Federal Regulations (CFR)
  - Title 47 CFR, Part 90: Private Land Mobile Radio
  - Title 47 CFR, Part 15: Class A
  - Title 49 CFR, Ch. II 220.0: Railroad Communications
  - Title 49 CFR, Part 236 Appendix E: Human-Machine Interface (HMI) Design



- 1 • Association of American Railroads (AAR) Standards Manual of Standards and  
2 Recommended Practices, Section K
- 3 • Americans with Disabilities Act (ADA)
  - 4 – 28 CFR Part 35: Title II Technical Assistance Manual
  - 5 – 28 CFR Part 36: Title III Standards of Accessible Design
  - 6 – 49 CFR Part 37: Transport Services for Individuals with Disabilities
- 7 • National Fire Protection Association (NFPA) Codes and Standards
  - 8 – NFPA 70: National Electrical Code
  - 9 – NFPA 72: National Fire Alarm Code
  - 10 – NFPA 130: Standard for Fixed Guideway Transit and Passenger Rail Systems
- 11 • American National Standards Institute (ANSI)
  - 12 – ANSI –J-STD-607-A: Commercial Building Grounding (Earthing) and Bonding  
13 Requirements for Telecommunications
  - 14 – ANSI/ICEA S-87-640: Standard for Outside Plant Communications Cable
  - 15 – ANSI/ICEA S-80-576: Communications Wire and Cable for Wiring of Premises
  - 16 – ANSI/ICEA S-83-596: Standard for Fiber Optic Premises Distribution Cable
  - 17 – ANSI/HFES 100: Human Factors Engineering Requirements for Visual Display  
18 Terminal (VDT) Work Stations
  - 19 – ANSI/IEEE C37.90.2: Standard for Tolerance of Radiated Electromagnetic  
20 Frequency Interference (RFI)
  - 21 – ANSI/BICSI 002: Data Center Design and Implementation Best Practices
- 22 • Institute of Electrical and Electronics Engineers (IEEE) Standards
  - 23 – IEEE 802.1: Bridging and Management
  - 24 – IEEE 802.3: Ethernet
  - 25 – IEEE 802.11: Wireless LANs
  - 26 – IEEE 828: Standard for Software Configuration Management Plans
- 27 • International Organization for Standardization (ISO) Standards
  - 28 – ISO/IEC 12207: Systems and software engineering -- Software life cycle processes
- 29 • Electronic Industries Association/Telecommunications Industry Association  
30 (EIA/TIA) Standards
  - 31 – TIA 603: Land Mobile FM or AM Communications Equipment Measurement and  
32 Performance Standards

- 1       – EIA-316: Minimum Standards for Portable/Personal Land Mobile
- 2       Communications FM or PM Equipment
- 3       – EIA-310: Cabinets, Racks, Panels, and Associated Equipment
- 4       – EIA 472: Generic Specifications for Fiber-optic cable
- 5       – EIA/TIA-598: Optical Fiber Cable Color Coding
- 6       – TIA-475.0000: Generic Specifications for Fiber Optic Connectors
- 7       – TIA/EIA-455: Standard Test Procedures for Optical Fibers, Cables, Transducers,
- 8       Sensors, Connecting and Terminating Devices, and other Fiber Optic
- 9       Components.
- 10      – TIA-102: Project 25 Digital Wireless Radio Communications
- 11      – TIA/EIA 568: Commercial Building Telecommunications Cabling Standards
- 12      – TIA/EIA 569: Commercial Building Standard for Telecommunications Pathways
- 13      and Spaces
- 14      – ANSI/TIA/EIA – 606-A-A, Administration Standard for Telecommunications
- 15      Infrastructure of Commercial Buildings.
- 16      • Underwriters Laboratories Inc. (UL) Publications
- 17      – UL 6: Rigid Metal Conduit
- 18      – UL 44: Thermoset-Insulated Wires and Cables
- 19      – UL 50: Standard for Enclosures for Electrical Equipment
- 20      – UL 62: Flexible Cords and Cables
- 21      – UL 94: Standard for Tests for Flammability of Plastic Materials for Parts in
- 22      Devices and Appliances
- 23      – UL 467: Grounding and Bonding Equipment
- 24      – UL 486 A-E: Wire Connectors
- 25      – UL 510: Standard for Polyvinyl Chloride, Polyethylene and Rubber Insulating
- 26      Tape
- 27      – UL 514A: Metallic Outlet Boxes
- 28      – UL 514B: Conduit, Tubing, and Cable Fittings
- 29      – UL 886: Outlet Boxes and Fittings for Use in Hazardous (Classified) Locations
- 30      – UL 1581: Reference Standard for Electrical Wires, Cables and Flexible Cords
- 31      – UL 1778: Uninterruptible Power Supply Equipment
- 32      • Telcordia (Bellcore)

- Network Equipment-Building System (NEBS) Level 3 requirements
- GR-20: Generic Requirement for Optical Fiber and Optical Cables
- GR-63: NEBS Requirements: Physical Protection
- GR-771: Generic Requirements for Fiber Optic Splice Closures
- U.S. Department of Defense (USDOD) Standards
  - MIL-STD-1472: Human Engineering
  - MIL-STD-781: Reliability, Test Methods, Plans, and Environments for Engineering, Development, Qualification and Production
  - MIL-STD-810: Department of Defense Test Method Standard for Environmental Engineering Considerations and Laboratory Tests
- National Transportation Communications for ITS Protocol (NTCIP) Standards
- International Union of Railways (UIC) Code / Union Industry of Signaling (UNISIG) Specifications
- European Integrated Radio Enhanced Network (EIRENE) Standards
- MOBILE radio for RAILway Networks in Europe (MORANE) Standards
- International Telecommunication Union (ITU) Telecommunication Standardization Sector (ITU-T) Standards

### **28.3 Related Documentation**

Related systems disciplines documents interfacing and/or related with the communications system shall be consulted to ensure compliance with required design criteria.

- Design Criteria Chapters
  - General
  - Trackway Clearances
  - Utilities
  - Structures
  - Stations
  - Support Facilities
  - Facility Power and Lighting Systems
  - Building Automation and Management Systems
  - Traction Power Supply System

- 1       – Overhead Contact System and Traction Power Return System
- 2       – Grounding and Bonding Requirements
- 3       – Automatic Train Control
- 4       – Electromagnetic Compatibility and Interference
- 5       – Supervisory Control and Data Acquisition Subsystems
- 6       – Rolling Stock-Core System Interfaces
- 7       – Reliability, Availability, Maintainability and Safety

## **28.4       Communications Systems**

8       The communications systems listed in Section 28.1 enable the Authority to provide a  
9       passenger-friendly, secure, safe, and efficient means of transportation to its ridership.  
10      The communications systems consist of field devices, centralized equipment, systems  
11      interface, integration and control software, and wired and wireless networks to provide  
12      connectivity. The proper design and operation of the communications systems is crucial  
13      to passenger and personnel safety and operations and maintenance.

### **28.4.1     General Communications Systems**

14      The communications systems are individual systems that function independently and in  
15      concert to deliver communications services and functionality to the CHSTP. The  
16      following design criteria and sections apply to all communications systems unless more  
17      specific or stringent criteria are listed in the individual sections.

#### **28.4.1.1   General Communications Systems Architecture**

18      The adherence to open system architecture concepts and non-proprietary standards is  
19      required, whenever it is available, to ensure interoperability of products from 2 or more  
20      manufacturers.

21      The CHSTP shall be implemented with an overarching Integration and Information  
22      Management Platform (IIMP) to provide a common front-end user interface that allows  
23      monitoring and control of various subsystems. This IIMP shall integrate the different  
24      types of subsystems (e.g., CCTV, ACS, TIS, SCADA) as well as systems of the same type  
25      but implemented using different manufacturer platforms (e.g., different manufacturer  
26      video management system software). This level of flexibility is especially required given  
27      the large scope and scale of the CHSTP and the phasing of implementation over many  
28      years. Refer to Section 28.4.8 for design criteria specific to the IIMP.

29      Standard commercial-off-the-shelf devices shall be used when such equipment and  
30      materials meet the functional and performance specifications. Special designs and

1 custom assemblies shall be confined to those devices that require such features to assure  
2 compliance with the specifications.

3 Each communications system shall have self-diagnostics built into the system whereby  
4 end-devices, inputs and components can be remotely interrogated to determine their  
5 health and operational status and allow fault diagnosis and repair in response to  
6 failures.

7 Electrical-mechanical (dry-contact), direct, or similar triggering interface between stand-  
8 alone communications systems (e.g., TIS and CCTV) shall be avoided when permitted  
9 by code. Interfaces and integration between communications systems and other systems  
10 shall be accomplished via the Integration Logic within the IIMP.

#### **28.4.1.2 General Communications Systems Spare Capacity, Expansion, and Obsolescence**

11 Communication system design shall minimize the effects of early component or  
12 subsystem obsolescence caused by advances in technology.

13 The communications systems shall be designed to facilitate future integration of  
14 additional Authority's facilities and right-of-way without interruption of the already  
15 operating system when these additions are implemented.

16 Communication systems, equipment and cabling shall be designed for an operational  
17 lifespan as declared in the *General* chapter.

18 The communications systems shall be designed to accommodate a full-scale deployment  
19 along the Authority's entire proposed right-of-way. Nothing in the design shall preclude  
20 the Authority from expanding the systems via subsequent, open procurements. The  
21 communications systems shall be designed to have a scalable, flexible, open, and non-  
22 proprietary architecture.

23 Over and above the above requirement for accommodating a full-scale deployment  
24 along the Authority's entire proposed right-of-way, the communications systems,  
25 equipment, infrastructure, support systems, rooms, and enclosures shall be designed  
26 and installed to have spare capacity as described in each section.

#### **28.4.1.3 General Communications Systems Command and Control**

27 In general, all users will utilize the IIMP as the front-end application that provides the  
28 ability to monitor and control the communications systems. There may be cases where  
29 certain system administration tasks (e.g., configuring a device) may require the user to  
30 utilize the native system graphical user interface (GUI) to perform the task; however,  
31 this should be avoided if possible.

32 In general, each communications system shall be designed to have multiple levels of  
33 command and control by users located at the Operations Control Center (OCC),

1 Regional Control Centers (RCC), stations, facilities, and yards. Each communications  
2 system shall be designed to have an application programming interface allowing the  
3 appropriate IIMP (refer to Section 28.4.8) to interface with the system to provide  
4 integration and interface functionality.

#### **28.4.1.4 General Communications Systems Physical, Enclosure and Power**

5 Equipment serving similar functions shall be in the same relative location in equipment  
6 cabinets wherever practicable.

7 Equipment cabinets within communication rooms shall be located so that rack-mounted  
8 equipment can be accessed via the front and the back of the equipment cabinet.  
9 Equipment mounted within wall-mounted enclosures or cabinets shall be mounted such  
10 that front and rear access to the equipment is possible via swing-out equipment rack  
11 mounting.

12 Each equipment cabinet and equipment enclosure shall have a main breaker that can  
13 shut off power to equipment in the cabinet or enclosure.

14 Equipment that is part of different systems and is to be maintained by maintainers with  
15 different skill sets should not be located within the same equipment cabinet or  
16 equipment enclosure.

17 Equipment enclosures that are not located within environmentally and particulate-  
18 controlled communications equipment rooms shall be NEMA-4 rated. These equipment  
19 cabinets and equipment enclosures shall be sealed and lockable and be designed to meet  
20 the environmental requirements of the housed equipment.

21 Equipment cabinets and enclosures shall be as specified in the *Electromagnetic*  
22 *Compatibility and Interference* chapter.

23 Communications system equipment in tunnel areas shall be located primarily in cross  
24 passages.

25 Equipment cabinet and enclosure doors shall be equipped with lockable handles. Locks  
26 shall be keyed alike. Doors shall be removable when unlocked. Electronic lock cylinders  
27 and electronic keys that allow access control to cabinets and enclosures shall be designed  
28 and included.

29 Normal diagnostics shall not require major disassembly of the equipment or of any  
30 other equipment.

31 Communications equipment enclosures, housings, conduit pathways and spaces shall  
32 not violate the clearances as specified in the *Trackway Clearances* chapter and shall be  
33 coordinated with the spatial and loading requirements in the *Tunnels* and *Structures*  
34 chapters.

Weight of communications equipment enclosures, housings, conduit pathways and cabling mounted on structures shall not exceed the load limits specified in the *Structures* chapter.

Refer to the *Seismic* and *Structures* chapters for additional slipstream and seismic requirements for Communications equipment mounting.

**A. Power and Uninterruptible Power Supply**

The communications systems shall be designed to operate on -48 VDC systems wherever possible and use 120 V ac, 1 phase, 60 Hz power sources only when necessary. DC power supplies shall be provided when needed and critical networking, fire, life, safety and radio equipment power supplies shall be designed to be N+1 redundant.

Refer to the *Traction Power Supply System* and *Facility Power and Lighting Systems* chapters for normal, backup and emergency power supply and uninterruptible power supply (UPS) details at buildings, facilities, stations and control centers.

At remote locations including wayside communications interface cabinets, communications shelters at traction power facilities, automatic train control facilities, standalone radio sites, the Designer shall provide uninterruptible power supplies to power communications systems equipment and support equipment for a minimum of 8 hours, after normal and/or backup power is lost.

Communications equipment shall be capable of automatic start-up following a power outage of any length without requiring manual re-initialization.

Communications equipment shall retain full status memory and process recall when operating on power from UPS, generator, battery, or inverter sources.

**28.4.1.5 General Communications Systems End-Devices**

Exposed cables, connectors, sockets, end-devices, and antennas that are supplied under the contract shall be tamper and vandal proof and aesthetically compatible with the surrounding environment. The installation of such equipment within facility and station buildings shall meet the aesthetic requirements of the respective architects.

Equipment shall, wherever practical, be designed to contain a self-diagnostic capability to permit the remote interrogation of the health and operational status of the unit and facilitate speedy repair after failure.

Onboard antennas shall be roof mounted and shall be positioned to not interfere with other antennas or exceed the current dynamic envelope of the vehicle, fixed equipment envelope and static gauge. The vehicles will operate under a wide variety of electrical operating environments, including under Overhead Contact System (OCS) equipment. Lightning arresters shall be provided for any roof-mounted equipment, including

1 antennas, to suitably protect against lightning strikes and accidental contact with the  
2 OCS. Any device near an OCS shall meet the clearance requirements as specified in the  
3 *Overhead Contact System and Traction Power Return System* chapter. Individual radio  
4 system onboard antenna design shall be done in coordination with the other onboard  
5 antenna design. A combined antenna package may be suitable if functional and  
6 performance requirements are met.

7 Onboard antenna and radio access points placed near the OCS shall be designed to  
8 mitigate, as much as practicable, the effects of near-field OCS interference with the radio  
9 systems.

10 Communications end device placement shall not violate the clearances as specified in  
11 the *Trackwork Clearances* chapter and shall be spatially coordinated with the  
12 requirements in the *Tunnels* and *Structures* chapters.

13 Loads of communications end devices mounted on structures shall not exceed the load  
14 limits specified in the *Structures* chapter.

15 Air gap required by radio or visual communications systems or end devices shall be  
16 coordinated with the fixed equipment envelope and the static and dynamic envelopes as  
17 specified in the *Trackwork Clearances* chapter. Communications end device locations shall  
18 be coordinated with structural and architectural elements. Refer to *Stations, Support*  
19 *Facilities* and *Structures* chapters. Airgap requirements shall be specified so as to fully  
20 support the functional and performance requirements of the overall communications  
21 system.

22 Mounting of end devices and conduits on concrete shall not interfere with rebar.

#### **28.4.1.6 Communications Systems and Supervisory Control and Data Acquisition Subsystems**

23 Refer to the *Supervisory Control and Data Acquisition Subsystems* chapter for design criteria  
24 for SCADA subsystems.

25 The communications systems that interface to the SCADA subsystems are limited to the  
26 transmission and display of SCADA subsystems status and control messages via any  
27 standard, approved data interfaces as required via the WAN or LAN. While it will be  
28 the responsibility of the WAN and LAN to meet the connectivity, security, latency,  
29 reliability metrics demanded by the various disciplines requiring this service, the WAN  
30 and LAN are a data delivery service only.

31 SCADA devices attached to the WAN and/or LAN networks may include wayside and  
32 station seismic, wind speed, and other hazard-monitoring devices, facilities control and  
33 alarm reporting, traction power facilities, SCADA, and other sensors, alarms and devices  
34 needing remote monitoring and control. The WAN and LANs are responsible for  
35 delivering SCADA data messages to the Master Terminal Units (MTUs).



## 28.4.2 End-to-End Design Criteria – the Transport Network

The foundation of the communications systems is the Transport Network, which consists of interconnected wired and wireless networks that securely and reliably move data between devices, applications, and equipment. The performance and availability of the Transport Network has direct correlation to the performance and availability of other systems that rely on the Transport Network to move data. For example, the performance (i.e., the response times, quality of video, up-time) of a CCTV system as seen by an end-user depends not only on the performance of the CCTV equipment (e.g., cameras, servers, video displays) but also on the performance of the Transport Network that transports the video streams between CCTV equipment.

The systems that make up the Transport Network are the WAN, LAN, and radio system. This chapter specifically discusses these 3 systems as 1 in order to capture the importance and design criteria of interface between the separate communications systems. The term “node” is used here to signify a single or group of interconnected equipment that together provide the required network functions at a particular location.

The WAN shall provide data connectivity to the entire HST system via core and edge rings. The WAN core nodes should exist at passenger stations, Rolling Stock Maintenance (RSM) and Maintenance of Infrastructure (MOI) facilities, and the RCC and OCC. The WAN edge nodes should exist at traction power facilities, Automatic Train Control (ATC) equipment houses, stand-alone radio sites, wayside communication interface cabinets, and tunnel ventilation facilities at tunnel portal sites and tunnel ventilation facilities at tunnel ventilation shafts.

The LANs shall provide short-range networking (e.g., in buildings or between nearby buildings) between local devices and equipment. The LANs shall exist at passenger stations, control centers, and facilities. LANs in a smaller configuration exist co-located with the WAN edge nodes to provide local end-device connectivity. Local end-devices shall connect to the Transport Network via LANs. The LAN shall support redundant, physically distributed connections to the WAN, to provide the backhaul connectivity described above. The LANs shall provide network connectivity between local devices/equipment and aggregate traffic from local devices that are destined for remote locations prior to delivering the traffic to the WAN, thus reducing the number of interfaces required on WAN nodes.

The WAN nodes (core and edge) shall be designed to allow transit traffic (i.e., traffic passing through the WAN node but not destined to any local devices) to be offloaded from modules or equipment responsible for routing packets. The WAN nodes shall also be designed to support traffic engineering to allow control of network paths.

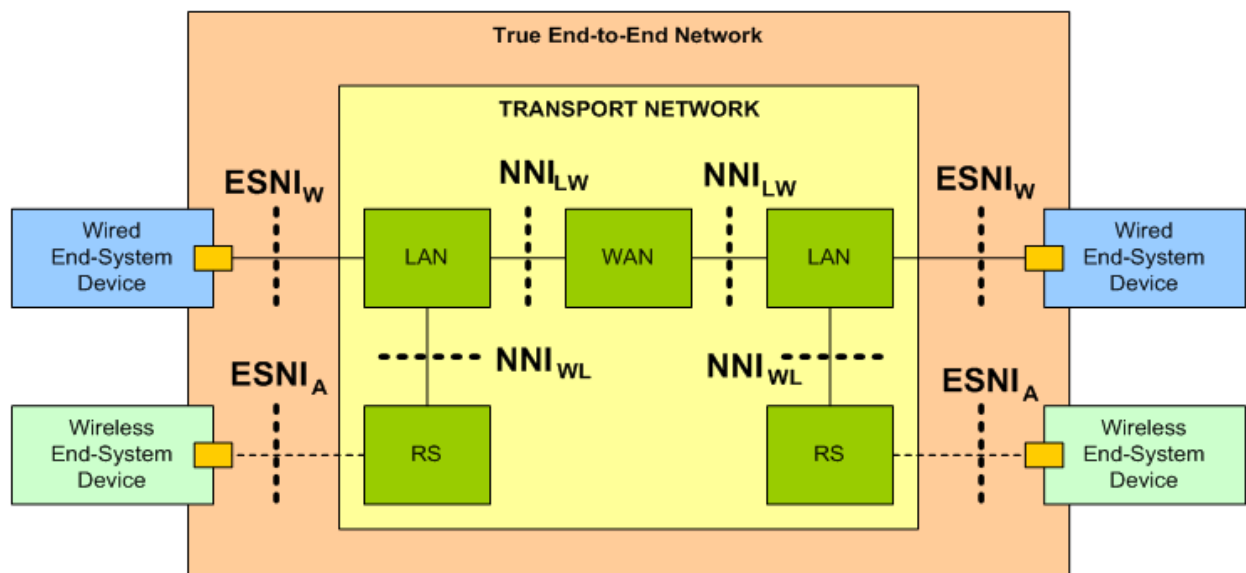
Two radio subsystems are part of the Transport Network. These separate radio subsystems provide wireless connectivity to mobile systems, equipment, and Authority users. These radio systems are the following:

- **Operations Radio System (ORS)** – voice and low-bandwidth data radio system for wireless communications between onboard equipment, mobile and portable users and fixed users and systems (if an integrated vital and non-vital radio system is used, the ORS may carry safety-critical train control data, refer to the *Automatic Train Control* chapter)
- **Broadband Radio System (BRS)** – high-bandwidth radio system for wireless communications between onboard high-bandwidth systems and fixed systems while trains are in motion, including at 250 miles per hour (mph).

#### 28.4.2.1 Transport Network Architecture

The Transport Network, as depicted on Figure 28-1, is a partition of the actual “end-to-end” network. The end-to-end network consists of the network level functions present in the end-system devices as well as the networking equipment. The Transport Network is the partition containing the networking equipment. The Transport Network partition itself consists of multiple partitions as depicted on Figure 28-1: the WAN, LAN, and radio system. The Transport Network shall provide network services to wired and wireless end-system devices. Wired end-system devices shall have a physical network connection to a LAN. Wireless end-system devices shall have a physical network connection over-the-air to the radio system. The radio system shall be connected to the WAN or LAN via a wired connection, and the LANs in different facilities shall be interconnected via the WAN.

**Figure 28-1: Transport Network Partitions**



Adaptation of application data to networking protocols and termination of networking protocols

The network interfaces between each of the network partitions are depicted on Figure 28-1 and listed below:

- End-System to Network Interface (Wired) – ESNI<sub>W</sub>
- End-System to Network Interface (Air) – ESNI<sub>A</sub>
- Network to Network Interface (radio system to LAN) – NNI<sub>WL</sub>
- Network to Network Interface (LAN to WAN) – NNI<sub>LW</sub>

The end-to-end network shall provide the transfer of data between the application processes running in the end-systems. The end-to-end network includes the network level functions present in the end-system devices as well as the networking equipment.

The end-system devices shall implement protocols that perform the functions throughout all 7 layers of the Open Systems Interconnection model (i.e., perform the application level processes associated with the top 2 layers, end-to-end logical connectivity functions associated with the Session and Transport layers, as well as the networking functions associated with the lower 3 layers).

The network equipment used to interconnect the end-system devices shall perform various functions associated with the lower 3 layers. End-system devices, which could be specified by non-communications disciplines and possibly procured under different contracts, also contain “network functions.” Therefore, the designers shall consider these end-devices as part of the end-to-end network and conduct design such that they are interoperable with the network equipment to ensure end-to-end delivery of the application data.

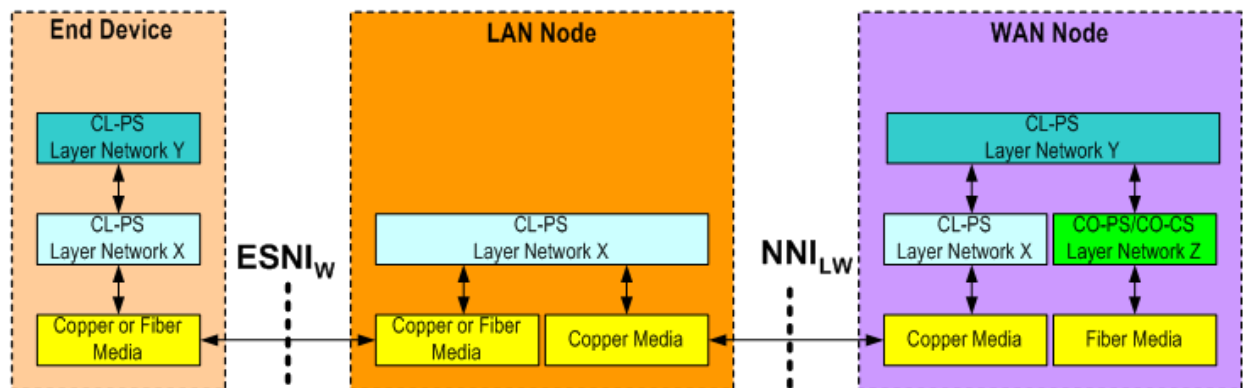
Network design shall minimize the different types of networking protocols and technologies while meeting the requirements for data transport across the network for applications. The Designer shall therefore design for a common network interface (or small set of network interfaces) to be defined and used by systems to connect to the networks. The Designer shall reduce the different types of equipment and technologies supported by the network to allow the Authority to maintain less spare equipment in storage, reduce the number of technologies and equipment types that maintenance personnel are required to be trained in, and, in general, permit a simpler network design.

It shall be the responsibility of the Designer to determine the best network protocol stacks to implement each of the above interfaces. Provided below are design guidelines and criteria that the Designer shall utilize when conducting network design and selecting the network protocols. The Designer shall note that different partitions of the Transport Network may be designed and installed by different designers and coordination will be required to ensure that the above interfaces are fully defined (i.e.,

protocol stack specified) to ensure interoperability and end-to-end functionality (based on the procurement plan).

Figure 28-2 illustrates the layer networks supported across the  $ESNI_W$  and the  $NNI_{LW}$  interfaces. Since protocols are not specified herein, the layer networks are referred to as Layer Network X, Layer Network Y, and Layer Network Z. The Designer shall note that Layer Network Z does not cross those interfaces that have been defined between different network partitions and remains solely within the WAN partition (i.e., is used solely between WAN nodes).

**Figure 28-2: Layer Networks across the  $ESNI_W$  and  $NNI_{LW}$  Interfaces**



Any layer network may be categorized as 1 of 3 distinct classes of networks;

- Connection-Less Packet-Switched (CL-PS)
- Connection-Oriented Packet-Switched (CO-PS)
- Connection-Oriented Circuit-Switched (CO-CS) networks

Layer Network Y, which is the end-to-end layer network beginning and ending on end-system devices, shall be a CL-PS layer network. A CL-PS network allows for any-to-any communications without having to configure a full-mesh of connections in the network and therefore is a more flexible solution for end-to-end communications. Layer Network Y shall support a scalable address format and routing mechanism suitable for WANs with hundreds of nodes and tens of thousands of end-points.

The address format supported by Layer Network Y shall support hierarchical addressing that signifies a location within a network topology. Such an address format will allow for address summarization, which allows the network to scale to support many end-points.

1 The routing mechanism supported by Layer Network Y shall support optimal routing  
2 across the network and not require packets to be broadcast out to all links before a route  
3 is learned.

4 Layer Network X, which is the layer network responsible for transporting data locally  
5 within a station or facility, shall be a CL-PS layer network as well. However, Layer  
6 Network X, being a local network, does not need to support the same level of scalability  
7 as described for Layer Network Y, and therefore a simpler, less expensive networking  
8 technology may be used to support Layer Network X (e.g., Ethernet bridging/switching).

9 Layer Network Z is a layer network that resides totally within the WAN partition.  
10 Unlike Layer Networks X and Y, Layer Network Z does not cross the ESNI<sub>W</sub> or NNI<sub>LW</sub>  
11 interfaces. Layer Network Z shall be either a CO-PS or CO-CS layer network. The  
12 functions provided by Layer Network Z shall be:

- 13 • Traffic Engineering
- 14 • Off-load of pass-through traffic at a WAN node from the Layer Y processor
- 15 • Deterministic network recovery mechanism to recover against failures to WAN links  
16 or equipment
- 17 • Disassociate the physical fiber topology from the logical Layer Y topology (e.g.,  
18 logical connections in Layer Z can be configured to provide Layer Y a logical mesh,  
19 while the underlying physical fiber topology may be a ring)

A. Types of Data Traveling on the Transport Network

20 The primary function of the Transport Network is to move data between the application  
21 processes running in the end-system devices. The data being transmitted and/or  
22 received by the application process may be in the form of messages, files, or real-time  
23 streams.

24 The Designer shall design a network that meets the performance requirements for the  
25 transport of messages, files, and real-time streams (both non-interactive and interactive)  
26 end-to-end across the interfaces and partitions defined on Figure 28-2.

B. Transport Network End-to-End Performance Objectives

27 Different partitions of the network will likely be designed and built by different  
28 designers, portions of the end-to-end performance objectives shall be allocated amongst  
29 the various partitions to ensure compliance of partitions as well as end-to-end  
30 compliance.

**28.4.2.2 Transport Network Spare Capacity, Expansion and Obsolescence**

31 There are no specific criteria. Refer to Section 28.4.1.2 and the individual chapters for  
32 WAN, LAN, and radio system.

### **28.4.2.3 Transport Network Command and Control**

1 There are no specific criteria. Refer to Section 28.4.1.3 and the individual chapters for  
2 WAN, LAN, and radio system.

### **28.4.2.4 Transport Network Physical, Enclosure and Power**

3 There are no specific criteria. Refer to Section 28.4.1.4 and the individual chapters for  
4 WAN, LAN, and radio system.

### **28.4.2.5 Transport Network End-Devices**

5 There are no specific criteria. Refer to Section 28.4.1.5 and the individual chapters for  
6 WAN, LAN, and radio system.

## **28.4.3 Wide Area Network**

7 The WAN consists of the hardware and software required to switch, manage, process,  
8 control, monitor, and deliver data traffic between field locations and central control  
9 facilities via the fiber-optic CI. The WAN shall deliver system data via the CI between  
10 any points on the network in a secure and reliable fashion. Data traffic includes voice,  
11 video, as well as any control, alarm, and status messages plus Network Management  
12 System (NMS) data used to support operations. Data traffic includes communications  
13 systems traffic and other disciplines' data traffic.

14 It shall be the responsibility of the WAN to meet the connectivity, security, latency, and  
15 reliability metrics requested by the various disciplines requiring data transfer service.

16 The WAN includes but is not necessarily limited to the following equipment/modules  
17 and subsystems:

- 18 • Optical network node equipment/modules
- 19 • Network switching equipment/modules
- 20 • Management System

### **28.4.3.1 Wide Area Network Architecture**

21 The WAN shall be designed to utilize fiber-optic links to connect the WAN nodes such  
22 that each WAN node has geographically diverse links connecting it to at least 2 other  
23 non-co-located WAN nodes so that no single point failure of the WAN equipment or CI  
24 can reduce the capability of the WAN to provide service with adequate quality of service  
25 (QoS). The WAN nodes shall utilize standard, non-proprietary protocols to transport  
26 data, automatically detect network faults, and provide automatic network recovery.

27 The network connectivity provided by the WAN shall be designed to be flexible to allow  
28 communications between any 2 WAN nodes and therefore provide a means to allow any  
29 control facility to take over system-wide operations in the event of a failure of the  
30 other(s).

1 WAN equipment shall be designed to support a configuration with multiple control  
2 modules. To support high availability, 2 control modules (built-in or modular) shall be  
3 furnished and installed for network equipment. These 2 control modules shall be  
4 provisioned to operate in a hot-standby capacity, such that if 1 control module fails the  
5 other shall take over without any loss of service. For modular control modules, the  
6 equipment shall operate such that if 1 control module is unplugged (e.g., for  
7 replacement) and replaced with another control module, the entire process shall take  
8 place with no loss of service.

9 WAN equipment shall be capable of supporting a configuration with multiple switching  
10 fabrics (i.e., multiple redundant forwarding backplanes).

11 The WAN shall be designed such that traffic is capable of being routed around any  
12 single equipment failure or cable failure.

13 The WAN shall be necessarily designed in segments across multiple contracts.  
14 Subsequent additions to the WAN shall be designed to be compatible and interoperable  
15 with the previous WAN segments.

16 Due to the wide area of coverage provided by the WAN, it is critical that the WAN  
17 equipment along with the WAN Element and Network Management Systems (EMS and  
18 NMS) provide the fault, configuration, administration, performance, and security  
19 functions as specified in ITU-T Recommendation M.3400, using standard protocols.

20 The Designer shall design a WAN NMS that will be used as a maintenance engineering  
21 tool for configuring, controlling, and monitoring the WAN. WAN NMS shall be used  
22 primarily for system configuration, detailed equipment diagnostic and fault-finding  
23 purposes, and shall include necessary equipment such as hardware and software.

24 The NMS and WAN EMS shall include health-status monitoring of the systems to a level  
25 of detail that typically includes fiber-optic transmit/receive multiplexers, repeaters,  
26 channel units, control modules, switching fabrics, and local and remote power supplies.  
27 The WAN NMS shall include equipment necessary for communications system  
28 monitoring and control.

#### **28.4.3.2 Wide Area Network Spare Capacity, Expansion, and Obsolescence**

29 The initial segments of the WAN shall be designed to support the traffic capacity of the  
30 eventual proposed, full build out of the CHSTP system, plus enough spare capacity to be  
31 able to reroute traffic around a single failure with no measurable degradation in QoS to  
32 be defined in the performance specifications, plus, over and above this capacity,  
33 additional spare capacity to support a 50 percent traffic growth. As new segments come  
34 on-line there shall not be any need to increase the capacity of links or equipment  
35 installed in previous WAN segments.

### **28.4.3.3 Wide Area Network Command and Control**

- 1 No specific end-device design criteria. Refer to Section 28.4.1.3.

### **28.4.3.4 Wide Area Network Physical, Enclosure, and Power**

- 2 WAN equipment shall support a configuration with multiple power supply modules. To  
3 support high availability, 2 power supplies (built-in or modular) shall be furnished and  
4 installed for network equipment. These 2 power supply modules shall be provisioned to  
5 operate in a hot-standby hot-swappable capacity, such that if 1 power supply fails, the  
6 other shall take over without any loss of service. For modular power supplies, the  
7 equipment shall operate such that if 1 power supply is unplugged (e.g., for replacement)  
8 and replaced with another power supply, the entire process shall take place with no loss  
9 of service.

### **28.4.3.5 Wide Area Network End-Devices**

- 10 There is no specific end-device design criteria. Refer to Section 28.4.1.5.

## **28.4.4 Local Area Networks**

- 11 The LAN is the communications network within a passenger station area, facility,  
12 traction power facility, stand-alone radio site, and ATC equipment house to which  
13 devices and systems interface to provide connectivity to the local control point, central  
14 head-end equipment, and the WAN.

### **28.4.4.1 Local Area Network Architecture**

- 15 Critical workstations and servers shall have 2 connections to the LAN and each  
16 connection shall be terminated on different LAN equipment.
- 17 The LAN shall support at least 2 connections to the WAN. Upon failure of 1 connection  
18 to the WAN, the LAN and the WAN shall be capable of automatically rerouting traffic  
19 onto the other connection using standard protocols. The Designer shall ensure that the 2  
20 connections are continually used with load balancing rather than only 1 connection  
21 actively carrying traffic at all times.
- 22 The LAN shall be designed such that traffic is capable of being routed around any single  
23 equipment failure or cable failure.
- 24 LAN equipment shall support a configuration with multiple control modules. To  
25 support high availability, 2 control modules (built-in or modular) shall be furnished and  
26 installed. These 2 control modules shall be provisioned to operate in a hot-standby  
27 capacity, such that if 1 control module fails the other shall take over without any loss of  
28 service. For modular control modules, the equipment shall operate such that if 1 control  
29 module is unplugged (e.g., for replacement) and replaced with another control module,  
30 the entire process shall take place with no loss of service.



1 The LAN EMS shall include health status monitoring of the systems, to a level of detail  
2 that typically includes switches, routers, power supplies and similar. The LAN EMS  
3 shall include equipment necessary for LAN monitoring and control.

#### **28.4.4.2 Local Area Network Spare Capacity, Expansion and Obsolescence**

4 The LANs shall be designed with 25 percent spare capacity, processing and connectivity  
5 to accommodate future increased usage, end-devices, and expansion.

6 LANs shall be designed with spare capacity to support the full build out of the system,  
7 and over and above this capacity, an additional 25 percent spare capacity.

#### **28.4.4.3 Local Area Network Command and Control**

8 There is no specific end-device design criteria. Refer to Section 28.4.1.3.

#### **28.4.4.4 Local Area Network Physical, Enclosure and Power**

9 LAN equipment shall support a configuration with multiple power-supply modules. To  
10 support high availability, 2 power supplies (built-in or modular) shall be furnished and  
11 installed. These 2 power supply modules shall be provisioned to operate in a hot-  
12 standby hot-swappable capacity, such that if 1 power supply fails the other shall take  
13 over without any loss of service. For modular power supplies, the equipment shall  
14 operate such that if 1 power supply is unplugged (e.g., for replacement) and replaced  
15 with another power supply, the entire process shall take place with no loss of service.

#### **28.4.4.5 Local Area Network End-Devices**

16 No specific end-device design criteria. Refer to Section 28.4.1.5.

#### **28.4.4.6 Wireless LAN**

17 The Wireless LAN (WLAN) is a wireless physical layer extension to the LAN. The  
18 WLAN shall be installed at indoor manned office locations at stations, facilities and  
19 control centers to provide secure WLAN access to Authority personnel via portable  
20 electronics (e.g., tablets and laptops). The WLAN shall also be deployed outdoors at the  
21 Yards and Terminal Stations for purposes of transmitting stored video, train health, and  
22 other stock data to maintenance facilities during out-of-service operations. This high-  
23 bandwidth WLAN shall be used to transmit large video, audio and telemetry data files  
24 while trains are stationary.

##### **A. Wireless LAN Architecture**

25 WLAN shall enable secure wireless access to the local LAN via wireless access points.

26 Coverage shall be provided for Authority personnel computing devices at the following:

- 27 • Back office areas, paid and unpaid areas, and platforms at stations
- 28 • Back office areas at facilities and control centers

Coverage shall be provided for offload of onboard data where trains are stored and maintained at the following:

- Terminal Stations
- RSM Yards
- MOI Yards

The data rate and capacity of this system shall be designed such that video and other data shall be downloadable within the maintenance window established by the operations and maintenance group for the maximum number of trains possibly present within the range of the access point(s).

Security mechanisms shall be provided to prevent unauthorized users access to the WLAN.

In the event of a security breach via the WLAN, access to critical systems shall not be possible.

The WLAN may be provided via unlicensed spectrum. Frequency coordination and interference studies shall be done to minimize impacts to the WLAN.

The WLAN shall interface with the LANs and WAN to support system-wide communications.

Radio frequency (RF) coverage simulations shall be performed to verify and validate wireless system infrastructure and wireless system parameters.

The Designer shall perform a capacity analysis study to determine the amount of bandwidth frequencies/channels required to support the WLAN wireless traffic requirements.

Radio system design shall ensure that restoration of lost data frames be accomplished by re-transmission of unacknowledged (lost) frames.

Wireless system propagation and antenna design shall be designed primarily to employ line-of-sight (LOS) radio links where possible.

Consideration shall be given to the characteristics of the onboard radio antennae. Wayside antennae shall be designed and deployed to maintain optimal and consistent radio signal strength regardless of the changes in orientation of the rolling stock antennae.

WLAN wireless nodes shall be designed that the failure of 1 node or other incident resulting in the loss of RF signal from that node shall be compensated by the 2 other adjacent access points.

- 1 WLAN connection must be seamlessly transferred as a mobile radio moves between  
2 different coverage areas within the same building, site, or location.

B. Wireless Local Area Network Spare Capacity, Expansion, and Obsolescence

- 3 The WLAN shall be designed with sufficient capacity to support the full build out of the  
4 system, plus, over and above this capacity, an additional 25 percent spare capacity,  
5 processing, and connectivity to accommodate future increased usage, end-devices and  
6 expansion.

C. Wireless Local Area Network Command and Control

- 7 There are no specific criteria. Refer to Section 28.4.1.3.

D. Wireless LAN Physical Enclosure and Power

- 8 There are no specific criteria. Refer to Section 28.4.1.4.

E. Wireless LAN End-Devices

- 9 There are no specific criteria. Refer to Section 28.4.1.5.

### 28.4.5 Radio System

- 10 The Radio System consists of separate radio subsystems that provide wireless  
11 connectivity to mobile systems, equipment, and users. These include:

- 12 • **Operations Radio System (ORS)** – voice and low-bandwidth data radio system for  
13 wireless communications between onboard equipment, mobile and portable users  
14 and fixed users and systems (if an integrated vital and non-vital radio system is  
15 used, the ORS may carry safety-critical train control data, refer to the *Automatic Train*  
16 *Control* chapter).
- 17 • **Broadband Radio System (BRS)** – high-bandwidth radio system for wireless  
18 communications between onboard high-bandwidth systems and fixed systems while  
19 trains are traveling at speeds up to 250 mph.
- 20 • **Public Safety Trench and Tunnel Radio System (PSTTRS)** – extends coverage of  
21 existing public safety/first responder radio system into the CHSTP trenches and  
22 tunnels to allow such first responders to access their respective radio systems as they  
23 normally do in their existing coverage areas.

- 24 The ORS and BRS shall include tunnel and trench wireless systems to extend respective  
25 RF coverage into tunnels and trenches as needed.

#### 28.4.5.1 Operations Radio System

- 26 The ORS will carry voice and low-bandwidth data for wireless communications between  
27 onboard equipment, mobile and portable users and fixed users and systems. While a

1 European Train Control System (ETCS)/GSM-R is desired, the radio system deployed  
2 may be a standard land mobile radio, GSM-R, or similar system, depending on  
3 requirements and RF spectrum availability. If an integrated vital and non-vital radio  
4 system, like ERTMS GSM-R is used, the ORS shall carry safety-critical train control data  
5 and shall have critical interfaces with the Train Control System, refer to the *Automatic*  
6 *Train Control* chapter. The ORS shall be designed with adequate bandwidth for voice  
7 and data including train control, alarm and event notification, onboard public address  
8 and passenger information signs, and other onboard communications subsystems.

9 The ATC component of the European Rail Traffic Management System (ERTMS) and the  
10 ETCS Level 2 defines continuous communications between the train and the radio block  
11 center for movement authority while not necessarily requiring wayside signals. The  
12 movement authority is communicated directly from a radio block center to the onboard  
13 unit using GSM-R. In-track balises are used to transmit only “fixed messages” such as  
14 location, gradient, speed limit, etc. A continuous stream of data from the wayside is  
15 transmitted to the onboard computer, which, via the Human-Machine Interface (HMI),  
16 informs the train engineer of the movement authorities and line-specific data on the  
17 route ahead, allowing the train to run at maximum or optimal speed while maintaining  
18 a safe braking distance.

19 GSM-R, the data bearer for ETCS Level 2, is a multi-function radio solution that supports  
20 critical train control data, train to wayside voice communication, multiple voice  
21 channels for railroad personnel, and passenger information. GSM-R includes various  
22 special railroad requested services such as emergency, group or broadcast calls,  
23 functional or location dependent addressing, call prioritizing, paging, telemetry, and  
24 other data in a single, integrated radio solution.

A. Operations Radio System Architecture

25 ORS shall be designed to utilize a radio tower infrastructure positioned at existing  
26 traction power facilities, signal instrument houses, stations, tunnel portal sites, and  
27 stand-alone radio sites—each spaced to give a nominal tower spacing of 2.5 miles. To  
28 achieve functional and performance requirements, the Designer shall look to antenna  
29 optimization, placement on existing commercial tower sites or buildings, and similar  
30 and, as a last resort, additional radio towers and supporting shelters and sites.

31 The ORS shall be implemented so that no single point failure of the ORS equipment can  
32 reduce the capability of the ORS to provide continuous connectivity with adequate QoS  
33 defined in the functional and performance specifications.

34 RF coverage simulations shall be performed by the Designer to verify and validate  
35 wireless system infrastructure and wireless system parameters (including continuous  
36 coverage along the Authority’s entire right-of-way), including validating continued  
37 coverage despite a single site failure. ORS shall be designed in such a manner that the

1 failure of 1 node (downing of an antenna or other incident resulting in the loss of RF  
2 signal from that node) shall be compensated by the 2 other adjacent stations.

3 The ORS System shall be provided via a licensed RF spectrum. This RF spectrum shall  
4 be for the exclusive use of the Authority.

5 The Designer shall perform a capacity analysis study to determine the number of  
6 frequencies/channels required to support the wireless traffic requirements.

7 Both uplink and downlink channels shall be designed to support train speeds of up to  
8 250 mph and shall be designed to provide rapid handover from wayside communication  
9 equipment without system impacts due to loss of data.

10 Signal strength value required to support system QoS and bit-error rate metrics shall be  
11 maintained during handoff by overlapping radio coverage between wireless  
12 communication points along the alignment.

13 Wireless system design shall ensure that wayside handoffs are “make before break.”  
14 This means that handoffs to a new base station shall provide that the connection shall  
15 not be dropped from the previous node until a new connection with the target base  
16 station is established.

17 Wireless system design shall ensure that restoration of lost data be accomplished by re-  
18 transmission of unacknowledged or lost data.

19 Wireless system propagation and antenna design shall be designed primarily to employ  
20 LOS radio links where possible. Antenna design, selection and placement shall place the  
21 main lobe of any directional antenna with the LOS of the alignment as much as feasible.  
22 The beamwidth of such antenna (up to 60 degrees) shall be selected based on the  
23 location of the next closest wayside wireless node antenna consistent with providing for  
24 LOS conditions under local curves of the track alignment.

25 Consideration shall be given to the characteristics of the onboard radio antenna at  
26 intermediate points between 2 wayside wireless transceivers. Wayside antennae shall be  
27 designed and deployed in such a manner to maintain optimal and consistent radio  
28 signal strength regardless of the momentary changes in orientation of the rolling stock  
29 antennae.

30 The ORS shall support operational and emergency voice communications and low-  
31 speed data communications.

32 ORS radios (portable and mobile type) for transit operations, maintenance, and police  
33 personnel shall include but not be limited to the following: voice and data logging,  
34 retrieval, and archiving systems.

1 The ORS (if GSM-R) shall be designed with interfaces and control such that when  
2 outside the RF coverage range of the ORS, mobiles and portables shall use commercial  
3 cellular GSM for non-critical voice and data.

4 The radio system shall be designed, furnished, installed, and maintained in accordance  
5 with the Association of American Railroads Manual of Standards and Recommended  
6 Practices, Section K.

**B. Operations Radio System Spare Capacity, Expansion, and Obsolescence**

7 The ORS shall be designed to accommodate future packet-switched network  
8 infrastructure capabilities.

9 The ORS shall be designed to support the traffic capacity of the eventual proposed, full  
10 build out of the CHSTP system and, over and above this capacity, an additional  
11 25 percent spare capacity, processing and connectivity to accommodate future increased  
12 users, usage, end-devices, and expansion.

**C. Operations Radio System Command and Control**

13 The HMI for ATC and voice radio shall comply with the relevant requirements of 49  
14 CFR Part 236 Appendix E and shall be developed with ergonomic considerations for  
15 visibility of radio displayed indications and radio functions that require Locomotive  
16 Engineer interaction with the display.

**D. Operations Radio System Physical Enclosure and Power**

17 There are no specific criteria. Refer to Section 28.4.1.4.

**E. Operations Radio System End-Devices**

18 In the event that the Contractor proposes an ERTMS Level 2 system, the air gap  
19 interfaces shall generally meet the requirements of UNISIG Subset-037; EuroRadio  
20 Functional Interface Specification, UNISIG Subset-046; Radio In-fill Form Fit and  
21 Function Interface Specification, UNISIG Subset-047; Trackside-Trainborne FIS, UNISIG  
22 Subset-092-1; ERTMS EuroRadio Conformance Requirements, and UNISIG Subset-092-2;  
23 ERTMS EuroRadio Test cases Safety Layer, UNISIG Subset-026; System Requirements  
24 Specification, UNISIG Subset-034; Functional Interface Specification for the Train  
25 Interface, UNISIG Subset-108; Interoperability consolidated requirements, UNISIG  
26 Subset-048; Trainborne Form Fit Functional Interface Specification for Radio In-Fill, and  
27 A11T6001 12 MORANE Radio Transmission Form Fit and Function Interface  
28 Specification for Euradio.

29 The design for the position of all antennas on the rolling stock shall be such that reliable  
30 communication is assured at the extremes of the track geometry and route geography  
31 that the rolling stock traverses. The movement and dynamic and static behavior of the  
32 rolling stock shall be taken into account.

The position of antennas on or about the vehicle roof and underneath the body of the train shall take into account antennas and other equipment and structural elements that might affect radio and transponder detection and communication.

#### **28.4.5.2 Operations Radio System at Trench and Tunnel Locations**

The ORS shall be extended into trenches and tunnels to provide seamless radio functionality for the trench and tunnel segments on the alignment. The Designer shall extend radio coverage of the ORS into tunnels and trenches where the at-grade ORS cannot provide coverage.

The system may include exposed, at-grade antennas for off-air interface, channelization equipment and RF-on-fiber transmission subsystem feeding the underground Bidirectional Amplifiers, which in turn will feed the underground distributed antenna system (DAS). The BDA system may also feed the underground DAS from base stations fed from the WAN. Bi-directional amplifiers located in the tunnel shall amplify, repeat, and distribute the signals. These signals shall be distributed using radiating coaxial, and use of power feeder cable, RF splitters, point-source antennae, cable taps, cross-band couplers, and other devices as necessary.

The system shall be designed so time delay interference from the over the air signals and the tunnel DAS system shall not interfere with communications. For instance, a train stopped at a tunnel portal shall have clear wireless communications back to dispatch. Clear communications is defined as DAQ level 3.4 or better for voice communications and a BER of 2 percent or less.

Only channels being used in the tunnel by either the Authority or first responders to tunnel incidents shall be brought into the tunnel.

##### **A. Operations Radio System at Trench and Tunnel Locations Architecture**

The ORS at trench and tunnel locations shall be designed to work efficiently in all trench and tunnel types, configurations, constructions, and combinations present.

Prior to the start of design, the tunnels and trenches shall be analyzed for determination of the proper level of signal amplification required to conform to the performance requirements.

The design shall consist of 2 redundant distributed signal feeds at opposite tunnel portals, with both sites available as backup to the other in the event of an equipment failure or antenna coaxial break. The ORS at trench and tunnel locations shall be implemented so that no single-point failure of the equipment can reduce the capability of the ORS at trench and tunnel locations to provide continuous connectivity with adequate QoS to be defined in the functional and performance specifications.

1 The system shall be designed to reduce, as much as practicable, the effects of near-field  
2 OCS interference with the radio system served by the DAS at all frequencies of  
3 operations. Any device near OCS shall meet the clearance requirements as specified in  
4 the *Overhead Contact System and Traction Power Return System* chapter.

5 Multiple DASs will need to be co-located in a trench, tunnel, or bore to provide multiple  
6 radio services in differing bands. Each system shall be designed in a manner that the  
7 bands of operation of 1 radio service shall not interfere with or degrade the performance  
8 of the ORS traffic.

9 Filtering and other mitigations shall be employed to reduce the risk of performance  
10 degrading interference by all co-located DASs.

B. Operations Radio System at Trench and Tunnel Locations Spare Capacity, Expansion and  
Obsolescence

11 There are no specific criteria. Refer to Section 28.4.1.2 and 28.4.5.1.

C. Operations Radio System at Trench and Tunnel Locations Command and Control

12 There are no specific criteria. Refer to Section 28.4.1.3 and 28.4.5.1.

D. Operations Radio System at Trench and Tunnel Locations Physical Enclosure and Power

13 There are no specific criteria. Refer to Section 28.4.1.4 and 28.4.5.1.

E. Operations Radio System at Trench and Tunnel Locations End-Devices

14 Leaky coaxial antenna radiating systems shall be free from co-locations with any metal  
15 objects that could perturb or attenuate the antenna RF radiating field per manufacturers'  
16 mounting instructions.

17 Leaky coaxial antenna radiating elements shall be mounted and powered such that  
18 adequate signal level can be transmitted to ORS antennas whether inside or outside the  
19 train, whichever case is worse and performance requirements can be met.

**28.4.5.3 Broadband Radio System**

20 The BRS is a high-bandwidth wireless system for wireless communications between  
21 onboard high-bandwidth systems and fixed systems while trains are traveling at speeds  
22 up to 250 mph. The primary data traffic transmitted via the BRS is CCTV video.

A. Broadband Radio System Architecture

23 Broadband wireless equipment shall be provided on the train to transmit and receive  
24 data wirelessly, with BRS access points located along the wayside, in tunnels, at stations,  
25 and yards to provide the required coverage.



1 The BRS shall be designed primarily to support the required bandwidth necessary to  
2 transfer real-time video data from/to the train in primary support of the onboard CCTV  
3 system.

4 The BRS shall interface with the LANs and WAN to support system-wide  
5 communications.

6 The BRS may be provided via unlicensed spectrum provided the Contractor can  
7 guarantee no interference from other users. The timeframe for the guarantee on no  
8 interference from other users shall be from 15 years of system acceptance. If this is not  
9 possible, licensed frequencies shall be used and the contractor shall be responsible for  
10 coordination. Frequency coordination and interference studies shall be done to minimize  
11 interference and blocking impacts to the BRS.

12 The BRS may be designed to optionally utilize an antenna-tower infrastructure  
13 positioned at existing traction power facilities, signal instrument houses, stations, tunnel  
14 portal sites and stand-alone radio sites, each spaced to give an ultimate tower spacing of  
15 approximately 2.5 miles or, based on spectrum used, BRS access points may need to be  
16 mounted on wayside poles to create “short-path” access point infrastructure.

17 RF coverage simulations shall be performed to verify and validate wireless system  
18 infrastructure and wireless system parameters.

19 The Designer shall perform a capacity analysis to determine the amount of spectrum  
20 required to support the BRS wireless traffic requirements.

21 Both uplink and downlink channels shall be designed to support train speeds of up to  
22 250 mph and shall be designed to provide rapid handover from wayside communication  
23 equipment without significant system impacts due loss of data.

24 Signal strength value required to support system QoS and bit error rate metrics shall be  
25 maintained during handoff by overlapping radio coverage between wireless  
26 communication points along the alignment.

27 Wireless system design shall ensure that wayside handoffs are ‘make before break” This  
28 means that handoffs to a new wireless node shall provide that the connection shall not  
29 be dropped from the previous node until a new connection with the target node is  
30 established.

31 Wireless system design shall ensure that restoration of lost data be accomplished by re-  
32 transmission of unacknowledged (lost) data.

33 Wireless system propagation and antenna design shall primarily be designed to employ  
34 LOS radio links where possible. Antenna design, selection and placement shall place the  
35 main lobe of any directional antenna with the LOS of the alignment as much as feasible,  
36 the beamwidth of such antenna (up to 60 degrees) shall be selected based on the location

1 of the next closest wayside wireless node antenna consistent with providing for LOS  
2 conditions under local curves of the track alignment.

3 Consideration shall be given to the characteristics of the onboard radio antenna at  
4 intermediate points between 2 wayside wireless transceivers. Wayside antennae shall be  
5 designed and deployed in such a manner to maintain optimal and consistent radio  
6 signal strength regardless of the momentary changes in orientation of the rolling stock  
7 antennae.

8 BRS wayside wireless nodes shall be designed in such a manner that the failure of 1  
9 node, downing of an antenna, or other incident resulting in the loss of RF signal from  
10 that node shall be compensated by the 2 other adjacent stations.

**B. Broadband Radio System Spare Capacity, Expansion and Obsolescence**

11 The BRS and shall be designed to support the traffic capacity of the eventual proposed,  
12 full build out of the CHSTP system and, over and above this capacity, an additional  
13 25 percent spare capacity, processing and connectivity to accommodate future increased  
14 users, throughput, usage, end-devices, and expansion.

**C. Broadband Radio System Command and Control**

15 There are no specific criteria. Refer to Section 28.4.1.3.

**D. Broadband Radio System Physical Enclosure and Power**

16 There are no specific criteria. Refer to Section 28.4.1.4.

**E. Broadband Radio System End-Devices**

17 BRS antenna infrastructure may require relatively closely spaced and short height  
18 towers, independent of the ORS tower infrastructure. Designer shall coordinate BRS  
19 antenna tower with OCS design to eliminate spatial and electrical spacing conflicts  
20 paying special consideration to avoiding spatial conflicts near phase breaks.

**28.4.5.4 Broadband Radio System at Trench and Tunnel Locations**

21 The BRS at trench and tunnel locations shall be designed and integrated with the BRS for  
22 seamless functionality for the trench and tunnel segments on the alignment. The BRS at  
23 trench and tunnel locations shall extend two-way coverage of the BRS into tunnels and  
24 trenches where the at-grade BRS cannot provide coverage.

25 Depending on whether the BRS at trench and tunnel locations utilizes a close-proximity  
26 propagation scheme (e.g., access points are mounted on short poles very close to the  
27 Authority's right-of-way), the same close-proximity propagation scheme may be  
28 suitable for coverage within trenches and tunnels.

A. Broadband Radio System at Trench and Tunnel Locations Architecture

Within trenches and tunnels, the BRS may simply be able to reuse the above-ground infrastructure scheme underground if the access points are mounted in very close proximity to the trains.

The BRS at trench and tunnel locations shall be designed to work efficiently in all trench and tunnel types, configurations, constructions, and combinations present within the CHSTP.

Prior to the start of design, the tunnels and trenches shall be analyzed for determination of the proper level of signal amplification required to conform to the performance requirements.

BRS at trench and tunnel locations shall be designed to reduce as much as practicable, the effects of near-field OCS interference with the radio system served by the DAS at all frequencies of operations. Any device near OCS shall meet the clearance requirements as specified in the *Overhead Contact System and Traction Power Return System* chapter.

Multiple DASs will need to be co-located in a trench, tunnel, or bore to provide multiple radio services in differing bands. Each system shall be designed in a manner that the bands of operation of 1 radio service shall not interfere with or degrade the performance of the ORS traffic, which is high-priority.

Filtering and other mitigations shall be employed to reduce the risk of performance degrading interference by both DASs.

B. Broadband Radio System at Trench and Tunnel Locations Spare Capacity, Expansion and Obsolescence

There are no specific criteria. Refer to Section 28.4.1.2 and 28.4.5.3.

C. Broadband Radio System at Trench and Tunnel Locations Command and Control

There are no specific criteria. Refer to Section 28.4.1.3 and 28.4.5.3.

D. Broadband Radio System at Trench and Tunnel Locations Physical Enclosure and Power

There are no specific criteria. Refer to Section 28.4.1.4 and 28.4.5.3.

E. Broadband Radio System at Trench and Tunnel Locations End-Devices

The BRS at trench and tunnel locations' radiating system shall be installed at a sufficient distance from metal objects to avoid perturbation or attenuation of the antenna RF radiating field per manufacturers' mounting instructions.

Leaky coaxial antenna radiating elements shall be mounted and powered such that adequate signal level can be transmitted to BRS antennas whether inside or outside the train, whichever case is worse and performance requirements can be met.

#### 28.4.5.5 Public Safety Trench and Tunnel Radio System

The Public Safety Trench and Tunnel Radio System (PSTTRS) shall extend coverage of existing public safety/first responder radio systems into the CHSTP trenches and tunnels to allow such first responders to access their respective radio systems as they normally do in their existing coverage areas. Such public safety/first responder radio systems are anticipated to fall into 1 of the following categories:

- VHF Public Safety
- UHF Public Safety
- 700 MHz for Public Safety Spectrum Trust and California 700 MHz Regional Planning Committees 5 and 6
- 700 MHz Public Safety Broadband
- Other state, local and municipal radio systems as appropriate

Design for the PSTTRS shall provide coverage and signal strength and Delivered Audio Quality levels consistent with the needs of the public safety community in areas of the alignment where such systems are deployed for in-tunnel repeaters. An assessment of the needs and capabilities of the local emergency response agencies will be made by the Fire/Life Safety and Security Committee for each trench or tunnel location where PSTTRS is to be deployed.

The design shall adhere to applicable Federal Communications Commission and National Public Safety Planning Administration Committee Region 6 (Northern California) and Region 5 (Southern California) planning regulations for 700 MHz Public.

##### A. Public Safety Trench and Tunnel Radio System Architecture

The PSTTRS design shall consist of 2 redundant distributed antenna feeds from the opposite tunnel portals, with both sites available as backup to the other in the event of an equipment failure or antenna coaxial break. The PSTTRS shall be implemented so that no single-point failure of the PSTTRS equipment can reduce the capability of the PSTTRS to provide continuous connectivity with adequate QoS to be defined in the functional and performance specifications.

The PSTTRS shall be designed to work efficiently in all trench and tunnel types, configurations, constructions, and combinations present.

Prior to the start of design, the tunnels and trenches shall be analyzed for determination of the proper level of signal amplification required to conform to the performance requirements.

System shall be designed to reduce as much as practicable, the effects of near-field OCS interference with the radio system served by the DAS at all frequencies of operations.

Any device near OCS shall meet the clearance requirements as specified in the *Overhead Contact System and Traction Power Return System* chapter.

Multiple DASs will need to be co-located in a trench, tunnel, or bore to provide multiple radio services in differing bands, each system shall be designed in manner that the bands of operation of 1 radio service shall not interfere with or degrade the performance of the ORS traffic, which is high-priority.

Filtering and other mitigations shall be employed to reduce the risk of performance degrading interference by multiple DASs.

**B. Public Safety Trench and Tunnel Wireless System Spare Capacity, Expansion and Obsolescence**

The system shall be expandable to accommodate 25 percent future public safety radio channels and capacity as they come in service.

**C. Public Safety Trench and Tunnel Wireless System Command and Control**

There are no specific criteria. Refer to Section 28.4.1.3.

**D. Public Safety Trench and Tunnel Wireless System Physical Enclosure and Power**

There are no specific criteria. Refer to Section 28.4.1.4.

**E. Public Safety Trench and Tunnel Wireless System End-Devices**

The PSTTRS leaky coaxial radiating system shall be installed at a sufficient distance from metal objects to avoid perturbation or attenuation of the antenna RF radiating field per manufacturers' mounting instructions.

Leaky coaxial antenna radiating elements shall be mounted and powered such that adequate signal level can be transmitted to transceivers inside the train and performance requirements can be met.

**28.4.5.6 Radio Interoperability with External Agencies and First Responders**

These requirements are developed based on pending requirements from Operations (including Authority relationship with statewide public safety radio systems).

**28.4.6 Cable Infrastructure**

The CI is the copper and fiber-optic cable, and related equipment supporting the communications systems.

The Designer shall properly design a structured CI to support a broad range of communications systems and applications. Structured cabling allows diverse applications to be served by the same cabling system. In a structured cabling system, separate facilities are not required for voice and data systems. The communications

rooms and closets shall be designed to accommodate the communications services required by the systems including expansion margins.

A combination of fiber and twisted-pair copper cables shall be used to support communications requirements.

The Designer shall employ equipment and fiber-optic cable in uses for which it is specified, where fiber will avoid the possibility of electromagnetic interference, and for cable runs that exceed the distance limitations of copper cable.

Along the wayside, fiber-optic and copper cabling shall be installed in cable troughs adjacent to the rail and in conduit feeding wayside sites. Fiber-optic cable shall be installed on both sides of the track within a protected cable trough to provide redundancy via physical diversity, so that no single point failure can result in a loss of WAN service. Fiber-optic cable will be provided along the Authority's entire right-of-way in both surface, aerial, trench and tunnel segments. Crosstrack connectivity shall be provided at each station and wayside facility requiring access to near and far cable trough per Section 28.5.6.

The primary communications system that interfaces with and uses the fiber-optic cable provided along the Authority's right-of-way is the WAN; however, some communications systems or other discipline systems may require dedicated fiber strands for their exclusive use.

The premises horizontal cabling maximum distance shall be 295 feet, independent of media type (UTP, coax, fiber). This allows 32 feet for patch cords and equipment cables. If a telecom outlet is used, the maximum distance will be reduced in accordance to TIA/EIA-568-B.1.

#### **28.4.6.1 Cable Infrastructure Architecture**

Premises backbone cabling shall be installed in a ring topology.

Horizontal cabling shall be installed in a star topology. Each cable shall be a home run without splices. Application-specific electrical components (such as impedance matching devices) shall not be installed as part of the horizontal cabling. These components, if necessary shall be added externally to the outlet.

The Designer shall comply with applicable structured cabling standards published by ISO/IEC and ANSI TIA/EIA related to designing, manufacturing, installing, and field testing CI consisting of unshielded and screened balanced twisted pair and fiber-optic cables and connectors.

Cables shall be designed and installed to be immune to electromagnetic interference (EMI) levels that are expected in high-speed rail environments per the *Electromagnetic Compatibility and Interference* chapter.

Cable and conduit located at seismic movement joints between structures and seismic fault crossings shall be designed with appropriate provisions to accommodate the expected long term and short term/large, single-event earth deflection standards.

#### **28.4.6.2 Cable Infrastructure Spare Capacity, Expansion and Obsolescence**

The Designer shall design for and implement the latest category twisted pair cabling available for twisted pair cabling, as appropriate.

Non-conduit cable pathways, raceways and troughs shall have at least 25 percent additional space for future needs. 25 percent additional empty conduit shall be installed to accommodate additional cabling.

The fiber-optic cable routed in the wayside trenches supporting the WAN shall be installed with additional 100 percent spare capacity.

When practical, 25 percent spare capacity for the addition of future equipment shall be included within equipment cabinets and enclosures.

Space for 25 percent spare equipment cabinets shall be added to the number of equipment cabinets required per equipment room. Equipment rooms shall be designed to support the needs of the eventual proposed, full build out of the CHSTP system, to accommodate the normal and spare amount of equipment cabinets, spare conduit penetrations and 25 percent additional wall mounted equipment.

#### **28.4.6.3 Cable Infrastructure Command and Control**

Not Applicable

#### **28.4.6.4 Cable Infrastructure Physical, Enclosure and Power**

All cables shall be tamper-proof. Premises cables shall not be run unprotected in exposed areas. When premises cables are run in exposed areas the cables shall be located within steel conduit for cable protection.

In concealed spaces at stations and other indoor facilities, premises cables shall be housed in Electrical Metal Conduits (EMT). Intermediate Metal Conduit (IMC) shall be used for cable runs in exposed passenger areas but not in a concealed space.

#### **28.4.6.5 Cable Infrastructure End-Devices**

Cables within cabinets and enclosures shall be neatly arranged using cable management accessories providing adequate clear space for equipment maintenance. Cabinets shall be tamper proof.

Cable terminations shall have cable tags identifying the cable number, the number of copper pairs or fiber strands in the cable, the distant end (where the cable goes) and the route that it takes.

1 Interior materials shall meet the requirement of non-flammability and smoke emission  
2 resistance such that no toxic gases are produced in the event of fire. Communications  
3 cables, including those used for the power supply, installed in regularly occupied,  
4 underground, and enclosed locations and tunnels shall be fire resistant, low smoke,  
5 non-toxic and halogen-free per applicable codes.. Cables used within systems used to  
6 aid in the evacuation of a station during a fire condition shall be able to maintain circuit  
7 integrity under fire conditions. The cable shall be rated to withstand the worst case heat  
8 levels that the cable would be exposed to for 3-hours given the various emergency  
9 evacuation scenarios (e.g., station fire, explosion). Dimensions, weights, sizes and  
10 quantities of cables run within cable troughs shall be coordinated with the cable trough  
11 dimensions as described in the *Structures* chapter.

12 Lateral cable runs entering or exiting or crossing the cable troughs shall not cross over  
13 longitudinal cable runs. Lateral cable runs shall pass below longitudinal cables.

14 Cable spool lengths shall be selected to avoid the need for splicing at an intermediate  
15 point between the manholes along the Authority's right-of-way. Splices should occur  
16 within entrance facility rooms at sites or at wayside sites.

17 If splicing is required at-grade, at an intermediate location, the splices shall be stored  
18 within a cabinet above ground on the wayside. Ten feet of cable slack shall be provided  
19 and stored in loop within the cabinet.

20 If splicing is required at an aerial section, at an intermediate location, splices may be  
21 conducted within a fiber-optic splice closure mounted in the void area or a cabinet  
22 mounted above the void area (the latter is preferred).

23 If splicing is required at an intermediate location, at a tunnel or trench segment, the  
24 splices shall be stored within a cabinet wall mounted above the cable trough. Ten feet of  
25 cable slack shall be provided and stored in loop within the cabinet.

26 Fiber-optic cable service loops of at least 50-feet long shall be installed every 500 feet  
27 within all sections of cable trough. The service loops shall be stored on frames mounted  
28 to the side wall of the cable trough adjacent to the cable. Service loops shall be installed  
29 as to not obstruct track-to-trough punch-outs at aerial segments.

30 Fiber-optic cable installed in cable trough shall be armored.

#### **28.4.7 Cable Labeling**

31 Cables shall be labeled according to ANSI/TIA/EIA – 606-A-A, Administration Standard  
32 for Telecommunications Infrastructure of Commercial Buildings, and other pertinent  
33 specifications.



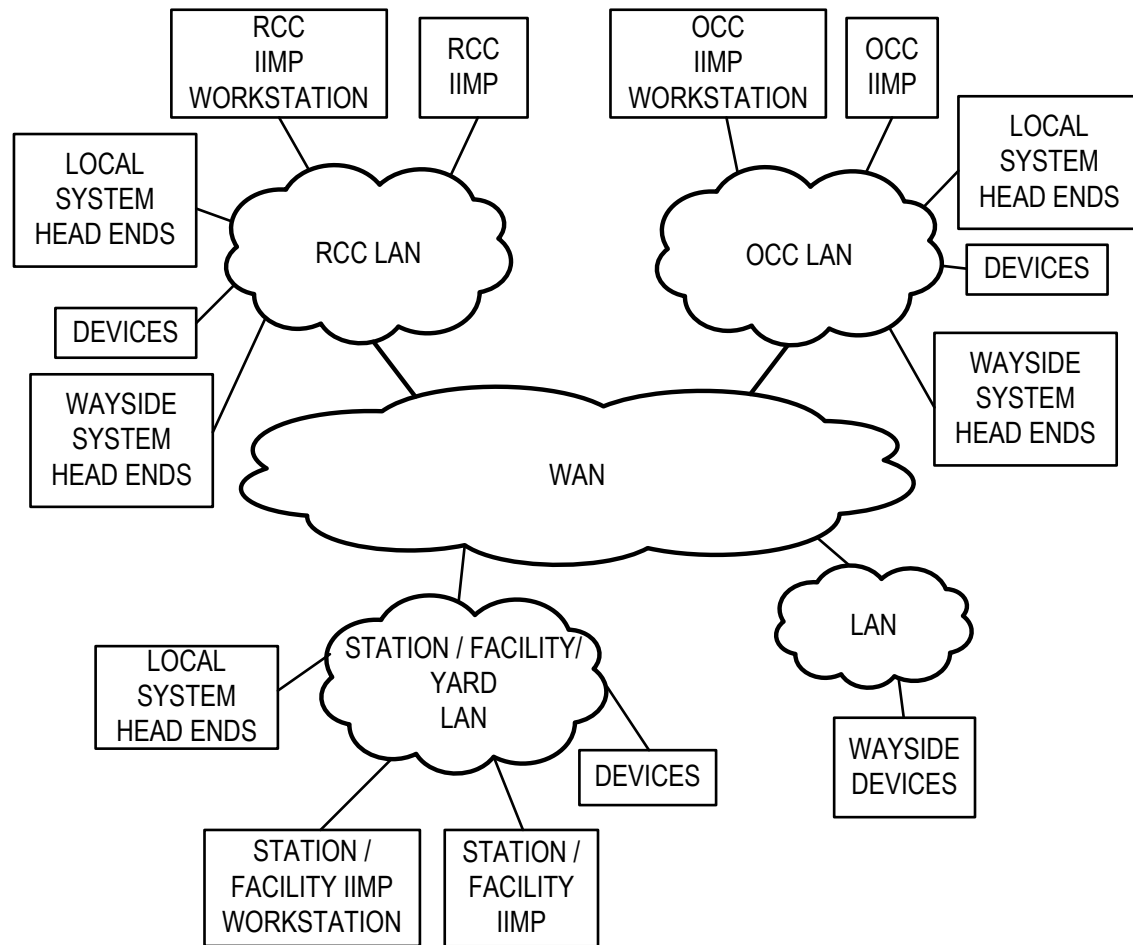
#### 28.4.8 Integrated Information Management Platform

Crucial to integrated operations and maintenance is the IIMP, which shall be designed and optimized to integrate and leverage information from multiple stand-alone communications systems and other devices and systems for streamlined and coherent operation, monitoring and control. IIMPs shall be installed at locations where human control and monitoring and/or system interface will be necessary: at OCCs and RCCs, stations, facilities, and yards.

IIMPs shall exist in multiple locations throughout the alignment and based on their location shall have a different scope of integration and control. IIMPs at the OCC and RCC shall provide integration, interface, and command and control for subtending OCC and RCC systems, wayside systems, and the local IIMPs throughout the alignment. Local IIMPs at stations and facilities shall provide integration, interface, and command and control only for subtending station and facilities systems. The local IIMPs shall pass data to the IIMPs at the OCC and RCC. Local IIMPs may be installed at unmanned facilities such as tunnel ventilation facilities as necessary. At unmanned facilities, these IIMPs will have appropriate workstation functionality.

Each type of IIMP shall support 3 basic functions: interfaces to subtending systems, logic to allow information from 1 system to cause specific actions by other systems, and the HMI or GUI allowing operators to control and monitor each of the subtending systems. IIMPs shall gather and present necessary data to automatically or manually allow the operator, dispatcher, security officer, station manager or maintenance manager to control, monitor and diagnose systems. Status reporting, information storage and retrieval, alarm processing, incident and operation reports shall be provided based on command and control assignments. The IIMPs deployed for the CHSTP shall have connectivity within the system as depicted on Figure 28-3.

1 **Figure 28-3: Integrated Information Management Platforms and Workstations**



2  
3  
4 Each IIMP shall support an interface with each of the subtending systems (e.g.,  
5 Electronic Access Control, Intrusion Detection) that allows the IIMP to communicate  
6 with the subtending systems to monitor and control those systems. The IIMP shall  
7 interface with the application programming interface of the subtending system in order  
8 to communicate, manipulate, and control the subtending systems via the device driver  
9 software within the IIMPs.

10 IIMPs shall be designed to integrate the following systems:

- 11 • Communications Systems  
12 • Communications Systems' NMS  
13 • Communications Systems' EMS  
14 • SCADA Subsystems  
15 • Business Systems including Automatic Fare Collection

• External Systems

The integration logic is the software processing of the IIMP, which shall contain the logic required to cause actions in 1 system based upon information in another system (e.g., electronic access control alarm causes video to be streamed to operator workstation). With this higher level integration platform, each installed communications or other system is required to support only a single interface thereby reducing the complexity of the architecture and making it easier to scale the architecture over time.

The IIMP shall also include the following components:

- Workstations
- HMIs (for audible input for TIS, radio and PACIS system input and similar)
- Videowall display system
- Printers and peripherals

**28.4.8.1 Integrated Information Management Platform Architecture**

IIMPs shall be designed to perform the system interface functions of the subtending systems based on the systems' requirements. Raw data streams shall be routed as efficiently as possible. The local IIMPs shall be designed to perform interface and integration without the need for transmitting raw, high-bandwidth data streams between remote systems and the OCC- and RCC-located IIMPs.

Local IIMPs shall be developed to operate in a stand-alone function continuing to interface and integrate subtending systems even when WAN connectivity to the central OCC or RCC IIMPs is lost.

**28.4.8.2 Integrated Information Management Platform Spare Capacity, Expansion and Obsolescence**

Additional interfaces may need to be added to modify existing ones or add new integration functions in the future. With the IIMP concept, only 1 new interface would need to be added if a new system is added: the interface between the new system and the IIMP. Additional logic shall be added to the IIMP to add new integration functions without needing to add new interfaces.

Over and above the projected, full-build capacity, the IIMP shall be designed to accommodate 25 percent additional subtending systems, 25 percent additional integration and interface functionality and 25 percent additional workstations.

**28.4.8.3 Integrated Information Management Platform Command and Control**

Control workstations at stations, yards, and facilities that interface with the OCC and RCC IIMPs shall be designed to also support manufacturers' stand-alone software

1 command and control GUI to permit local control of local communications systems in  
2 the case that connectivity to the OCC and RCC IIMP is lost.

3 IIMPs shall be designed to integrate control and monitoring for subtending NMSs and  
4 EMSs.

5 The HMI shall be consistent between all similar IIMPs to reduce training.

#### **28.4.8.4 Integrated Information Management Platform Physical, Enclosure and Power**

6 There are no specific criteria. Refer to Section 28.4.1.4.

#### **28.4.8.5 Integrated Information Management Platform End-Device**

7 There are no specific criteria. Refer to Section 28.4.1.5.

#### **28.4.9 Global Positioning System Network Timing System**

8 The GNTS shall be used to synchronize the clocks of communications systems, network  
9 equipment, stand-alone digital clocks, devices, and on-train equipment such that time is  
10 measured and recorded uniformly and consistently throughout the system. The GNTS  
11 receiver shall generate the timing signal required to synchronize systems.

12 In addition to digital clocks and employee computers, the GNTS shall synchronize  
13 clocks in other communication systems that rely on timing functions. These systems  
14 include:

- 15 • Closed Circuit Television (CCTV) System
- 16 • Intrusion Detection (ID) System
- 17 • Electronic access control System (EACS)
- 18 • Fire Alarm System (FAS)
- 19 • Public Address and Customer Information Sign System (PACIS)
- 20 • SCADA subsystems
- 21 • Radio System
- 22 • WAN and LANs
- 23 • Onboard Systems
- 24 • Operations and Maintenance (O&M) Systems

#### **28.4.9.1 Global Positioning System Network Timing System Architecture**

25 The GNTS shall be based on the time data transmitted by the GPS satellite system with  
26 backup oscillators. GPS inputs and backup oscillators will exist at the OCC and RCCs.

1 The GNTS shall be designed to automatically fail-over during failure of the active time  
2 source.

3 The GNTS shall be designed to synchronize clocks even in the event that the GPS signal  
4 is lost.

5 Failure of the clock system shall not contribute to the failure of any communications or  
6 other systems.

7 In the event that a master clock is unable to transmit a time signal to systems, these time-  
8 based systems and stand-alone digital clocks shall operate on their own internal clock.

9 User groups inside and outside of CHSTP will rely on the timing provided by GNTS for  
10 various operational, investigational, and maintenance purposes. Given that the  
11 communication systems above shall be synchronized to a common time source, this will  
12 permit the correlation of time-coded events in 1 system to time-coded events in another.  
13 For example, security user groups may easily correlate an intrusion alarm to CCTV  
14 video of the perpetrator, as these systems are synchronized.

15 The system shall include a performance management and monitoring system, which  
16 shall be integrated into the IIMP.

#### **28.4.9.2 Global Positioning System Network Timing System Spare Capacity, Expansion and Obsolescence**

17 The system shall be designed to support the eventual proposed, full build out of the  
18 CHSTP system and over and above this capacity, additionally include 25 percent spare  
19 capacity and capability to expand number of synchronized and synchronizing devices.

#### **28.4.9.3 Global Positioning System Network Timing System Command and Control**

20 There are no specific criteria. Refer to Section 28.4.1.3.

#### **28.4.9.4 Global Positioning System Network Timing System Physical, Enclosure and Power**

21 Wall clocks shall be powered by UPS. If Ethernet protocol is used by the LANs, wall  
22 clocks shall be powered by Power-over-Ethernet.

#### **28.4.9.5 Global Positioning System Network Timing System End-Devices**

23 Wall clocks shall be digital, with 7-segment or similar display to be specified in the  
24 functional and performance specifications. In public areas, hour-minute clocks shall be  
25 wall mounted near the passenger service agent booth in stations. In back office areas,  
26 hour-minute-second clocks shall be wall mounted in each regularly staffed room.  
27 Hallways shall have hour-minute-second clocks mounted at 1 per 250 feet of linear  
28 hallway. Hallway clocks shall be mounted such that the readable face is perpendicular  
29 to the wall.

1 Each station shall have a clock designed to the aesthetics of the station and shall be  
2 driven by the GNTS. Refer to the *Stations* chapter for details on placement of public  
3 clocks

#### **28.4.10 Fire Alarm System**

4 The FAS shall be designed to perform the following functions:

- 5 • To monitor the station areas, control centers, facilities and ancillary areas including  
6 spaces located within tunnels
- 7 • Initiate alarms (both visible and audible notifications) and activate the fire  
8 suppression systems
- 9 • Alert the monitoring and response organizations to the incident
- 10 • Emergency Voice Alarm Communication System (EVACS) to provide pre-recorded  
11 and manual voice messaging to building occupants

##### **28.4.10.1 Fire Alarm System Architecture**

12 A designated street entrance to each station, facility and building shall be provided with  
13 an alarm annunciator panel indicating the fire alarm zones and alarm status, per code.  
14 FASs shall be addressable and integrated into the building management systems  
15 (ventilation, elevators, etc.) and EACS (unlock doors).

16 The FAS shall comply with the latest versions of the National Fire Alarm codes (NFPA  
17 72) and standards regarding the application, design, installation, location, performance,  
18 testing, inspection, and maintenance of FASs and their components. The FAS shall  
19 include alarm initiating devices (detection), alarm notification appliances (horns, only in  
20 areas where PA elements are not present, and strobes), fire alarm control panels,  
21 auxiliary control panels, integrated annunciator panels, and other components to  
22 provide a complete system.

23 The FAS shall be designed per NFPA 72, which permits the transmission of evacuation  
24 announcements via compliant mass notification systems (e.g., the Public  
25 Address/Customer Information Sign System [PACIS]) permitting a more integrated  
26 system where the fire alarm notifications will be transmitted via the PACIS system. To  
27 be compliant with the code the cables and loudspeakers of the PA system shall need to  
28 comply with UL requirements.

##### **28.4.10.2 Fire Alarm System Spare Capacity, Expansion and Obsolescence**

29 The FAS shall be designed to support the eventual proposed, full build out of the  
30 CHSTP system and over and above this capacity, accommodate 25 percent additional  
31 fire initiating devices and 25 percent additional fire notification appliances.

### **28.4.10.3 Fire Alarm System Command and Control**

- 1 There are no specific criteria. Refer to Section 28.4.1.3.

### **28.4.10.4 Fire Alarm System Physical, Enclosure and Power**

- 2 There are no specific criteria. Refer to Section 28.4.1.4.

### **28.4.10.5 Fire Alarm System End-Devices**

- 3 Under NFPA 72, the voice notification system (PACIS) shall pass intelligibility tests by  
4 either objective or subjective testing. For objective testing, the system shall score a 0.5 or  
5 better reading on the speech transmission index for public address (PA) systems (STI-  
6 PA). The Designer shall perform the objective tests to ensure the system complies with  
7 the intelligibility requirements of NFPA 72.

### **28.4.11 Public Address and Customer Information Sign System**

- 8 The PACIS system shall be designed to give clear, accurate, synchronized, and timely,  
9 audible, and visual information to customers and personnel in the station areas, back-  
10 office areas, and yards. The PACIS shall serve as the notification element in routine  
11 operations and in emergency situations. The PACIS shall be designed to meet ADA  
12 requirements; voice announcements shall have an equivalent visual announcement.

- 13 The PA subsystem shall utilize a distributed speaker system in each location. The PA  
14 subsystem shall utilize signal processing techniques, ambient noise monitoring, and a  
15 physical layout designed to ensure audio broadcasts will produce clear, intelligible  
16 speech within the area of coverage.

- 17 The Customer Information Sign subsystem shall include 2 types of dynamic message  
18 visual displays placed strategically throughout stations. Customer Information Displays  
19 (CIDs) shall be low-resolution, limited-color, high-visibility Light-Emitting Diode (LED)  
20 or similar format. Customer Information Monitors shall be large-format, high-color,  
21 high-resolution Liquid-Crystal Display (LCD) or similar format. CID signs shall be used  
22 to display customer-oriented textual information including train arrival and delay  
23 information delivered by the PACIS headend using data from the ATC system. The CIM  
24 signs shall be placed at locations on platforms and mezzanines and generally above  
25 passenger service agent booths to provide scheduling and information textually and  
26 graphically to passengers.

### **28.4.11.1 Public Address and Customer Information Sign System Architecture**

- 27 The PACIS shall be designed to meet the NFPA requirements for an EVACS. Refer to  
28 Section 28.4.9. The EVACS system will automatically halt normal PACIS system  
29 announcements to deliver emergency announcements during emergency situations.

1 The PACIS shall provide a processing to display/announce messages in a prioritized  
2 manner and be capable of automatically overriding current messages/announcements  
3 when an emergency or higher priority message shall be broadcast.

4 The PACIS shall be designed to operate in a zoned manner to allow live and pre-  
5 recorded announcements (both audible and visual) to be directed to individual zones, as  
6 well as “All-Call” general broadcast announcements.

7 The PA subsystem shall utilize automatic volume control in stations to automatically  
8 compensate for a high wind or high background (crowd, traffic, train) noise  
9 environment.

#### **28.4.11.2 Public Address and Customer Information Sign System Spare Capacity, Expansion and Obsolescence**

10 The PACIS shall be designed to support the eventual proposed, full build out of the  
11 CHSTP system and, over and above this capacity, accommodate 25 percent more  
12 speakers, CIDs, CIMs and 25 percent more message storage.

#### **28.4.11.3 Public Address and Customer Information Sign System Command and Control**

13 Local (at station or facility) and central (OCC, RCC) control shall be provided for each  
14 PACIS system.

#### **28.4.11.4 Public Address and Customer Information Sign System Physical, Enclosure and Power**

15 PACIS equipment shall be enclosed in vandal and environmental proof enclosures and  
16 may require heating depending on placement.

#### **28.4.11.5 Public Address and Customer Information Sign System End-Devices**

##### **A. Public Address Subsystem Audio Coverage**

17 The PA subsystem shall be designed to provide high speech intelligibility, even when  
18 operating in a reverberant and raised ambient noise environment. The system shall be  
19 designed and tested to meet the intelligibility requirements in NFPA 72 for an EVACS.

20 The PA subsystem shall be designed such that sound projects from 1 direction only. The  
21 Designer shall not locate speakers in the ceiling and walls in the same area. The Designer  
22 shall not locate speakers on walls directed towards each other.

23 The spacing of speakers shall assure no dead spots or sound pressure level (SPL)  
24 variance of more than  $\pm 2$  dB SPL in the listening zone to be defined in the performance  
25 specifications.

26 The placement of speakers shall allow coverage at the station platform levels, public  
27 concourse areas (both paid and non-paid areas), passage walkways, non-public back-  
28 office spaces at stations, and parking areas.



The placement of speakers shall allow coverage throughout work areas, including but not limited to security offices, maintenance, communications, mechanical, and electrical rooms, hallways, and restrooms within the control centers and maintenance facilities.

Configuration of loudspeaker zones will vary according to the architectural design of each station and building.

At outdoor locations, the speaker type and placement shall minimize the sound spill of amplified messages into adjacent residential neighborhoods.

**B. Customer Information Signs Visual Coverage**

Customer Information Signs shall be provided at station platforms and concourse areas.

Customer information displays on platforms shall be located precisely along the platform and spaced such that each sign is co-located with a train car entrance. One double-sided sign shall be allocated per train car door, at the same location at each train car door when train is stops at a station. Along with arrival, delay and general information, CIDs will display reserved seat information to allow passengers to queue at their appropriate train car.

Customer information displays and monitors within the concourse area shall be located to provide coverage to persons in waiting areas and near ticket sales offices. The provided coverage shall allow a person sitting in any seat within the waiting areas to have a clear view of a CID or CIM screen and the text displayed is legible according to standards NEMA TS4:2005 and EN 12966-1:2005.

The number of displays required per platform depends on various factors including, length of platform, size of characters to be displayed, viewing angles, and height of sign.

CIDs and CIMs shall be located and operate in compliance with ADA requirements.

Audio message announcements from the PA subsystem within a PA subsystem zone shall be displayed as scrolling text messages on the CIDs and/or CIMs located in that zone. The audio and text messages shall be coordinated so that playback to the public occurs at the same time. It shall also be possible to transmit audio and text messages independent of each other.

CIDs and CIMs shall display the time-of-day, synchronized to the GNTS.

**28.4.12 Telephone and Intercom System**

The TIS shall provide wired voice communication functions for personnel, Authority police personnel, Contract Police and Security personnel, 3<sup>rd</sup>-party emergency responders and passengers. The system shall include several subsystems for specific user groups and applications. Various types of telephone and/or intercom devices shall

1 be deployed at locations along the Authority's right-of-way, public areas of the  
2 passenger station, and in selected ancillary rooms associated with the passenger stations  
3 and utility buildings.

#### **28.4.12.1 Telephone Subsystem**

4 The telephone subsystem shall include the following subsystems:

- 5 • Administrative Telephones shall provide fixed voice communications between  
6 employees to conduct daily operations. Administrative Telephones shall be located  
7 at each workspace, in communications rooms, electrical and mechanical rooms,  
8 ticket offices, security offices, conference rooms, and similar.
- 9 • Emergency Telephones are adjacent to the motor-operated power disconnects.  
10 Emergency Telephones will be used by staff to contact the traction electrification  
11 power coordinator to report emergencies and coordinate OCS power shut off.
- 12 • Sound Powered Telephones (SPT) System is a stand-alone communications system  
13 that does not rely on network connectivity or a power source. SPT shall use electro-  
14 mechanical transducers that convert voice into an electrical signal for  
15 communication, without use of any external power sources and shall be used as  
16 backup in the event of network or radio failures. SPTs shall be located in trenches  
17 and tunnels and used by 3<sup>rd</sup>-party first responders and other personnel under  
18 emergency circumstances.

##### A. Telephone System Architecture

19 Voice communications carried by the telephone systems shall be recorded and time-  
20 stamped.

##### B. Telephone System Spare Capacity, Expansion and Obsolescence

21 The telephone system shall be designed to support the traffic capacity of the eventual  
22 proposed, full build out of the CHSTP system and over and above that, to accommodate  
23 25 percent more telephones, throughput and calls.

##### C. Telephone System Command and Control

24 There are no specific criteria. Refer to Section 28.4.1.3.

##### D. Telephone System Physical, Enclosure and Power

25 Telephone hardware shall be robust, vandal resistant, and designed to withstand and  
26 function in the operating environment.

##### E. Telephone System End-Devices

27 SPTs shall be terminated above ground at the emergency command posts at both ends of  
28 trenches and tunnels.

1 An administrative telephone allowing communication to the OCC and RCC, wayside  
2 equipment enclosures, other TPF, and the control room of the concerned power utility  
3 grid substation shall be provided at each prefabricated 25 kV switchgear room and each  
4 wayside power control cubicle.

#### **28.4.12.2 Intercom Subsystem**

5 The Intercom Subsystem shall include the following:

- 6 • Customer Assistance Intercoms (CAIs) shall be installed for passengers to report  
7 emergencies and obtain general travel information. CAIs shall have 2 buttons for  
8 these 2 different functions.
- 9 • Emergency Intercoms (EIs) shall be installed for passengers to report emergencies  
10 and shall have a single button to perform this function.
- 11 • Station Intercoms shall allow the station attendant to communicate with passengers  
12 at fare collection equipment, fare-barrier equipment or through protective glass.

##### A. Intercom System Architecture

13 Voice communications carried by the intercom systems shall be recorded and time-  
14 stamped.

##### B. Intercom System Spare Capacity, Expansion and Obsolescence

15 The Intercom system shall be designed to support the traffic capacity of the eventual  
16 proposed, full build out of the CHSTP system and over and above that, to accommodate  
17 25 percent more intercoms, throughput and calls.

##### C. Intercom System Command and Control

18 There are no specific criteria. Refer to Section 28.4.1.3.

##### D. Intercom System Physical, Enclosure and Power

19 Intercom hardware shall be robust, vandal resistant, and designed to withstand and  
20 function in the operating environment.

##### E. Intercom System End-Devices

21 CAI and EI shall be placed platform and mezzanine locations and coordinated with  
22 CCTV camera viewing fields.

#### **28.4.13 Closed Circuit Television System**

23 The CCTV system shall be designed to monitor critical locations along the Authority's  
24 right-of-way and in facilities and stations.

1 The CCTV system is an essential component to providing safety and security to  
2 customers and Authority personnel. The CCTV system shall be designed to allow the  
3 Authority and/or law enforcement authorities to deter and prevent crimes and attacks,  
4 identify perpetrators and provide a video record useful for any legal proceedings that  
5 may follow. Information gathered by the CCTV system will be used to manage assets  
6 and alert and dispatch security forces.

7 The CCTV system shall provide video to Authority users at each station and facility and  
8 remotely at the OCC, RCCs, and Terminal Control Facilities. The CCTV system shall be  
9 flexible to allow any camera within the system to be viewed by a user at any IIMP  
10 depending on the privileges of the user currently logged into the system at the viewing  
11 device. Typically, users at stations will only need to view live or recorded video  
12 captured by cameras that are local to the station, while users located at the OCC and  
13 RCCs shall be able to view live or recorded video captured by cameras that are local as  
14 well as remote. When viewing live video captured by local cameras or recorded video  
15 stored locally, the video streams shall be transmitted via the LAN only. When viewing  
16 live video captured by remote cameras or recorded video stored remotely, the video  
17 streams shall be transmitted via the WAN as well as the LANs.

#### **28.4.13.1 Closed Circuit Television System Architecture**

18 Video from cameras except those located onboard trains shall be continuously recorded  
19 and stored at the nearest video storage location (i.e., station, maintenance facility,  
20 control center) and storage shall be designed to store video for a minimum of 90 days  
21 (see Section for spare capacity requirements). This includes wayside cameras and  
22 cameras located at smaller facilities including traction power facilities, signal equipment  
23 houses and stand-alone radio sites.

24 The CCTV system shall be designed to NTCIP 1208 Standards.

25 Video captured by the cameras shall be transmitted to local storage. For cameras at the  
26 wayside, video will be stored at the nearest video storage location. Video will be  
27 selectively transmitted back to OCC and RCCs based on exception (e.g., alarm triggers  
28 the transmission of live video, or the operator manually selects a camera for viewing).

#### **28.4.13.2 Closed Circuit Television System Spare Capacity, Expansion and Obsolescence**

29 The CCTV system shall be designed to support the eventual proposed, full build-out of  
30 the CHSTP system and, over and above that, an additional 25 percent more capacity to  
31 accommodate more cameras, storage and processing.

#### **28.4.13.3 Closed Circuit Television System Command and Control**

32 The CCTV system shall be designed to the specific chain of custody demands as  
33 specified in the *Systems Safety and Security* chapter.

#### 28.4.13.4 Closed Circuit Television System Physical, Enclosure and Power

CCTV cameras shall be enclosed in vandal and environmental proof enclosures and may require heating depending on placement.

#### 28.4.13.5 Closed Circuit Television System End-Devices

CCTV images shall be available for viewing on individual workstations (multiple views available on 1 screen) or on selected parts of wall display. Image display shall be highly flexible and customizable.

Placement of the CCTV cameras shall be designed to maximize the operational and security objectives while providing the most efficient and cost-effective solution. The CCTV System shall be designed to provide the coverage and fields of views specified below. Each field of view is categorized under 1 of the general CCTV functions listed below:

- **Monitor** – The target person occupies at least 5 percent of the video monitor screen height, and the scene portrayed is not unduly cluttered. From this level of detail an observer shall be able to monitor the number, direction, and speed of movement of people, providing their presence is known to the observer (i.e., the target does not have to be searched for within the overall scene). For 4CIF (480 pixels vertically) resolution and a 5-foot-6-inch person, 5 percent of the video monitor screen height would be equivalent to approximately 4.5 pixels per foot.
- **Detect** – The target person occupies at least 10 percent of the video monitor screen height. After an alert, an observer shall be able to search within the displayed scene and ascertain with a high degree of certainty whether or not a person is present. For 4CIF (480 pixels vertically) resolution and a 5-foot-6-inch person, 10 percent of the video monitor screen height would be equivalent to approximately 9 pixels per foot.
- **Recognize** – The target person occupies at least 50 percent of video monitor screen height. An observer can determine with a high degree of certainty whether or not an individual shown is the same as someone they have seen before. For 4CIF (480 pixels vertically) resolution and a 5-foot-6-inch person, 50 percent of the video monitor screen height would be equivalent to approximately 40 pixels per foot.
- **Identify** – The target person occupies at least 120 percent of the video monitor screen height. The picture quality and detail shall be sufficient to enable the identity of an individual to be established beyond reasonable doubt. For 4CIF (480 pixels vertically) resolution and a 5-foot-6-inch person, 120 percent of the video monitor screen height would be equivalent to approximately 100 pixels per foot.

The minimum percentages of the video monitor screen height given above that the target shall occupy shall be obtained without the observer needing to zoom into the scene and assumes resolutions of at least 4CIF.

1 The above categories provide direction to the Designer for appropriate image sizes to  
2 when choosing the placement of cameras and the type of lenses to obtain the required  
3 field of view. The above categories do not imply that it will be impossible to recognize or  
4 identify an individual if the image size is smaller than the 50 percent or 120 percent  
5 figures suggested. There is also no guarantee that individuals will be identifiable simply  
6 because they occupy >120 percent of the screen. Other factors, such as lighting, angle of  
7 view, and video compression will also have an influence. Therefore, the above categories  
8 shall be used as a guide to the Designer in determining preliminary locations of cameras  
9 and required lens characteristics. Fields of view shall be tested in the field to confirm  
10 whether the live and recorded images meet the intended function.

A. Field of View at Stations

11 The CCTV design shall provide the following fields of view at stations:

- 12 • Pedestrian Entrances/Exits
  - 13 – Description: Fixed color cameras shall be located to provide fields of view
  - 14 showing the frontal view of persons both entering and exiting stations
  - 15 – Function: Recognize
  - 16 – Resolution: 4CIF
  - 17 – Frame Rate: 5 frames per second
- 18 • Situational Awareness (Non-Platform)
  - 19 – Description: Pan-Tilt-Zoom color cameras shall be located to provide fields of
  - 20 view showing an overview of the station public areas. 100 percent coverage of
  - 21 public space is required to be covered, with an emphasis placed on the following
  - 22 locations: areas near entrances/exits, passageways, tops and bottoms of
  - 23 stairways, and areas in front of elevator and escalator landings, to monitor
  - 24 passenger movement.
  - 25 – Function: Monitor
  - 26 – Resolution: 4CIF
  - 27 – Frame Rate: 5 frames per second
- 28 • Situational Awareness (Platform)
  - 29 – Description: Pan-Tilt-Zoom color cameras shall be located on platforms to
  - 30 provide fields of view showing an overview of the platform areas. 100 percent
  - 31 coverage of the platform is required to be covered, with an emphasis placed on
  - 32 the following locations: platform edges, tops and/or bottoms of stairways, areas
  - 33 in front of elevator and escalator landings, to monitor passenger movement.
  - 34 – Function: Monitor

- 1       – Resolution: 4CIF
- 2       – Frame Rate: 15 frames per second
- 3       • Platform Edges
  - 4       – Description: Fixed color cameras shall be located to provide fields of view
  - 5       showing the edge of the platform along the entire length of the platform for the
  - 6       purpose of providing the train door operator video to aid his determination that
  - 7       train doors to platform are clear and can be closed.
  - 8       – Function: Detect
  - 9       – Resolution: 4CIF
  - 10      – Frame Rate: 15 frames per second
- 11      • Fare Control Areas (e.g., turnstiles)
  - 12      – Description: Fixed color cameras shall be located to provide fields of view
  - 13      showing the frontal view of persons (head to knees) both entering and exiting
  - 14      to/from the paid areas.
  - 15      – Function: Identify
  - 16      – Resolution: 4CIF
  - 17      – Frame Rate: 5 frames per second
- 18      • Inside Elevator Cab
  - 19      – Description: Fixed color cameras shall be located inside elevators to obtain full
  - 20      coverage of the entire cab interior to monitor passenger activity.
  - 21      – Function: Monitor
  - 22      – Resolution: 4CIF
  - 23      – Frame Rate: 5 frames per second
- 24      • Trespass Control (Unsecured Side)
  - 25      – Description: Fixed color cameras shall be located to provide fields of view of the
  - 26      unsecured side in front of an access control point (e.g., doorway, gate) to capture
  - 27      video of persons using the electronic access control device (e.g., card reader) and
  - 28      potential intruders entering the secured area.
  - 29      – Function: Detect
  - 30      – Resolution: 4CIF
  - 31      – Frame Rate: 5 frames per second
- 32      • Trespass Control (Secured Side – Identification)

- 1       – Description: Fixed color cameras shall be located inside the secured area to  
2       provide fields of view of the secured entrance point (e.g., doorway, gate) to  
3       capture video of persons entering the secured area for the purpose of  
4       identification.
- 5       – Function: Identify
- 6       – Resolution: 4CIF
- 7       – Frame Rate: 15 frames per second (15 frames per second are specified here  
8       assuming intruders may be moving quickly through the access control point to  
9       avoid being seen.)
- 10      • Trespass Control (Secured Side - Monitoring)
- 11       – Description: Pan-Tilt-Zoom color cameras shall be located inside the secured  
12       room to monitor the activity of persons inside the room.
- 13       – Function: Monitor
- 14       – Resolution: 4CIF
- 15       – Frame Rate: 5 frames per second
- 16      • Customer Assistance/Emergency Intercoms
- 17       – Description: Fixed color cameras shall be located near CAI and EI Intercoms to  
18       monitor the activity of persons using the intercom. CAI/EI cameras may be Pan  
19       Tilt Zoom cameras able to monitor multiple CAI/EI devices and bring field of  
20       view surrounding of CAI/EI activated by button press. Details of this functional  
21       interface are to be covered in the functional and performance specifications.
- 22       – Function: Recognize
- 23       – Resolution: 4CIF
- 24       – Frame Rate: 5 frames per second
- 25      • Ticket sales offices/ Ticket Vending Machines
- 26       – Description: Fixed color cameras shall be located near ticket sales offices and  
27       ticket vending machines to capture video suitable to recognize the person  
28       making the transaction.
- 29       – Function: Recognize
- 30       – Resolution: 4CIF
- 31       – Frame Rate: 15 frames per second

**B. Field of View at Parking Areas**

- 32      The CCTV design shall provide the following fields of view at parking areas:



- 1 • Vehicular Entrances/Exits
  - 2 – Description: Fixed color cameras supporting progressive scan shall be located to
  - 3 provide fields of view showing the driver as well as the license plate of the
  - 4 vehicle when the vehicle is entering and exiting the parking area. The area of
  - 5 interest (e.g., face or license plate) shall cover approximately 15 percent or more
  - 6 of the camera's field of view.
  - 7 – Function: Identify
  - 8 – Resolution: 4CIF
  - 9 – Frame Rate: 30 frames per second
- 10 • Parking Revenue Collection Sites
  - 11 – Description: Fixed color cameras shall be located to provide fields of view to
  - 12 capture video suitable to identify the person making the transaction.
  - 13 – Function: Identify
  - 14 – Resolution: 4CIF
  - 15 – Frame Rate: 15 frames per second
- 16 • General Observation of Parking Area
  - 17 – Description: Pan-Tilt-Zoom cameras supporting progressive scan shall be located
  - 18 to provide wide fields of view to capture video suitable to monitor the area
  - 19 where vehicles are parked. One-hundred percent coverage of the parking area is
  - 20 required.
  - 21 – Function: Monitor
  - 22 – Resolution: 4CIF
  - 23 – Frame Rate: 30 frames per second

C. Field of Views along the Right-of-Way

24 The CCTV design shall provide the following fields of view along the Authority's right-  
25 of-way:

- 26 • Tunnel Portals
  - 27 – Description: Fixed color cameras supporting progressive scan shall be located to
  - 28 provide fields of view to capture video of persons or vehicles entering and
  - 29 exiting the tunnel portal.
  - 30 – Function: Detect
  - 31 – Resolution: 4CIF
  - 32 – Frame Rate: 30 frames per second

- 1 • Traction Power Facilities
- 2 – Refer to the requirements for Field of View at Stations for Trespass Control:
- 3 Unsecured Side, Secured Side – Identification, and Secured Side – Monitoring
- 4 • Enclosures at Standalone radio Sites
- 5 – Refer to the requirements for Field of View at Stations for Trespass Control:
- 6 Unsecured Side, Secured Side – Identification, and Secured Side – Monitoring
- 7 • ATC Equipment Houses
- 8 – Refer to the requirements for Field of View at Stations for Trespass Control:
- 9 Unsecured Side, Secured Side – Identification, and Secured Side – Monitoring
- 10 • Tunnel Ventilation Facilities at Portal Sites and Ventilation Shafts
- 11 – Refer to the requirements for Field of View at Stations for Trespass Control:
- 12 Unsecured Side, Secured Side – Identification, and Secured Side – Monitoring
- 13 • Tunnel Exit Shafts
- 14 – One-hundred percent of Tunnel emergency exit shafts shall be covered. Refer to
- 15 the requirements for Field of View at Stations for Trespass Control: Unsecured
- 16 Side, Secured Side – Identification, and Secured Side – Monitoring

D. Operations Control Center /Regional Control Center

17 The CCTV design shall provide the following fields of view at the OCC/RCCs:

- 18 • Access Control Points
- 19 – Refer to the requirements for Field of View at Stations for Trespass Control:
- 20 Unsecured Side, Secured Side – Identification, and Secured Side – Monitoring

E. Field of View at Yards

21 The CCTV design shall provide the following fields of view at the yards:

- 22 • Transition tracks
- 23 – Refer to the requirements for Field of View at Stations: Unsecured Side, Secured
- 24 Side – Identification, and Secured Side – Monitoring Sections
- 25 • Yard throat (where yard tracks come together at signaled area)
- 26 – Refer to the requirements for Field of View at Stations: Unsecured Side, Secured
- 27 Side – Identification, and Secured Side – Monitoring Sections
- 28 • General view of yard tracks to show occupancy and movement
- 29 – Refer to the requirements for Field of View at Stations: Unsecured Side, Secured
- 30 Side – Identification, and Secured Side – Monitoring Sections

- Transfer tracks between transportation and rolling stock maintenance departments
  - Refer to the requirements for Field of View at Stations: Unsecured Side, Secured Side – Identification, and Secured Side – Monitoring Sections

**F. Field of View at Maintenance Facilities**

The CCTV design shall provide the following fields of view at the MOI/RSM sites:

- Access Control Points
  - Refer to the requirements for Field of View at Stations for Trespass Control: Unsecured Side, Secured Side – Identification, and Secured Side – Monitoring Sections
- Fuel Dispensing Area
  - Description: Color cameras shall be located at the Fuel Dispensing Area to monitor the activity of personnel.
  - Function: Monitor
  - Resolution: See 4CIF
  - Frame Rate: 5 frames per second

**28.4.14 Intrusion Detection System**

The IDS shall be designed to monitor critical locations along the Authority's right-of-way and in facilities including perimeters and building interiors.

The IDS shall automatically detect physical, human intrusions at the following locations:

- Entrances into OCC and RCC buildings, maintenance facilities, stations, yards
- Traction Power Facilities
- Standalone radio site enclosures
- Equipment cabinets and enclosures
- Entrances to the back office areas (non-public areas) at stations
- Tunnel portals
- Tunnel Emergency Exit shafts
- Entrances to the following protected rooms and locations
  - Police and Security Offices
  - Ticket Sales Offices
  - Station Agent Offices

- 1       – Entrance Facility Rooms
  - 2       – Train Control and Communications Rooms
  - 3       – Station Computer Rooms
  - 4       – Communications Closets
  - 5       – Mechanical Rooms
  - 6       – Electrical Rooms
  - 7       – Fire Protection Rooms
  - 8       – Plumbing Rooms
  - 9       – Elevator/Escalator Machine Rooms
  - 10      – Battery Rooms
  - 11      – Station Control Rooms
  - 12      – Terminal Control Facilities
  - 13      – Rooms containing money and safes
  - 14      • Fenced areas
  - 15      • Other locations considered necessary and as determined at a later date
- 16   The IDS shall also detect the intrusion of animals/livestock of significant size into the  
17   Authority's right-of-way.
- 18   The IDS shall not cause false alarms when trains cross tunnel portals.
- 19   The IDS shall not cause false alarms when rodents or other small debris cross tunnel  
20   portals.
- 21   The IDS shall be integrated with the EACS such that authorized persons may enter the  
22   above secure locations without generating an alarm, as appropriate.

#### **28.4.14.1 Intrusion Detection System Architecture**

- 23   There are no specific criteria. Refer to Section 28.4.1.1.

#### **28.4.14.2 Intrusion Detection System Spare Capacity, Expansion and Obsolescence**

- 24   The IDS shall be designed to support the eventual proposed, full build out of the CHSTP  
25   system and, over and above that, to accommodate 25 percent more intrusion detection  
26   devices and processing of alarms.

#### **28.4.14.3 Intrusion Detection System Command and Control**

- 27   There are no specific criteria. Refer to Section 28.4.1.3.

#### 28.4.14.4 Intrusion Detection System Physical, Enclosure and Power

- 1 Enclosures for intrusion detection equipment shall be tamperproof.

#### 28.4.14.5 Intrusion Detection System End-Devices

- 2 IDS end-devices shall be concealed and inaccessible by potential intruders.

#### 28.4.15 Electronic Access Control System

3 The EACS shall control which personnel is permitted access to facilities and restricted  
4 areas; which areas they can access (e.g., rail yard facilities, communications rooms  
5 within stations); and when they can access these areas (e.g., certain days of the week or  
6 shifts). In addition to controlling passage in and out of facilities or areas, and  
7 determining who belongs and who does not, the EACS shall provide the ability to  
8 observe and track movement in and out of controlled areas. The EACS shall be designed  
9 to keep records of access for amount of time specified in the *Systems Safety and Security*  
10 chapter.

11 The EACS shall be designed to provide easy access to public areas of facilities during  
12 hours of service operation while at the same time limiting access to non-public areas to  
13 authorized personnel only. During non-service hours, access to public areas shall also be  
14 controlled and granted to authorized persons only.

15 The EACS shall not interfere with fire protection and life safety systems.

16 Power over Ethernet (PoE) use is subject to codes, Authority Having Jurisdiction, and  
17 the amount of power delivered by PoE source versus the amount of power required by  
18 the electric lock.

19 The EACS shall conform to ADA requirements.

20 The EACS entry control shall be based upon a combination of the following:

- 21 • An agency-provided special ID card/badge to work with automatic readers (based  
22 on what the users **HAVE**)
- 23 • A code, such as a personal identification number (PIN), for entering on a keypad  
24 (based on what users **KNOW**)
- 25 • A biometric device for feature recognition, such as fingerprint identification (based  
26 on who users **ARE**)

27 Different admission controlled barriers (e.g., doors, gates, portals) shall require different  
28 combinations of the above 3 entry-control mechanisms. Less secure areas may require  
29 only the use of an agency-provided identification card to grant access to the secure zone  
30 while more secure areas may require a combination of all 3 entry control mechanisms.

- 1 The EACS shall be designed to allow any combination of the above entry-control  
2 mechanisms at admission controlled barriers even if a single-entry control mechanism is  
3 deemed sufficient and installed during the initial deployment.

#### **28.4.15.1 Electronic Access Control System Architecture**

- 4 There are no specific criteria. Refer to Section 28.4.1.1.

#### **28.4.15.2 Electronic Access Control System Spare Capacity, Expansion and Obsolescence**

- 5 The EACS system shall be designed to support the eventual proposed, full build out of  
6 the CHSTP system and, over and above that, to accommodate 25 percent more electronic  
7 access control devices and processing of access requests and alarms.

#### **28.4.15.3 Electronic Access Control System Command and Control**

- 8 There are no specific criteria. Refer to Section 28.4.1.3.

#### **28.4.15.4 Electronic Access Control System Physical, Enclosure and Power**

- 9 There are no specific criteria. Refer to Section 28.4.1.4.

#### **28.4.15.5 Electronic Access Control System End-Devices**

- 10 There are no specific criteria. Refer to Section 28.4.1.5.

### **28.5 Communications Physical Support Rooms, Enclosures, Shelters and Equipment**

- 11 The following sections include design criteria for:

- 12 • Communications Rooms  
13 – Entrance Facility Rooms  
14 – Train Control and Communication Rooms  
15 – Communications Closets  
16 • Communications Interface Cabinets  
17 • Communications Shelters  
18 • Standalone Radio Sites  
19 • Radio Towers  
20 • Systems Conduits at Track

### 28.5.1 Communications Rooms supporting Communications Systems and Equipment

These sections contain the design criteria for rooms, pathways and spaces used to house and distribute cable, conduit, and equipment.

Rooms shall be defined and designed for all buildings, including stations, facilities, tunnel ventilation facilities at portal sites and tunnel ventilation shafts, control centers, and similar. Grounding and bonding of communications rooms, conduit, cabling, shields, cabinets, enclosures, and similar shall be as in the *Grounding and Bonding Requirements* chapter. ANSI/TIA/EIA-568-A Standard categorizes structured cabling rooms into 6 areas:

- Entrance Facility
- Equipment Room
- Backbone Cabling (connects entrance facilities, equipment rooms, and closets)
- Telecommunications Closet
- Horizontal Cabling (extends from the closets to the work areas and end devices)
- Work Area

Table 28-1 provides the nomenclature equivalency between the ANSI/TIA/EIA and project elements.

**Table 28-1: Room Nomenclature**

ANSI/TIA/EIA-568-A Designation	Project Equivalent
Entrance Facility	Entrance Facility Room
Equipment Room	Train Control and Communications (TCC) Room
Equipment Room	3 <sup>rd</sup> Party Telecom Room
Telecommunications Closet	Communications Closet
Work Area	Terminal Control Facility
Work Area	Station Control Room
Work Area	Control Room
Work Area	Incident Command Post
Work Area	Incident Room

**Table 28-2: Communications Related Spaces**

Site Location	Rooms/Shelters/Equipment Location	Provisioning
OCC	<ul style="list-style-type: none"> <li>Entrance Facility Room</li> <li>TCC Rooms (dual redundant)</li> <li>Communications Closet(s)</li> <li>3<sup>rd</sup> Party Telephone Room</li> </ul>	110/220 V ac 600A utility power, UPS, HVAC, waterless fire suppression, security.
RCC	<ul style="list-style-type: none"> <li>Entrance Facility Room</li> <li>TCC Room</li> <li>Communications Closet(s)</li> <li>3<sup>rd</sup> Party Telephone Room</li> </ul>	110/220 V ac 600A utility power, UPS, HVAC, waterless fire suppression, security.
Intermediate Passenger Station	<ul style="list-style-type: none"> <li>Entrance Facility Room</li> <li>TCC Room</li> <li>Communications Closet(s)</li> <li>3<sup>rd</sup> Party Telephone Room</li> </ul>	110/220 V ac 300A utility power, UPS, HVAC, waterless fire suppression, security
Terminal Passenger Station	<ul style="list-style-type: none"> <li>Entrance Facility Room</li> <li>TCC Room</li> <li>Communications Closet(s)</li> <li>3<sup>rd</sup> Party Telephone Room</li> </ul>	110/220 V ac 400A utility power, UPS, HVAC, waterless fire suppression, security
Rolling Stock Maintenance or Maintenance of Infrastructure Facility	<ul style="list-style-type: none"> <li>Entrance Facility Room</li> <li>TCC Room</li> <li>Communications Closet(s)</li> <li>3<sup>rd</sup> Party Telephone Room</li> </ul>	110/220 V ac utility power, UPS, HVAC, waterless fire suppression, access security.
Traction Power Facility	<ul style="list-style-type: none"> <li>Communications Shelter</li> <li>Radio Tower</li> </ul>	110/220 V ac utility power, UPS, HVAC, waterless fire suppression, security
ATC Equipment Houses	<ul style="list-style-type: none"> <li>Communications Shelter</li> <li>Radio Tower</li> </ul>	110/220 V ac utility power, UPS, HVAC, waterless fire suppression, security
Standalone Radio Site	<ul style="list-style-type: none"> <li>Communications Shelter</li> <li>Radio Tower</li> </ul>	Grading, drainage, access road, 110/220 V ac utility power UPS, HVAC, concrete slab, drainage access roads, access restriction
Outdoor Yard	<ul style="list-style-type: none"> <li>Yard CICs: location based on necessary WLAN RF coverage and other end devices</li> </ul>	110/220 V ac utility power, UPS, HVAC, underground conduit, drainage, concrete slab, security fence.
Tunnel Ventilation Facility	<ul style="list-style-type: none"> <li>Entrance Facility Room</li> <li>TCC Room</li> <li>Communications Closet(s)</li> </ul>	110/220 V ac utility power, UPS, HVAC, waterless fire suppression, access security.



### 28.5.1.1 Communications Rooms

Each building (not including communications shelters) shall have an Entrance Facility Room where Authority and third-party service and cabling enters the building. The primary function of the Entrance Facility Room is demarcation from service providers and customer premises cabling and between inside plant and outside plant Authority cabling.

The Train Control and Communications Room (TCCR) houses the train control and communications equipment supporting wayside system and contains the bulk of communications equipment supporting the station or facility building. This room shall be shared between the train control equipment and communications equipment.

Communications closets are smaller communications rooms and are smaller than the TCCRs and generally contain less communications equipment than the main communications room and are placed based on horizontal cabling distance limitations. The primary function of the communications closet shall be termination and cross-connection of horizontal and backbone cables.

In general, train control and communications rooms and communications closets shall house communications (and other discipline) equipment in equipment cabinets. Equipment cabinets may be free-standing or wall mounted and shall be designed to the criteria listed in Section 28.4.1.4.

### 28.5.1.2 Train Control and Communication Rooms, Entrance Facility Rooms and Communications Closets

Train Control and Communications Rooms, entrance facility rooms, and communications closets and contents shall be designed to the seismic and structural requirements declared in the *Structures* chapter. The rooms shall have a minimum floor loading as described in the *Structures* chapter. If raised access flooring is used in the room, it must be rated accordingly.

Provide ac-grade plywood, 8 feet high with a minimum thickness of 0.75 inches (3/4") around the perimeter of the room. Plywood shall be fire-rated and may NOT be painted. The bottom of the plywood shall be mounted 6 inches above the finished floor.

The rooms shall be dedicated to the TCCR function and related support facilities and not shared with electrical or HVAC installations, nor shall water pipes, ductwork, pneumatic tubing, or similar pass through the Entrance Facility room, TCCR, or communications closet.

At minimum, the rooms shall be sized based on BICSI standards and the number of work areas and end devices plus expansion as detailed in Section 28.4.1.2.

The rooms shall not be located below water level and shall be free of water supply or drain pipes unless needed to support the installed equipment.

- 1 A floor drain shall be provided within the room if risk of water ingress exists.
- 2 The room shall be located with ready access to the main HVAC delivery system or
- 3 dedicated HVAC equipment.
- 4 The room shall be located away from sources of EMI such as electrical transformers,
- 5 motors, and induction sealing devices.
- 6 Environmental control equipment, such as power distribution or conditioner systems,
- 7 and UPS up to 100 kVA shall be permitted in the room.
- 8 Doors providing access to other areas of the building through the room shall be avoided
- 9 to limit access to authorized personnel only.
- 10 Minimum clear height in the rooms shall be 9 feet without obstructions.
- 11 If contaminants are present in greater concentrations as indicated in ANSI/TIA/EIA-569-
- 12 A, the room shall be provided with vapor barriers, positive room pressure, or absolute
- 13 filters.
- 14 If sprinklers are required by local code, the heads shall be provided with wire cages to
- 15 prevent accidental operation and drainage troughs shall be placed under sprinkler pipes
- 16 to prevent leakage onto equipment. Consideration shall be given to the installation of
- 17 alternate fire-suppression systems (i.e., “clean agent”).
- 18 Communications equipment requires the HVAC 24 hours per day, 365 days per year
- 19 including during times when equipment is powered by UPS or other emergency or
- 20 standby power source.
- 21 Rooms require at minimum a double door 72 inches wide and 90 inches high without
- 22 doorsill and with a removable center post.
- 23 Firestopping shall be used whenever a fire-rated wall, ceiling, or floor is penetrated by
- 24 communications facilities. The firestop shall be installed to maintain the rating of the
- 25 structure penetrated; refer to the *Structures* chapter. A qualified system or design
- 26 approved and tested by an independent laboratory for use in the specific penetration
- 27 shall be used.
- 28 Floor materials having antistatic properties shall be selected.

A. Entrance Facility Rooms

- 29 If third-party and Authority cabling terminations are co-located within the entrance
- 30 facility room, there shall be a secure, physical separation between spaces accessible by
- 31 third-party personnel and Authority equipment.

- 32 At each Entrance Facility Room the Designer shall provision for the following:

- 1 • Outside Plant Cable Termination
- 2 • Inside Plant Cable Termination

B. Train Control and Communications Rooms

- 3 Avoid placing main rooms in building areas that limit expansion such as elevators, core,  
4 outside walls, or other fixed building walls. The room shall be accessible for delivery of  
5 large equipment.

C. Communications Closets

- 6 A communications closet shall be located on each floor as close as practical to the center  
7 of the floor space and preferably in the core area of the building. Additional  
8 communications closets shall be provided when the floor area being served exceeds  
9 10,000 square feet or the horizontal cabling distance to the end devices exceeds 295 feet.  
10 Multiple communications closets on a floor shall be interconnected by a minimum of 2  
11 Trade Size 4 conduits or equivalent pathway.

**28.5.2 Communications Interface Cabinets**

- 12 Provide Communications Interface Cabinets (CICs), which are stand-alone cases that  
13 only house equipment and cabling and are used primarily for providing network  
14 connectivity to end devices. CICs are located indoors as wall-mounted CICs. CICs may  
15 be located outdoors at facilities or located along the Authority's right-of-way and in  
16 tunnel cross-passages. Minimum lateral space provision for CICs of 2'-6" depth shall be  
17 provided at all sections and locations along the right-of-way, thereby allowing a CIC to  
18 be placed at any longitudinal location on both sides of the track. Cable routing  
19 provisions to connect nearest cable trough to CIC shall be provided or not precluded at  
20 all sections and locations along the right-of-way.

- 21 CIC weight and location shall be coordinated with the structural loading specified in the  
22 *Structures* chapter.

- 23 CICs shall be designed to protect enclosed equipment per the criteria described in  
24 Section 28.4.1.4.

**28.5.3 Communications Shelters**

- 25 Communications provides network connectivity for communications and other  
26 disciplines' equipment at various sites throughout the Authority's right-of-way via a  
27 communications shelter. A communications shelter is relatively larger than a CIC and  
28 can accommodate personnel as well as equipment. Communications shelters may be  
29 located at traction power facilities, tunnel portal sites, ATC equipment houses and other  
30 locations. Generally the communications shelter provides a stable environment for radio  
31 system equipment. Communications shelters shall be generally located at-grade.

1 Communications shelters shall be designed to the seismic, structural and physical  
2 design criteria listed in the *Structures* chapter.

3 Communication shelters and equipment cabinets and enclosures shall be access  
4 controlled and remotely monitored at OCC and RCC for intrusion and environmental  
5 status.

#### **28.5.4 Standalone Radio Sites**

6 The ORS must be configured as a secure, multi-functional network to connect wayside  
7 elements with mobile units throughout the geography of operation.

8 Space accommodations shall be provided for a communications shelter and radio tower  
9 at a spacing of approximately 5 miles, accomplished through the co-location of radio  
10 tower within or adjacent to Traction Power Facilities (TPFs) (Substations, Paralleling  
11 Stations, and Switching Stations), ATC equipment houses, and at tunnel portal sites.  
12 Additional stand-alone radio sites (SRS) containing radio towers and supporting  
13 communications shelters shall be placed approximately in between these co-located sites  
14 with a minimum site dimension of 40' x 25'. These SRSs shall be placed only relative to  
15 the TPFs, ATC equipment houses, and tunnel ventilation facilities at tunnel portal sites  
16 such that the linear distance between any 2 radio towers is nominally 2.5 miles and no  
17 greater than 3 miles.

18 If spacing between adjacent TPFs, ATC equipment houses and tunnel portal sites is:

- 19 • Greater than 0 miles and less than or equal to 3 miles – no standalone radio site  
20 required (do nothing)
- 21 • Greater than 3 miles and less than or equal to 6 miles – place a single standalone  
22 radio site equidistant between adjacent sites
- 23 • Greater than 6 miles – multiple standalone radio sites shall be placed between the  
24 existing sites to achieve a uniform spacing no greater than 3 miles between all sites.

25 Standalone radio sites shall not be placed for tunnel sections. Standalone radio sites shall  
26 be easily accessed by maintainers to support MTTR criteria.

27 Access to each stand-alone radio site shall be required both during construction and for  
28 operation and maintenance purposes. SRS access roads may also be used for  
29 maintenance access to the trackway, and for safety egress.

30 Refer to the *Civil* chapter for access road, parking, and gate and fence criteria. Refer to  
31 the *Drainage* chapter for drainage requirements.

32 Stand-alone radio sites shall be designed to minimize the adverse visual impact on the  
33 areas in which they are located and to be in compliance with the appropriate  
34 architectural, environmental and commercial guidelines as specified in the

Environmental Assessment/Final Environmental Impact Report and local jurisdictional ordinances.

Safety signage shall be provided at stand-alone radio sites in accordance with applicable codes and standards.

#### 28.5.5 Radio Towers

Radio towers are located at the sites described in Section 28.5.54 and may also be located at stations and facilities based on coverage needed determined by final design. Generally radio towers shall be located at-grade.

The ORS radio towers shall be maximum 100-foot-tall monopoles or equivalent.

The Designer shall confirm that all site selection, environmental approvals, zoning board approvals, planning commission, Federal Aviation Administration and Federal Communications Commission approvals are complete as part of the design process. The Authority prefers towers not require lighting and marking where possible. If elements are incomplete, unapproved, or non-compliant, the Designer shall bring the radio site to full compliance with all applicable codes. A radio tower should not be placed within 250 feet of overhead power lines.

The Designer shall calculate wind load and ice accumulation to the latest version of Electronics Industry Association Structural Standards for Steel Antenna Tower and Antenna Supporting Structures. As of this writing this is - EIA/TIA-222-E.

To factor in the proper ice loading consideration, the tower designer shall refer to the EIA/TIA-222-F Code appendix. The Designer must consider ice load and wind velocity.

The Designer shall conduct and provide a geological analysis of the soil at the radio tower site. The Designer shall design the tower foundation to perform to the functional and performance specs and soil conditions. The radio tower foundation shall be designed to the seismic, structural and physical design criteria listed in the *Structures* chapter.

The Designer shall determine and incorporate Federal Aviation Administration requirements for marking and lighting the radio tower if required. The lighting shall be remotely monitored via the appropriate SCADA subsystem as discussed in the *Supervisory Control and Data Acquisition Subsystems* chapter.

The Designer shall design a transmission line bridge to support and protect the cable for the horizontal run from the tower to the building entry ports. This cable must be resistant to climatic and field environmental factors and be vandal proof.

### 28.5.6 Systems Conduits at Track

HST Systems (Traction Electrification, Overhead Contact System, Automatic Train Control and Communications) require cable conduits for low-voltage, data and control cables to pass from 1 side of the track to the other at selected wayside sites and facilities. Conduits shall provide cable paths between the cable troughs, surface mounted devices and the wayside site or facility. Depending on the site and section features, these pathways may consist of the following elements:

- At-Grade, embankment, cut, fill – manholes, undertrack conduit ductbank, at-grade trough segments to join the cable troughs
- Retained fill – Manholes and cable ductbank at tracklevel, within the retention walls
- Trench – manholes, underground conduit ductbanks, undertrack conduits, surface-mounted conduits, pullboxes and/or non-structural members carrying conduit above track to end devices and troughs
- Aerial – structure mounted conduit, pullboxes and underground conduit ductbank
- Tunnels – To be determined by Tunnel Design

The Designer shall provide these systems conduits at track at the following locations:

- O&M Facilities
  - Operations Control Center/Heavy Maintenance Facility
  - Regional Control Centers
  - Rolling Stock Maintenance Facilities
  - Maintenance of Infrastructure Facilities
- Stations
  - At the ends of each platform (locations of manholes and ductbanks shall be coordinated with the *Civil* and *Stations* chapters)
- Wayside Sites
  - Traction Power Facilities
  - Train Control Facilities
  - Standalone Radio Sites
- Tunnel Ventilation Facilities

- 1        - Tunnel Ventilation Facilities at Portals
- 2        - Tunnel Ventilation Facilities at Ventilation Shafts

#### 28.5.6.1 At-Grade, Embankment, Cut Systems Conduits at Track

3 The Designer shall provide concrete-encased, undertrack conduit ductbank consisting of  
4 Trade Size 4 conduits terminating at manholes at the locations and quantities shown in  
5 Table 28-3 for at-grade, embankment and cut sections.

6 Manhole covers shall be lockable and be labeled “California High-Speed Rail – Low  
7 Voltage Cabling”

**Table 28-3: Quantities of Undertrack and Underground Conduit at At-Grade and Trench Sites and Locations**

Wayside Site, Location or Facility	Minimum Number of Trade Size 4 Conduits
Traction Power Facility	12
Train Control House	20
Standalone Radio Site	12
Tunnel Ventilation Facilities	12
Operations Control Center / Heavy Maintenance Facility	12 Or Two Banks Of 12*
Regional Control Center	12 Or Two Banks Of 12*
Rolling Stock Maintenance Facility	12 Or Two Banks Of 12*
Maintenance of Infrastructure Facility	12 Or Two Banks Of 12*
Station	Two Banks Of 12**

8 Notes:

9 \* Provide a single 12 conduit ductback for an O&M Facility wholly existing on 1 side of the main line  
10 tracks. In the case that the main line track divides an O&M Facility, install 2 12 conduit undertrack  
11 conduits spaced 1000 feet apart, centered at the facility.

12 \*\* At each end of a station platform, provide an undertrack conduit ductbank containing 12 conduits.

13 Conduit ductbank and manholes shall be designed, manufactured and tested in  
14 accordance with, NEC, NEMA, ASTM and EIA/TIA 758. There shall be a maximum of 2  
15 90 degree (or equivalent) bends in low-voltage systems conduits between pull boxes,  
16 handholes, manholes and cable vaults. Manholes shall be designed to accommodate  
17 connection to cable trough and connection of the same number and size of conduits  
18 within ductbank to wayside site or facility.

19 Conduit ductbank containing 12 conduits shall be arranged 4 conduits wide by 3  
20 conduits high. Conduit ductbank containing 20 conduits shall be arranged 4 conduits  
21 wide by 5 conduits high.

A. Longitudinal Location of Conduit Ductbank and Manholes

1 Locate conduit ductbank and manholes longitudinally within the limits of the site as  
2 shown on the contract drawings. If alternative systems site locations exist at time of  
3 construction locate conduit ductbank and manholes within the limits of each site  
4 alternative.

5 When a relocated roadway separates the Authority's right-of-way and a wayside site or  
6 facility location, additionally install a conduit ductbank of the same configuration  
7 beneath the roadway to a third manhole located outside of the relocated roadway's  
8 right-of-way.

9 Conduit ductbank shall be located at least 2 feet away from other 25kV underground  
10 ductback.

B. Vertical Location of Conduit Ductbank

11 Conduit ductbank shall be located minimum 6 feet below top of rail and when not  
12 underneath rail, conduit ductbank shall be located minimum 3 feet below grade.

C. Horizontal Location of Manholes

13 Manholes shall be located within the Authority's right-of-way or wayside site or facility  
14 land (in the case of providing underground ductbank beneath a relocated roadway.)

D. Vertical Location of Manholes

15 Manhole top surfaces shall nominally be a minimum of 2 inches above grade.

**28.5.6.2 Trench Systems Conduits at Track**

16 At trenches, the Designer shall provide concrete-encased, underground conduit  
17 ductbank, consisting of Trade Size 4 conduits terminating at pullboxes at trench side and  
18 at manholes at the site side at the locations and quantities shown in Table 28-3. At trench  
19 sections, if underground conduit ductbank penetrates trench wall, waterproofing of  
20 trench wall shall be maintained. Refer to the *Structures* chapter for provisions for  
21 installing conduit embedment on site specific walls of earth retaining structures.

**28.5.6.3 Retained Fill Systems Conduits at Track**

22 The Designer shall design the retained fill sections of trackway to accommodate  
23 concrete-encased, undertrack conduit ductbank consisting of Trade Size 4 conduits  
24 terminating at manholes at the locations and quantities shown in Table 28-3. These  
25 manholes shall be at or near track elevation and within the retaining walls. Manholes  
26 shall be designed to accommodate connection to cable trough and connection of the  
27 same number and size of conduits within ductbank to the wayside site or facility. Refer  
28 to the *Structures* chapter for provisions for installing conduit embedment on site specific  
29 columns of site specific walls of earth retaining structures.



#### 28.5.6.4 Aerial Systems Conduits at Track

The Designer shall design the aerial structures to accommodate the surface mounting of conduit and cables, or equivalent, between opposing cable troughs and wayside sites and locations at the quantities and locations shown in Table 28-3. Refer to the *Structures* chapter for provisions for installing conduit embedment on site specific columns of aerial structures.

Underground conduit ductbank design at aerial sections shall be coordinated with underground elements of aerial structures per the *Structures* chapter.

Aerially mounted conduits shall be coordinated with the loading specified in the *Structures* chapter.

#### 28.5.6.5 Tunnel Cable Conduits

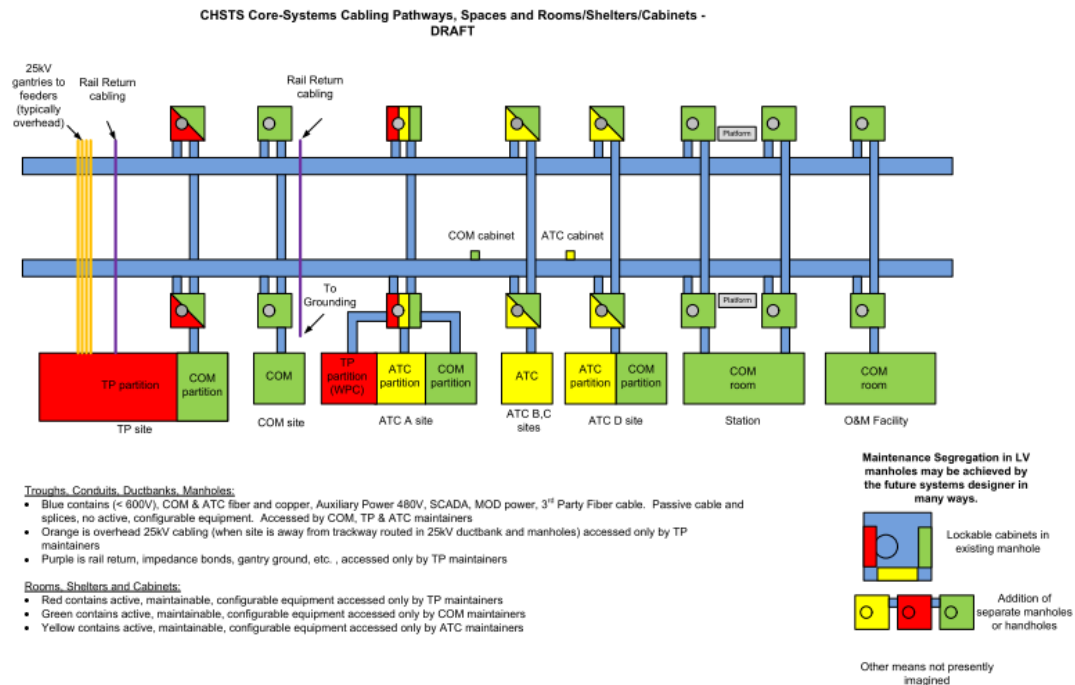
Undertrack conduits at tunnel sections are to be integrated as part of the tunnel design.

#### 28.5.7 Conductor Routing and Segregation

- Cables for 25kV catenary feeder and negative feeder shall not be routed in any common pathway (e.g., conduit, duct, ductbank) or space (e.g., cabinet, handhole, manhole) with cables of other functionality.
- Cables for facility power (i.e., electrical power service for buildings) shall not be routed in any common pathway or space with cables of other functionality.
- Cabling for rail return shall not be routed in any common pathway or space with cables of other functionality.
- Cabling for low voltage services including low voltage auxiliary power (i.e., power for systems wayside facilities and equipment), signaling and train control, SCADA and communications shall not be routed in any common pathway or space with cables of other, higher voltage services.
  - Low voltage cabling in common cabling pathways and spaces shall be segregated to provide maximum feasibility for maintenance regimes such as Traction Power Maintainers, Signaling and Train Control Maintainers, and Communications Maintainers.
    - Cabling to be maintained by separate maintenance regimes shall be routed in separate handholes, cabinets and ducts or single conduits.
    - Cabling to be maintained by separate maintenance regimes routed in common vaults or manholes shall be segregated by maintenance regime and access controlled.
    - If maintenance segregation and access control cannot be achieved within a common low voltage manhole or vault, then the manhole or vault shall be

- 1                   used only as ‘transition or pass-through zones’ for cables and should not be  
2                   used for splices, connections or terminations.
- 3               ○ Low voltage cabling within cable troughs shall be separated by the  
4               maintenance regime classification using dividers, sections or similar
- 5       • Cabling of different voltage ratings or may be routed in the same concrete-encased  
6       ductbank (e.g., medium voltage auxiliary power and low voltage auxiliary power)  
7       but cables of each voltage rating shall be in separate conduits, and adequate  
8       separation shall be maintained between conduits per EMC guidelines. There shall be  
9       not less than 3 inches of concrete between conduits.
- 10   • Cables of different insulation ratings but same functionality shall not be routed in  
11   common ducts, troughs or single conduits. All cables shall be insulated for the  
12   highest insulation rating in the common route.
- 13   • Rail return cables shall be installed in concrete encased ductbanks in the track  
14   subgrade or integrated within the track slab.
- 15   • All splicing and terminating of cabling shall only occur in vaults, manholes, shelters  
16   or cabinets, and shall be segregated and access controlled by maintenance regime.
- 17   Figure 28-4 depicts the conceptual level cable routing for Core Systems in different  
18   equipment rooms, shelters and cabinets for systems equipments.

# 1 Figure 28-4: Conceptual Level Cable Routing for Core Systems



## 28.6 External Services

### 28.6.1 Telecommunications Services

The Designer shall accommodate and design interfaces to the following telecommunications services:

- Voice-grade telephone service at the control centers, stations, maintenance facilities, and yards to allow Authority personnel to make outgoing calls and receive incoming calls from/to selected Authority ATELS
- VPN data service used to interconnect control centers as a backup data communications path in case a control center becomes isolated from the WAN
- Internet Access
- Cellular Telephone Service for management staff

In addition to normal voice-grade telephone service, the Designer shall accommodate and design interfaces to the Government Emergency Telecommunications Service (GETS). GETS is a White House-directed emergency phone service provided by the National Communications System (NCS). GETS supports federal, state, local, and tribal

1 government, industry, and non-governmental organizations personnel in performing  
2 their National Security and Emergency Preparedness (NS/EP) missions. GETS provides  
3 emergency access and priority processing in the local and long distance segments of the  
4 Public Switched Telephone Network. It is intended to be used in an emergency or crisis  
5 situation when the Public Switched Telephone Network is congested and the probability  
6 of completing a call over normal or other alternate telecommunication means has  
7 significantly decreased.

8 If wireless cellular service is required, the Designer shall also design interfaces to the  
9 Wireless Priority Service (WPS), which is another National Communications System-  
10 sponsored program that increases the ability to make calls when cellular networks are  
11 congested.

12 External telecommunications services that shall be interfaced with include Priority  
13 Restoration Level X as defined under the Telecommunications Service Priority (TSP)  
14 program. The TSP program is a federally-established program under which the Office of  
15 Priority Telecommunications in the Executive Office of the President prioritizes the  
16 restoration and provisioning of telecommunications services—including services to  
17 private companies and institutions—that support NS/EP. Priority Restoration  
18 designation establishes priorities for restoring NS/EP service in the event of an outage or  
19 failure of multiple services. According to the Priority Restoration service level, the  
20 telecommunications service provider will dispatch personnel outside normal business  
21 hours if necessary to restore circuit(s) (and provide any related special construction)  
22 assigned a Priority Restoration level of 1, 2, or 3. The telecommunications service  
23 provider will dispatch personnel outside normal business hours to restore circuits (and  
24 provide any related special construction) assigned a Priority Restoration level of 4 or 5  
25 only when the next business day is more than 24 hours away.

#### **28.6.1.1 Backup Virtual Private Network Connections**

26 A VPN telecommunications service shall be interfaced to and used to interconnect  
27 control centers to provide a backup communications path allowing communications  
28 between control centers in the event that any control center loses network connectivity  
29 via their primary network path over the WAN. Consideration shall be given to  
30 telecommunications services that allow for dynamic bandwidth provisioning, where the  
31 Authority can request the service provider to increase the provided bandwidth when the  
32 primary communications path via the WAN is not available.

#### **28.6.2 External Notification Services**

33 The Designer shall integrate into the IIMPs the following emergency notification  
34 services on behalf of the Authority:

- 35 • Commercial Mobile Alert System
- 36 • National Weather Service

- 1 • Emergency Managers Weather Information Network
- 2 • Naval Meteorology and Oceanography

### **28.6.3 Infotainment**

- 3 The Authority ultimately may facilitate onboard broadband information and
- 4 entertainment (infotainment) access to passengers via a data radio service separate from
- 5 the Authority radio systems described previously. These infotainment services may be
- 6 provided by an independent franchisee. The Designer shall not preclude the addition of
- 7 this service.



## Chapter 29

### Rolling Stock–Core Systems Interfaces

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	Oct 2013	Rolling Stock Procurement
2	Feb 2014	Revision 2





## Table of Contents

<b>29</b>	<b>ROLLING STOCK – CORE SYSTEMS INTERFACES</b>	<b>29-1</b>
29.1	Scope	29-1
29.2	Regulations, Codes, Standards, and Guidelines	29-1
29.3	General Interface Design Requirements	29-3
29.4	Traction Power Interfaces	29-3
29.4.1	Purpose	29-3
29.4.2	Power Supply	29-3
29.4.3	Train Power and Current Limitation	29-4
29.4.4	Regenerative Braking	29-4
29.4.5	Power Factor	29-5
29.4.6	Electrical Protection Coordination	29-5
29.4.7	Overvoltages Generated by Harmonics	29-6
29.5	Overhead Contact System Interfaces	29-6
29.6	Automatic Train Control Interfaces	29-6
29.6.1	Purpose	29-6
29.6.2	General Requirements	29-6
29.6.3	Interface Requirements	29-7
29.6.4	Automatic Train Control Technology Information	29-8
29.6.5	Rolling Stock Interface Requirements to Onboard Automatic Train Control	29-8
29.6.6	Safe Braking Design	29-9
29.6.7	Rolling Stock Parameters	29-10
29.6.8	Environmental	29-11
29.6.9	Mechanical Interfaces	29-11
29.6.10	Electrical / Functional Interfaces	29-14
29.6.11	Commands from Automatic Train Supervision to Rolling Stock	29-18
29.6.12	Train Subsystems Interface	29-18
29.6.13	Track Circuit – Rolling Stock Electromagnetic Compatibility	29-18
29.6.14	Rolling Stock - Electromagnetic Field Exposure	29-19
29.7	Communications Interfaces	29-19
29.7.1	Interface Requirements	29-20

## Tables

Table 29-1:	Power Factor of Trains	29-6
-------------	------------------------	------

## Figures

Figure 29-1:	Open Systems Interconnection Reference Model.....	29-21
Figure 29-2:	Wayside Communications and Onboard Systems Interface Diagram .....	29-22
Figure 29-3:	Example Protocol Stack and Interfaces .....	29-22

## Acronyms

AREMA C&S	American Railway Engineering and Maintenance-of-Way Association Communications and Signals
ATC	Automatic Train Control
ATO	Automatic Train Operation
ATS	Automatic Train Supervision
CHST	<i>Not Used; use HST for California High-Speed Train</i>
CHSTP	California High-Speed Train Project
EIRENE	European Integrated Railway Radio Enhanced Network
EMC	Electromagnetic Compatibility
EMF	Electromagnetic Field
ERTMS	European Rail Traffic Management System
HST	High-Speed Train
ICD	Interface Control Documents
IEC	International Electrotechnical Commission
IEEE	Institute of Electrical and Electronics Engineers
IIMP	Integrated Information Management Platforms
OBS	Onboard Systems
OCC	Operation Control Center
OCS	Overhead Contact System
RAMS	Reliability, Availability, Maintainability, and Safety
RS	Rolling Stock
SS	Substation
TES	Traction Electrification System
TPS	Traction Power Supply System
UIC	International Union of Railways
WCS	Wireless Communications Systems
$U_m$	mean useful voltage
$U_n$	nominal voltage
$U_{max1}$	highest permanent voltage
$U_{max2}$	highest non-permanent voltage
$U_{min1}$	lowest permanent voltage
$U_{min2}$	lowest non-permanent voltage



## 29 Rolling Stock – Core Systems Interfaces

### 29.1 Scope

The passenger Rolling Stock (RS) shall interface with the core systems consisting of Traction Power, Overhead Contact System (OCS), Automatic Train Control (ATC), and Communications. This chapter addresses these core systems interfaces only. The design criteria for passenger RS interfaces with infrastructure including clearances, wheel/rail interaction, structure loading requirements, noise, shock, and vibration are described in the Next Generation Amtrak/CHSRA Trainsets Performance Specification.

The design criteria for passenger RS interfaces shall cover the functional, mechanical, electrical, and logical interface requirements between the passenger RS and the core systems that allow the California High-Speed Train (HST) System to meet the functional and performance requirements of the project, including Reliability, Availability, Maintainability, and Safety (RAMS). RAMS requirements are described in the RAM chapter and the specific discipline chapters of these design criteria and in the Next Generation Amtrak/CHSRA Trainsets Performance Specification.

### 29.2 Regulations, Codes, Standards, and Guidelines

Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.

- International Union of Railways (UIC) Code
  - UIC 612-0 – Driver Machine Interfaces for EMU/DMU, Locomotives, and Driving Coaches – Functional and system requirements associated with harmonized Driver Machine Interfaces
- American Railway Engineering and Maintenance-of-Way Association Communications and Signals (AREMA C&S) Manual of Recommended Practices
  - Part 17 – Recommended
- U.S. Department of Defense (USDOD) Standards
  - MIL-HDBK-217F – Military handbook – Reliability Prediction of Electronic Equipment
  - MIL-STD-785B – Reliability Program for System and Equipment Development and Production
  - MIL-STD-756B – Reliability Modeling and Prediction
  - MIL-STD-1629A – Procedures for Performing a Failure Mode, Effects and Criticality Analysis
- European Committee for Electrotechnical Standardization (CENELEC) Standards

- EN 50121-3-1 – Railway applications - Electromagnetic compatibility. Rolling stock. Train and complete vehicle
- EN 50121-3-2 – Railway applications - Electromagnetic compatibility. Rolling stock. Apparatus
- EN 50121-5 – Railway applications - Electromagnetic compatibility. Emission and immunity of fixed power supply installations and apparatus.
- EN 50121-4 – Railway applications – Electromagnetic compatibility – Part 4: Emission and immunity of the signaling and telecommunications apparatus
- EN 50125-1 – Railway applications – Environmental conditions for equipment – Part 1: Equipment on-board rolling stock
- EN 50125-3 – Railway applications – Environmental conditions for equipment – Part 3: Equipment for signalling and telecommunications
- EN 50126-1 – Railway applications – The specification and demonstration of Reliability, Availability, Maintainability and Safety – Part 1: Basic requirements and generic process (1999)
- CLC/TR 50126-2 Railway applications – The specification and demonstration of Reliability, Availability, Maintainability and Safety (RAMS) – Part 2: Guide to the application of EN 50126-1 for safety (2007)
- EN 50128 – Railway applications – Communications, signalling and processing systems – Software for railway control and protection systems (2010)
- EN 50129 – Railway applications – Communications, signalling and processing systems – Safety related electronic systems for signalling (2003)
- EN 50159 – Railway applications – Communications, signalling and processing systems – Safety-related communication in transmission systems (2010)
- EN 50238 - Railway applications - Compatibility between rolling stock and train detection systems (2003)
- EN 50388:2005 – Railway applications – Power supply and rolling stock. Technical criteria for the coordination between power supply (substation) and rolling stock to achieve interoperability
- International Electrotechnical Commission (IEC) Standards
  - IEC 60812 – Analysis techniques for system reliability – Procedures for failure mode and effect analysis
- European Rail Traffic Management System (ERTMS) Specifications
  - EEIG 96S126 – ERTMS/ETCS RAMS Requirements Specification
- Institute of Electrical and Electronics Engineers (IEEE) Standards

- IEEE 519 - IEEE recommended Practices and Requirements for Harmonic Control in Electrical Power Systems
- IEEE 1558 – IEEE Standard for Software Documentation for Rail Equipment and Systems
- ANSI/IEEE 1028 – IEEE Standard for Software Reviews – The Institute of Electrical and Electronics Engineers
- IEEE 1003 – IEEE Standard for Information Technology – Portable Operating System Interface (POSIX)
- IEEE 1008 – IEEE Standard for Software Unit Testing
- IEEE 1228 – IEEE Standard for Software Safety Plans
- Federal Transit Administration (FTA)
  - UMTA-MA-06-0153-87-2 – Conductive Interference in Rapid Transit Signaling Systems Volume II: Suggested Test Procedures
  - UMTA-MA-06-0153-85-8 – Inductive Interference in Rapid Transit Signaling Systems Volume II: Suggested Test Procedures
  - UMTA-MA-06-0153-85-11 – Radiated Interference in Rapid Transit Signaling Systems Volume II: Suggested Test Procedures

### 29.3 General Interface Design Requirements

The design of interfaces between the passenger RS and core systems shall be reflected in the individual systems and subsystems designs and shall be recorded in a series of Interface Control Documents (ICDs), per the CHSTS Interface Management Plan.

### 29.4 Traction Power Interfaces

#### 29.4.1 Purpose

The purpose is to define the Traction Power Supply System (TPS) interface requirements with the passenger rolling stock (RS).

#### 29.4.2 Power Supply

- System Voltage

Refer to the *Traction Power Supply System* chapter Sections 20.6.1 *System Voltage* and 20.6.2 *Voltage Related Requirements* for the range of system voltages under which the RS shall function normally.
- Frequency

1 Refer to the *Traction Power Supply System* chapter Section 20.6.3 *Frequency* for the range of  
2 system frequencies under which the RS shall function normally.

### 29.4.3 Train Power and Current Limitation

3 The maximum allowable train current including auxiliaries, for both traction and regeneration  
4 modes, shall be equal to the maximum current capacity of the Traction Electrification System  
5 (TES), which is 1500 amperes (refer to Section 7 of European Standard 50388).

6 In order to facilitate stable operation in abnormal operating conditions, trains shall be equipped  
7 with an automatic device that adapts the level of the power consumption depending upon OCS  
8 voltage in steady state conditions as explained below:

9 If  $U$  = OCS voltage at any instance in kV,

10  $I_{\max}$  = maximum current consumed by the train at 25 kV,

11  $I_{\text{aux}}$  = current consumed by the auxiliaries of the train, and

12  $I$  = current drawn by the train at voltage  $U$ , then

$$I = I_{\text{aux}} + (I_{\max} - I_{\text{aux}}) * (U - 17.5) / 22.5 - 17.5 \text{ for } 17.5 \text{ kV} \leq U \leq 22.5 \text{ kV}$$

14 and

$$I = I_{\max} \text{ for } 22.5 \text{ kV} < U \leq 29 \text{ kV}.$$

16 This equation does not apply in regenerative braking mode.

### 29.4.4 Regenerative Braking

17 General conditions on the use of Regenerative Brakes include the following:

- 18 • Trains shall not continue to use their regenerative brakes if the following occurs (also refer  
19 to Section 12 of EN 50388):
  - 20 - There is a loss of supply voltage or an OCS-rail/ground short-circuit on the same section  
21 fed by the substation
  - 22 - The OCS fails to absorb the energy
  - 23 - The line voltage is higher than 29 kV; under these conditions trains may continue to use  
24 regenerative braking to supply energy to their auxiliary loads, the remaining energy  
25 shall be addressed by the onboard power management system.
- 26 • If feedback energy absorption by other consumers is not available, the RS shall revert to  
27 other brake systems.
- 28 • TPS shall be designed in such a way that regenerative braking can be used as a service brake  
29 and as an emergency brake.



- Regenerative braking mode shall be permitted from the beginning of train operations.

#### 29.4.5 Power Factor

The RS shall conform to the following power factor requirements:

- Inductive Power Factor

Table 29-1 gives the requirements of inductive power factor,  $\lambda$ , of any train. (Refer also to Section 6 of EN 50388):

**Table 29-1: Power Factor of Trains**

Instantaneous Train Power 'P' at the Pantograph (in MW)	$\lambda$
$P > 6$	$\geq 0.95$
$2 < P \leq 6$	$\geq 0.93$

In order to control the total power factor of the auxiliary load of a train during the coasting phases, the overall average ' $\lambda$ ' (traction plus auxiliaries) shall be higher than 0.85 over a complete time table journey.

For yards and maintenance facilities, when a train is stationary, with traction power off, and the active power taken from the OCS is greater than 10 kW per vehicle, the total power factor resulting from the train load shall not be less than 0.8 but with a target value of 0.9.

During regeneration, inductive power factor may be allowed to decrease freely in order to keep voltage within limits.

- Capacitive Power Factor

During traction mode, capacitive power factor is allowed in order to keep voltage within limits.

- Within the range of voltage from 19.0 kV and 27.5 kV, capacitive power factors are not limited

- Within the range of voltage from 27.5 kV and 29.0 kV a train shall not behave like a capacitor

#### 29.4.6 Electrical Protection Coordination

Refer to the *Traction Power Supply System* chapter Section 20.7.5 *Electrical Protection Coordination* for details of the electric protection coordination between RS and traction power supply system.

- Conformity assessment shall be carried out for design and operation of substations and traction units (RS) in accordance with EN 50388:2005, clause 14.6.

#### 29.4.7 Overvoltages Generated by Harmonics

- 1 The acceptable impacts of harmonics on the system are detailed in the *Traction Power Supply*  
2 *System* chapter *Section 20.7.7 Harmonic Distortion Limit*.

### 29.5 Overhead Contact System Interfaces

- 3 For the technical criteria regarding the design of the OCS, from which the RS pantograph must  
4 collect power, refer to the *Overhead Contact System and Traction Power Return System* chapter  
5 *Section 21.6 Overhead Contact Line Design*.
- 6 The requirements for the very critical dynamic interaction between the passenger RS  
7 pantograph and the overhead contact line are detailed in the *Overhead Contact System and*  
8 *Traction Power Return System* chapter *Section 21.10 Dynamic Behavior and Quality of Current*  
9 *Collection*.
- 10 The requirements for the spacing between multiple in service pantographs and the design of the  
11 phase breaks in the overhead contact line are detailed in the *Overhead Contact System and*  
12 *Traction Power Return System* chapter *Section 21.12.1 Pantograph Spacing for Design of the Overhead*  
13 *Contact Line* and *Section 21.12.2 Phase Breaks* respectively.

### 29.6 Automatic Train Control Interfaces

#### 29.6.1 Purpose

- 14 The purpose is to define the ATC interface requirements with the passenger RS.

#### 29.6.2 General Requirements

- 15 This document defines the minimum ATC interface requirements that shall be provided with  
16 the RS.
- 17 • Interfaces are divided into 4 general categories: mechanical, electrical, functional, and  
18 electromagnetic:
- 19 - The mechanical interfaces cover space provisions (lockers) and mounting points  
20 (including brackets).
- 21 - The electrical interfaces cover wiring and cabling, grounding and bonding, as well as  
22 power and voltage requirements.
- 23 - The functional interfaces describe protocols for specific functions and data exchanges  
24 required at the RS subsystems interfaces to ensure that the overall system functional  
25 requirements are met.

- Electromagnetic interfaces cover design provisions to ensure that RS conductive, inductive, and radiated emissions do not interfere with the safe and dependable operation of the ATC equipment, particularly track circuits and transponders.

### 29.6.3 Interface Requirements

#### 29.6.3.1 Interface Requirements of the Rolling Stock Contractor

The RS Contractor shall perform the following:

- Provide physical and functional interfaces to the RS equipment to allow the ATC equipment to perform ATP, ATS, ATO, berthing/docking, and other functions
- Work with the ATC Contractor to establish RS conductive, inductive, and radiated emission limits, which ensure the safe and dependable operation of the ATC equipment under normal and worst-case failure conditions
- Exchange required protocols (e.g., operational data, transfer functions, tolerances, failure modes, and any other data necessary) with the ATC Contractor for evaluating safe braking distances and implementing correct functional operation
- Provide space and attachment points for all RS-mounted ATC equipment; space shall include sufficient volume of unoccupied space to allow for equipment temperature considerations
- Provide compartments with the required environment for all interior-mounted ATC equipment
- Provide interconnecting conduit, wiring, terminal strips, connectors, trainlines, and related wiring items to both interconnect ATC equipment sets and connect the RS with the ATC equipment
- Participate in RS static and dynamic investigative, qualification, and acceptance tests (i.e., participate in the development of procedures, participate in the tests, and participate in the development of the test reports that demonstrate that the interfaces function properly and that the RS performs properly with the integrated systems)
- Develop and maintain the detailed ATC-RS interface information in a controlled document format compliant with the CHSTS Interface Management Plan
- Prepare test/inspection plans and perform test/inspection procedures for type testing, modeling, verifying, and validating that the ATC-RS interface requirements are met prior to the RS delivery

#### 29.6.3.2 Interface Requirements of the Automatic Train Control Contractor

The ATC Contractor shall perform the following:

- Provide physical and functional interfaces in the ATC equipment to allow the RS to operate under ATP, ATS, ATO, and other functions

- 1 • Work with the RS Contractor to establish RS conductive, inductive, and radiated emission  
2 limits, which ensure the safe and dependable operation of the ATC equipment under  
3 normal and worst-case failure conditions
- 4 • Provide space and attachment point requirements for all RS-mounted ATC equipment.  
5 Identify the volume of unoccupied space needed to allow for equipment temperature  
6 considerations
- 7 • Identify the required environment for all interior-mounted ATC equipment
- 8 • Provide interconnecting terminal strips, connectors, trainlines, and related wiring items to  
9 both interconnect ATC equipment sets and connect the RS with the ATC equipment
- 10 • Participate in RS static and dynamic investigative, qualification, and acceptance tests (i.e.,  
11 participate in the development of procedures, participate in the tests, and participate in the  
12 development of the test reports that demonstrate that the interfaces function properly and  
13 that the RS performs properly with the integrated systems)
- 14 • Develop and maintain the detailed ATC-RS interface information in a controlled document  
15 format compliant with CHSTS Interface Management Plan
- 16 • Prepare test/inspection plans and perform test/inspection procedures for type testing,  
17 modeling, verifying, and validating that the ATC-RS interface requirements are met prior to  
18 the RS delivery

#### **29.6.4 Automatic Train Control Technology Information**

19 The onboard requirements are defined in the Signal and Control and Infrastructure Interface  
20 Specification sections of the Next Generation Amtrak/CHSRA Trainsets Performance  
21 Specification, and shall generally consist of a set of vital processor equipment, data  
22 communications equipment for networking around the train on ATC dedicated networks and  
23 for interfacing to the train control networks, data radios and antennas, speed sensors, track  
24 circuit code pick-up antennas, and transponder (balise) reading antennas. GPS receivers may  
25 also be provided by the ATC contractor. Voice and other data radios for communications  
26 between operations and maintenance personnel may be provided by the ATC contractor if  
27 ERTMS, which uses the European Integrated Railway Radio Enhanced Network (EIRENE)  
28 specifications, is supplied with the train control system. Alternatively, voice and other data  
29 radios may also be provided by the communications contractor if the voice and other data  
30 transmission are not integrated with the ATC system.

#### **29.6.5 Rolling Stock Interface Requirements to Onboard Automatic Train Control**

31 The RS contractor shall provide terminal blocks in an ATC terminal panel adjacent to the ATC  
32 enclosure and install wires that connect from this terminal block to all the ATC system  
33 peripherals including antennas, sensors, ATC specific trainlines, and RS subsystems including  
34 networks. The ATC and the RS contractors shall determine the applicable wire sizes, types,  
35 numbers, spares, placement, and specifications. All ATC specific trainlines shall be wired to the

ATC terminal panel by the RS contractor. The RS contractor shall provide wiring and make connections to the ATC terminal panel for all functions necessary to control the train. This shall include trainlines and local wiring. Any specialty/custom connectors and wiring shall be provided by the ATC contractor.

The functions that need to be controlled and/or sensed by ATC include the following:

- Propulsion
- Brakes
- Doors
- Low voltage power
- Train event recorder
- Train health monitoring and maintenance diagnostic subsystem
- Opening and closing of main circuit breaker for phase break operations
- Pantograph up and down controls
- Air vent controls
- Flange lubrication controls
- Onboard system abnormality detectors including vehicle instability, hot bearings, etc.

Switching of connections between non-ATC and ATC control modes shall be achieved within the ATC enclosure. In order to allow this to be done, the RS contractor shall connect both the control wiring to the train subsystems (normally trainlines) and the manual control wiring for these functions, to the ATC termination panel. Prior to the installation of the ATC equipment, appropriate jumpers shall be installed by the RS Contractor at the ATC terminal blocks to ensure reliable connections when the RS is test run prior to ATC installation.

An additional 10 percent spare number of terminals shall be provided.

#### **29.6.6 Safe Braking Design**

The RS contractor shall provide the following information in a timely manner:

- Service brake rates
- Urgency (Full service) and emergency brake rates
- Brake effort build up times
- Maximum acceleration of the train
- Single point brake system failure modes and other impacts on emergency brake rates

### 29.6.7 Rolling Stock Parameters

The RS contractor shall supply the following core set of data (nominal, minimum, and maximum characteristic values will be required):

- Complete tractive effort characteristics as a function of speed and load for motoring and braking. These characteristics shall take into consideration distance, gradients, degraded conditions, etc.
- Complete acceleration characteristics corresponding to the tractive effort curves
- Response time from penalty brake applications to traction power removal
- Response time from non-penalty brake applications to traction power removal
- Response time from emergency brake initiation to full emergency brake application and a failsafe propulsion removal (e.g., activation of the power cut-off switch [PCS])
- Response time from urgency brake initiation to full urgency brake application
- Response times from ATC command change to tractive effort change
- Rate of tractive effort changes under all conditions
- Transfer function from ATC command to tractive effort output
- Requirements and limitations for signals transmitted across couplers
- Wheel spin/slide system operational characteristics in normal and degraded mode
- Percentage brake rate reduction resulting from brake system failures – This calculation shall be performed as a probabilistic risk assessment for critical circuits and systems, including brake component failures, in order to assess what braking rate should be used in the safe braking analysis.
- Selected degraded, cut-out, and failure mode information for other circuits and systems
  - Propulsion and braking test data, including tests under both normal and low adhesion conditions)
  - Eddy current brake performance data including Failure Mode, Effects, and Criticality Analysis information, if installed on the trainsets
  - Electromagnetic compatibility (EMC) characteristics and parameters
  - Design information for speed and acceleration limiting functions to protect end-of-line approaches
  - Thermal characteristics and requirements of RS equipment to comply with trainset duty cycles
- Low adhesion conditions compounded by gradients and degraded/cut-out/failed equipment that impact all brake rates shall be used in the ATC design. These variables and the

1 associated brake rates and distances shall be defined and agreed upon between the RS and  
2 the ATC contractors.

3 Minimum mean deceleration values are defined in the Braking section of the Next Generation  
4 Amtrak/CHSRA Trainsets Performance Specification.

#### 29.6.8 Environmental

5 Onboard ATC equipment inside cubicles shall be functional within the temperature range of  
6 25C to 75C.

#### 29.6.9 Mechanical Interfaces

7 Sufficient space/volume shall be provided to accommodate the mechanical dimensions,  
8 including provisions for air cooling circulation of ATC equipment.

##### A. ATC Control and Radio Equipment

9 **ATC Electronics** – Provide a securable locker at each end of the train, adjacent to the cabs for  
10 the main ATC electronics unit. The locker shall include space for all electronics and other  
11 equipment, an ATC terminal panel allowing ATC supplied harnesses to be connected to the  
12 ATC terminal panel and to be plugged into the ATC equipment, and sufficient volume for air  
13 circulation. The RS contractor shall cable between the RS subsystems and the ATC terminal  
14 panel.

15 The ATC interface specifications shall identify the required dimensions of the locker.

16 The ATC radio equipment shall be co-located in the ATC locker with the ATC electronics unit.

17 The ATC interface specifications shall identify a weight to be assumed for each of the ATC  
18 equipment sets.

19 A space on the floor of the ATC equipment locker of a minimum 6 inches by 12 inches with  
20 clearance height of 4 inches immediately below or adjacent to the ATC volume for mounting an  
21 ATC accelerometer shall be provided along the longitudinal. The 12-inch dimension shall be  
22 parallel to the train side.

23 **ATC Communications Interface Unit** – Provide a securable locker at each end of the train,  
24 adjacent to the ATC equipment locker for ATC communications interfaces equipment. The  
25 locker shall include space for all electronics and other equipment, a terminal panel allowing  
26 ATC supplied harnesses to be connected to the ATC communications interface terminal panel  
27 and to be plugged into the ATC communications interface equipment, and sufficient volume for  
28 air circulation. The RS contractor shall cable between the RS subsystems and the ATC  
29 communications interface terminal panel.

30 The ATC interface specifications shall identify the dimensions of the locker.

1 Assume a minimum of 16 pounds for each set of ATC communications interface equipment.

2 This locker shall be within a maximum of 10 feet cable distance from the main ATC enclosure.

3 **Mounting Arrangements** – The RS Contractor shall provide floor mounting points for the ATC  
4 equipment rack to be mounted on.

5 **Maintenance Access** – Provide access to the allocated space for the ATC equipment for front  
6 and rear access for easy installation, test, and subsequent maintenance. Provide access to the  
7 cable termination and relay areas.

8 **Bonding** – Provide a train ground bonding point and termination to allow the ATC contractor  
9 to bond the equipment frame (rack) to the train ground.

10 **Heat Dissipation and Thermal Interfaces** – Provide sufficient natural ventilation capability  
11 such that the ATC equipment when outputting 500 W will perform within specifications in an  
12 inside cubicle temperature range of -25 °C [-13 °F] to +75 °C [167 °F].

13 **ATC Console Controls** – Provide space on the console for the installation of a control panel  
14 containing a set of ATC control switches, an audible alarm, and associated wiring access and  
15 terminations. The panel location shall be readily accessible to a seated Locomotive Engineer,  
16 when facing forward. The placement of devices and equipment shall be agreed upon by the  
17 Authority's representative and the RS and the ATC contractors during the design phases and  
18 cab mock-up. The volume provision for the ATC controls panel shall be identified in the ATC  
19 interface specifications. The ATC and the RS contractors shall determine the applicable wire  
20 sizes, types, numbers, spares, placement, and specifications.

21 The ATC Contractor shall assemble the panel with the required number and type of switches,  
22 etc. for mode selection and other ATC functions for the Locomotive Engineer's input and  
23 acknowledgement.

24 The Locomotive Engineer's console has been described in the Cab and Trainset Exhibits and  
25 Mock-Ups sections of the Next Generation Amtrak/CHSRA Trainsets Performance  
26 Specification, and shall be designed in compliance with UIC 612 – *Driver Machine Interfaces for*  
27 *EMU/DMU, Locomotives and Driving Coaches – Functional and System Requirements Associated with*  
28 *Harmonized Driver Machine Interfaces*.

29 **ATC Display** – Make provision to mount an ATC display in a location where the Locomotive  
30 Engineer can readily see this display, while looking forward, from a seated position. The ATC  
31 display shall be supplied by the ATC contractor and be integrated into the Locomotive  
32 Engineer's console in accordance with UIC 612.

33 Cable and wiring details shall be identified in the ATC-RS interface specifications.

34 Provision shall be made for the ATC contractor to install a sealed FRA-approved bypass switch  
35 in the cab.



**B. Speed Sensors**

**Locations** – Provide for a minimum of 2 speed sensor positions per trainset, preferably on non-powered axles and not on the leading bogies (relative to trainset direction), for ATC. These speed sensors must be dedicated to ATC and shall preferably be located close to each of the ATC cabinets.

Should Doppler radar devices be needed, the ATC and the RS contractors shall determine the number required and the mounting location.

**Wiring** – The RS contractor shall provide cabling and wiring between the sensor points and the ATC cabinet terminal block. Specialty/custom connectors and wiring shall be provided by the ATC contractor.

**C. ATC Data Radio Antenna**

**Locations** – Provide a location for an antenna mounting preferably on the roof of the cab car at each end of the trainset above the location of the ATC electronics enclosure.

**Dimensions** – The exterior dimensions of the antenna shall be identified by the ATC contractor and shall be compliant with the rolling stock/infrastructure clearance requirements. The antenna shall be mounted longitudinally on the roof. A support bracket shall be used to mount the antenna on the car.

**Mounting Arrangements** – The RS contractor shall provide the mounting point to allow the ATC contractor to secure the bracket to the roof. The ATC antenna mount can be adapted to a design of the attachment points provided by the RS contractor.

**Weight** – The combined weight of the antenna and bracket shall be identified in the ATC interface specifications.

**Maintenance Access** – Provide access to the space above the ceiling for installation of the antenna, running and terminating a large coaxial cable, and access for long term maintenance.

**Wiring** – The RS contractor shall provide cabling and wiring between the antenna points and the ATC cabinet terminal block. Any specialty/custom connectors and wiring shall be provided by the ATC contractor. The cable requirements shall be defined in the ATC interface specifications.

**D. ATC Transponder (Balise) Interrogator Antenna**

**Locations** – The RS contractor shall provide a mounting point for a transponder interrogator antenna. The minimum distance required between the antenna and any propulsion related equipment, including cables and motors, and bottom carbody covers and panels shall be identified in the ATC interface specifications. The antennas and its mounts shall be accessible and all adjacent panels and covers shall be permanently bonded.

1 Below the antenna, a clear, material free(metal and otherwise) path is required from the bottom  
2 face of the antenna to the track bed in a square cone expanding out from the bottom of the  
3 antenna at an angle of 30 degrees in all 4 directions.

4 The cable requirements between the antenna location and the ATC locker terminal panels shall  
5 be defined in the ATC interface specifications.

#### E. ATC Cab Signal Antennas

6 **Locations** – The RS contractor shall provide mounting points for 2 antennas, 1 in front of each  
7 wheel on the number 1 axle on each cab car. The mounting points shall be protected to give  
8 physical protection to the antennas from loose objects on the ballast and ties. Clearances  
9 required between the ATC cab signal antennas and any propulsion related equipment,  
10 including cables and motors shall be identified in the ATC interface specifications.

11 Below the antenna a clear, material free (metal and otherwise) path is required from the bottom  
12 face of the antenna to the track bed in a square cone expanding out from the bottom of the  
13 antenna at an angle of 30 degrees in all 4 directions.

14 The cable requirements between the antenna locations and the ATC locker terminal panels shall  
15 be identified in the ATC interface specifications.

16 **Bonding** – The RS contractor shall provide a bonding termination adjacent to the antenna  
17 mounting position for the ATC contractor to connect to.

### **29.6.10 Electrical / Functional Interfaces**

#### A. Bypass Mode

18 Provide for an ATC Bypass (or cutout) mode in which there is no ATC supervision or  
19 enforcement. The RS subsystem shall enforce a speed limit of 59 miles per hour (mph) on train  
20 operation in this mode.

21 Trainset speed shall be displayed at all times, even during ATC Bypass.

#### B. Network and Discrete Trainlines

22 The following minimum trainline provisions shall be provided in the RS design for ATC  
23 functions:

- 24 • Bypass – 2 discrete
- 25 • Twisted Shielded Pairs (TSPs) – 4; for ATC communication – 2; and radio network – 2
- 26 • Mode selection – a total of 6
- 27 • Master Controller status – cab active – 2 vital lines to ATC
- 28 • ATO start – 2

- Door enable – Correct side door enable/supervision will be a function of ATC and the passenger RS contractor shall provide additional trainlines for this function. In addition to the 2 already allocated for the car, an additional 2 shall be provided for vital side control.

- Provision – 4 trainlines for function transfer from cab to cab

C. Power Supply Interfaces

Provide power and associated circuit protection to each ATC cabinet as specified by the ATC contractor.

D. Interfaces to Operators Console

Provide routing between the operator's console and the local ATC cabinet based on the agreed-upon wire sizes, types, numbers, spares, placement, and specifications. If there is an additional RS supplied speedometer installed that is not part of the ATC onboard, it shall be inhibited when the train is operating in ATC, ATC-ATO, and Restricted Manual modes.

E. Master Controller Key-Up

Redundant inputs shall be provided to the ATC indicating that the local cab is the controlling cab.

F. Master Controller Position

Provide a logical input to ATC that indicates that the Master controller is in the Service Brake, Urgency Brake, or Emergency Brake position, at a minimum. The propulsion brake reference (PBR) signal shall be provided to the ATC system for Master Controller Position.

G. Mode Selection Interfaces

The ATO mode selection switches shall be provided by the ATC contractor and mounted on the console.

H. ATO Start Pushbuttons

The ATO start buttons shall be provided by the ATC contractor and mounted on the console. These switches shall be connected to the ATC cabinet by means of cables installed by the RS contractor.

I. Emergency and Urgency Brake Interfaces

Urgency brake commands initiated by the Master controller or an urgency brake device shall be retrievable by the operator unless ATC deems the trainset to be in an unsafe condition.

Provide an ATC interface to the emergency brakes such that ATC can hold off the applications of emergency brakes when operating in an ATC mode, in some operational scenarios, such as passenger initiated and the Locomotive Engineer confirmed application of emergency brakes (e.g., due to fire on train). This is to enable the ATC onboard to assist the Locomotive Engineer in avoiding the trainset stopping in the no-stopping zones, such as phase breaks, tunnels, long aerial structures, etc., in case of an emergency brakes application.

J. In Bypass mode, the RS shall prevent ATC from applying the emergency brakes. The maximum speed of 59 mph shall be enforced by the RS subsystem  
Service Brake Interfaces

1 Provide a logical (network) interface to ATC that will enable the train subsystems to respond to  
2 braking commands from ATC. The train shall respond to ATC braking commands if they are of  
3 higher demand than currently being requested by the master controller (operator input). The  
4 braking system shall also be capable of sending status to the ATC.

K. Propulsion System Interfaces

5 Provide a logical (network) interface to ATC that will respond to tractive effort commands from  
6 ATC when operating in ATO.

7 The train shall also provide to ATC an input when the master controller is in the brake position.  
8 The train shall respond to ATC braking commands if the train detects that ATC is operating in  
9 ATO, the train shall recognize the status of the ATO start trainline to indicate that ATO is  
10 operational.

11 ATC shall be capable of requesting the cut-off of propulsion.

L. Train Door System Interfaces

12 Provide for both opening and closing, side enable lines that are redundant to enable ATC to  
13 treat them vitally. The requirement is to provide 2 trainlines for each side that shall be operated  
14 by ATC to ensure that the doors can only be opened on that specific side when the train is  
15 under ATC control in ATC territory and during docking.

M. Event and Incident Detectors

16 The RS shall provide input to the onboard ATC equipment for events and incidents occurring  
17 onboard that the ATC shall respond to and/or transmit an alarm/event announcement to the  
18 ATC-ATS. The details of specific responses shall be determined during the Contractor's design  
19 phase. The requirements for the event recorder and ATC data recorder have been identified in  
20 the On-Train Monitor and Diagnostics System section of the Next Generation Amtrak/CHSRA  
21 Trainsets Performance Specification.

22 The interfaces provided shall include the following:

- 23 • Hot bearing detectors
- 24 • Penalty and emergency brake applications
- 25 • Locomotive Engineer alerter activated
- 26 • Car body and truck accelerometers for instability detection.
- 27 • Ten additional detectors to be defined during the detailed design phase

N. Maintenance/Diagnostic System Interfaces

28 Provide a logical (network) interface between the ATC and the onboard maintenance and  
29 diagnostic system. The requirements have been identified in the On-Train Monitor and

Diagnostics System and Control/Command and Onboard Data System sections of the Next Generation Amtrak/CHSRA Trainsets Performance Specification. The network interface shall be sized to allow for a minimum of 200 elements relating to onboard subsystems (1 element equating to door system failure in Car 1 for example). The interface shall also be sized to allow for transfer of a minimum of 40 ATC elements including software versions of ATC onboard equipment.

The onboard systems data shall be transmitted via the communications and/or the ATC wayside for monitoring of train systems health status to the Operations Control Center (OCC) and/or the maintenance depots.

The following events will be transmitted by ATC to the control center ATS:

- Low Voltage Power Supply /Battery Charger Failed
- Low Battery
- Up to 10 additional diagnostic interfaces to be determined during the detailed design phase.

O. Event Recorder Interfaces

In addition to the requirements for the event recorder that have been identified in the On-Train Monitor and Diagnostics System section of the Next Generation Amtrak/CHSRA Trainsets Performance Specification, the RS contractor shall provide discrete and logical (network) interfaces to provide the following ATC related data to the onboard event recorder:

- ATC Operating Mode
- ATC penalty brake application.
- Urgency Brake Application
- Emergency Brake Application
- ATC Tractive and Braking Effort Level Command
- ATC Measured Speed
- ATC Door enable left
- ATC Door enable right
- ATC Bypass Switch status
- ATC Mode Switch status
- ATC Time Signal
- ATC Fault Codes
- ATC Spare 1
- ATC Spare 2

- ATC Spare 3

### 29.6.11 Commands from Automatic Train Supervision to Rolling Stock

The following commands and groups of commands can be sent from the ATS workstations to the train for action by the RS subsystems:

- **Announcement and Passenger Sign Interfaces** – Provide a logical (network) interface to enable ATC and/or communications to send triggers to the RS audible announcement and visual message display systems to activate pre-recorded announcements. The actual text of the announcements shall be determined during the detailed design phase.

### 29.6.12 Train Subsystems Interface

Provide a logical interface to enable ATC to send commands to the train subsystems for the following:

- To close and open vents and other elements for ventilation control when approaching and leaving tunnels
- For switching off high voltage power breakers as necessary for phase breaks
- To lower and raise pantographs

For switching these items and other equipment, the commands shall be sent from the onboard ATC to the RS subsystems based on physical location on the right-of-way.

### 29.6.13 Track Circuit – Rolling Stock Electromagnetic Compatibility

The ATC and RS contractors shall jointly establish specific immunity limits to protect ATC equipment, particularly track circuits and track-based data communication circuits, from train-generated interference. These limits shall be implemented by coordinated action by the RS and ATC contractors during ATC and RS design, implementation, and qualification testing. The RS shall accommodate the electromagnetic environment requirements identified in the Electromagnetic Compatibility and Infrastructure Interface Specification sections of the Next Generation Amtrak/CHSRA Trainsets Performance Specification.

For EMC between RS and ATC track circuits, EN 50238 – *Railway Applications – Compatibility between Rolling Stock and Train Detection Systems* shall govern the relationship. The ATC contractor shall use EN 50238 procedures to document and convert the electromagnetic immunity characteristics of planned wayside equipment into corresponding emission limits which will apply to RS. The RS contractor shall ensure that the RS emissions comply with the limits developed by the ATC contractor. If necessary, the contractors shall modify ATC and RS designs to ensure compatibility.

U.S. Department of Transportation Federal Transit Administration test procedures govern testing of the compatibility of trains and track circuits. The RS contractor shall perform field

1 qualification tests to demonstrate that the RS conforms to the emission limits using the  
2 following procedures:

- 3 • UMTA-MA-06-0153-85-6 – Conductive Interference in Rapid Transit Signaling Systems  
4 Volume II: Suggested Test Procedures
- 5 • UMTA-MA-06-0153-85-8 – Inductive Interference in Rapid Transit Signaling Systems  
6 Volume II: Suggested Test Procedures
- 7 • UMTA-MA-06-0153-85-11 – Radiated Interference in Rapid Transit Signaling Systems  
8 Volume II: Suggested Test Procedures

#### 29.6.14 Rolling Stock - Electromagnetic Field Exposure

9 Implanted medical device manufacturers and U.S. research laboratories have established  
10 exposure limits for people with implanted medical devices such as pacemakers. The  
11 recommended limit is 1.0 G for frequencies up to 10 kHz, including the Project traction power  
12 frequency of 60 Hz.

13 The RS contractor shall determine whether an electromagnetic field (EMF) level above the  
14 recommended 1.0 G limit can occur inside the RS passenger compartments. The RS contractor  
15 shall perform a detailed design assessment during the design and construction phases to  
16 determine worst case level. The analysis shall confirm EMF levels inside the passenger  
17 compartment, considering the worst case EMF due to OCS and rail currents and the shielding  
18 effect of the railcar body. After construction, the contractor shall test the RS to determine the  
19 actual worst case EMF level inside the passenger compartment.

20 If the actual worst case EMF level exceeds the established limit, the Project safety program will  
21 ensure that passengers or employees with implanted medical devices are aware of the potential  
22 exposure to a magnetic field above the established limit by placing suitable warning signs in the  
23 passenger compartments and on platforms.

### 29.7 Communications Interfaces

24 Passenger RS to wayside communications interface design criteria give the Designer the  
25 guidance to define the RS system interfaces to function and perform with the wayside  
26 communications systems. The *Communications* chapter contains design criteria for wayside  
27 communications systems.

28 The RS will be specified with multiple onboard systems (OBSs) that will have an interface  
29 and/or need to be integrated with the wayside communications systems (described in the  
30 *Communications* chapter). OBSs shall interface to the wayside communications systems to  
31 provide seamless functionality between equipment at fixed locations and onboard devices.  
32 Proper interface design and control is required to achieve this functionality.

1 The RS train-to-wayside radio equipment and RS GPS Network Time System equipment will be  
2 designed and installed by the wayside communications Designer (or possible 3<sup>rd</sup> party for an  
3 infotainment system).

4 Dependent on the final design of the wayside systems, it is anticipated that there will be as  
5 many as 6 train-to-wayside radio systems installed onboard the RS. These train-to-wayside  
6 radio systems may include the following:

- 7 • Global Positioning System radio receiver
- 8 • Operations Radio System (voice and non-vital low-bitrate data radio)
- 9 • ATC Radio System (vital data radio) (refer to Signaling and Train Control Interfaces section)
- 10 • Broadband Radio System (non-vital high-bitrate data radio for real-time data [i.e., CCTV  
11 video] off-load while mobile at 250 mph)
- 12 • Wireless Local Area Network (WLAN) radio (non-vital high-bitrate data transfer while  
13 stationary)
- 14 • Infotainment radio (fee-driven passenger internet / entertainment delivered by 3<sup>rd</sup> party for  
15 fee) (this system is TBD). Infotainment train-to-wayside radio will be provided by the  
16 wayside systems contractor or other 3<sup>rd</sup> party, however onboard infotainment equipment  
17 shall be provided by the RS Contractor.

### **29.7.1 Interface Requirements**

18 An interface is identified as any point where 2 or more systems meet. This interface point may  
19 include internal hardware, circuitry, external peripherals, networks, logical interfaces between  
20 networks, users, or similar. All interfaces shall be identified, managed during changes, and  
21 tracked during the design process. Furthermore, to ensure the constructed system satisfies the  
22 system requirements, these interfaces shall be tested and verified by the system integrator after  
23 construction.

24 Interface requirements include the following:

- 25 • Physical – Physical interfaces include space provisions (e.g., cabinets, conduits and cabling  
26 pathways) and mounting locations and hardware (e.g., brackets and exterior and interior  
27 antenna mounting locations).
- 28 • Mechanical – Mechanical interfaces include HVAC for equipment cooling.
- 29 • Electrical – The electrical interfaces include power, voltage and uninterruptable power  
30 supply (UPS) requirements and grounding and bonding.
- 31 • Electromagnetic – Electromagnetic interfaces cover design provisions to ensure that the RS  
32 conductive, inductive, and radiated emissions do not interfere with the safe and dependable  
33 operation of the equipment.

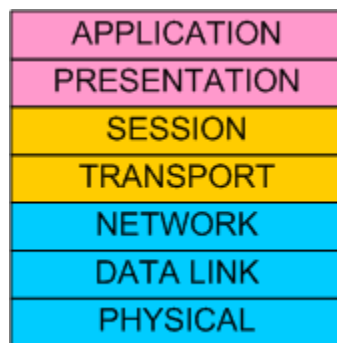


- Functional – The functional interfaces include protocols for specific functions and data exchanges required at the RS subsystems interfaces to ensure that the end-to-end system functional requirements are met. More details on functional interfaces are included in the next section.

#### 29.7.1.1 Functional Interfaces

OBSs shall interface to the fixed wayside communications systems to achieve end-to-end functionality. In conducting the design of these systems and interfaces, the designers shall coordinate multiple interfaces encompassing multiple protocols between the RS systems and wayside communications systems including wireless networks, wired networks, and control systems (Integrated Information Management Platforms [IIMPs]).

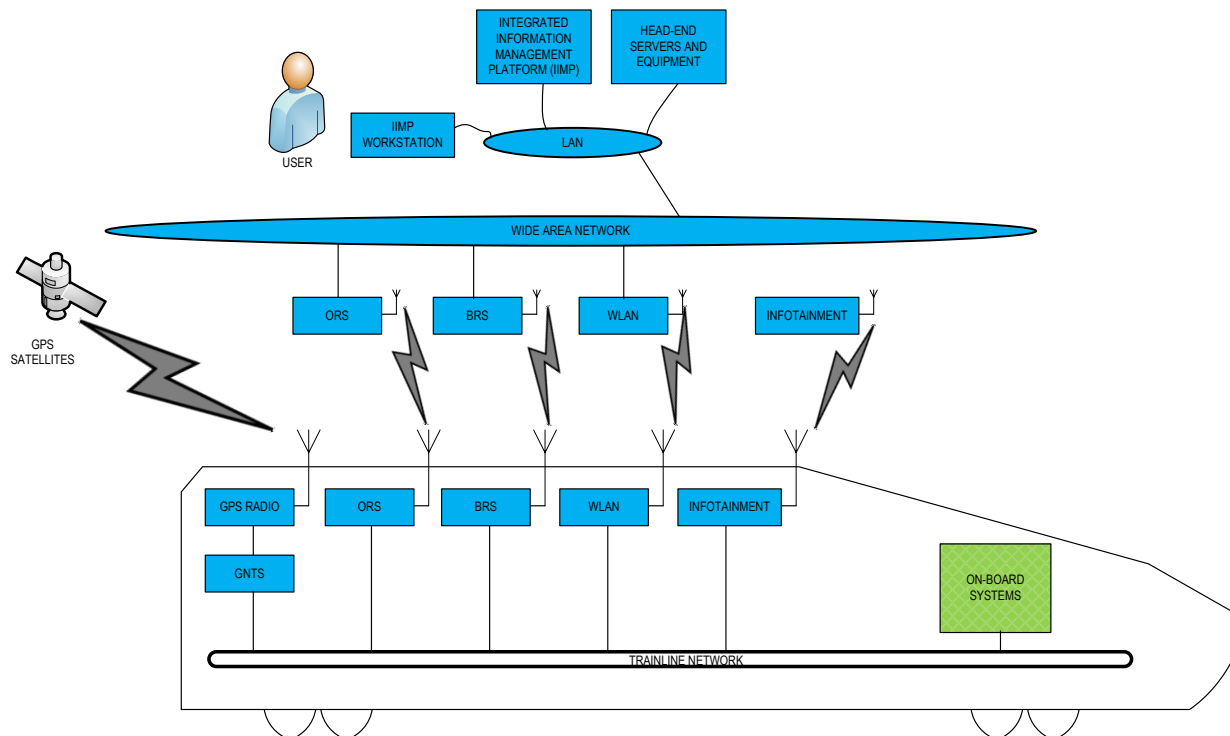
**Figure 29-1: Open Systems Interconnection Reference Model**



With reference to Figure 29-1; the end-system device shall coordinate protocols that perform the functions throughout all 7 layers (i.e., perform the application level processes associated with the top 2 layers, end-to-end logical connectivity functions associated with the Session and Transport layers, as well as the networking functions associated with the bottom 3 layers). The network equipment used to interconnect the end-system devices will perform various functions associated with the lower 3 layers. End-system devices, which could possibly be procured under different contracts, also contain “network functions”. These end-systems are therefore part of the end-to-end network and shall be interoperable with the network equipment to ensure end-to-end delivery of the application data.

When considering interfaces between the wayside communications systems and the onboard systems, it is important to consider the system boundaries. A high-level, end-to-end system diagram for onboard systems to wayside communications systems is depicted on Figure 29-2.

**Figure 29-2: Wayside Communications and Onboard Systems Interface Diagram**



Communications systems provided by the wayside communications contractor are indicated with a solid fill and the systems to be provided by the RS Contractor are crosshatched.

To support the end-to-end exchange of data, components shall support an interface with each other and exchange data using a common protocol across the interfaces. The interface may include a physical connection between 2 communicating components (e.g., between a server and network switch) or shall be via a logical communications path across a network when the 2 communicating components are connected to a common wired or wireless network. Agreement between multiple protocols is required to support fully end-to-end communications between 2 components.

Consideration of the multiple interfaces and protocol compatibilities to achieve functionality is crucial to ensuring the onboard systems interface to the wayside communications systems and provide full functionality. Please refer to the *Communications* chapter for the “Transport Network”.

### 29.7.1.2 Interface Requirements of the Rolling Stock Contractor

The RS Contractor shall make provisions for and provide interfaces for equipment to be provided by a separate, multiple-vendor procurement.

- RS Contractor shall provide the following physical interfaces:
  - Provide antenna locations for 6 antennas to be installed at the exterior of the vehicle.

- 1        - Provide conduit and/or pathways for antenna cables to be installed between the  
2        antennas and the radio equipment cabinets.
- 3        - Provide equipment cabinet space for radio head end equipment for 6 radio systems, and  
4        the onboard GNTS equipment.
- 5        • RS Contractor shall provide the following mechanical interface:
  - 6        - Provide cooling or ventilation to maintain specified equipment safe operating  
7        temperature during normal and emergency power operation.
- 8        • RS Contractor shall provide the following electrical interfaces:
  - 9        - Provide conditioned power and back-up power to meet the power consumption  
10       requirements of the equipment.
  - 11       - Provide grounding and bonding per the requirements of the *Grounding and Bonding*  
12       chapter.
- 13       • RS Contractor shall provide the following electromagnetic interfaces:
  - 14       - Design spatial separation, grounding, shielding and bonding such that conductive,  
15       inductive, and radiated emissions do not interfere with the safe and dependable  
16       operation of the equipment.
- 17       • RS Contractor shall provide the following functional interfaces:
  - 18       - Each interface shall be designed to deliver end-to-end functionality.
  - 19       - Coordinate multiple interfaces encompassing multiple protocols between the RS and the  
20       wayside communications system, including radio networks, wired networks, and  
21       control systems.
  - 22       - Provide documentation for higher level (layers 7-6) protocols for all onboard systems  
23       provided. The onboard systems shall be designed with application programming  
24       interfaces (APIs) to allow the control and monitoring functionality required in the  
25       specifications to be accessed via the IIMPs.
  - 26       - Provide systems with open standard protocols for lower level (layers 1-5) for all systems  
27       provided.
  - 28       - The onboard systems shall be designed in coordination with the wayside IIMP to  
29       provide users seamless control of onboard systems from operations facilities.
  - 30       - The onboard passenger information / intercom system, passenger information sign  
31       systems and internal and external electronic displays system shall be designed to receive  
32       and deliver aural and/or visual messages from users at operations facilities to the  
33       onboard passenger areas in real time.
  - 34       - The onboard fire alarm system shall be designed to deliver onboard alarm notifications  
35       to users at operations facilities in real time.

- The onboard passenger emergency intercom and Passenger Alarm System shall be designed to deliver onboard passenger voice calls to users at operations facilities in real time.
- The onboard Internal and External Camera Video Recorder system shall be designed to deliver video feeds by remote operation, manual onboard operation or automatic trigger to users at operations facilities in real time. The onboard Internal and External Camera Video Recorder system shall be designed to automatically offload stored video when trains are stationary in yards or terminals wirelessly.

### **29.7.1.3 Interface Requirements of the Wayside Systems Contractor**

The Wayside Systems Contractor shall provide interfaces to RS equipment to be provided by a separate, multiple-vendor procurement.

- Wayside Systems Contractor shall provide the following physical interfaces:
  - Provide all radio systems and equipment and cabling spatial requirements to the RS Contractor
  - Provide all onboard GNTS equipment and cabling spatial requirements to the RS Contractor
- Wayside Systems Contractor shall provide the following mechanical interface:
  - Provide cooling or ventilation requirements to maintain specified equipment safe operating temperature during normal and emergency power operation to the RS Contractor.
- Wayside Systems Contractor shall provide the following electrical interfaces:
  - Provide conditioned power and back-up power requirements to the RS Contractor.
  - Provide grounding and bonding per the requirements of the *Grounding and Bonding* chapter and the equipment to the RS Contractor.
- Wayside Systems Contractor shall provide the following electromagnetic interface:
  - Provide emissions and susceptibility requirements to the RS Contractor.
- Wayside Systems Contractor shall provide the following functional interfaces:
  - Each interface shall be designed to deliver end-to-end functionality.
  - Coordinate multiple interfaces encompassing multiple protocols between the RS and the wayside communications system, including radio networks, wired networks, and control systems.

## Chapter 30

### Reliability, Availability, and Maintainability

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	Sept 2013	Rolling Stock Procurement
2	Feb 2014	Revision 2



## Table of Contents

30	<b>RELIABILITY, AVAILABILITY, AND MAINTAINABILITY .....</b>	<b>30-1</b>
30.1	Scope .....	30-1
30.2	Regulations, Codes, Standards, and Guidelines .....	30-1
30.3	CHSTS RAM Program .....	30-2
30.4	RAM Design Requirements.....	30-3
30.4.1	Automatic Train Control RAM Design Criteria .....	30-3
30.4.2	Traction Power Supply System and Overhead Contact System RAM Design Criteria ...	30-4
30.4.3	Communications RAM Design Criteria.....	30-6
30.4.4	Station RAM Design Criteria .....	30-8
30.4.5	Rolling Stock RAM Design Criteria .....	30-10
30.4.6	Infrastructure RAM Design Criteria .....	30-15

## Acronyms

ATC	Automatic Train Control
ATS	Automatic Train Supervision
CAHSR	California High-Speed Rail
CCTV	Closed Circuit Television
CENELEC	European Committee for Electrotechnical Standardization
CFR	Code of Federal Requirements
CHSRA	California High Speed Rail Authority
CHST	California High-Speed Train
CHSTP	California High-Speed Train Project
CHSTS	California High-Speed Train System
CM	Corrective Maintenance
CRAM	CHSTS RAM Metrics document
CTS	Communications Transmission System
DCM	Design Criteria Manual
EAC	Electronic Access Control
EMC	Electromagnetic Compatibility
EMI	Electromagnetic Interference
EPS	Extruded Polystyrene
FAS	Fire Alarm System
FCC	Federal Communications Commission
FRA	Federal Railroad Administration
HV	High Voltage
HVAC	Heating, Ventilation, and Air Conditioning
IDS	Intrusion Detection System
IEC	International Electrotechnical Commission
IEEE	Institute of Electrical and Electronics Engineers
IIMP	Integrated Information Management Platform
HST	High-Speed Train
KPI	Key Performance Indicator
LAN	Local Area Network
MTBF	Mean Time Between Failures
MTBSI	Mean Time Between Service Interruptions
MTTRS	Mean Time To Restore Service
NF	Negative Feeder
OCC	Operations Command Center
OCS	Overhead Contact System



PACIS	Public Address and Customer Information Signs
PA/VMS	Public Address / Visual Message Signs
PM	Preventive Maintenance
PMT	Project Management Team
PS	Paralleling Station
RAM	Reliability, Availability, and Maintainability
RAMPP	CHSTS RAM Program Plan
RAMS	Reliability, Availability, Maintainability, and Safety
RCC	Regional Control Center
RiAC	Reliability Information Analysis Center
RS	Rolling Stock
SCADA	Supervisory Control and Data Acquisition Subsystems
SS	Substation
SWS	Switching Station
TES	Traction Electrification System
TPF	Traction Power Facility
TPS	Traction Power Supply System
UPS	Uninterruptible Power Supplies
WAN	Wide Area Network
WPC	Wayside Power Cubicle



## 30 Reliability, Availability, and Maintainability

### 30.1 Scope

The Reliability, Availability, and Maintainability (RAM) chapter summarizes design criteria for the CHSTS necessary to achieve the reliability, availability, and maintainability targets set in the CHSTS RAM Metrics document (CRAM). The design criteria cover Systems, Stations, Rolling Stock, and Infrastructure.

### 30.2 Regulations, Codes, Standards, and Guidelines

Refer to the General chapter for requirements pertaining to regulations, codes, and standards.

- European Committee for Electrotechnical Standardization (CENELEC)
  - EN 50126-1:1999: Railway applications – The specification and demonstration of Reliability, Availability, Maintainability, and Safety (RAMS) – Part 1: Basic requirements and generic processes
  - CLC/TR 50126-2:2007: Railway applications – The specification and demonstration of Reliability, Availability, Maintainability, and Safety (RAMS) Part 2: Guide to application of EN 50126-1 for safety
  - CLC/TR 50126-3:2008: Railway applications – The specification and demonstration of Reliability, Availability, Maintainability, and Safety (RAMS) Part 3: Guide to application of EN 50126-1 for Rolling Stock RAM
  - EN 50128:2011: Railway applications – Communication, signalling and processing systems – Software for railway control and protection systems
  - EN 50129 – Railway applications – Communications, signalling and processing systems – Safety related electronic systems for signalling (2003)
- International Electrotechnical Commission (IEC)
  - IEC 60812: Analysis techniques for system reliability – Procedures for failure mode and effects analysis
  - IEC 60863: Presentation of reliability, maintainability, and availability predictions
  - IEC 61025: Fault Tree Analysis
  - IEC 61078: Analysis techniques for dependability – Reliability block diagram

method

- IEC 61165: Application of Markov Techniques

- U.S. Department of Defense (US DOD) Standards

- MIL-STD-785B: Reliability Program for Systems and Equipment, Development and Production

- MIL-STD-217F Note 2: Reliability Prediction of Electronic Equipment

- MIL-STD-470B: Military Standard: Maintainability Program for Systems and Equipment

- MIL-STD-810G: Environmental Engineering Considerations and Laboratory Tests

- MIL-STD-471A: Maintainability Verification / Demonstration / Evaluation

- Reliability Information Analysis Center (RiAC)

- Reliability Toolkit: Commercial Practices Edition

- NPRD-2011: Nonelectronic Parts Reliability Data

- California High-Speed Rail Authority (CAHSR)

- TM 300.09 CHSTS RAM Program Plan

- TM 1600.01: CHSTS Verification and Validation Management Plan

- TM 1600.02 CHSTS Interface Management Plan

- CHSTS RAM Metrics

- California High-Speed Rail Program Revised 2012 Business Plan

- HSR 11-16: Design Criteria, Chapter 32

### **30.3 CHSTS RAM Program**

Each contractor or supplier of systems, equipment, facilities, and infrastructure shall establish and perform a RAM program compliant with the CHSTS RAM Program Plan (RAMPP).

## 30.4 RAM Design Requirements

### 30.4.1 Automatic Train Control RAM Design Criteria

The Automatic Train Control system (ATC) equipment and functions shall use rugged, reliable, fault tolerant, and service-proven equipment to comply with CRAM high-level Key Performance Indicator (KPI) targets. The ATC design and construction shall minimize the number and severity of failures that can affect passenger service or cause a service interruption.

ATC equipment design and implementation shall ensure that, when needed because of a failure, Corrective Maintenance (CM) can be completed quickly and with no impact on train service beyond that caused by the failure. ATC and infrastructure design and implementation shall enable fast detection, response, access, correction, and service restoration if a service interrupting failure occurs. All ATC equipment shall meet Mean Time Between Service Interruption (MTBSI), Mean Time To Restore Service (MTTRS), Mean Time Between Failure (MTBF), and Mean Time To Repair (MTTR) requirements from the CRAM.

The following RAM ATC design criteria augment requirements for ATC equipment in the ATC chapter of these criteria:

#### General

- ATC spaces with active equipment shall be designed with HVAC, and with HVAC backup power to maintain operational temperature in case of normal power failure.

#### Power and Electrical

- All ATC equipment shall be served by an uninterruptible power supply (UPS) which can power the equipment for at least 4 hours, to ensure continued operation if normal power fails.

#### Track Circuits

- Track circuits shall be highly reliable with service-proven design.
- Safety-critical ATC system design shall enable continued safe train operations in case of a track circuit failure. ATC system shall detect and annunciate any conflict between track circuits and other elements of the ATC system.
- Track circuit control software shall be suitable for safety-critical application and highly reliable.

#### Interlockings

- Interlocking equipment shall have a fault-tolerant configuration.
- Interlocking equipment shall preclude common-mode failures.

- Interlockings with more than 4 switch motors shall be in a zoned configuration, so that independently controlled switches at crossovers allow straight moves, even with certain failures on the adjacent track.

#### Switch Machines

- Switch machines shall be highly reliable, to prevent disruption of revenue service.
- Switch movement failure detection systems shall monitor switch movements and detect failures.
- Switches shall be manually lockable in case of a failure, and the lockable feature shall be arranged with the interlocking equipment to allow continued operation with minimum impact over a manually locked switch.

#### Transponders

- The ATC system, if it uses transponders, shall safely and dependably interpolate train location and allow undisturbed operation if 1 transponder fails. The ATC system shall promptly annunciate the failed transponder.

#### Automatic Train Supervision (ATS)

- The ATS shall have a fault tolerant design at every location, including the Operations Command Center (OCC) and Regional Control Centers (RCCs), so that no single failure of the ATS can interrupt or disrupt service.
- Redundant ATS equipment shall be hot swappable so that restoration of a failed unit does not interrupt or disrupt service.

### **30.4.2 Traction Power Supply System and Overhead Contact System RAM Design Criteria**

The Traction Power Supply System (TPS) equipment and functions shall be fault tolerant, and the Overhead Contact System (OCS) equipment and functions shall be robust, service-proven, and highly reliable. The TPS and OCS shall meet the CRAM high-level KPI targets. The TPS and its utility supply shall provide double contingency protection against failures. The TPS and OCS shall minimize the number and severity of failures that can affect passenger service or cause a service interruption.

TPS and OCS equipment design and implementation shall ensure that, when needed because of a failure, CM can be completed quickly and with no impact on train service beyond that caused by the failure. TPS, OCS, and infrastructure design and implementation shall enable fast detection, response, access, correction, and service restoration if a service interrupting failure occurs. All TPS and OCS equipment shall meet MTBSI, MTTRS, MTBF, and MTTR requirements from the CRAM.

The TPS and utility supply system double contingency design shall ensure that no first failure of the TPS or utility supply can disrupt train service. No second TPS or utility supply failure shall stop train service.

The *Traction Power Supply System* chapter provides the TPS system design criteria. *Overhead Contact System and Traction Power Return System* chapter provides the OCS and *Traction Power Return* system design criteria. The following detailed RAM design criteria augment TPS and OCS requirements.

#### General

- Electrified TPS and OCS components shall be electrically isolatable for fast, safe maintenance.

Traction Power Substations (SS) Each SS shall have at least 2 high voltage (HV) traction power transformers, on separate, independent circuits. Each HV traction power transformer shall be capable of handling the full load of the SS.

- Each HV traction power transformer shall be fed from the utility grid via separate, independent utility feeds.
- For each SS, at least 1 utility feed shall be independent from those at each adjacent SS.
- In the event of a loss of power on a single circuit at an SS, the remaining circuit(s) shall supply the full load of the SS.
- In the event of a total power loss to an SS, a double contingency design shall supply power from an alternate SS via a switching station (SWS).
- The circuit breaker connections from the utility to the SS shall be of the 1.5 CB configuration.

#### Switching Stations

- To protect from bridged feeds between operating blocks, phase breaks at SWS shall be normally open to isolate the feed sections.
- Normally open trackside, motorized, load-break disconnect switches installed at the SWS phase break shall provide electrical continuity in case of a failure.
- The SWS shall be equipped with switchgear and motorized disconnect and bypass switches to allow isolation of Autotransformers in case of a failure.

#### Paralleling Stations (PS)

- The Autotransformer at each PS shall be completely electrically isolatable from the system by switchgears and disconnect switches in case of a failure.

1 Wayside Power Cubicles (WPC)

- 2     • Each WPC shall be powered by at least 2 sources, in addition to an eight hour battery  
3        backup.

4 OCS and Traction Power Return Systems

- 5     • Includes Catenary Conductors (Section 21.8), Other Overhead Conductors (Section 21.9),  
6        Sectionalizing and Switching (Section 21.12), and OCS Structural (Section 21.15).
- 7     • All equipment shall be service proven and meet all CRAM requirements.
- 8     • Auto-tensioning devices on OCS lines shall maintain appropriate tension levels to  
9        prevent line breakage.
- 10    • The OCS shall ensure even wear on Rolling Stock (RS) pantographs.
- 11    • The OCS shall minimize possible impact on adjacent systems and equipment in case of  
12      an OCS structure failure.

**30.4.3 Communications RAM Design Criteria**

13 The Communications system equipment and functions shall be fault tolerant and service-  
14 proven to meet the CRAM high-level KPI targets. The Communication system shall minimize  
15 failures that can affect passenger service or cause a service interruption.

16 Communication equipment design and implementation shall ensure that, when needed because  
17 of a failure, CM can be completed quickly and without impact on train service beyond that  
18 caused by the failure. Communication and infrastructure design and implementation shall  
19 enable fast detection, response, access, correction, and service restoration if a failure occurs. All  
20 Communication equipment shall meet MTBSI, MTTRS, MTBF, and MTTR requirements from  
21 the CRAM.

22 The OCC and each RCC shall be fully independent and each shall be capable of providing  
23 command and control of all CHSTS operations in case of a failure at another facility.

24 The *Communications* chapter provides the Communications System design criteria. The  
25 following additional detailed RAM Communications design criteria augment the DCM chapter  
26 28 criteria.

27 General

- 28     • Communications spaces with active equipment shall be designed with HVAC and  
29        HVAC backup power to maintain operational temperature in case of normal power  
30        failure.
- 31     • Communications equipment shall use off-the-shelf components to ensure the availability



of replacements over the specified lifetime and to permit fast, safe maintenance.

## Power and Electrical

- All critical and essential Communications equipment shall be powered by a UPS or battery backup to ensure continued operation if normal power fails.
- All Communications equipment shall be protected by surge protection devices to ensure continued operation in overvoltage events.

## Transport Network

- The Transport Network includes Wide Area Networks (WAN) (Section 28.4.3), Local Area Networks (LAN) (Section 28.4.4), Radio (Section 28.4.5), and Cable Infrastructure (Section 28.4.6)
- The Transport Network system shall be fault tolerant, diverse, and redundant to prevent any single failure from causing a service interruption. The Transport Network shall include redundancy in wired and wireless networks.
- The Transport Network shall incorporate fault tolerant provisions including double ring network topology and overlapping radio coverage.
- The Transport Network shall transmit data over fault tolerant networks simultaneously to eliminate data loss in case of a single failure.
- The fiber cabling physical routing shall have a fault tolerant configuration, with fault tolerant local loops from each interlocking, that allows undisturbed operations in case of any first failure.
- Communications control equipment shall incorporate fault tolerant provisions, such as hot standby servers and hot standby network switches.
- Communications control equipment shall include hot swappable components, including network switches and transceivers, for fast, safe maintenance.

## Public Address and Customer Information Signs System (PACIS) – Section 28.4.10

- Audio and visual passenger communications systems shall be fault tolerant, so that no single failure can disrupt communications between the operator and passengers.
- PACIS systems shall fully overlap in coverage areas so that no single failure can prevent adequate communication between CHSTS users and operators.

## Telephone and Intercom System – Section 28.4.11

- Backup communications methods such as radio or cellular telephone shall be provided for all telephone subsystems.

- Recording equipment for voice communications on the telephone system shall be fault tolerant.

### Security and Monitoring

- Security and Monitoring includes Fire Alarm System (FAS) (Section 28.4.9), Closed Circuit Television (CCTV) (Section 28.4.12), Intrusion Detection System (IDS) (Section 28.4.13), and Electronic Access Control System (EACS) (Section 28.4.14).
- Security and Monitoring equipment shall be fault tolerant to ensure sufficient safety and security in the event of a single failure.
- FAS shall include manual fire alarm switches to trigger alarm in case of a sensor failure.
- EACS shall include a manual key override to access facilities protected by EACS in case of a failure.
- CCTV shall have at least 2 cameras monitoring each critical covered area, to provide fault tolerant coverage in case of a camera failure.
- CCTV shall be recorded on fault tolerant devices.

### Integrated Information Management Platform (IIMP)

- IIMPs for critical and essential subsystems and equipment shall be in a redundant configuration to prevent loss of function in the event of a single hardware or software failure.

### GPS Network Timing System (GNTS)

- The GNTS design shall preclude it from causing a service failure.

### Supervisory Control and Data Acquisition (SCADA) Systems – Chapter 27

- SCADA System design shall include fault tolerant servers so that no first failure can affect SCADA functionality.

#### **30.4.4 Station RAM Design Criteria**

Station equipment and functions shall be fault tolerant and service-proven to meet the CRAM high-level KPI targets. Station design and construction shall ensure that equipment or structural failures are highly unlikely to interrupt passenger service. Provisions shall include diverse and fault tolerant systems and highly reliable equipment.

Station equipment design and implementation shall ensure that, when needed because of a failure, CM can be completed quickly and without impact on train service. Station equipment and infrastructure design and implementation shall enable fast detection, response, access, correction, and service restoration if a failure occurs. All Station equipment shall meet MTBSI,

MTTRS, MTBF, and MTTR requirements from the CRAM.

The *Station* chapter provides the Station design criteria. The following additional detailed RAM design criteria augment the requirements for the Station equipment.

#### Fare System Equipment

- Fare machines shall be highly reliable and service-proven.
- Each station shall have at least 1 fare machine above the requirement to satisfy maximum projected demand, so the station can provide unrestricted service in case of a first failure.
- Stations shall provide all needed facilities and equipment so staff can sell tickets manually when needed for special events or because of fare machine failures.

#### Elevators and Escalators

- Each station shall have at least 1 escalator above the requirement to satisfy maximum projected demand, so the station can provide unrestricted service in case of a failure.
- Escalators shall be reversible to sustain passenger movements during peak hours and during maintenance.
- Elevators shall be service-proven and fault tolerant to meet CRAM requirements.
- Elevators and Escalators shall provide easy access to all serviceable components for fast, safe maintenance.

#### Lighting and Electrical – Sections 14.3.7.1, 14.3.7.2

- The Lighting System shall have redundant power supplies, to avoid a loss of station lighting due to a power distribution failure.
- Backup power shall be supplied to Station equipment by UPS, battery, or generator in the event of a normal power loss as required by Sections 17.4 and 17.8.
- Station electrical circuit protection boxes shall be easily accessible to personnel, and shall use circuit breakers.

#### Plumbing – Section 14.3.7.3

- Each fixture shall have a local shut-off valve to stop flow in case of a failure.
- Each room with a concentration of plumbing fixtures, such as a bathroom or employee shower room, shall be fitted with floor drains to ensure water does not build up on floor as a result of a failure.

1 Safety Equipment

- 2 • The Fire Alarm System shall have manual switches to trigger an alarm in case of a  
3 sensor failure.
- 4 • Each safety alarm shall operate on an independent circuit, so that even in case of failure,  
5 at least 1 safety alarm will operate.

6 Heating, Ventilation, and Air Conditioning (HVAC) – Section 14.3.7.7

- 7 • Areas critical to train operations shall have redundant HVAC equipment, so that Train  
8 Control and Communications equipment does not overheat in case of a first failure.  
9 HVAC equipment shall have adequate backup power to maintain operational  
10 temperature for critical equipment.
- 11 • HVAC equipment shall have local shut-off switches in case of a failure.
- 12 • HVAC equipment shall provide easy access to all serviceable components for fast, safe  
13 maintenance.

14 Public Address/Visual Messaging Sign (PA/VMS) System – Section 14.3.9

- 15 • PA/VMS subsystems shall each be fault tolerant, so that no single failure can disrupt  
16 communications between the operator and passengers.
- 17 • PA/VMS equipment shall consist of off-the-shelf components to ensure the availability  
18 of replacements over the specified lifetime and to permit fast, safe maintenance.
- 19 • PA/VMS subsystems shall fully overlap in coverage areas so that no single failure can  
20 prevent adequate communication between the operator and passengers.

21 CCTV

- 22 • At least 2 cameras shall monitor each critical area, to provide fault tolerant coverage in  
23 case of a camera failure.
- 24 • CCTV shall be recorded on fault tolerant devices.

**30.4.5 Rolling Stock RAM Design Criteria**

25 Rolling Stock systems, equipment, and functions shall use rugged, highly reliable, fault tolerant,  
26 and service-proven equipment to meet the CRAM high-level KPI targets. Rolling Stock design  
27 and construction shall ensure that failures are highly unlikely to cause service interruptions.

28 Rolling Stock design and implementation shall ensure that, when needed because of a failure,  
29 CM can be completed quickly and with no impact on train service beyond that caused by the  
30 failure. Rolling Stock and infrastructure design and implementation shall enable fast detection,  
31 response, access, correction, and service restoration if a failure occurs. The Rolling Stock and its

equipment shall meet MTBSI, MTTRS, MTBF, and MTTR requirements from the CRAM.

Rolling Stock shall include a Monitoring and Diagnostic System (MDS) which alerts the operator of any potentially service interrupting failure and the location of the failure on the train. Fault tolerant provisions of Rolling Stock systems shall include multiple independent propulsion and braking systems, door systems, HVAC, communications systems, and all other systems necessary for adequate passenger train service.

DCM chapter 29 and the Next Generation Amtrak/CHSRA Trainsets Performance Specification (Trainset Specification) provide the Rolling Stock system design criteria. The following additional detailed RAM Rolling Stock design criteria augment the DCM chapter 29 and Trainset Specification criteria.

#### General

- Train systems and equipment shall have documented maintainability provisions for fast detection, response, access, correction, and service restoration in case of a failure.

#### Door Control System and Doors

- Door controls shall be highly reliable and service-proven.
- Each exterior doorway control and vestibule door controls shall have manual overrides in case of failure.
- Doors shall be manually openable, closable, and lockable in case of failure.

#### Communications Systems/Passenger Information Signs

- Onboard Communications systems shall be fault tolerant, diverse and redundant. They shall include alternative and independent communication means such as portable radio or cellular phones in addition to onboard radio.
- Passenger information systems shall each be fault tolerant, to ensure communications between the operator and passengers are available after any first failure.
- Intercom and Passenger Information Signs shall be fault tolerant, so that no single failure can disrupt communications between the operator and passengers.
- Communications Systems equipment shall consist of off-the-shelf components to ensure the availability of replacements over the specified lifetime and to permit fast, safe maintenance.

#### CCTV

- At least 2 cameras shall monitor each critical area, to provide fault tolerant coverage in case of a camera failure.

- CCTV shall be recorded on fault tolerant devices.

#### Event Recorder

- Event Recorder equipment shall be fault tolerant to ensure critical train information is not lost in case of a failure.
- Event Recorder equipment shall be served by a UPS to ensure continued operation in case of a power failure.

#### Monitoring and Diagnostic System (MDS)

- MDS shall be fault tolerant and service-proven.
- MDS shall be powered by a UPS to ensure continued operation in case of a power failure.

#### HVAC System

- Each car shall have fault tolerant HVAC units, to mitigate effects of a single failure.

#### Primary Power Distribution and Auxiliary Power System, Low Voltage System, Trainlines and Train and Car Control, and Pantograph Current Collector

- Pantograph shall be highly reliable and service-proven.
- Pantograph shall include a manual, mechanical means of raising and lowering in case of a failure.
- Pantograph shall retract or sacrificially fail in case it interferes with and may damage the OCS.
- An Auxiliary Power System shall automatically take over and power vital train systems in case of a Primary Power Distribution failure.
- The onboard Power Distribution System shall include surge protection devices to protect electrical components against damage due to overvoltage.
- Propulsion/door interlock design shall include a manual override for restrictive failures per specification requirements.

#### Propulsion System

- The Propulsion System shall be highly dependable and service-proven.
- The Propulsion System shall have a secondary, operator activated method of cutting power if propulsion fails to cease when commanded.
- The Propulsion System design shall tolerate the failure of a single unit without affecting

the function of any other unit.

- If a propulsion failure occurs, the propulsion unit shall not apply any torque load other than rolling friction.

#### Onboard ATC

- The ATC equipment shall be fault tolerant and service proven.
- The ATC design shall include elements that can safely control train operations in case of an ATC failure.
- The train shall be forced to stop in case of an ATC failure that increases the risk of personal injury or equipment damage.

#### Friction Brake System, Compressed Air System, Parking Brake System, and Adhesion Management

- The Friction Brake system shall be highly dependable and service-proven.
- The Friction Brake system design shall have a secondary, operator activated method of releasing brakes in case brakes will not release. Train operation with brakes released by the secondary method shall be regulated to a safe speed.
- The Spin/Slide control system shall have independent units on each car to minimize the effects of any single failure.
- The Compressed Air system shall be fault-tolerant, so the train operation is not disrupted by a single air compressor failure.
- Communications from cab to brake units on each car shall be fault tolerant, diverse and redundant, with connections over both wired and wireless networks.
- The Friction Brake system shall enforce automatic application of emergency brakes in the case of a failure of electronic brake controls or a loss in brake pipe (or equivalent) continuity.
- The Friction Brake system design shall tolerate the failure of a single unit without affecting the function of any other unit.

#### Carbody

- The Carbody and all components (e.g. window and door seals) shall have sufficient strength and durability to withstand daily high speed train service.

#### Interior Furnishing and Finishes

- All interior furnishings shall be modular to enable simple and rapid repair and

1 replacement.

- 2 • Interior finishes shall be easy to clean and resistant to scratches and markings.
- 3 • Vestibule doors shall be manually openable, closable, and lockable in case of failure.

#### 4 Lighting

- 5 • Light fixtures and circuits shall be easily accessible to maintenance staff.

#### 6 Couplers

- 7 • The Coupler system shall be highly reliable and service-proven.
- 8 • Coupler design shall have a secondary, operator activated method of decoupling, if a
- 9 failure prevents decoupling.

#### 10 Truck Assemblies

- 11 • Truck Assemblies shall be highly reliable and service-proven.

#### 12 Fire Protection Systems and Extinguishing

- 13 • Fire Protection Systems shall be fault tolerant to ensure sufficient safety in the event of a
- 14 single failure.
- 15 • Fire alarms shall have manual switches to trigger an alarm in case of a sensor failure.

#### 16 Café Equipment

- 17 • Café equipment shall consist of off-the-shelf components to ensure the availability of
- 18 replacements over the specified lifetime and to permit fast, safe maintenance.
- 19 • Café equipment shall be sufficiently ventilated to prevent the buildup of smoke and
- 20 odors.
- 21 • Café equipment shall consist of off-the-shelf components to ensure the availability of
- 22 replacements over the specified lifetime and to permit fast, safe maintenance.

#### 23 Cab Controls (not ATC)

- 24 • Cab Controls shall be fault tolerant, diverse and redundant to prevent any single failure
- 25 from causing a service disruption.
- 26 • The Horn and Bell shall have independent controls to ensure that1 is available even if
- 27 the other fails.

#### 28 Water and Waste Water Systems



- The Water and Waste Water Systems shall be fault tolerant to prevent any single failure from causing a service disruption.

#### 30.4.6 Infrastructure RAM Design Criteria

Infrastructure design shall use service-proven concepts to meet the CRAM high-level KPI targets. Infrastructure design and implementation shall enable fast detection, response, access, and correction of failures or degradation if it occurs. The Infrastructure shall meet maintainability requirements from the CRAM.

The RAM Program Plan for Infrastructure shall recognize that all maintenance activities which require access to the track area can only occur during a maintenance window. The maintenance window is a time period scheduled in advance during CHSTS non-operating hours.

The RAM Program Plan for Infrastructure shall address the following general RAM design topics, as applicable to the contractor's scope:

- Design life of Infrastructure and equipment
- Design features and methods to ensure design life
- Identification and mitigation of availability affecting factors
- Accessibility to Infrastructure and equipment for maintenance
- Maintainability features to prolong equipment life
- Inspection plan
- Overhaul and replacement plans

#### Maintainability and Access

- Each structure shall be safely maintainable with service and repair times compatible with the CHST operating schedule and limited time per day for on-track access.
- Fast, easy access to equipment and structural elements that require routine maintenance, inspection, cleaning, replacement, or are otherwise vulnerable to wear or failure shall be provided. This includes physical access to confined spaces. This access shall not require the removal, deconstruction, or dismantling of structures or equipment.
- Access to equipment and structural elements shall minimize the need to isolate the OCS, for specialized equipment, or for occupation of the track.
- Due to the presence of high voltage equipment and supporting structures, special attention shall be paid to avoid the need to access the underside of bridges and viaducts and ceilings of tunnels directly above tracks for inspection and maintenance purposes.

- Visible fault indicators shall be provided to assist in the physical monitoring or repair of structural elements and equipment.
- Handling provisions shall be facilitated, including provisions such as lifting lugs for removal or replacement of heavy items or assemblies, forklift compatibility, and lifting limitations for manual handling.
- Provisions shall be made to allow for adjustment or alignment of equipment such that it can be done with no isolation or occupation of the operating tracks.

#### Aerial Structures (Rail Bridges, Road Bridges, Overpasses, etc.)

- Open type expansion joints shall have a trenched design to protect support components from weathering.
- Bearings shall be of a service proven design, and widely commercially available to ensure the supply of replacements.
- Rail bridges shall have sufficient stiffness and damping to protect from damage from excessive vertical acceleration and dynamic response.
- Each structure shall mitigate and minimize potential damage from seismic events.
- Aerial Structures shall include sensors strategically fitted on vulnerable points, such as load-bearing supports, to detect potential issues including but not limited to excessive bridge deck sag or vibration, allowing for targeted PM.
- Concrete characteristics shall match structural requirements and environmental conditions, to prevent degradation by acid attack, chloride-induced corrosion, alkali-aggregate reactions, and other wear mechanisms that weaken structures.
- Contractor shall use admixtures to enhance concrete durability.
- Repair or replacement of aerial structures for anything less than a catastrophic event shall not cause a train service interruption.

#### Earthworks (Trenches, Tunnels, Retaining Walls, Cuts, Embankments, etc.)

- Cuts and hillside modifications shall include erosion control measures and protection from falling rocks and debris where applicable.
- Earthworks shall include sensors strategically fitted on vulnerable points, such as load-bearing supports and walls, to detect potential issues including but not limited to lateral movement in retaining walls, drainage issues, or excessive vibration in tunnels, allowing for targeted PM.
- Tunnel ventilation systems shall be resilient to obstructions and shall allow fast, safe

1 access for cleaning.

- 2 • Drainage shall be designed to collect and remove excess water in case of accidental  
3 waterproofing failure.
- 4 • Concrete characteristics shall match structural requirements and environmental  
5 conditions, to prevent degradation by acid attack, chloride-induced corrosion, alkali-  
6 aggregate reactions, and other wear mechanisms that weaken structures.
- 7 • Contractor shall use admixtures to enhance concrete durability.
- 8 • Repair or replacement of earthworks for anything less than a catastrophic event shall not  
9 cause a train service interruption.



## Chapter 31

### Systems Assurance

RFP No. HSR 14-32 – INITIAL RELEASE - 05/27/2015

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	Sept 2013	Rolling Stock Procurement



## Table of Contents

31	SYSTEMS ASSURANCE (TBD) .....	31-1
----	-------------------------------	------

### Tables

No table of figures entries found.

### Figures

No table of figures entries found.

## Acronyms




## **31 Systems Assurance (TBD)**

- 1 TBD. This topic may be included in a future revision of the Design Criteria or will be included
- 2 in another document.



## Chapter 32

### System Safety and Security

RFP No.: HSR 14-32 – EXECUTION VERSION

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
2	Feb 14	Revision 2
3	Feb 2016	Revision 3



## Table of Contents

1	<b>32</b>	<b>SYSTEM SAFETY AND SECURITY</b>	<b>32-1</b>
2	32.1	Safety and Security Policy Statement	32-1
3	32.2	Scope	32-1
4	32.3	Regulations, Codes, and Standards	32-2
5	32.4	Goals	32-5
6	32.5	Hazard and Security Risk Management	32-5
7	32.5.1	Hazard Management	32-5
8	32.5.2	Security Risk Management	32-5
9	32.5.3	Safety and Security Certification	32-6
10	32.6	Construction Safety and Security	32-6
11	32.7	Designing for Safety and Security	32-7
12	32.7.1	Design Principles and Strategies	32-7
13	32.8	Safety and Security Design Criteria	32-11
14	32.8.1	Access Management	32-11
15	32.8.2	Fire Life Safety	32-13
16	32.9	Stations	32-15
17	32.9.1	General	32-15
18	32.9.2	Platforms	32-16
19	32.9.3	Public/Non-public Spaces	32-17
20	32.9.4	Walking Surfaces	32-17
21	32.9.5	Railings and Guardrails	32-17
22	32.9.6	Elevators, Escalators, and Stairs	32-17
23	32.9.7	Emergency Power	32-18
24	32.10	Parking Garages and Lots	32-22
25	32.11	Radio and SCADA Communications	32-23
26	32.12	Support Facilities	32-23
27	32.13	Traction Electrification	32-23
28	32.13.1	Overhead Contact System (OCS)	32-23
29	32.13.2	Traction Power Facilities	32-24
30	32.13.3	Emergency Power	32-24
31	32.14	Train Control	32-25
32	32.14.1	General	32-25
33	32.14.2	Automatic Train Control (ATC)	32-25
34	32.15	Operations Control Center (OCC)	32-25
35	32.15.1	TBD Communications	32-25
36	32.15.2	Displays and Controls	32-25
37	32.15.3	Alarms	32-25
38	32.15.4	Other Design Features	32-25
39	32.15.5	Physical Security of the OCC	32-25
40	32.16	Maintenance and Storage Facility	32-25
41	32.16.1	Yards	32-25
42	32.16.2	Shops	32-25
43	32.17	Rolling Stock	32-26
44	32.17.1	Onboard Diagnostic System	32-26
45	32.18	Alignment Configurations	32-26

**Chapter 32 – System Safety and Security**

---

1	32.18.1	General.....	32-26
2	32.18.2	At Grade Alignments .....	32-29
3	32.18.3	Embankments and Retained Fills .....	32-30
4	32.18.4	Trench Structures.....	32-30
5	32.18.5	Tunnels.....	32-31
6	32.18.6	Elevated Structures (Bridges and Aerial Structures) .....	32-32

## Acronyms

ADA	Americans with Disabilities
ATC	Automatic Train Control
Authority	California High-Speed Rail Authority
CBC	California Building Code
CCTV	Closed Circuit Television
Certification	Safety and Security Certification
CHST	<i>Not Used; use HST for California High-Speed Train</i>
CHSTS	California High-Speed Train System
CPTED	Crime Prevention Through Environmental Design
CPUC	California Public Utilities Commission
EEW	Earthquake Early Warning
HRI	Hazard Risk Index
HST	High-Speed Train
HVAC	Heating, Ventilation and Air Conditioning
HVM	Hostile Vehicle Mitigation
ICP	Incident Command Post
NFPA	National Fire Protection Agency
OCC	Operations Control Center
OCS	Overhead Contact System
OSFM	Office of the State Fire Marshal
PHA	Preliminary Hazard Analysis
PMT	Program Management Team
PtD	Prevention through Design
RAMS	Reliability, Availability, Maintainability and Safety
RCC	Regional Control Center
SiSHA	Site-Specific Hazard Analysis
SSI	Sensitive Security Information
SSMP	CHSTS Safety and Security Management Plan
TVA	Threat and Vulnerability Assessment
V&V	Verification and Validation
VVMP	CHSTS Verification and Validation Management Plan





## 32 System Safety and Security

### 32.1 Safety and Security Policy Statement

1 It is the policy of the California High-Speed Rail Authority (Authority) to perform work on the  
2 California High-Speed Train System (CHSTS) in a manner that ensures the safety and security  
3 of employees, contractors, emergency responders, and the public. The application of system  
4 safety and security is a fundamental hazard and vulnerability management process that  
5 incorporates the characteristics of planning, design, construction, testing, operational readiness,  
6 and subsequent operation of the high-speed rail system. Safety and security are priority  
7 considerations in the planning and execution of all work activities on the CHSTS.

8 All trains, facilities, systems and operational processes must be designed, constructed, and  
9 implemented in a manner that promotes the safety and security of persons and property. The  
10 design, construction, testing, and start-up of the CHSTS will comply with applicable safety and  
11 security laws, regulations, requirements and railroad industry practices. The Authority will  
12 maintain or improve upon the public transit and railroad industry standards for safety and  
13 security. Through the Reliability, Availability, Maintainability, and Safety (RAMS) Program, a  
14 standard of safety will be established that is as safe as or safer than conventional U.S. railroad  
15 operations. The design, construction, testing, and start-up of the CHSTS will be accomplished in  
16 compliance with this standard.

17 The Authority is committed to providing a safe and secure travel and work environment.  
18 Therefore safety, accident prevention, and security breach prevention must be incorporated into  
19 the performance of every employee task. All Authority, Program Management Team (PMT),  
20 and Contractor personnel, subcontractors, and employees are charged with the responsibility  
21 for ensuring the safety and security of employees, contractors, emergency responders, and the  
22 public who come in contact with the CHSTS. Each individual and organization is responsible  
23 for hazard and vulnerability management, for applying the processes that are designed to  
24 ensure safety and security, and for maintaining established safety and security standards,  
25 consistent with their position and organizational function. Through a cooperative team effort  
26 and the systemic application of safety and security principles, the CHSTS will be designed,  
27 constructed, tested, and placed into service in a safe and secure manner.

### 32.2 Scope

28 The processes for ensuring that the CHSTS is designed, built and prepared for operation in a  
29 safe and secure manner are described in the CHSTS Safety and Security Management Plan  
30 (SSMP). This chapter presents general design guideline processes for safety and security  
31 considerations for the CHSTS trackway and facilities, and technical design criteria for safety  
32 and security considerations not found elsewhere in the respective technical chapters of the  
33 Design Criteria.

### 32.3 Regulations, Codes, and Standards

- 1 Refer to the *General* chapter for requirements pertaining to regulations, codes, and standards.
- 2 Applicable codes and regulations include but are not limited to the following list:
- 3 - Occupational Safety and Health Administration (OSHA) CFR Title 29, Part 1910 *General*
- 4 *Industry*, and Part 1926 *Construction*
- 5 - Federal Railroad Administration (FRA)
- 6 - CFR Title 49, Part 270 *System Safety Program* (under development)
- 7 - CFR Title 49, Part 236, subparts H and I
- 8 - CFR Title 49, Part 1520 *Protection of Security Sensitive Information*
- 9 - CFR Title 49, Part 1580 *Rail Transportation Security*
- 10 - Federal Transit Administration (FTA)
- 11 - Circular 5800.1 Safety and Security Management Guidance for Major Capital Projects
- 12 - Handbook for Transit Safety and Security Certification
- 13 - Transportation Security Administration (TSA) security directive SD RAILPAX-04-01 dated
- 14 May 20, 2004 for existing railroads, security recommendations for high-speed rail (under
- 15 development)
- 16 - Americans with Disabilities Act of 1990 (ADA), 42 USC 12131-12165
- 17 - California Code of Regulations (CCR)
- 18 - Title 8, Tunnel Safety Orders
- 19 - Title 24, Part 2 *California Building Code (CBC)*
- 20 - Title 24, Part 9 *California Fire Code 2010 (CFC) Based on the 2009 International Fire Code*
- 21 - Title 24, Part 433 *Fixed Guideway Transit Systems (Stations)*
- 22 - California Health and Safety Code
- 23 - Chapter 6.95, Article 1, Sections 25500-25520 Hazardous Materials Business Plans
- 24 - California Public Utilities Commission (CPUC) General Orders
- 25 - U.S. Department of Defense

- 1       – MIL STD-882E, System Safety Program Requirements
- 2       – MIL-STD-1629A Procedures for Performing a Failure Mode, Effects, and Criticality
- 3       Analysis
- 4       - EN 50126-1 Railway applications - The specification and demonstration of Reliability,
- 5       Availability, Maintainability and Safety (RAMS)
- 6       - Hazard Analysis Guidelines for Transit Projects, January 2000, U.S., Department of
- 7       Transportation/FTA
- 8       - American National Standards Institute (ANSI)
- 9       – Standard Z590.3-2011 Prevention Through Design
- 10      – Standard A117.1. Accessible and Useable Buildings and Facilities
- 11      - National Fire Protection Association (NFPA)
- 12      – NFPA 1: Fire Code
- 13      – NFPA 3: Recommended Practice on Commissioning and Integrated Testing of Fire
- 14      Protection and Life Safety Systems
- 15      – NFPA 14: Standard for the Installation of Standpipe and Hose
- 16      – NFPA 70: National Electrical Code
- 17      – NFPA 101: Life Safety Code
- 18      – NFPA 130: Standard for Fixed Guideway Transit and Passenger Rail Systems as
- 19      applicable to the particular characteristics of the high-speed train system
- 20      - American Public Transportation Association (APTA)
- 21      – Recommended Practice for CCTV Camera Coverage and Field of View Criteria for
- 22      Passenger Facilities
- 23      – SS-SIS-RP-001-10 Recommended Practice for Security Lighting for Transit Passenger
- 24      Facilities
- 25      – SS-SIS-RP-002-10 Recommended Practice for Security Lighting for Nonrevenue Transit
- 26      Facilities
- 27      – SS-SIS-RP-003-10 Fencing Systems to Control Access to Transit Facilities

- 1       – SS-SIS-RP-004-10 Chain Link, Mesh or Woven Metal Fencing to Control Access to
- 2       Transit Facilities
- 3       – SS-SIS-RP-005-10 Gates to Control Access to Revenue and Nonrevenue Transit Facilities
- 4       – SS-SIS-RP-007-10 Crime Prevention Through Environmental Design (CPTED)
- 5       – SS-SIS-RP-009-12 Anti Vehicle Barriers for Public Transit
- 6       – SS-SIS-S-010-13 Security Considerations for Public Transit
- 7       – SS-SIS-RP-013-13 Physical Security for Public Transit
- 8       – SS-PS-002-98 Standard for Emergency Signage of Access/Egress of Passenger Rail
- 9       Equipment
- 10      - Crime Prevention Through Environmental Design, Timothy D. Crowe, National Crime
- 11      Prevention Institute
- 12      - Federal Transit Administration (FTA)
- 13       – Public Transportation System Security and Emergency Preparedness Planning Guide –
- 14       Report Number FTA-MA-25-5019-03-10
- 15       – Transit Security Design Guidelines – Report Number FTA-MA-26-7085-05
- 16       – Transit Security Handbook – Report Number FTA-MA-90-9007-98-1
- 17       – Recommended Emergency Preparedness Guidelines for Rail Transit Systems
- 18      - National Parking Association, Security Design for a Parking Facility
- 19      - Transit Cooperative Research Program (TCRP)
- 20       – Synthesis 21, Improving Transit Security
- 21       – Synthesis 27, Emergency Preparedness for Transit Terrorism
- 22       – Synthesis 80, Transit Security Update
- 23       – Report 86, Volume 12, Making Transportation Tunnels Safe and Secure
- 24       – Report 86, Volume 4, Intrusion Detection for Public Transportation Facilities Handbook

## 32.4 Goals

The goals of System Safety and Security are to achieve acceptable levels of hazard and security risk resulting in the following:

- Prevention of fatalities or injuries to passengers, employees, emergency responders, and the general public
- Prevention or minimization of damage to infrastructure and interruptions in service
- Protection of people (employees, contractors, emergency responders and passengers) and Authority property (facilities and equipment) from criminal acts

## 32.5 Hazard and Security Risk Management

### 32.5.1 Hazard Management

A hazard is a condition or circumstance that could lead to an unplanned or undesired event, which, when it occurs, can cause injury, illness, death, damage or loss of equipment or property, or severe environmental damage.

Hazards shall be managed to an acceptable level of risk in order to provide the Authority with a reasonable assurance that the CHSTS is designed, built, and placed into service in a safe and secure manner. An acceptable level of risk is achieved through the following:

- The identification of hazards that can reasonably be expected to occur during the life-cycle of the CHSTS
- Analyzing the hazards for severity and probability in order to assign a Hazard Risk Index (HRI)
- Application of appropriate mitigations that reduce the HRI to an acceptable level
- Tracking the application of mitigations to ensure that efforts to reduce the HRI to an acceptable level are effective and complete

Detailed procedures for hazard management, including roles and responsibilities of Designers, can be found in the Safety and Security Management Plan (SSMP).

### 32.5.2 Security Risk Management

Security risk is determined by evaluating potential threats against system vulnerabilities. Threats are defined as specific intentional acts that may damage the system, its facilities, or endanger passengers, employees or the general public. Threats include intentional actions that detract from overall security. These range from terrorist attack and major crime to crimes that

1 impact quality of life such as theft of services, pick pocketing, graffiti and vandalism.  
2 Vulnerability is defined as the susceptibility of the system to a particular type of security threat.  
3 The identified design security mitigations are incorporated in the respective technical chapters  
4 of the Design Criteria.  
5 Security risk management information, including the Threat and Vulnerability Assessment  
6 (TVA), shall be protected under the Authority's Sensitive Security Information (SSI) Policy and  
7 shared only with those persons with a need to know.

### **32.5.3 Safety and Security Certification**

8 The goal of the Safety and Security Certification Program (Certification) is to verify that  
9 identified safety and security requirements have been met, and to provide evidence that the  
10 CHSTS is safe and secure for revenue service. With respect to the Final Design and Construction  
11 phases, the objective of Safety and Security Certification is to document that safety and security  
12 certifiable elements of facilities, equipment, and operations have been designed, constructed,  
13 manufactured, inspected, installed, and tested in accordance with safety and security  
14 requirements. Safety and security certifiable elements are those elements that are to be proven  
15 to be safe and secure prior to the start of revenue service.

16 Safety and Security Certification shall be applied at the beginning of each project phase, and  
17 completion of certification activities for that project phase is required for advancing project  
18 elements into the next project phase.

19 Mitigations identified through the Hazard Management and Security Risk Management  
20 processes described in Sections 32.5.1 and 32.5.2, respectively shall be verified for completion  
21 and validated for effectiveness using the Verification and Validation (V&V) process described in  
22 the Verification and Validation Management Process (VVMP). Each safety and security  
23 certifiable element shall be considered Safety and Security Certified when the V&V process is  
24 successfully completed.

25 Detailed procedures and forms for Safety and Security Certification can be found in the Safety  
26 and Security Management Plan (SSMP).

## **32.6 Construction Safety and Security**

27 The CHSTS shall be designed so as to avoid, eliminate, or minimize the safety hazards and  
28 security risks to employees, contractors, the general public, Authority facilities and the  
29 environment during the Construction Phase.

## 32.7 Designing for Safety and Security

### 32.7.1 Design Principles and Strategies

Design of infrastructure shall promote and support a safe and secure high-speed train system. Safety is a primary requirement and many system components have functional requirements pertaining to safety. The implementation of safety and security measures improves both actual and perceived safety and security. The goal is to manage safety hazards and security risks so as to achieve a level of safety and security that is acceptable to the Authority. Design strategies for achieving an acceptable level of safety and security include Prevention through Design (PtD), Crime Prevention through Environmental Design (CPTED), and Protective Security Measures.

**Prevention through Design** – PtD is the concept of addressing safety and health needs in the design process to prevent or minimize the hazards and risks associated with the construction, manufacture, use, maintenance, retrofitting, and disposal of facilities, processes, materials, and equipment. PTD prescribes the application of a hierarchy of controls as follows:

1. Avoidance
2. Elimination
3. Substitution
4. Engineering Controls
5. Warnings
6. Administrative Controls
7. Personal Protective Equipment

**Crime Prevention through Environmental Design (CPTED)** – CPTED will be applied as appropriate to prevent and mitigate crime. CPTED practices will be integrated early in the design process. Design shall focus on natural surveillance, natural access control, territorial reinforcement, maintenance and activity support. By incorporating CPTED principles and considerations early in site planning, perimeter definition, sightlines, lighting, landscaping and other site design elements, implementation of alternate, more expensive engineering solutions can be avoided during later stages of design.

CPTED principles include improving sightlines and eliminating areas of concealment. Areas, spaces, or structures that provide concealment shall be avoided, particularly in stations, station sites, rolling stock yards and maintenance facilities, wayside facilities, parking facilities, bridges, tunnels, and other structures that can be improved through organization of space, architecture and lighting.

**Protective Security Measures** – Protective security measures are systems or activities that together contribute to a layered security program. These measures provide attributes that provide assistance with threat deterrence, delay, detection, response and/or recovery. Protective security measures can be design or procedural.

### 32.7.1.1 Safety Strategies

**Hazard Management** – Hazards shall be identified and assessed for their potential probability of occurrence and consequential effects to the system and hazard risk shall be mitigated to an acceptable level.

Hazards shall be assessed at the program level using Preliminary Hazard Analysis (PHA) techniques, and then assessed, as required, at the site-specific level using Site-specific Hazard Analysis (SiSHA) techniques. The hazard management program is described in detail in the SSMP.

**Site Selection** – The site selection for facilities shall consider safety hazards during the site selection process. Consideration shall be given to appropriate adjacent land uses and application of perimeter buffer zones to isolate the CHSTS from hazards where practicable.

**Separation** – High-speed train operations shall be isolated from outside influences to the extent practicable. Public at-grade highway-rail grade crossings on exclusive use corridors shall be eliminated through closure or grade separated from the trackway. CHSTS alignments located above, below, or adjacent to other railroad systems and roadways shall use lateral and/or vertical separation (where supported by hazard assessment) to prevent potential intrusion of vehicles into the trackway and to protect HST operating infrastructure. Additional physical elements, such as earthen berms and barrier walls, shall be used at locations where deemed appropriate through site-specific hazard analysis. Refer to the *Rolling Stock and Vehicle Intrusion Protection* chapter for technical requirements.

**Access Control** – Access to the trackway shall be controlled in order to mitigate the hazards associated with higher speeds. Non-employee access will be restricted to public areas in which engineering controls and administrative controls are applied to promote the safety of the public.

**Redundancy** – Safety-critical systems shall be robust and redundant with sufficient space for expanded, additional, and contingency measures. Alternative operating facilities shall be identified to allow for on-going operation of essential functions.

**Hazard Detection** – System-wide hazard detection shall identify those hazardous conditions that could affect the safety of the CHSTS including intrusion detection for persons, vehicles and large objects, rolling stock conditions, fire and smoke, and natural phenomena including earthquakes, climatic conditions (temperature, wind, and rain), wildfire, mudslides, falling rocks, and high water. Hazard detection systems shall be applied at high-risk locations as identified by the SiSHA.

**Monitoring** – The CHSTS shall be equipped with an integrated system for detecting, monitoring, surveillance, and response to environmental conditions, the railroad operating system, normal operations, and emergencies in real time. Monitoring detectors shall be linked to the central control system, the Operations Control Center (OCC) and/or Regional Control Centers (RCCs), the Automatic Train Control (ATC) system, and the traction power supply and



distribution systems. Unsafe conditions shall be conveyed through communications systems so that appropriate action may be taken.

**Communications** – Wired and wireless data transport networks, command and control systems, and detection and monitoring systems shall detect, notify and respond to safety and security incidents. High-speed train communications systems shall be interoperable and functional across the system and with applicable third party emergency responders.

**Lighting** – Facilities, such as , stairways, access/egress points, station platforms, sidewalks, and other public areas shall be well-lit to mitigate potential hazards identified in the site-specific hazard analysis. Power supply shall be redundant and able to operate in a compromised mode.

**Fire Protection** – The fire protection system shall include fire prevention, fire detection, fire suppression, and mitigation from fire damage including sufficient and safe evacuation capacity. Facilities shall be constructed of non-flammable or fire resistant materials, where practicable, and include fire detection, suppression and smoke removal systems.

**Emergency Evacuation** – Facilities shall provide capacity for safe emergency evacuation and compliance with emergency procedures. This includes the sizing and location of access and egress points, support infrastructure such as walkways, stairways, and access roads, and support systems such as lighting and communications.

**Signage** – Passenger and staff warning signs shall be used where appropriate or required. Unless required for safety reasons, signs identifying sensitive areas shall not be used for security reasons. Safety-critical signage shall be functional in abnormal lighting conditions.

### 32.7.1.2 Security Strategies

**Threat and Vulnerability Analysis (TVA)** – The TVA is initiated during the preliminary phases of the HST system development and reviewed and updated at each subsequent phase.

The TVA process will identify the likelihood of specific threats that may endanger CHSTS assets (people, property and information); the potential vulnerabilities associated with the design of the CHSTS; and mitigation efforts that can be designed into the CHSTS to reduce the risk and to minimize the consequences of identified potential criminal and terrorism activities.

Site-specific TVAs may be required to identify security design enhancements for specific locations or systems, to address additional requirements that may be required.

**Access Management** – Access management includes physical components and procedures that provide control of assets and activities around CHSTS facilities and restricted areas.

Access and egress points shall be minimized and sized for normal and emergency operation. The design shall strive to achieve fewer, more functional, and more maintainable access points.

1 **Access Control** –Access points to non-public areas, including the right-of-way, non-public  
2 facilities, non-public rooms within facilities accessed by the public, information systems and  
3 other designated shall be secured with access control technology.

4 Stations, maintenance facilities, system’s sites and offices of the CHSTS shall have the ability to  
5 be “locked down” to prevent entry by any unauthorized person.

6 **Full Security Access Check Points** – Design of HSR passenger stations shall provide space for  
7 implementation of security check points. If implemented, these activated check points shall be  
8 capable of accommodating queuing passengers for security checks prior to boarding.

9 **Barriers** – Barriers, including fences, gates, walls or other enclosures to deter unauthorized  
10 access and serve to demarcate and secure physical areas. The design of a barrier shall be  
11 commensurate with the risk from unauthorized intrusion. Determination of what type of barrier  
12 is based on several factors including whether the material is resistant to cutting; limits someone  
13 from crawling or burrowing beneath; and the requirement for visibility from and to both sides  
14 of the barrier. Design and application of barriers is defined further in the Civil chapter.

15 **Intrusion Detection** – Intrusion detection for designated restricted areas shall be included in the  
16 design and construction to mitigate the security threat against the system.

17 **Lighting** – Use of illumination can deter or allow detection of criminal or terrorist activity.  
18 Facilities, including tunnel portals, restricted access points, revenue collection, and parking lots  
19 shall be well-lit to mitigate potential vulnerabilities and, if required, provide for CCTV  
20 monitoring.

21 **Natural Surveillance** – Clear lines of sight and open walkways -shall be utilized to enhance  
22 security. Glass should be judiciously used as it is considered a source of secondary injury.  
23 Minimize impediments to natural surveillance such as alcoves, blind spots, and curved  
24 walkways. Where natural surveillance is restricted, mechanical surveillance (CCTV) shall be  
25 considered.

26 **Natural Access Control** – Street furniture, landscape design, walkways, walls or other design  
27 elements shall be utilized to provide natural access control and encourage people through  
28 public routes and discourage access to non-public or restricted areas. Entryways shall be sized  
29 to accommodate the normally expected ingress and egress demands.

30 **Territorial Reinforcement** – Territorial reinforcement is the use of the physical design to create  
31 or extend a sphere of control over that space. This concept includes use of signage, landscaping,  
32 pavement designs, and fences/barriers to define spaces and encourage the designated use of the  
33 space. Spaces shall be designed for their intended use, such as equipping emergency routes,  
34 stairs, exits with alarms, panic bars, time egress delays and no exterior handles.

35 **Detection Devices** – CCTV, intrusion detection or other such technological devices detect  
36 unauthorized access in the system. These can be used in conjunction with perimeter fencing or

other barriers and shall be placed where easily visible, but not easily accessible. Visibility adds to the sense of perceived risk by offenders.

**Monitoring** – An integrated system for surveillance and protection of critical infrastructure shall include intrusion detection, closed-circuit television and access control, reporting both locally and to the OCC and the appropriate RCCs.

**Structures** – Access to tunnels, sub-grade alignment (trenches), elevated structures and other wayside facilities shall be secured and include intrusion detection. Publicly accessible enclosures, such as bike lockers, or vendor kiosks shall be placed away from public gathering locations and station entrances. Hostile vehicle mitigation (HVM) placement may be appropriate to provide barriers to access of hostile or non-approved vehicles posing a threat to a structure. The HVM treatment could include raised curbs, bollards, permanent or pop-up barriers, vehicle gates or road calming measures that result in reduced speeds.

**Fire Resistant Material** – Fire resistant materials reduce the impacts of both accidental fire incidents but also incendiary or explosive attacks. Fire resistant materials provide heat protection and limit the amount of combustible material.

**Air Intakes** – Air intakes shall be elevated and secured. Station and facilities HVAC equipment will be designed in accordance with appropriate security considerations.

**Critical Systems Infrastructure** – Critical systems infrastructure, including signal system elements and communication, shall be protected and secured from intentional or unintentional breach.

**Utilities** – Incoming utilities shall be protected and secured. Essential utilities shall be redundant to the extent practical. Further information may be found in the *Utilities* chapter.

## 32.8 Safety and Security Design Criteria

### 32.8.1 Access Management

In the design of the CHSTS facilities, the Authority's Access Management Plan (TBD) determines the system's assets and what areas of the system and facilities shall be controlled. This will provide the ability to determine access requirements within a controlled facility or asset. Basic principles of access management include the following:

- Limiting the number of access points
- Identifying and dedicating secure areas
- Providing transitions between secure and non-secure areas
- Minimizing interference with the movement of passengers and operations

- Not interfering with fire protection and life safety systems
  - Conforming to Americans with Disabilities Act (ADA) requirements
  - Layering of security systems
  - Using protective measures addressing all threat phases – deterrence, detection, defense, mitigation, response and recovery
  - Providing an audit trail and/or transaction reporting capability
- Design of facility and infrastructure shall consider and implement the principles of access management.
- Refer to the *Civil* chapter for more detailed requirements.

#### **32.8.1.1 Access Control**

The trackway shall be fully access-controlled to allow only authorized persons. Emergency exits, rooms containing fixed equipment, corridors, stairwells and other controlled areas in at-grade, aerial, trench, and tunnel trackways shall have doors and/or gates with a lock system capable of preventing unauthorized access. A release mechanism that opens the doorway or gate from the inside in the event of an evacuation shall be provided for enclosed spaces such as rooms, buildings, stairwells, tunnels, trench structures, and aerial structures, and for at-grade access/egress points where the need is identified by a site-specific hazard analysis.

Public spaces, such as those at stations, shall be able to be fully closed outside of operational hours or during emergencies. Trains and some spaces within stations (e.g., platforms, paid areas) shall only be accessible to members of the public holding a valid ticket. In order to protect non-revenue or secure areas from unauthorized access, secured perimeters, including fencing, walls, or other physical barriers, electronic barriers (e.g., sensors and alarms), and natural barriers shall be implemented. The location and design of the secured perimeter shall consider effectiveness and functionality.

Doors shall be controlled by electronic control devices such as card readers or pin pads. Doors used for normal operations shall have natural surveillance where practicable. Emergency exit doors shall have both audio and visual alarms and shall comply with appropriate fire codes.

Devices such as turnstiles, tire shredders, roll-up gates, pop-up barriers or drop gates, may be used to prevent unauthorized passage of vehicles into parking areas. Stations shall be designed to promote maximum visibility over the surrounding area.

Electronic access points, including automatic gates, doors with access controls, sensor line gates and automated portals, shall be utilized over mechanical or staffed access control.

Access/egress points shall be secured from means of being opened by forcible entry tools, and shall also be equipped with a means of controlled access that allows entry by emergency

responders in the event of an emergency. Access/egress control systems shall comply with the California Building Code (CBC).

### 32.8.1.2 Public Spaces

The public interface occurs at specific, controlled locations: station facilities (including platforms, public concourse areas for ticketing, waiting, and vendor facilities, drop-off and pickup areas, and parking facilities), aboard trains, and at roadway and pedestrian walkway overcrossings/undercrossings. These spaces shall be open to public access, although station facilities will be segregated into paid and non-paid areas in order to control ticket possession and validation. Use of Authority-controlled public spaces shall be subject to conformance to Authority policy for use, including hours of access, congregations of people (intentional or not), demonstrations, vending, and other factors affecting the safety and flow of passengers.

Stations shall be equipped with barriers to prevent unauthorized entry into or across the trackway from the station platforms. Refer to the *Civil* and *Stations* chapters for detailed requirements. Barriers delineating the limits of public access shall be equipped with appropriate signage for trespass and warning of hazards.

Roadway overheads shall be equipped with a combination of rigid traffic barrier and access-restricting fencing to prevent the intrusion of vehicles, persons, and debris. Access-restricting fences at roadway overheads shall have a curved top section to deter the throwing of objects onto the trackway. In addition, a solid barrier shall be installed directly above the trackway to prevent contact with the OCS wires.

Refer to the *Rolling Stock and Vehicle Intrusion Protection* and *Overhead Contact System and Traction Power Return System* chapters and Standard and Directive Drawings for more detailed requirements.

### 32.8.1.3 Non-public Spaces

Facilities, rooms, and spaces used in the operation of high-speed trains shall be secured with locks and detection systems. Traction power facilities, ATC housings, communications equipment, and other core systems facilities, where located outside the right-of-way, shall be protected by an access-restricting fence or wall enclosure with secured gates.

Buildings, systems, and other facilities that are critical to high-speed train operation shall be made secure when located adjacent to public entrances, public vehicle circulation, public parking or other publicly accessible areas. These components include emergency generators, fuel storage, fire pumps, universal power supply facilities, critical heating, ventilation and air conditioning systems, and emergency power distribution feeders.

## 32.8.2 Fire Life Safety

Achieving an acceptable level of fire and life safety is a principal goal for the CHSTS. Each type of high-speed train facility shall have location-specific fire and life safety infrastructure, plans, and procedures. The principal guidance for fire and life safety infrastructure is found in NFPA

130, modified as appropriate to the peculiar characteristics of high-speed train operations and as amended by other NFPA standards. Fire protection strategies and designs shall focus on fire and smoke prevention, retardation of fire growth, detection, suppression, containment, and evacuation in order of precedence. CHSTS facilities shall be constructed of fire resistant materials, where practicable, and comply with State and local fire codes where applicable. The Office of the State Fire Marshal (OSFM) is the authority having jurisdiction, but local fire codes shall be applied where deemed appropriate by OSFM. Fire detection, suppression, and smoke removal systems shall be implemented. Response to fire and other emergency operations on trains or within the trackway must be first coordinated with the OCC for the protection of passengers, employees, and emergency responders through the control of trains and the high voltage power in the OCS.

**Access/Egress** - Access requirements for emergency response operations shall be provided for incidents such as fire, derailment/collision, or major passenger or employee injury. The design of the means of emergency access shall consider the typical fire apparatus size and operating characteristics as defined by the California Fire Code. Evacuations of a large number of people from underground structures or via aerial ladder truck are challenging, time-consuming, and difficult, especially when conducted from an extreme height. Access/egress gates or stairways are preferred. See Section 32.18 for requirements for access/egress points.

The design of the means of egress must consider the worst-case scenario in terms of passenger load (up to 1,000 people per train) and reasonable adverse conditions (night time, inclement weather). Typical passengers include the elderly, children, and disabled passengers, and egress design must allow for their particular safety needs. Egress analysis shall be performed to support the site-specific access/egress design requirements.

**Fire Prevention and Mitigation** - The prevention of fire is achieved through the use of non-flammable materials in the construction of infrastructure and rail vehicles. The effects of a fire, should one occur, shall be mitigated through the installation of on-board fire suppression systems in the high-speed trains, compartmentalization in the design of the trains and interior spaces of infrastructure such as tunnels, stations, maintenance facilities, system's sites, employee facilities, and traction-power facilities, standpipe systems in tunnel and sub-grade (trench) alignments, ventilation systems in tunnels greater than 1,000 feet in length as defined by Fire Hazard Analysis, and access/egress facilities appropriate for and specific to the site under consideration. Fire suppression strategies shall consider that the use of water and conductive chemicals is not compatible with the OCS.

**Smoke Removal** – In the event of a fire, a smoke removal system in enclosed spaces such as tunnels, underground stations, station concourses, and other confined environments, is critical for the safe and efficient passage of passengers and employees to a place of safe refuge. A mechanical ventilation system shall be provided in tunnels greater than 1,000 feet in length as defined by Fire Hazard Analysis. Smoke removal equipment shall be located away from high-risk areas, protected, and connected to emergency power. Separate smoke removal systems for different areas of a facility may be considered. Smoke removal equipment shall have the ability

to be controlled by both the OCC/RCC and local control panels in case the remote connection is lost.

**Reliability** – Fire prevention and mitigation systems shall have redundancies in order to overcome a local failure, and shall be encased, buried and otherwise protected from potential damage. Where appropriate, a dual fire pump arrangement shall be considered with one electric pump and one diesel pump.

## 32.9 Stations

### 32.9.1 General

Stations, by their nature, are locations where activities with the potential for fire or other hazards are present. In addition, the presence of large concentrations of passengers leads to unique requirements. Worst-case scenarios for occupant load and incidents must be considered. NFPA 130 is the principal guidance document except where modified by CBC 433.

Refer to the *Stations* chapter for more detailed technical requirements for station facilities.

#### 32.9.1.1 Fire Life Safety

An automatic sprinkler protection system shall be provided in areas of stations used for concessions, storage areas, trash rooms, and in the steel truss areas of all escalators and other similar areas with combustibles loadings, except trackways. Equipment rooms with electrical and electronic equipment, such as communications and ATC equipment rooms, shall have special fire protection provisions. Fire protection standpipe and hose systems shall be installed in accordance with NFPA 14 and as modified by NFPA 130. Fire hose cabinets and portable fire extinguishers shall be provided as required by NFPA 130. Stations shall be provided with a fire command center and emergency communications systems in accordance with NFPA 72 and as modified by NFPA 130. The Station Control Room at each station shall be configurable to an Incident Command Post for use by local emergency responders.

#### 32.9.1.2 Access/Egress

The guidance for access/egress at station facilities can be found in California Fire Code, NFPA 130, and CBC section 433 (as corrected to reflect the six minute evacuation rule).

Access points shall be located to preclude traffic congestion. Traffic patterns for vehicles and pedestrians shall be clearly marked.

Traffic patterns and site layouts shall be structured to permit rapid and easy access to all portions of the site and station by law enforcement and emergency responders. All portions of the interior of the stations shall be accessible to emergency personnel through the use of access controlling devices and procedures. Fire lanes shall be designated in the vehicle roadways and parking areas surrounding the station. At least one entrance shall be designated as the Emergency Access Entrance and identified as such in the Emergency Action Plan for the facility.

1 For security, access and egress points shall be minimized and sized for normal and emergency  
2 operation. The design shall strive to achieve fewer, more functional, and more maintainable  
3 access points.

4 Access into the paid areas (platforms) of the stations must allow for the set up and deployment  
5 of a physical security checkpoint, if required. If deployed, checkpoints shall accommodate all  
6 passenger traffic.

### **32.9.2 Platforms**

7 Special consideration shall be made for the hazards peculiar to station platforms including  
8 platform configuration, walking surfaces and markings, platform edge treatment, the effects of  
9 passing trains, Americans with Disabilities Act (ADA) requirements, and other mitigations for  
10 platform hazards.

11 Platform configurations shall be site-specific and conform to safety guidelines including  
12 minimum widths for two-track platforms and single-track platforms, height that allows for  
13 level boarding to the CHSTS equipment, cross-slope away from the track, longitudinal slope in  
14 conformance to the rail alignment.

15 A tactile warning strip, in conformance with ADA, shall be provided at the platform edge. The  
16 platform edge material shall be non-flammable, slip resistant, and of contrasting color to  
17 distinguish it from the main platform area. An under-platform safety refuge area shall be  
18 constructed to provide a place of refuge.

19 Platform obstructions shall be assessed on a site-specific basis so that these do not present visual  
20 or physical barriers to passengers, train operators, or emergency responders.

21 Signage at the ends of all platforms will be posted to warn of danger and restrictions on entry.

22 End of platform areas shall have gates separating the platform and public areas from the non-  
23 public, restricted area. Unauthorized entry through the platform end gates shall cause an  
24 intrusion alarm to be sent to the OCC.

25 Intrusion detection system shall be provided along the platform edge and detect accidental or  
26 intentional entry into the trackway. Upon detection, an alarm shall be sent to the OCC for  
27 appropriate operational response.

28 OCS support poles shall be located away from the platform areas where possible. If an  
29 energized conductor has to be located above a station platform, the wire height shall be set to  
30 meet the minimum clearance requirements specified in the *Grounding and Bonding Requirements*  
31 chapter.

32 Inter-track fences shall be installed to mitigate the effects of trains passing through the station  
33 facility on the center tracks at maximum authorized speed and to prevent people from crossing  
34 the tracks. The inter-track fences are described in detail in the *Civil* chapter.



**Evacuation** - Evacuation routes must be available to evacuate all passengers from the affected station platform (Platform Occupant Load) in four minutes or less and from the most remote point on the platform to a point of safety in six minutes or less. Once the last passenger has set foot upon a platform escalator or stair, sufficient escape routes shall be provided to enable evacuation to a point of safety within the remainder of the total 6 minutes. At least 2 means of egress, remote from each other shall be provided from each station platform.

Evacuation routes shall be planned so that a passenger confronted by an outbreak of fire can turn away and make a safe escape. To achieve this goal the maximum travel distance to an escape route, such as an escalator, stair, passageway or entrance, shall not exceed 325 feet. Fare gates shall assume an emergency exit mode in the event of loss of power to the fare gates or upon actuation of a manual or remote control.

Refer to the *Stations* chapter for additional requirements

### **32.9.3 Public/Non-public Spaces**

Differentiation between public and non-public space within the station facilities shall be determined with all non-public spaces access controlled and clearly identified as such. Specific areas within non-public spaces may require further control as determined by space usage.

### **32.9.4 Walking Surfaces**

Walking surfaces, including public areas, pedestrian access bridges, and ancillary spaces shall be constructed of materials selected for their slip resistant qualities in both wet and dry states.

### **32.9.5 Railings and Guardrails**

Safety railings and guardrails shall be provided to prevent falls from elevations four feet or greater.

Railings and guardrails shall meet the applicable municipal codes regarding heights and loadings. The design shall not permit dropped objects to roll underneath and fall to the lower level(s).

### **32.9.6 Elevators, Escalators, and Stairs**

#### **32.9.6.1 Elevators**

Elevators shall meet the requirements of ANSI 117.1.

Hands-free, two-way communications shall be provided between patrons within the cab and OCC.

Elevator hoistway doors at the surface and the elevator machine room door shall be equipped with intrusion alarm sensors to detect unauthorized entry. The alarm shall annunciate locally and at the OCC.

1 Remote elevator indicators shall be provided at the OCC. Each elevator cab shall have a camera  
2 mounted in the cab, with the intent to obtain full coverage and field of view of the cab interior  
3 and entrance to monitor and identify passengers.

4 Elevators shall be glazed or have transparent panels to allow an unobstructed view both inside  
5 and outside of the car.

6 Elevators shall be passenger-activated, with a means provided to allow control from the OCC.

#### **32.9.6.2 Escalators**

7 Escalators shall meet the requirements of ANSI 117.1

8 Adequate queuing shall be provided at the top and bottom landings.

9 Signing and graphics shall be provided to enable patrons to determine the direction of escalator  
10 motion prior to their arrival at, and clear of, the landing plate.

11 OCC shall have the capability to monitor the status of escalators at each station.

#### **32.9.6.3 Stairs**

12 Stair tread-riser relationship shall be one that easily accommodates travel in both directions, is  
13 usable under all types of weather, and minimizes the hazard of tripping or falling. Additionally,  
14 the following shall be provided:

- 15 • Tread-riser relationship shall meet the requirements of the CBC, Chapter 10, Section 1009  
16 and 1012.
- 17 • Stairs shall be of a slip-resistant material with an eased nosing that is distinct and meets the  
18 requirements of 28 CFR, Part 36 4.9.3 (ADA).
- 19 • When gutters/runnels are provided, they shall be protected by the handrails.
- 20 • Handrails shall be continuous and meet the requirements of 28 CFR, Part 36 4.26 and 49  
21 CFR, Part 38 38.83(13) (ADA) and CBC, Chapter 10, Section 1009.

22 Emergency stairs shall be properly designated with signage and have CCTV surveillance and  
23 monitoring coverage to observe personnel entering the emergency stairs. Entrance into or  
24 through these areas shall send an intrusion alarm to the OCC.

#### **32.9.7 Emergency Power**

26 Stations shall be equipped with emergency power systems with a minimum capability of  
27 supplying power for 6 hours.

28 As a minimum, emergency power shall be provided for the following functions:

- 1 • Emergency lighting
- 2 • Fire command center and station control center
- 3 • Emergency communication systems
- 4 • Train control systems directly associated with station operations
- 5 • Automatic and manual fire suppression systems
- 6 • Fire detection and alarming system
- 7 • Station ventilations systems
- 8 • Security detection and alarming system
- 9 • CCTV system cameras
- 10 • Emergency egress and exit signage
- 11 • Equipment shall be placed in locations that do not impede the movement of passengers or
- 12 staff.
- 13 • During power failures, emergency power shall be available at designated locations of each
- 14 station and for those functions considered critical.
- 15 • At each station, a quick-connect provision for a generator system shall be provided for use
- 16 in the event of extended local utility power interruption.
- 17

#### **32.9.7.1 Lighting**

18 Lighting is an important element in the development of safety and security requirements.  
19 Lighting allows for walkway hazards and other obstructions to be seen more clearly, provides  
20 guidance for appropriate pathways, and enhances security sight lines by improving natural  
21 surveillance. Lighting shall complement CCTV systems to ensure CCTVs are an effective tool  
22 for surveillance and investigation. Lighting shall be installed in vandal-resistant coverings.  
23 Designers shall reference the Illuminating Engineering Society of North America (IESNA) for  
24 recommended safety and security lighting practices.

25 CHSTS facilities shall be illuminated to provide emergency egress for passengers and access for  
26 employees and emergency responders. Facilities include access/egress points, emergency  
27 vehicle assembly areas, walkways in tunnels, tunnel portal facilities, traction power facilities,  
28 system sites, and maintenance facilities.

29 Emergency lighting for passenger areas and emergency egress pathways shall be provided in  
30 accordance with NFPA 101 and as modified by NFPA 130 and CBC.

### 32.9.7.2 Landscaping

Landscaping shall conform to CPTED principles. Landscaping that could allow for concealment or reduce sightlines shall be avoided. Landscaping shall permit law enforcement, security and operations personnel to observe activities throughout the facility. Landscaping may be used, as appropriate, to direct the movement of people or keep people away from an area.

Trees shall have, at a minimum, a 6-foot canopy at the lower edge; hedges should not be taller than 2 feet. Perimeter fencing and walls shall have a minimum 10-foot clearance from the nearest planting, allowing for easy observation of potential trespassers or weapons concealment.

### 32.9.7.3 Chemical, Biological, and Radiological (CBR) Detection

Chemical, Biological, and Radiological detection systems (CBR) shall be installed at high-risk station locations as identified in a site-specific TVA.

### 32.9.7.4 Closed Circuit Television (CCTV)

CCTV will be used throughout the system at high-risk locations as identified in a site-specific TVA or hazard analysis to monitor and record activities.

Each station shall have a sufficient number of cameras to provide an overview of the facility and monitor patron activities. Camera position shall take into consideration bi-directional flow.

Security cameras shall be encased in vandal- and weather-resistant housings. Lenses shall be easily replaceable with the proper tools.

To the maximum extent possible, security camera locations shall be unobtrusive, to further minimize the probability of vandalism.

Cameras shall be mounted as high as possible to maximize the field of view and reduce accessibility by vandals.

Cameras shall avoid obstructions such as structures, shelters, trees and vehicles.

CCTV camera fields of view coverage and monitoring requirements are defined in the Communications chapter.

Access control, intrusion alarm systems, and emergency telephones locations, including blue light stations, shall be integrated with the CCTV system. Upon an alarm condition, access indication, or emergency telephone use, an alarm and access indication shall annunciate in the OCC, and the images of the camera monitoring that location shall be immediately displayed in the OCC.

Passenger assistance telephone locations shall be integrated with the CCTV system. Upon passenger assistance telephone use, an alarm and indication shall annunciate in the OCC, and the images of the camera monitoring that location shall be immediately displayed in the OCC.

- 1 Camera monitors shall function on emergency power during loss of primary power.
- 2 CCTV camera images shall be transmitted to the OCC monitors and local security facilities and  
3 be integrated with access control and intrusion detection equipment.
- 4 All CCTV camera images shall be recorded. The security camera system shall have digital video  
5 recording minimum capability of 31-day storage, based on first in/first out.
- 6 Other facility specific requirements for CCTV shall be identified through site-specific analysis  
7 and be addressed in the specific design criteria chapter.
- 8 In spaces where CCTV lighting is inappropriate due to environmental impacts, infrared  
9 receptors with infrared cameras or other appropriate technologies may be used.

#### **32.9.7.5 Public Address System**

- 10 Speakers and amplifiers shall provide coverage to all public areas.

#### **32.9.7.6 Emergency Telephones and Alarms**

- 11 Emergency call buttons, alarms, or telephones shall be installed in stations, on station sites and  
12 parking areas, in tunnels, on aerial structures and bridges, and in other select places along the  
13 alignment including emergency access/egress points. The location of such telephone systems  
14 shall be designated with a blue light and integrated with the emergency power supply.

#### **32.9.7.7 Materials, Surfaces and Fasteners**

- 15 Sufficient clear space shall be provided around overhead and side projections and corners to  
16 reduce the potential for bumping and walking into protrusions.

#### **32.9.7.8 Security Alarms, Locks and Gates**

- 17 Differentiation of public and non-public spaces within station facilities shall be achieved using  
18 design and access controls to define public access and non-public spaces. Secure areas requiring  
19 further security measures, such as station control rooms, shall additional controls to limit access  
20 and detect intrusion.

#### **32.9.7.9 Overall Architectural Features for Visibility**

- 21 By making customers more visible to one another, to operations and maintenance personnel,  
22 and to the general public, much of the opportunity for criminal activity is removed. High  
23 visibility facilitates increased detection of unlawful behavior and, if required, rapid response by  
24 station and law enforcement personnel. At a minimum the following shall be considered with  
25 the architectural features:

- 26 • Public areas of stations, including platforms, shall be as open as possible with long,  
27 unbroken lines of sight, eliminating dark or obscure areas. Nooks and crannies shall be  
28 avoided.

- 1 • Columns and other structures shall be kept to a minimum so as not to impair sightlines  
2 within the station areas.
- 3 • Obstructions to visual and electronic surveillance shall be minimized.
- 4 • Horizontal surfaces and “shelves” shall be sloped so as not to permit the placement of  
5 packages or other objects.
- 6 • Special attention shall be paid to areas where there are changes from one area to another.  
7 Avoid 90 degree corners by angling corners or severely curved areas where possible. The  
8 use of mirrors or other reflective materials shall be used to eliminate or reduce “blind”  
9 corners.
- 10 • Entrances to stations shall be readily identifiable.

#### 32.9.7.10 Security and Policing Facilities

11 Facilities to support security and policing operations must be included in the CHSTS  
12 infrastructure. Stations shall have offices, locker rooms and vehicle parking spaces. Specific  
13 stations will have detention and interview facilities. Security spaces shall have additional access  
14 control to prevent unauthorized access. Requirements for Police and Security space are found  
15 in the Stations chapter.

### 32.10 Parking Garages and Lots

16 Protected parking can enhance the reality and perception of security. The design shall be  
17 guided by the National Parking Association’s publication *Security Design for A Parking Facility*.  
18 At a minimum the following shall be included when designing parking facilities:

- 19 • Parking lots and garages shall be fenced where appropriate and required by local municipal  
20 code, and provide a high degree of visibility by passers-by and law enforcement. They shall  
21 be designed to maintain natural surveillance (openness) and territorial reinforcement.
- 22 • Parking lots and parking structure aisles, passageways, and pedestrian walkways shall be  
23 uniformly lighted. Refer to the *Facility Power and Lighting Systems* chapter.
- 24 • Within the structure, pedestrian movements shall be directed along the driving aisles to the  
25 primary vertical circulation element(s). Their route of travel shall be direct and clearly  
26 defined. Where pedestrians must cross vehicular traffic inside the parking/garage or lot,  
27 clearly defined crosswalks shall be provided.
- 28 • Controlled access to the parking lot or garage shall be provided whenever possible by the  
29 use of parapets, fencing, wire mesh partitions, and/or spiny landscaping materials.
- 30 • Avoid dead end spaces and dark corners in parking areas.

- 1 • Lighting shall be in accordance with applicable local municipal code, to prevent the creation  
2 of a dark area when the sun is down, and shall be uniform.
- 3 • Emergency lighting shall be provided in the parking structures and lots.
- 4 • CCTV coverage and field of views shall include pedestrian and vehicular entrances and  
5 exits, and an overview of the parking areas.
- 6 • CCTV systems in garages and parking lots shall conform to CCTV systems criteria as  
7 described in Section 32.9.8.4. Refer to the Communications chapter for additional  
8 requirements.
- 9 • Elevators shall conform to the criteria for station elevators in Section 32.9.6.1.
- 10 • Emergency telephone stations shall be provided near all stairways and elevators at garages  
11 and shall be centrally located at all parking lots. The location of such telephone systems  
12 shall be designated with a blue light and integrated with the emergency power supply.
- 13 • Security mirrors or reflective surfaces shall be used in stairwells and other corners to allow  
14 pedestrians the opportunity to observe what is around corners without being surprised,  
15 providing both a sense of security as well as reducing the opportunity for potential  
16 confrontations with undesirable persons. This reduces vulnerability to an ambush-type of  
17 offense.

## 32.11 Radio and SCADA Communications

18 TBD

## 32.12 Support Facilities

19 Design of access/egress for all support facilities shall be in compliance with the requirements of  
20 California Code of Regulations, Title 24, Part 9 *California Fire Code*. Support facilities are  
21 considered to be non-public spaces and shall have controlled access on all points of entry.

## 32.13 Traction Electrification

### 32.13.1 Overhead Contact System (OCS)

22 The operation of the OCS shall be controlled from the OCC but shall have the ability to be  
23 isolated from the OCC or other remote locations. Generally, grounding of the conductors shall  
24 be performed locally by qualified railroad employees. Emergency responders will be required  
25 to confirm isolation and grounding of the OCS prior to commencing operations that might  
26 interfere with or otherwise bring them into contact with the OCS.

1 Special considerations for the OCS system shall be included in the development of emergency  
2 response plans for all infrastructure elements. OCS poles shall be located outside of the  
3 dynamic train envelope and the walkway envelope.

4 The *Overhead Contact System and Traction Power Return System* chapter identifies in greater detail  
5 the operation of and design requirements for the OCS system.

### 32.13.2 Traction Power Facilities

6 TBD

#### 32.13.2.1 Access Control to Traction Power Facilities

7 Access to Traction Power facilities shall be restricted to properly trained and authorized  
8 personnel only.

9 Entrances shall be locked, and entry shall be by means of an access control system.

10 An intrusion detection system shall be provided to detect unauthorized entry. An unauthorized  
11 entry shall be annunciated in the OCC.

12 A CCTV camera shall be provided to monitor entries into the facility and shall be integrated  
13 with the access control and intrusion detection systems. Camera images shall be transmitted to  
14 the OCC and recorded. The exterior of the facility shall be illuminated to supplement the CCTV  
15 system.

16 Traction Power facilities shall be fenced to prevent access from both the outside and from the  
17 HSR trackway.

#### 32.13.3 Emergency Power

18 **Distributed Emergency Power** – Emergency power infrastructure shall be installed separately,  
19 and as far apart as possible from normal electrical infrastructure. This includes electric panels,  
20 conduits and switchgear as well as electric distribution.

21 **Normal and Emergency Fuel Storage** – Fuel storage shall be access restricted and protected.  
22 Main fuel storage shall be located away from loading docks, entrances and parking. Emergency  
23 fuel storage shall be sized based on the facility it is supporting. Emergency batteries or  
24 uninterrupted power supply may replace or supplement emergency generators at some sites.

25 **Emergency Power** – An emergency generator shall be located away from the station building,  
26 loading docks, entrances, parking, and areas prone to flooding. The facility housing the  
27 generator shall be secured. Conduit and cabling shall be installed outside of a facility where  
28 practicable to enable mobile generator support of the facility's electrical system.



## **32.14 Train Control**

1 TBD

### **32.14.1 General**

2 TBD

### **32.14.2 Automatic Train Control (ATC)**

3 TBD

## **32.15 Operations Control Center (OCC)**

### **32.15.1 TBD Communications**

4 TBD

### **32.15.2 Displays and Controls**

5 TBD

### **32.15.3 Alarms**

6 TBD

### **32.15.4 Other Design Features**

7 TBD

### **32.15.5 Physical Security of the OCC**

8 TBD

## **32.16 Maintenance and Storage Facility**

9 TBD

### **32.16.1 Yards**

10 TBD

### **32.16.2 Shops**

11 TBD

## 32.17 Rolling Stock

### 32.17.1 Onboard Diagnostic System

1 Onboard diagnosis shall be installed to monitor the CHSTS rolling stock for performance and to  
2 detect the presence of hazards such as fire and smoke inside the carbody, on under-carbody  
3 electrical cabinets, propulsion system elements, and wheel bearings. Information from the on-  
4 board diagnostic systems shall be transmitted to the train operator, OCC, and vehicle  
5 maintenance facilities.

## 32.18 Alignment Configurations

### 32.18.1 General

#### 32.18.1.1 Walkways

6 For egress pathways the CHSTS shall be equipped with a continuous walkway to allow  
7 passengers to evacuate a train to a station or place of safety at any point along the trackway.  
8 Walkways shall be located adjacent to each track where practicable, and may be located  
9 between two adjacent tracks. Walkways will consist of a uniform, slip-resistant surface.  
10 Crosswalks connecting the two sides of the trackway with a walking surface uniform to the top  
11 of rail shall be located at access/egress points, special trackwork (switches and crossovers), and  
12 cross passageways. Raised walkways shall be provided with a continuous handrail along the  
13 side opposite the trackway. Refer to the *Trackway Clearances* and *Civil* chapters for detailed  
14 technical requirements.

#### 32.18.1.2 Access/Egress Points

15 Access/egress points for all alignment configurations shall be provided at a minimum of  
16 nominal 2.5 mile intervals, with a maximum 3.0 mile interval, co-located with traction power,  
17 train control, communication, or other CHSTS systems sites where practicable. Access/egress  
18 points will be located at more frequent intervals where they can be easily accommodated (e.g. at  
19 dead-end streets, adjacent roadways, etc.). The requirements found in this section apply to all  
20 alignment types. Further requirements for at-grade alignments, raised embankments, below-  
21 grade structures, tunnels, and elevated structures are found later in section 32.18.

22 Access/egress points will alternate sides of the trackway where possible. Crosswalks connecting  
23 the two (2) sides of the trackway with a walking surface uniform to or level with the top of rail  
24 shall be located at access/egress points and more often where reasonable feasible or where  
25 required by site-specific hazard analysis. Where several alignment configurations are present  
26 (at-grade alignments, raised embankment, aerial structure, etc.) the nominal distance shall be  
27 calculated over the combination of the various alignment configurations, retaining a continuous  
28 walkway configuration not to exceed the maximum 3.0 mile interval. Signage mounted on the  
29 handrail, fence or retaining wall structure at 1,000-foot intervals will direct passengers to the  
30 nearest access/egress point. Access and egress points shall have doors and/or gates with a lock

1 system capable of preventing unauthorized access. A release mechanism that opens the  
2 doorway or gate from the inside in the event of an evacuation shall be provided for enclosed  
3 spaces such as rooms, buildings, stairwells, tunnels, trench structures, and aerial structures, and  
4 for at-grade access/egress points where the need is identified by a site-specific hazard analysis.

5 Access/egress points that open to the trackway shall be equipped with basic facilities for  
6 emergency response: access road, emergency vehicle turning and staging areas (per the  
7 requirements of the *Civil* chapter), access-control gate, lighting, a communications link to the  
8 OCC and nearest RCC, and a traction power indication panel.

### 32.18.1.3 Intrusion Detection

9 Intrusion detection shall be located along the right-of-way and at CHSTS facilities identified as  
10 high-risk locations. Intrusion detection shall detect access to elevated trackways, air intakes,  
11 cross passages, subsurface to surface access locations, ATC interlocking houses,  
12 communications houses, traction power facilities, station platform edges, tunnel portals, and  
13 other critical infrastructure areas. Intrusion detection shall be employed at high-risk locations  
14 for theft, vandalism, accidental and intentional entry or other types of intrusion where required  
15 by site-specific hazard or threat/vulnerability analysis. The intrusion detection system will not  
16 be contiguous for the whole of the route. Intrusion detection systems shall report to the OCC  
17 and appropriate RCC as well as local administrative and/or security offices depending on the  
18 location.

19 **Fall Intrusion Detectors** shall be located at overheads and underpasses used by roadway  
20 vehicles, animals, and non-CHSTS equipment. Data output from the fall intrusion detectors  
21 shall be fed to the ATC system and available to railroad operations personnel.

22 **Rock Fall Intrusion Detectors** shall be located at tunnel portals and in areas adjacent to  
23 retained embankments and cut slopes that are prone to rock falls or other landslides. Data  
24 output from the fall intrusion detectors shall be fed to the ATC system and available to railroad  
25 operations personnel.

26 **Wayside Intrusion Detectors** shall be installed in the fencing along the CHSTS right-of-way to  
27 detect intrusion to the maximum extent practical. This system shall be installed in all areas  
28 where intrusion may occur, as supported by site-specific hazard analysis or threat/vulnerability  
29 assessment. These detectors shall be connected to the ATC system and to the OCC and shall  
30 alert the OCC when intrusion occurs.

### 32.18.1.4 Roadway and Rail Vehicle Intrusion Protection

31 The *Rolling Stock and Vehicle Intrusion Protection* chapter identifies in greater detail the  
32 assessment processes and infrastructure elements required for addressing protection against  
33 intrusion into the CHSTS trackway by railroad rolling stock or roadway vehicles.

34 Protection against intrusion by adjacent rail and mass transit operations shall be achieved  
35 through the implementation of possible mitigations, including horizontal separation, vertical

1 separation, installation of berms and/or ditches, or installation of intrusion prevention crash  
2 walls. Location and configuration of the intrusion protection infrastructure shall be supported  
3 by site-specific hazard analysis.

4 Protection against intrusion by roadway vehicles from adjacent roadways shall be achieved by  
5 locating the CHSTS trackway beyond a Clear Recovery Zone from the edge of the roadway, as  
6 identified by Caltrans in its Highway Design Manual. Where such separation cannot be met, a  
7 site-specific hazard analysis shall be conducted to determine the requirements for a physical  
8 barrier to avert intrusion. The barrier shall meet the requirements of Caltrans Traffic Manual,  
9 Chapter 7 and the *Rolling Stock and Vehicle Intrusion Protection* chapter.

#### **32.18.1.5 Protection against Hydrologic Hazards**

10 The CHSTS trackway shall be designed so as to mitigate the effects of water, considering the  
11 hydrology of the surrounding environment and the hydraulic requirements for drainage that  
12 preserves the safe operating conditions of the trackway. Hydrologic studies shall identify the  
13 ambient flood plain and determine the required mitigations including raising the trackway  
14 above the flood plain or the installation of barriers to prevent intrusion of flood waters into the  
15 trackway. Hydrologic studies shall also identify the potential runoff and determine the required  
16 mitigations, including drainage system requirements and outflow.

17 The *Drainage* chapter identifies in greater detail the assessment processes and infrastructure  
18 elements required for addressing hydraulic and hydrologic issues.

#### **32.18.1.6 Communications**

19 Stand-alone radio sites will be located along the CHSTS trackway at nominal 2.5 mile intervals,  
20 co-located with traction power or train control sites where possible. The facilities shall share  
21 vehicle access roadways with access/egress points as described in Section 32.18.1.2. Access  
22 control measures shall be provided as described in Section 32.8.1.1.

23 Maintenance of radio towers shall not interfere with CHSTP operating envelope.

#### **32.18.1.7 Safety and Security Signage**

24 Appropriate safety and security signage shall be used throughout the system to ensure safety  
25 for passengers, staff, and the public. Signage will be clear, consistent and implemented  
26 wherever directional or information might be needed by the public or passengers. Where  
27 applicable, signage shall cite California laws. Signage shall reflect general signage and  
28 wayfinding standards (TBD). Safety signage, such as those pertaining to warnings, trespassing,  
29 danger, and loitering, shall be used in stations, at bridges and overheads, at right-of-way edges,  
30 on and around trackways, and around and inside tunnels. Unless required for safety reasons or  
31 by other standards, signs shall not identify sensitive areas.

32 Signage shall follow ADA, APTA SS-PS-002-98, CPUC, NFPA 130, and CFR Title 29, Part 1910  
33 standards and guidance.

Emergency exit signs shall be provided in buildings per the CBC. Emergency exit signs shall also be provided on aerial structures and bridges, and in trenches and tunnels.

### 32.18.1.8 Potential Hazard Adjacency

**Wind Farms Setback Requirements** – Horizontal separation between the center line of the nearest CHSTS track and any wind turbine shall be no less than 1.5 times the overall wind turbine machine height (measured from grade to the top of the structure, including the uppermost extension of any blade).

**Oil and Gas Well Setback Requirements** – Active oil and gas wells shall be relocated outside the CHSRP right-of-way. Abandoned oil and gas wells located with the CHSRP right-of-way shall be re-abandoned per current State requirements.

**Adjacent Trees/Vegetation** - Adjacent properties shall be assessed for the presence of large trees or other vegetation that could reasonably have the potential to fall into or otherwise damage the CHSTS right-of-way in the event of a large storm event. Every effort shall be made to work with adjacent landowners to mitigate the effects of identified hazardous trees/vegetation.

**Adjacent Structures** – Adjacent properties shall be assessed for the presence of buildings and other tall structures that could reasonably have the potential to fall into or otherwise damage the CHSTS right-of-way during an earthquake or other event that compromises the integrity of the structure. Every effort shall be made to work with adjacent landowners to mitigate the effects of identified hazardous structures.

**Adjacent Hazardous Materials Facilities** - Adjacent properties within 500 feet of the CHSTS right-of-way shall be assessed for the presence of hazardous materials storage, processing, or distribution facilities, as defined by the California Health and Safety Code, Sections 25500-25520. Identified properties shall be analyzed for conditions that could reasonably have the potential to effect the safe operation of the CHSTS trains and every effort shall be made to work with adjacent landowners to mitigate the effects of identified hazardous materials facilities.

**Other Adjacent Hazardous Facilities** – Other hazardous facilities located adjacent the CHSTS trackway shall be analyzed for their potential to impact the safety or security of the CHSTS.

## 32.18.2 At Grade Alignments

### 32.18.2.1 Access Roads

Access from public roadways to the access/egress points shall be made via access roads per the criteria found in the *Civil* chapter. Access roads shall lead directly to the access point for the trackway and be protected from unauthorized use by an access control system. Access roads will continue inside the CHSTS fence line where practicable.

### 32.18.2.2 Walkways

- 1 Walkways shall be located outside the OCS support poles on both sides of the trackway, but
- 2 close enough to be accessible directly from the train via portable ramp or ladder equipment
- 3 carried aboard the trainset.

### 32.18.2.3 Access/Egress

- 4 Access/egress points for at-grade alignments shall be co-located with traction power facilities
- 5 (substations, switching stations, and paralleling stations), interlocking houses, stand-alone radio
- 6 sites, and other wayside facilities where practicable. Service roads will continue along the
- 7 interior of the trackway where practicable.

## 32.18.3 Embankments and Retained Fills

### 32.18.3.1 Walkways

- 8 Walkways shall be located outside the OCS support poles on both sides of the trackway, but
- 9 close enough to be accessible directly from the train via portable ramp or ladder equipment.
- 10 Sufficient shoulder on the raised embankment or retained fill shall be provided for the
- 11 walkways at track level. Retained fills with a precipice of greater than 30 inches shall be
- 12 provided with a fall-restraint barrier such as handrail or parapet walls. The barrier design shall
- 13 conform to the requirements of CCR Title 8, Chapter 4, Section 3210 Guardrails at Elevated
- 14 Locations.

### 32.18.3.2 Access/Egress

- 15 Access/egress points for raised embankments or retained fills shall be provided by stairways
- 16 that meet the minimum requirements of the CBC at nominal 2.5 mile intervals with a maximum
- 17 3.0 mile interval and co-located with traction power facilities, interlocking houses, stand-alone
- 18 radio sites, and other wayside facilities where practicable. Such stairways shall alternate sides
- 19 where possible and shall be located within 100 feet of the emergency vehicle staging area and
- 20 protected by an access-control pedestrian gate with a minimum 48-inch opening.

## 32.18.4 Trench Structures

- 21 Refer to the *Structures* chapter for detailed technical requirements.

### 32.18.4.1 Walkways

- 22 Walkways shall be located along the trench walls on the same side as the access/egress points
- 23 where possible. Walkways shall be illuminated to provide safe passage in the event of an
- 24 evacuation in accordance with NFPA 130.

### 32.18.4.2 Access/Egress

- 25 Access/egress for sub-grade trackway alignments shall be provided by stairways that meet the
- 26 minimum requirements of the CBC at nominal 2,500 feet intervals with a maximum 3,000 feet
- 27 interval. Such stairways shall be located within 100 feet of the emergency vehicle staging area
- 28 and protected by an access-control pedestrian gate with a minimum 48-inch opening.

### 32.18.4.3 Ambient Gas Detectors

Ambient gas detectors shall be installed in tunnels, trenches, underground stations, and other below-grade locations identified by site-specific hazard analysis, and at locations adjacent to solid-waste disposal sites regulated by California Code of Regulations Title 27, Chapter 6, Section 20921 and as supported by site-specific hazard analysis.

### 32.18.5 Tunnels

Tunnels, by their nature, present unique safety, security and fire life safety challenges. Detailed technical requirements are found in the *Tunnels* chapter.

#### 32.18.5.1 Walkways

Walkways shall be located along the tunnel walls on the same side as the access/egress points or cross passageways where possible. Walkways shall be illuminated to provide safe passage in the event of an evacuation, in accordance with the requirements of NFPA 130.

#### 32.18.5.2 Access/Egress

Access/egress from twin bore single-track tunnel structures shall be provided by passage to a place of refuge at 800 feet intervals. Egress from single bore double-track tunnels equipped with center walls shall be provided by fire doors opening to the opposite track. Egress from single bore two-track tunnels not equipped with center walls shall be provided by lateral or vertical exits to the surface every 2,500 feet.

#### 32.18.5.3 Doors

Doors shall be hinged to open in the exit direction where possible, or sliding doors if required due to obstructions on the other side of the door or where the doorway opens directly to another trackway. Doors shall be 1.5 hour fire-rated and equipped with self-closing mechanisms. Emergency egress doors shall not allow access into controlled areas.

#### 32.18.5.4 Cross Passages

Cross passageways shall be illuminated in the event of an incident requiring evacuation. Cross passageways shall be equipped with fire doors with self-closing mechanisms at each opening

#### 32.18.5.5 Tunnel Portals

Designing tunnel portal layout and facility requirements shall anticipate the requirements of response to an incident where there may be evacuees, emergency responders and potential smoke discharge.

A site-specific hazard analysis shall be conducted for each tunnel portal to determine the appropriate fire and life safety infrastructure to be provided at each specific tunnel portal. Consideration shall be given to tunnel length, proximity to other tunnels, surrounding topography, and access/egress routes. The *Tunnels* chapter provides details on how to determine the appropriate requirements of fire and life safety infrastructure for different categories of tunnels.

- 1 Security infrastructure at tunnel portal facilities includes intrusion protection fence, security
- 2 gates, intrusion detections systems, and landscaping designed with CPTED principles in mind.
- 3 Tunnel access/egress points leading to public space shall be access controlled.

#### **32.18.5.6 Fire**

- 4 Fire prevention and mitigation infrastructure shall include dry standpipe systems with
- 5 universal fire hose connections and a minimum 1-hour water supply.

#### **32.18.5.7 Communications**

- 6 Radio communications systems shall be provided for railroad operations and fire and life safety
- 7 operations separately, hard-wired communications stations at each cross passageway or other
- 8 access/egress points.

#### **32.18.5.8 Lighting**

- 9 Lighting for the walkways shall be provided in all tunnels in excess of 250 feet in length. NFPA
- 10 130 is the primary source of guidance.

#### **32.18.5.9 Signage**

- 11 Signage shall be provided directing passengers to the nearest egress point.

#### **32.18.5.10 Ventilation**

- 12 Ventilation systems shall be installed in tunnels greater than 1,000 feet in length where
- 13 supported by a Fire Hazard Analysis report. Ventilation systems shall be sized to allow
- 14 maintenance of a smoke-free environment for duration sufficient to allow passenger egress to a
- 15 place of safe refuge.

### **32.18.6 Elevated Structures (Bridges and Aerial Structures)**

#### **32.18.6.1 Walkways**

- 16 Walkways shall be located on the outer edges of the aerial structures, inside of the OCS support
- 17 poles. Aerial structures shall be equipped with a fall-restraint barrier such as handrails or
- 18 continuous parapet walls to prevent persons from falling from the structure. The barrier design
- 19 shall conform to the requirements of CCR Title 8, Chapter 4, Section 3210 Guardrails at Elevated
- 20 Locations.

#### **32.18.6.2 Access/Egress**

- 21 Access/egress for aerial alignments shall be provided by stairways that meet the minimum
- 22 requirements of the CBC at nominal 2.5 mile intervals with a maximum 3.0 mile interval and co-
- 23 located with traction power facilities, interlocking houses, stand-alone radio sites, and other
- 24 wayside facilities where practicable. Such stairways shall alternate sides where possible and
- 25 shall be located within 100 feet of the emergency vehicle staging area and protected by an
- 26 access-control pedestrian gate or enclosure with a minimum 48-inch opening.



1 In addition to access/egress stairways at nominal 2.5 mile intervals, alignments with restricted  
2 access to the right-of-way through the fence (aerial structures, trench structures) require  
3 additional opportunities for emergency access. Access to aerial structures by aerial ladder  
4 trucks shall be afforded at nominal 2,500 feet intervals with a maximum interval of 3,000 feet.  
5 Access can be made from public roadways, but provision must be made for access through  
6 private property where public property access is not available within the required interval  
7 distances.

8 Every effort must be made to design facilities that allow for egress from aerial structures by  
9 other than aerial ladder truck.

### 32.18.6.3 Spaces underneath Elevated Structures

10 Spaces under elevated structures shall be owned and controlled by the Authority with the  
11 following potential exceptions:

- 12 • Controlled public spaces, inclusive of Fire or Police Stations or Public Transportation  
13 facilities where safety and security measures can be determined to be effective and  
14 maintained ; or
- 15 • Uncontrolled public spaces that minimize combustible materials, including landscape  
16 materials, and provide for the creation of safe spaces appropriate for public gatherings.

17 In no case shall space underneath elevated structures include hazardous materials, explosives,  
18 flammable materials, or other elements that might pose a safety or security risk to the CHSTS  
19 operation.

20 Public structures or activities that are considered for placement under elevated structures shall  
21 undergo a site-specific safety and security assessment to ensure that the structure or activity  
22 does not increase the risk to the Authority.

23 Uncontrolled private buildings or spaces will not be allowed under the elevated ROW without  
24 review and approval by the Authority.

25 Requests for waivers to this policy will be considered through a safety and security risk  
26 assessment to determine the risk of the specific structure or activity on the operation and assets  
27 of the Authority.

## Appendix

### Design Terms

Revision	Date	Description
0	02 Mar 12	Initial Release, R0
1	Feb 2014	Revision 1



## Design Terms

2x25 kV Autotransformer System:

A Power Supply scheme for electrified railways that utilizes a Catenary energized at 25 kV to ground and an along track Negative Feeder also energized at 25 kV to ground. The 25 kV catenary and 25 kV negative feeder supplies are derived via connections to an Autotransformer or the secondary winding of a Main Transformer, which results in a phase difference of 180° between the voltages, giving 50 kV between the high voltage bushings.

## A

Abandoned Mines:

A collective term referring to the mapped or otherwise known presence of subsurface voids resulting from man-made mining or other subsurface tunneling activities.

Absolute Block:

A section of track into which a train is not permitted to enter while it is occupied by another train, except as provided for by the rules.

Access Control:

Restriction of access to or from a highway or railway right-of-way or facilities.

Access Control System:

System which enables the Authority to control access to areas and resources in a physical facility or area.

Accessible Voltage:

That part of the Rail Potential under operating conditions which can be bridged by persons, the conductive path being conventionally from hand to both feet through the body, or from hand to hand (horizontal distance of one meter to a touchable point).

Accessibility:

The ease with which a site or facility may be reached by passengers and others necessary to the facility's intended function. Also, the extent to which a facility is usable by persons with disabilities, including wheelchair users.

Active Fault:

A fault that has either known or is suspected of having had tectonic movement within Holocene time (past 11,000 years).

Administrative Telephone (ATEL):

Telephones which provide fixed voice communications between Authority employees to conduct daily operations. Part of the Telephone and Intercom System (TIS).

<u>Aerial Ground Wire:</u>	<i>See Static Wire.</i>
<u>Aerial Structure:</u>	Trackway section placed on a structure, other than a culvert, which spans above earthen, paved, or water surfaces including roadways, railroads, and water channels. <i>Also, called elevated guideway.</i>
<u>Aerodynamic Force:</u>	In regard to pantograph, additional vertical force applied to the pantograph as a result of air flow around the pantograph assembly.
<u>Alignment:</u>	The horizontal and vertical route of a transportation corridor or path.
<u>Ambient Noise Sensor:</u>	Sensor which detects background noise to automatically adjust Public Address output level to maintain audible output level.
<u>Ambient Temperature:</u>	Outdoor air temperature measured with a thermometer (or other temperature measuring device) and located so that it is protected from direct sunlight and wind effects.
<u>Americans with Disabilities Act (ADA):</u>	Federal regulation establishing legal requirements for accessibility. The Act prohibits discrimination on the basis of disability in employment, State and local government, public accommodations, commercial facilities, transportation, and telecommunications.
<u>Approximate Location:</u>	In regard to underground facilities, the “approximate location of subsurface installations” being a strip of land not greater than 24 inches on either side of the exterior surface of the subsurface installation. “Approximate Location” does not define depth.
<u>Arc:</u>	(Electrical arc) Electrical discharge which breaks down the insulation characteristic of air, permitting current to jump across the space between two contacts.
<u>Arcing:</u>	In regard to pantograph, the flow of current through an air gap between a pantograph contact strip and the contact wire, which results in erosion of both elements and is usually indicated by the emission of intense light, and which can result in radio frequency interference.
<u>Area of Impact:</u>	The area of required utility construction or relocation due to the California High-Speed Train Project (CHSTP).

<u>Area of Influence:</u>	The area parallel to and 900 ft transverse to the centerline of the nearest track, or as defined in the Environmental Document, whichever is greater.
<u>At-Grade:</u>	At ground surface level; used to describe roadways, river crossings, and track alignments.
<u>Attenuation Relationship:</u>	Semi-empirical relationship to predict ground motions from a specific seismic source and event.
<u>Attenuation Time:</u>	The time required for the vehicle motion to stabilize after crossing a point of change in the nature of the alignment.
<u>Aspect:</u>	The appearance of a wayside signal, conveying an indication, as viewed from the direction of an approaching train; the appearance of an Onboard Cab Display as viewed in the cab.
<u>Authority:</u>	California High-Speed Rail Authority.
<u>Authorized Person:</u>	Any person who has been authorized by the Agency to enter restricted areas of the property.
<u>Automatic Train Control (ATC):</u>	The collective name for the train control subsystems that comprise the Automatic Train Protection, the Automatic Train Operation, and Automatic Train Supervision sets of functions that govern train operations on the main tracks.
<u>Automatic Train Control Mode (ATC Mode):</u>	A mode of operation which allows the Locomotive Engineer to control the acceleration and braking of a train subject to supervision of Automatic Train Protection.
<u>Automatic Train Control Bypass Mode (ATC Bypass Mode):</u>	A mode of operation which allows trains to proceed under the manual control of the engineer when there is an onboard ATC failure. Permission to break the ATC seal and enter this mode must be granted by the train dispatcher. Movement must not exceed the speeds prescribed by the operating rules and procedures.
<u>Automatic Train Control – Automatic Train Operation Mode (ATC-ATO Mode):</u>	A mode of operation in which the ATC system controls the acceleration and braking of a train subject to supervision of Automatic Train Protection.
<u>Automatic Train Control Territory:</u>	Territory equipped with Automatic Train Control (ATC) wayside equipment.

Automatic Train Operation (ATO): The functional set responsible for the automatic operation of throttle and brake commands to move trains between stations and other stopping locations (including those required due to the proximity of other trains and signal status) within the constraints imposed by the Automatic Train Protection functional set. Provides dwell timing at stations and the control or prompting of the opening and closing of train doors.

Automatic Train Protection (ATP): The functional set responsible for the safety-critical functions including those of interlocking, train detection, signal aspects, broken rail detection, hazard detectors (if implemented as part of the ATC system), and movement authorities (including speed limit and cab signal commands if appropriate) that are sent to the train and acted upon by the onboard train control to enforce safe limits. The ATP functional set includes the enforcement of the safety-critical functions. Positive Train Control functions are part of ATP.

Automatic Train Supervision (ATS): The functional set within the automatic train control system that is responsible for the centralized supervision and control of train movements; the ATS monitors trains, adjusts the performance of individual trains to maintain schedules, and provides data to adjust service to minimize inconveniences otherwise caused by irregularities. ATS also provides automatic and manual route setting at interlockings and the identification and tracking of trains, the display of alarms and events, and logging and storage of event data.

Autotransformer: Apparatus which helps boost the Overhead Contact System voltage and reduce the running rail return current in the 2x25 kV autotransformer feed configuration. It uses a single winding having three terminals. The intermediate terminal located at the midpoint of the winding is connected to the rail and the static wires, and the other two terminals are connected to the catenary and the negative feeder wires, respectively.

## **B**

Backslope: Resultant excavation face located between outer shoulder line and natural ground line.

Back-of-House Area: Area dedicated to station operational and support functions, with access restricted to station employees.

<u>Backwater:</u>	An unnaturally high state in stream caused by obstruction or confinement of flow, as by a dam, a bridge, or a levee. Its measure is the excess of unnatural over natural stage, not the difference in state upstream and downstream from its cause.
<u>Ballast:</u>	A selected material, usually crushed rock without fines, placed in a track to hold its position, distribute weight, dissipate force, and provide drainage.
<u>Ballasted Track:</u>	Track constructed with ties supported by and normally embedded in ballast
<u>Ballast-less Track:</u>	<i>See Non-Ballasted Track.</i>
<u>Barrier:</u>	A device intended to contain or redirect an errant vehicle by providing a physical limitation through which a vehicle would not typically pass.
<u>Barrier Offset Distance:</u>	The lateral distance from the centerline of the track to the face of the barrier, trackside, or other roadside feature.
<u>Base Flood:</u>	The flood having a one percent chance of being equaled or exceeded in any given year. This is the regulatory standard also referred to as the "100-year flood".
<u>Betterment:</u>	Improvements to the capacity and/or functionality of a utility system that is not required for safety, operation, and construction of the California High-Speed Train (CHST) System.
<u>Blanket:</u>	A layer of coarse grained material between ballast and subgrade, spread over entire width. It may be required over the formation where the subgrade soil is of poor quality, rainfall is heavy, and traffic density is high, as the absence of blanket in such cases can lead to problems in service.
<u>Block:</u>	A length of track of defined limits on which train movements are governed by information provided by the Onboard Cab Display, by interlocking signals, or by authorized manual methods.
<u>Blockage Ratio:</u>	Ratio of train cross section area to tunnel cross section area.
<u>Brake Horsepower:</u>	Actual horsepower applied to a fan shaft by the motor.
<u>Broadband Radio System:</u>	Part of the Radio System, designed to transmit high-bandwidth data between moving high-speed trains and the fixed wayside.



## C

<u>Cab:</u>	The location on a locomotive or high-speed train set from which the engine or train is operated by the locomotive engineer. The location on an Infrastructure Maintenance Vehicle from which on-track maintenance vehicle is operated by the driver.
<u>Cable Infrastructure (CI):</u>	The fiber optic and copper infrastructure and supporting equipment used to interconnect systems and field device equipment.
<u>Cantilever:</u>	In regard to Overhead Contact System (OCS), A frame for supporting and registering the OCS conductors, often including solid core insulators; for auto tensioned systems, the cantilever connections at the pole are hinged to accommodate along track movement of the conductors, thereby allowing the end of the cantilever away from the pole to swing.
<u>Capable Fault:</u>	A mapped or otherwise known Quaternary fault with evidence of Holocene displacement, structural relationship to related Holocene faults, and/or where data are not sufficient to rule out the presence of Holocene movement.
<u>Casualty:</u>	A serious injury or fatality.
<u>Catenary:</u>	An assembly of overhead wires consisting of, as a minimum, a messenger wire, carrying vertical hangers that support a solid contact wire which is the contact interface with operating electric train pantographs, and which supplies power from a central power source to an electrically-powered vehicle, such as a train.
<u>Center Platform:</u>	Passenger platform in-between two station tracks.
<u>Closed Circuit Television (CCTV):</u>	The use of video cameras to transmit a visual signal to specific places with limited viewing and recording.
<u>Clothoid Spiral:</u>	The most common type of spiral. The radius increases at a linear rate over the length of the spiral. <i>Also known as constant rate spiral.</i>
<u>Code:</u>	A type of legislation that purports to exhaustively cover a complete system of law on a specific subject matter to define a procedure or performance requirement.

<u>Cohesive Subgrade:</u>	Subgrade constructed with soils having cohesive behavior, i.e., soils where shear strength is predominantly derived from cohesion of the soil is termed as cohesive subgrade. Normally, soils having particles finer than 75 micron exceeding 12% exhibit cohesive behavior. All fine grained soils and GM, GM-GC, GC, SM, SM-SC and SC types of soils exhibit cohesive behavior. Acronyms are from the Uniform Soil Classification System.
<u>Cohesionless Subgrade:</u>	Subgrade constructed with cohesionless, coarse-grained soils, i.e., soils where shear strength is predominantly derived from internal friction of the soil and is termed as cohesionless subgrade. GW, GP, SW and SP types of soils fall in this category. Acronyms are from the Uniform Soil Classification System.
<u>Communications Interface Cabinet (CIC):</u>	A stand alone cabinet containing racks used for Communications and Supervisory Control and Data Acquisition equipment.
<u>Concourse:</u>	Open space for the gathering or passage of patrons.
<u>Connectivity:</u>	Describes the degree of “connectedness” of a transportation system such as a transit network, and the ease with which passengers can move from one point to another within the network, or points outside the network.
<u>Consist:</u>	A set of rolling stock that forms a train in service, a consist for HST can be a single trainset or more than one coupled together.
<u>Constant Rate Spiral:</u>	<i>See clothoid spiral.</i>
<u>Contact Force:</u>	The sum of forces for all contact points of one pantograph.
<u>Contact Point:</u>	Point of mechanical contact between a pantograph contact strip and a contact wire.
<u>Contact Wire:</u>	A solid grooved, bare aerial, overhead electrical conductor of an OCS that is suspended above the rail vehicles and which supplies the electrically powered vehicles with electrical energy through roof-mounted current collection equipment - pantographs - and with which the current collectors make direct electrical contact.
<u>Contact Wire Height:</u>	Height of the underside of the contact wire above top of rail level when not uplifted by the pantograph of an electric train.

<u>Containment:</u>	Engineered structure (steel, concrete or earthworks) designed to maintain a vehicle within a defined area.
<u>Contours:</u>	A variable curve that connects points with the same elevation value used to depict surface elevations on a contour map.
<u>Control:</u>	An established point on the earth's surface with a known position in the X, Y, Z coordinates and used for reference and mapping of field surveys.
<u>Control Center:</u>	<p>The location from which remote control signal appliances and switches are operated and operational decisions are made. On the CHST system these control centers are designated as:</p> <ul style="list-style-type: none"><li>• Operations Control Center (OCC) – The main control center that will have direct control of all main track operations outside those areas controlled by the Regional Control Centers and general supervisory oversight of all railroad operations system-wide.</li><li>• Regional Control Center (RCC) – Two RCCs, one each located in Northern and Southern California will have direct control of all main track operations on the Peninsula Corridor and LOSSAN Corridor, respectively.</li><li>• Yard Control Center (YCC) – The yard component of the Heavy Maintenance Facility (HMF) and the Terminal Storage and Maintenance Facilities (TSMF) will each have a YCC staffed by a yardmaster and a train dispatcher who will be jointly responsible for direct control of operations, including all remotely controlled signals and switches, within the limits of the yard.</li></ul>
<u>Controlled Access:</u>	Refer to "Access Control".
<u>Conventional Rail:</u>	Traditional intercity passenger rail services of more than 100 miles with as little as 1 to as many as 7-12 daily frequencies; may or may not have strong potential for future high-speed rail service. Top speeds of up to 79 mph generally on shared track. Intended to provide travel options and to develop the passenger rail market for further development in the future.
<u>Counterpoise:</u>	A buried wire or a configuration of wires constituting a low resistance grounding system or portion of a grounding system.

<u>Crossover:</u>	A pair of turnouts connecting parallel tracks.
<u>Customer Assistance Intercom (CAI):</u>	Intercoms installed for passengers to report emergencies or obtain general travel information from local or remote Authority personnel. CAIs have two buttons for these two different functions. Part of the Telephone and Intercom System.
<u>Customer Information Monitor:</u>	A visual information device that is full color and able to display detailed image information from the Public Address and Customer Information Sign (PACIS) system. Part of the PACIS system.
<u>Customer Information Sign:</u>	A visual information device that is monochrome and able only to scroll text information from the Public Address and Customer Information Sign (PACIS) system. Part of the PACIS system.
<u>Cut-and-Cover:</u>	Construction technique in which a trench is excavated, infrastructure is installed, and the trench is closed.
<u>Cut and Fill:</u>	Construction technique involving excavation or grading followed by placement and compaction of fill material.

## D

<u>Datum:</u>	A reference from which measurements are made for establishing horizontal and vertical control.
<u>Dead Load:</u>	Static Load that is relatively constant throughout the life of a structure.
<u>Dedicated Corridor:</u>	A segment of right of way within the California High-Speed Train (CHST) System where the main tracks are used exclusively for High-Speed Train (HST) operations only, designated as such in the operating rules, and where these main tracks are completely separated physically from all other railroad tracks. The operation of trains (passenger and freight), other than the HST, over these tracks, is strictly prohibited in the operating rules and by regulation.
<u>Dedicated Track:</u>	A main track designated in the operating rules for the exclusive use of CHST operations. All other train movements, passenger and freight, are prohibited and restricted by the operating rules and by regulation. It may or may not be in a Dedicated Corridor.

---

**Design Terms**

<u>Degree of Curve:</u>	The central angle turned by a curve in 100 feet. It is closely approximated by $Dc = 5730 \text{ feet} / \text{Radius}$ . Railroad curves are defined by the Chord Definition, in which the length is described by a 100 foot long tangent between two points on the arc of the curve. The exact formula for chord definition curves is $Dc = 2 * \arcsin(50 / \text{Radius})$ .
<u>Derail:</u>	A track safety device designed to guide a locomotive or car off the rails at a selected spot as a means of protection against collisions and other accidents.
<u>Design Criteria:</u>	The direction for design of the system. California High-Speed Train Project Design Criteria establishes requirements and guidelines for the design of CHSTP Infrastructure and Systems elements.
<u>Design Frequency:</u>	The recurrence interval for hydrologic events used for design purposes.
<u>Design Life:</u>	The projected period of time for which a design element will perform while meeting minimum specifications under a particular maintenance regimen.
<u>Design Method:</u>	Load and Resistance Factor Design (LRFD) methods are preferred for force based structural and geotechnical design.
<u>Design Speed:</u>	The maximum permissible speed along a segment of alignment based on the design specification of the track infrastructure, signaling system characteristics, and the maintenance specifications for that class of track.
<u>Design Standards:</u>	Refer to the <i>General</i> chapter for detailed explanation of Design Standard Classifications.
<u>Design Storm:</u>	That particular storm which contributes runoff which the drainage facilities were designed to handle.
<u>Digital Terrain Model:</u>	A three-dimensional model of digital surfaces of topographic features.
<u>Directivity and Near Source Effects:</u>	The effects of direction of fault rupture and closeness to the fault on ground motion.
<u>Disconnect Switch:</u>	A no-load interrupting type electrical switch for disconnecting electrical power from a line section.

<u>Diverging Speed:</u>	Maximum speed for a train using the diverging route through a turnout.
<u>Dual Control Switch:</u>	A power operated switch that may also be operated by hand. Dual control switches are found only within the limits of a yard.
<u>Dwarf Signal:</u>	A low wayside signal with minimal preview that is used to provide adequate preview of the aspect displayed to high-speed trains.
<u>Dwell:</u>	The time from wheel stop to wheel start of a train performing a scheduled stop at a station.
<u>Dynamic Envelope:</u>	<p>In regard to pantograph, a clearance envelope around the pantograph static profile that takes into account the pantograph sway and pantograph uplift under dynamic conditions.</p> <p>In regard to vehicles and tracks, it is the trace of the maximum limits of movement of the vehicle in normal service. This outline is defined by the limits of motion due to wear of various components to their limits and includes deficiencies, such as deflated / overinflated airbags, etc. When defined from the perspective of the vehicle, it normally does not include any track deviations. When defined from the perspective of the infrastructure, track deviations are included. <i>Also called Kinematic Envelope.</i></p>
<b>E</b>	
<u>Earthwork:</u>	A general term applying to excavations and embankments, and the movement of soil and rock.
<u>Electric Lock Switch:</u>	A hand-operated switch, typically restricted to yards, that is equipped with an electrically controlled device that restricts the movement of the switch.
<u>Electrical Clearance – Dynamic (Passing):</u>	The minimum permissible clearance distance between the Overhead Contact System (OCS) messenger wire, contact wires, pantograph, or other live parts of either the vehicle or OCS and the grounded vehicle load gauge, overhead structure, or other adjacent fixed structure under dynamic operating conditions, such as during the passing of a train or the movement of the conductors due to heating or climatic conditions.

**Design Terms**

<u>Electrical Clearance – Static:</u>	Minimum clearance between live parts of either a vehicle pantograph or the Overhead Contact System, and grounded (earthed) parts of either a vehicle or adjacent fixed structure, while the vehicle and the live parts are stationary.
<u>Electrical Clearance - Safety:</u>	The distance in a straight line between a standing surface accessible to persons and energized parts necessary to prevent direct contact with energized parts, as defined in EN 50122-1: 1997 Section 5.
<u>Electrical Section:</u>	This is the entire section of the Overhead Contact System which, during normal system operation, is powered from a substation (SS) circuit breaker. The SS feed section is demarcated by the phase breaks of the supplying SS and by the phase breaks at the adjacent switching station or line end. An electrical section maybe subdivided into smaller elementary electrical sections.
<u>Elementary Electrical Section:</u>	This is the smallest section of the Overhead Contact System power distribution system that can be isolated from other sections or feeders of the system by means of disconnect switches and/or circuit breakers.
<u>Electromagnetic Field (EMF):</u>	The force field that extends outward from any moving electrical current, consisting of both a magnetic field and an electric field.
<u>Electromagnetic Interference:</u>	An electrical emission or disturbance that causes degradation in performance or results in malfunctions of electrical or electronic equipment, devices, or systems.
<u>Emergency Intercom:</u>	Intercoms installed for passengers to report emergencies and have a single button to perform this function. Part of the Telephone and Intercom System.
<u>Emergency Telephone (ETEL):</u>	Telephones adjacent to the motor-operated Overhead Contact System disconnect switches. These telephones are used by Authority staff to contact the Traction Electrification Power coordinator to report emergencies. Part of the Telephone and Intercom System.
<u>Embankment or Fill:</u>	In regard to earthwork for trackbed, artificial mound of imported material generally made of selected earth, gravel, or stone; built to support the HST when the reference line of the longitudinal profile is above the natural ground.

<u>Entrance Facility:</u>	A room in a building where cabling terminates. Cabling may be Authority cabling, third party cabling, or both.
<u>Epoch:</u>	As used in surveying, a specific date (time stamp) that all positions are based upon.
<u>Equilibrium Superelevation:</u>	The calculated superelevation that exactly balances the lateral force of the train on the curve at the defined speed. Normally called Balancing Cant or Equilibrium Cant in European publications.
<u>Erosion:</u>	The loosening, dissolving, or wearing away of earth materials in response to weathering, interaction with flowing water, wave action, or wind.
<u>Exclusive Use Corridor:</u>	<i>See Dedicated Corridor.</i>
<u>Expansive Soils:</u>	Soils that undergo swelling and shrinkage when wetted and dried.
<b>F</b>	
<u>Fail safe:</u>	<p>For railroad related safety: A design principle the objective of which is to eliminate the hazardous effects of a failure of a component or system.</p> <p>For non-railroad safety related design: A design feature that ensures that the system remains safe or in the event of a failure will cause the system to revert to a state which will not cause a mishap.</p>
<u>Fare Collection Line:</u>	Demarcation between Free Area and Paid Area.
<u>Fare Gate Array:</u>	Physical barrier which requires a valid CHST ticket to pass.
<u>Fare Gates:</u>	Physical barrier which requires a valid CHST ticket to pass. Also referred to as a Fare Gate Array.
<u>Fault Hazard Zone:</u>	Overall zone within which deformations related to fault rupture may occur and should be considered in the design.
<u>Feasible:</u>	Capable of being implemented.
<u>Feeder:</u>	A current carrying electrical connection between the Overhead Contact System and a traction power facility (substations, switching stations, and paralleling stations).



<u>Feeder Route:</u>	Branch routes that feed into main (arterial) routes.
<u>Fiber Optic Cable System:</u>	A data transmission technology that relies on light rather than electricity, conveying data through a cable consisting of a central glass core surrounded by layers of plastic. Part of Cable Infrastructure.
<u>Fire Alarm System (FAS):</u>	System which monitors the station areas, control centers, facilities, and ancillary areas including spaces located within tunnels for fire; initiates alarms; activates the fire suppression systems; alerts the monitoring and response organizations to the incident; and assists in the fire emergency evacuation processes.
<u>Flyover:</u>	A bridge that carries one road or rail alignment aerially over another.
<u>Footprint:</u>	Area of the ground surface covered by a facility or affected by construction activities.
<u>Foreslope:</u>	In fill sections, the resultant slope of the fill that allows to safely support track and road subgrade and that places the subgrade at safe height above the maximum water and flooding level.
<u>Formation:</u>	It is a general term referring to the whole of blanket, subgrade, and subsoil.
<u>Formation Top:</u>	Boundary between ballast and top of blanket or subgrade (where blanket layer is not provided).
<u>Free Area:</u>	Areas within a station which are open to the general public.
<u>Freeboard:</u>	The vertical distance between the level of the water surface usually corresponding to the design flow and a point of interest such as a bridge beam, levee top or specific location.
<u>Free Cross Section Area:</u>	The standard tunnel cross section area excluding clearance for tunnel design details and fixed equipment.
<u>Frequency:</u>	The number of times a field, such as an electromagnetic field, changes direction in space each second. Also, the number of trains, flights, or other transportation service occurring in a given time period.

Frog (Turnout):

- Fixed Frog: Term essentially synonymous with Frog. “Fixed” is sometimes used as part of the name on railroad systems that also use spring frogs and swing nose frogs in order to clarify the type of frog used in a given situation.
- Spring Frog: A frog without a fixed open flangeway on one side between the frog point and wing rail that has springs holding that wing rail up against the frog point on that side so as to provide unbroken wheel support for the main track. The other wing rail is fixed. Main track traffic travels on the fixed wing side of the frog, not moving the frog. The wheels of diverging side traffic opens the sprung wing rail which is then forced closed by the spring after the wheel has passed. Spring frogs are either right handed or left handed. These devices are normally used only where the traffic on the side springing the wing rail is 20 percent or less of the total traffic over the frog. These devices are generally unknown outside North America.
- Swing Nose Frog: A frog in a turnout with a movable frog point connected to a switch machine for manipulation relative to the switch position.
- Point of Frog: In American terminology, the point where the gauge lines are 1/2 inch apart, or the point located one-half the distance in inches from the intersection of the gauge lines of the rails through the frog. In European terminology, the theoretical point of intersections of the gauge lines of the rails through the frog. The point, as defined in European terminology, is usually called the theoretical point of frog in American terminology.
- Heel of Frog: End of rails that are part of the frog assembly on the end away from the switch
- Toe of Frog: End of rails that are part of the frog assembly on the end toward the switch.
- A frog is commonly called a Crossing in European terminology.

## G

<u>Gantry:</u>	Portal frame spanning a railroad track or tracks for supporting and displaying signals, or installed parallel to the track(s) at traction power facilities (TPF) to support disconnect switches and for connecting feeder cables from the TPF to the Overhead Contact System.
<u>Global System for Mobile Communications - Railway (GSM-R):</u>	An international wireless communications standard for railway communications. GSM-R is a sub-system of European Rail Traffic Management System (ERTMS). It is used for communication between train and railway regulation control centers for communication and control.
<u>Geographic Information System (GIS):</u>	An information management system designed to store and analyze data referenced by spatial or geographic coordinates.
<u>GEOID09:</u>	Gravimetric hybrid geoid height model developed by National Geodetic Survey (NGS) containing the separation between NAD83 and NAVD88 and is the basis for elevations (orthometric heights) using Global Position System (GPS) survey methods.
<u>Geosynthetics:</u>	<p>Structural elements made of synthetic materials for use in earthworks and construction of trackbed layers. A distinction is made between:</p> <ul style="list-style-type: none"><li>• Geotextiles: Geosynthetics (woven or non-woven), which may be used for separation, filtering, drainage and reinforcement.</li><li>• Geomembranes: Geosynthetics (synthetic or bituminous layer) impermeable to water, which may be used for protection of sensitive subgrade against penetration of surface water or for protecting ground water against pollution.</li><li>• Geogrids: Fine or coarse mesh geosynthetics, which may be used for separation and reinforcement.</li><li>• Geocomposite: Compound structure made of at least two layers of geosynthetic materials.</li></ul>
<u>Global Positioning System (GPS):</u>	A space-based global navigation satellite system that provides location and time information in all weathers and at all times anywhere on or near the Earth when and where there is an unobstructed line of sight to four or more GPS satellites.

<u>GPS Network Timing System (GNTS):</u>	Timing system which provides accurate time-of-day synchronization to devices and systems within the CHST System. For example, wall clocks will be field devices of the GNTS.
<u>Grade Crossing:</u>	The intersection of a railroad and a highway at the same elevation (grade); an intersection of two or more highways; an intersection of two railroads.
<u>Grade, Gradient:</u>	The slope of changes in elevation, defined in percentage %, as feet of rise in 100 feet. Sometimes defined in European publication as millimeters of rise in one meter, in which case it is normally written as ‰.
<u>Grade Separation Structures:</u>	<p>In respect to HST:</p> <ul style="list-style-type: none"><li>• Underpass: HST passes under roadway or other railroad.</li><li>• HST Overpass: HST passes over roadway or other railroad.</li><li>• HST Aerial Structure: HST is elevated. HST passes over roadways, bikeways, and other railroads where they occur.</li><li>• HST Bridge: HST passes over water feature.</li></ul> <p>In respect to Roadway Structures: Refer to Caltrans Nomenclature.</p>
<u>Grade-Separated:</u>	At different elevations; on separate levels.
<u>Grid:</u>	A system of interconnected power generators and power transmission lines that is managed to meet the requirements of electrical energy users connected to that transmission system at various points.
<u>Ground Grid (Mat):</u>	A buried grid for installations, such as substations and disconnect switch platforms, which provides a low resistance path to ground and reduces touch-and-step potentials for operators of the equipment.
<u>Ground Potential Rise (GPR):</u>	The maximum electric potential that a substation grounding grid may attain relative to a distant grounding point assumed to be at the potential of remote earth. The GPR is equal to the maximum grid current times the grid resistance to earth.

## Design Terms

---

<u>Ground Rod:</u>	A metal rod driven into the ground with ground wire connection to structures or equipment to disperse currents to ground (earth).
<u>Ground Wire:</u>	A conductor installed for the purpose of providing electrical continuity between a device or equipment and a grounding system.
<u>Grounded:</u>	Connected to earth through a ground connection or connections of sufficiently low impedance and having sufficient current-carrying capacity to limit the build-up of voltages to levels below that which may result in undue hazard to persons or to connected equipment.
<u>Groundwater:</u>	Water contained and transmitted through open spaces within rock and sediment below the ground surface.
<u>Guard Rail:</u>	A length of rail placed adjacent to the rail across from a frog to guide wheels through the frog. Some guard rails are fabricated from a special section produced for that purpose.
<u>Guard Railing:</u>	A metal railing acting as a safety barrier at the side of a freeway, highway or road to prevent errant vehicles from leaving the traveled way.
<u>Guideway:</u>	A track or riding surface that supports and physically guides vehicles specially designed to travel exclusively on it. For the CHSTP, use “trackway” in lieu of guideway.
<u>Guidelines:</u>	Non-mandatory, recommended, and supplemental information regarding generally acceptable methods to satisfy provisions of a regulation, code, or standard.

## H

<u>Half-Sine Spiral:</u>	A spiral with a defined variation in the change of radius, usually in the form of a sine wave curve so as to reduce the entry and exit change in the rate of change. Recommended in high-speed operation, particularly if the track is on a concrete base. <i>Also called variable rate spiral.</i>
<u>Hazard:</u>	Hazards encompass all aspects of technology or activities that produce risk. Hazards include the characteristics of things and the actions or inactions of people.

<u>Hazardous Fault:</u>	A fault that meets the following criteria: $\geq 1.0$ mm/year Slip Rate (SR) and/or $\leq 1,000$ year Recurrence Interval (RI).
<u>Hazardous Minerals:</u>	Naturally occurring minerals contained within soil or rock that contain minerals known to be harmful if inhaled, ingested, or in contact with skin.
<u>Headspan:</u>	An across-track support arrangement comprising two or more wires that provide support for one or more OCS equipments. Headspans can be attached to two separated poles or to wayside buildings or other fixed structures.
<u>Headway:</u>	The time between trains at a given point. For example, a 15-minute headway means that one train arrives, departs or passes every 15 minutes.
<u>Heavy Maintenance Facility (HMF):</u>	A yard facility that provides overnight and mid-day rolling stock storage and Level 1 to 5 maintenance capabilities.
<u>Help Point Intercom (HPI):</u>	Intercoms installed for passengers to communicate with Authority personnel to report emergencies and obtain general travel information. HPIs have two buttons for these two different functions. Part of the Telephone and Intercom System.
<u>High Risk Utility:</u>	<p>Utilities/Facilities conducting the following materials, whether encased or not, are considered to be High Risk:</p> <ul style="list-style-type: none"><li>• Petroleum products.</li><li>• Oxygen.</li><li>• Chlorine.</li><li>• Toxic or flammable gases or liquids.</li><li>• Natural gas in pipelines of any size.</li><li>• Underground electric supply lines, conductors or cables that have a potential to ground of more than 300 volts, either directly buried or in duct or conduit, which do not have concentric grounded or other effectively grounded metal shields or sheaths.</li><li>• Water in pressured pipeline 8 inches or greater in diameter or pipelines of any size with normal operating pressure greater than 80 psi.</li><li>• Other utilities that could disrupt the operation of CHST system.</li></ul>

## Design Terms

<u>High Signal:</u>	A signal located such that its aspect can be detected by train operators sufficiently in advance of the time the train passes the signal such that the train operator can identify the aspect and take proper action by the time the train reaches the signal.
<u>High-Speed Main Tracks:</u>	<i>See main track.</i>
<u>High-Speed Train (HST):</u>	Train designed to operate safely and reliably at speeds near 200 mph (320 km/h).
<u>High-Speed Railroad:</u>	A railroad system utilizing steel-wheel-on-steel-rail technology with a regular operating speed greater than 125 mph (200 km/h).
<u>Highway-Rail (Hi-Rail) Vehicle:</u>	A type of Infrastructure Maintenance Vehicle equipped with both rubber tires and steel wheels allowing it to operate on either a highway or railroad track.
<u>Holocene Fault:</u>	Fault with most recent movement within the past 11,000 years.
<u>Home Signal:</u>	A signal (wayside or virtual) at the entrance to an interlocking to govern trains entering the interlocking.
<u>Human Machine Interface (HMI):</u>	The user interface to systems or equipment. It is the space where interaction between humans and machines occurs.
<u>Hut:</u>	Small self contained enclosure to protect and secure specialized equipment.
<b>I</b>	
<u>Impedance Bond:</u>	An electrical device located between the rails consisting of a coil with a center tap used to bridge insulated rail joints in order to prevent track circuit energy from bridging the insulated joint while allowing the traction return current to bypass the insulated joint. The center tap can also be used to provide a connection from the rails to the static wire and/or traction power facilities for the traction return current.
<u>Indication:</u>	The information conveyed by the appearance of the Onboard Cab Display in the cab. The information conveyed by the aspect of a fixed wayside signal.
<u>Infrastructure Maintenance Vehicle (IMV):</u>	Infrastructure maintenance equipment operated on track for inspection or maintenance that may not shunt track circuits or operate signals.

<u>Insulated Joint:</u>	A joint in the running rail used to prevent track circuit energy on one side of the joint from leaking to the other side of the joint.
<u>Insulated Overlap:</u>	A sectionalizing point in the catenary formed by cutting insulation into the out-of-running conductors of the two adjoining and overlapping catenaries, having between them in the overlap span an electrical clearance realized by an air gap. The contact and messenger wires of these two overlapping tension lengths that terminate at opposite ends of the overlap section create a sectionalizing point in the catenary as required for operational and maintenance reasons, and permit the passage of pantographs under power from one energized electrical sub-section to the next, both supplied by the same traction power source.
<u>Integrated Information Management Platform (IIMP):</u>	Platform which integrates and leverages information from multiple stand-alone communications systems and other devices and systems for streamlined and coherent operation, monitoring, and control.
<u>Interceptor Ditches:</u>	Above a cut slope, they carry runoff from the watershed served and prevent surface runoff from entering the cut.
<u>Interlocking:</u>	An arrangement of signals (wayside and/or virtual) and switch appliances so interconnected that their movement must succeed each other in proper sequence and for which interlocking rules are in effect.
<u>Interlocking Signals:</u>	Fixed signals which govern the movement of trains through Interlockings that are observed by the train operator under Automatic Train Control failure conditions at reduced speed.
<u>Intermediate Station:</u>	Any station between two terminal stations. Intermediate HST stations will include additional tracks to allow for through running express services.
<u>Intermodal:</u>	Describes transportation that involves more than one means (walk, bike, auto, transit, taxi, train, bus, air, etc.) during a single journey.



<u>Interoperability:</u>	In the context of the European High Speed Lines, the capability of the European High-Speed lines railway network to permit high speed trains to run safely and continuously with specified performances. It is based on legal, technical and operational conditions that must be fulfilled to satisfy the necessary requirements. Thus, for example, a German high-speed train satisfying the requirements of the Rolling Stock Technical Specification for Interoperability (TSI) is able to run safely and continuously on a French High-Speed Line, the infrastructure of which satisfies the requirements of the various infrastructure Technical Specifications for Interoperability. These TSI design standards were developed specifically for the design, construction and operation of interoperable high-speed railways in Europe and are based on European and international best practices.
<u>Intrusion:</u>	Entry of errant vehicles, goods, objects and people into the operating space of HST or other transportation system. An errant vehicle's exit out of its right-of-way and entry into the operating space of another transportation system's right-of-way.
<u>Intrusion Detection System:</u>	An electronic system that alerts the Control Center of an intrusion event and may result in train movement restriction.
<u>Intrusion Protection:</u>	Physical structure or space which will prevent entry of errant vehicles, goods, objects, and people into the operating space of CHST or other transportation system.
<u>Island Platform:</u>	<i>See Center Platform.</i>
<b>K</b>	
<u>Karst Terrain:</u>	A type of topography that is formed by subsurface dissolution of minerals, including mapped or otherwise known subsurface naturally occurring or man-induced voids.
<u>Kiss-and-Ride:</u>	Facility for private vehicles to drop off or pick up CHST patrons.
<b>L</b>	
<u>Land Subsidence:</u>	The gradual downward settlement or sinking of the ground surface.

<u>Landslide:</u>	Mapped or otherwise known rock falls, mud flows, debris flows, landslides, and other forms of slope failures.
<u>Lead (Turnout):</u>	The distance from the actual point of switch to the 1/2 inch point of frog.
<u>Level of Service (LOS):</u>	A rating using qualitative measures that characterize operational conditions within a traffic stream and their perception by motorists and passengers.
<u>Life Cycle:</u>	All phases of the system's life including design, research, development, test and evaluation, production, deployment (inventory), operations and support, and disposal.
<u>Line Side Drains:</u>	Drains which collect and discharge surface water, seepage water, and ground water into a controlled outlet. Generally a distinction is made between buried drains, open channels, and side ditches.
<u>Liquefaction:</u>	Reduction of soil strength because of excess pore water pressure due to earthquake ground shaking when saturated.
<u>Live:</u>	An electrically energized circuit or component.
<u>Live Load:</u>	Load that varies due to dynamic factors within the normal operating cycle, but excluding seismic effects.
<u>Live Part:</u>	A part or component connected to an energized circuit and therefore live and not insulated from the energized circuit.
<u>Local Area Network (LAN):</u>	A network which provides network connectivity between terminals, servers, switches, sensors, and other electronic/optical equipment within a station or operational facility.
<u>Longitudinal:</u>	A facility located parallel to and within highway, existing railway, or proposed Authority right-of-way.
<u>Low Risk Utility:</u>	Facilities that are not covered under the definition for "High Risk Utility" are considered to be Low Risk Utilities.

## M

<u>Mainline:</u>	In respect to CHST system, the mainline is the main route typically consisting of two main tracks. Mainline also includes passing track and station track. Mainline does not include maintenance sidings and yard track. <i>Also, main line, revenue tracks, and revenue service tracks.</i>
<u>Main Track:</u>	A track designated for the movement of trains at normal commercial speed having their movement protected by a control system. Tracks for the primary purpose of access to stations, yards, and other auxiliary facilities are not main tracks regardless of the presence or absence of movement protection system on those tracks. On CHST system, scheduled stops of any kind, including station stops will not normally be permitted on main tracks. On CHST system main tracks, all movements are protected by the Automatic Train Control system.
<u>Maintenance:</u>	Regular activities that are required to support safe operations and the intended use of the high-speed train system such as inspection and correction of deviations from the design along the track.
<u>Maintenance Management Information System (MMIS):</u>	A system that provides various management and statistical functions to support the users in organizing the planning of rolling stock maintenance work and providing the relevant information necessary for decision making.
<u>Maintenance Siding:</u>	A track dedicated to parking maintenance equipment and trains and normally connected to a passing track.
<u>Major Utility:</u>	Any subsurface, above ground, or overhead facility used for transmission, regardless of size, shape, or method of conveyance.
<u>Maximum Considered Earthquake (MCE):</u>	Ground motions corresponding to greater of (1) a probabilistic spectrum based upon a 10 percent probability of exceedance in 100 years (i.e., a return period of 950 years); and (2) a deterministic spectrum based upon the largest median response resulting from the maximum rupture (corresponding to $M_{max}$ ) of any fault in the vicinity of the structure.

<u>Maximum Authorized Speed (MAS):</u>	The highest speed that is permitted over a specific portion of the railroad alignment. It may be authorized by special instructions of the current timetable, operating rules, or any other publication authorized by the chief operating officer.
<u>Maximum Contact Force:</u>	The maximum value of the contact force exerted by the pantograph on the contact wire. <i>Sometimes, maximum force.</i>
<u>Mean Contact Force:</u>	The statistical mean value of the contact force exerted by the pantograph on the contact wire. <i>Sometimes, mean force.</i>
<u>Medical Health Criterion:</u>	Maximum pressure variation (peak-to-peak value) in the tunnel (outside of the train) independent of time.
<u>Messenger Wire:</u>	In catenary construction, the Overhead Contact System Messenger Wire is a longitudinal bare stranded conductor that physically supports the contact wire or wires either directly or indirectly by means of hangers or hanger clips and is electrically common with the contact wire(s).
<u>Minimum Contact Force:</u>	The minimum value of the contact force exerted by the pantograph on the contact wire. <i>Sometimes, minimum force.</i>
<u>Minimum/Maximum:</u>	<i>See Design Standards.</i>
<u>Minor Utility:</u>	Any subsurface, above ground, or overhead facility used as distribution lines or service laterals to individual parcels or properties
<u>Modal:</u>	A transportation system defined on the basis of specific rights-of-way, technologies, and operational features.
<u>Movement Authority:</u>	The vital information used by the onboard Automatic Train Control (ATC) system to determine the position on the track(or limit) to which the train can safely move under ATC supervision, including the speed limits both permanent and temporary that must be observed between its current position and the Movement Authority limit.

## N

<u>National Spatial Reference System (NSRS):</u>	Datum, defined and managed by the National Geodetic Survey, and the foundation for the National Spatial Data Infrastructure (NSDI).
--	---

<u>Negative Feeder (NF):</u>	An overhead conductor supported on the same structure as the catenary conductors, which is at a voltage 25 kV with respect to ground but 180° out-of-phase with respect to the voltage on the catenary. Therefore, the voltage between the catenary conductors and the negative feeder is 50 kV nominal. The negative feeder connects successive feeding points, and is connected to one terminal of an autotransformer in the traction power facilities via a circuit breaker or disconnect switch. At these facilities, the other terminal of the autotransformer is connected to a catenary section or sections, via circuit breakers or disconnects.
<u>Network Management System (NMS):</u>	A system used to provision, test, and maintain network connectivity.
<u>Neutral Leads:</u>	The wires connecting the center tap of impedance bonds to other impedance bonds and/or to traction power ground circuits.
<u>Neutral Section:</u>	<i>See Phase Break.</i>
<u>Nominal Voltage:</u>	Voltage by which an installation or part of an installation is designated. The operating voltage of the Overhead Contact System may differ from the nominal voltage within defined permissible tolerances.
<u>Non-Ballasted Track:</u>	Rail lines installed over concrete slabs for support.
<u>Non-operating Condition:</u>	The environmental/climatic conditions under which trains will not be permitted to maintain continuous operation and revenue service will cease.
<u>Non-public Area:</u>	Station areas accessible only to station staff and secured against unauthorized entry with lockable doors.
<u>Non-Vital:</u>	A designation placed on a system, subsystem, element, component, or function denoting that satisfactory operation of such is not mandatory for safety.
<u>North American Datum of 1983 (NAD 83):</u>	The horizontal control datum for the United States based on the Geodetic Reference System 1980 and with a geocentric origin.
<u>North American Vertical Datum of 1988 (NAVD 88):</u>	The vertical control datum established for surveying elevations in the United States based on the General Adjustment of the North American Datum of 1988.

## O

<u>OCS Pole:</u>	Vertical structural element supporting the overhead contact system equipment, which provides physical support, registration and/or termination of the Overhead Contact System conductors including auxiliary wires.
<u>Onboard Cab Display (OCD):</u>	Display located in the cab of a train or Infrastructure Maintenance Vehicle that displays to the locomotive engineer, or operator the current speed, allowed speed, approaching speed restrictions ahead within a defined distance, the required braking curve that must be adhered to in order to maintain schedule, and other necessary operating information. OCD provides the status of the Automatic Train Control system and the safe limits within which the train may operate. Generally OCD incorporates a group of displays including train radio, electronic timetable, control command, and technical and diagnostic displays.
<u>Operating Basis Earthquake (OBE):</u>	Ground motions corresponding to a probabilistic spectrum based upon an 86 percent probability of exceedance in 100 years (i.e., a return period of 50 years).
<u>Operating Condition:</u>	The environmental/climatic conditions under which trains will be permitted to maintain continuous operation, and for which the Overhead Contact System shall provide full, uninterrupted, and acceptable performance.
<u>Operating Envelope:</u>	A zone delineated by HST tracks and Overhead Contact System.
<u>Operating Infrastructure:</u>	Infrastructure that is required for HST operation. This includes infrastructure within operating envelope and other HST facilities such as traction power facilities, wayside power cubicles, train control rooms, communication rooms, and cable troughs.
<u>Operations Radio System (ORS):</u>	The voice and low-bandwidth data radio system for wireless communications between onboard equipment, mobile and portable users, and fixed users and systems. (If an integrated vital and non-vital radio system is used, ORS may carry safety-critical train control data).

<u>Operating Speed:</u>	The highest in-service speed that is achievable by a trainset technology on a segment of alignment that conforms to all of the requirements specified for that class of track. <i>See also Maximum Authorized Speed.</i>
<u>Operations Control Center (OCC):</u>	<i>See Control Center.</i>
<u>Outboard Platforms:</u>	Side boarding platforms located directly opposite one another, each serving one track.
<u>Overhead Contact System (OCS):</u>	<p>OCS is comprised of:</p> <ul style="list-style-type: none"><li>• The aerial supply system that delivers 2x25 kV traction power from substations to the pantographs of high-speed electric trains, comprising the catenary system, messenger and contact wires, stitch wires and hangers, associated supports and structures (including poles, portals, headspans and their foundations), manual and/or motor operated isolators, insulators, phase breaks, conductor termination and tensioning devices, downguys, and other overhead line hardware and fittings.</li><li>• Portions of the Traction Power Return System consisting of the negative feeders and aerial static wires, and their associated connections and cabling.</li></ul>
<u>Overhead Contact Line Zone and Pantograph Zone:</u>	The zone whose limits, in general, are not exceeded by a broken overhead contact line in the event of a dewirement or by a damaged pantograph or broken fragments thereof which are energized.
<u>Overlap:</u>	<i>See Uninsulated Overlap and Insulated Overlap.</i>
<u>Owner:</u>	In the context of utility coordination, the owner of the underground or above ground utility or its authorized agent.
<u>Ownership:</u>	Any interest in land, real estate, or the improvements on it.

## P

<u>Paid Area:</u>	Areas on the platform side of the fare-paid line where possession of a valid CHST ticket is required.
-------------------	---

<u>Pantograph:</u>	Current collector apparatus consisting of spring-loaded hinged arms mounted on top of electrically powered rail vehicles that provides a sliding electrical contact and collects current from the contact wire of the overhead contact system. The pantograph is designed to follow changes in the vertical height and lateral offset of the contact wire, and to provide for essentially vertical movement of the pantograph collector head.
<u>Pantograph Clearance Envelope:</u>	A clearance envelope around the pantograph static profile.
<u>Pantograph Current:</u>	Current that flows through the pantograph.
<u>Pantograph Head:</u>	Pantograph equipment comprising the current collector strips and their mountings.
<u>Pantograph Sway (Pantograph lateral displacement):</u>	Lateral displacement of the pantograph induced, under the dynamic passage of the electric vehicle, by vehicle and pantograph lateral displacements that include gauge deviation, roll and lateral vehicle shock loads, and cross-track tolerance.
<u>Parallel Feeder:</u>	<i>See Negative Feeder.</i>
<u>Paralleling Station (PS):</u>	An installation which helps boost the Overhead Contact System (OCS) voltage and reduce the running rail return current by means of the autotransformer feed configuration. The negative feeders and the catenary conductors are connected to the two outer terminals of the autotransformer winding at this location with the center terminal connected to the traction return system. The OCS sections can be connected in parallel at PS locations.
<u>Parcel:</u>	A distinct, continuous portion or tract of land.
<u>Park and Ride:</u>	Facility where CHSTP patrons can park and leave personal vehicles prior to transfer to HST.
<u>Passenger Aural Comfort Criteria:</u>	Maximum pressure change inside the train within a specified period of time to limit the discomfort on passengers' ears when passing through a tunnel.



---

**Design Terms**

<u>Passing Track:</u>	A designated track connected to a main track on both ends for the purpose of allowing a train to clear the main track as a part of normal operations, usually for the purpose of accessing a station platform, allowing train overtaking, or allowing trains to clear the main tracks to minimize delay in case of operational issues. For regulatory and signaling purposes the passing track is treated the same as a main track.
<u>Peak Period:</u>	Time period during the day with the greatest volume of CHSTP patrons.
<u>Performance Based Design:</u>	In regard to seismic design, a design based on specific performance criteria in addition to building code based safety criteria.
<u>Performance Criteria:</u>	<p>In regard to seismic design, for primary structures there are two levels of Performance Criteria:</p> <ul style="list-style-type: none"><li>• No Collapse Performance Level (NCL) for which structures are able to undergo the effects of the Maximum Considered Earthquake with no collapse. Significant damage may occur which requires extensive repair or complete replacement of some components. Occupants not on trains are able to evacuate safely. Damage and collapse due to train derailment is mitigated through containment design. If derailment occurs, train passenger and operators are able to evacuate derailed trains safely.</li><li>• Operating Performance Level (OPL) for which structures are able to withstand the effects of the Operating Basis Earthquake with elastic response with no spalling, and response within structural deformation limits, in order to limit rail stresses and protect against derailment. No derailment occurs; trains are able to safely brake from the maximum design speed to a safe stop; passengers and operators are able to evacuate stopped trains safely. Disruption of service for all systems supporting HST operations is minimal. Train operation resumes within a few hours and possibly at reduced speeds.</li></ul>

<u>Phase Break:</u>	An arrangement of insulators and grounded or non-energized wires or insulated overlaps, forming a neutral section, which is located between two sections of Overhead Contact System that are fed from different phases or at different frequencies or voltages, under which a pantograph may pass without shorting or bridging the phases, frequencies or voltages.
<u>Photogrammetry:</u>	The art, science, and technology of obtaining reliable information about physical objects and the environment through process of recording, measuring, and interpreting images and patterns of electromagnetic radiant energy and other phenomena.
<u>Pick-Up and Drop-Off:</u>	Facility for private and semi-private vehicles to drop off or pick up CHSTP patrons, which could include facilities for taxis, private shuttles, and rental cars.
<u>Plat:</u>	A plan or map of a plot of ground.
<u>Platform:</u>	Station area adjacent to tracks where trains stop to allow passengers to board and alight.
<u>Portal:</u>	<i>In regard to Overhead Contact System, see Portal Structure.</i>
<u>Portal Structure:</u>	An Overhead Contact System (OCS) structure consisting of a crossbeam or truss supported by two separate OCS poles usually placed to the outside of multiple tracks to support OCS conductors. OCS support brackets or drop pipes are attached to the beam or truss to support the OCS cantilever frames.
<u>Positive Train Control (PTC):</u>	FRA-mandated train control requirement that automatically enforces train separation, collision avoidance, speed restrictions, and movement authority. On CHST Automatic Train Protection fulfills this requirement.
<u>Potentially Hazardous Fault:</u>	Fault having known or documented Holocene activity or known Quarterly faults with suspected Holocene activity.
<u>Pothole / Test Pit:</u>	An excavation to expose an underground facility.
<u>Power Operations Controller (POC):</u>	The authorized person in a Control Center who is permitted to operate and control TES equipment through the Supervisory Control and Data Acquisition system and by voice commands to authorized field personnel and emergency response personnel, as applicable.

<u>Power Transformer:</u>	A device which transforms power on in ac system from one voltage level to another (e.g., from 115 kV to 25 kV).
<u>Prepared Subgrade:</u>	The upper layer of the subgrade is formed into a prepared subgrade layer, which normally has a cross slope. This layer is made of imported or treated material depending of the quality of the upper part of embankment or the bottom of the excavation. Its quality and compactness shall be better than the material below. Its function is to minimize the deformation of the upper part of the embankment or the bottom of the excavation and to prevent water that has passed through the subballast layer from penetrating to the earthwork below.
<u>Pressure Comfort:</u>	Conditions where there is no passenger ear discomfort due to pressure change.
<u>Pressure Tightness Coefficient:</u>	Time in which the difference between internal and external pressures upon a stepwise pressure change decrease from 100% to approximately 38% of the initial pressure difference.
<u>Primary Type 1:</u>	Primary Type 1 elements are those that directly support HST track. Refer to the <i>Seismic</i> chapter for detailed explanation of this general classification.
<u>Primary Type 2:</u>	Primary Type 2 elements are those that do not directly support HST track, but have the potential to affect HST service. Refer to the <i>Seismic</i> chapter for detailed explanation of this general classification.
<u>Private Utility:</u>	Utility infrastructure owned by a private corporation or public or private entities. They may not be regulated by the public or government agency.
<u>Public Address and Customer Information Sign(PACIS) System:</u>	System which provides synchronized audio and visual information to passengers and Authority personnel using Public Address speakers and Visual Signs.
<u>Public Area:</u>	Station free areas and paid areas, accessible to the general public.
<u>Public Transportation:</u>	Shared passenger transportation service available for use by the general public. Public transportation modes include buses, ferries, trolley buses, and various forms of rail transit including light rail, people movers, and grade separated "rapid transit" (metro/subways/elevated). Intercity public transportation includes airlines, buses, and intercity rail.

Public Utility: Utility infrastructure that are operated and maintained for public service. Public Utilities can be either publicly or privately owned and involve natural monopolies in sectors specially regulated by the California Public Utilities Commission.

## Q

Qualified Analyst: In regard to seismic design, an individual with the knowledge of engineering seismology and at least 5 years of experience in performing site specific deterministic and probabilistic seismic hazard analyses (DSHA and PSHA) in California.

Qualified Person: In regard to Overhead Contact System (OCS) and Traction Electrification System (TES) equipment, an authorized person who has been trained in and has demonstrated adequate knowledge of the installation, construction, maintenance, and operation of the OCS lines and TES equipment and the hazards involved, including identification of and exposure to electric supply and communications lines and equipment in or near the workplace.

Quality Level: A level of accuracy scale used for identifying the location of underground and above ground utility facility information needed to develop capital projects, and for acquiring and managing that level of information during the project development process. Four Levels of Quality Measurement are used ranging from Level A to Level D.

Quaternary Fault: Fault with evidence of movement in the past 1.6 million years.

Queuing Area: Station area where passengers wait or line up to use a device or circulation element such as a ticket machine, fare gate, stair, elevator, or escalator. Queuing areas should be designed to accommodate waiting passengers without disrupting other passenger flows. Also, area provided to accommodate peak passenger surges.

## R

Radio Frequency: The frequency range of the electromagnetic spectrum that is used for radio communication.

**Design Terms**

<u>Radio System:</u>	Communications systems which use radio propagation to transport voice and data between fixed entities and systems and mobile entities and systems.
<u>Rail Shared Corridor:</u>	<i>See Shared Rail Corridor.</i>
<u>Rail Return:</u>	The combination of track structure, jumpers, impedance bonds, grounds, and cables, each of which provides part of the electrical return path from the wheel-sets of the traction units to a substation.
<u>Rail Potential:</u>	The voltage between running rails and ground occurring both under operating conditions when the running rails are utilized for carrying the traction return current or under fault conditions.
<u>Redundant Utility Supply Circuits:</u>	A configuration of two supply circuits from the utility supply company that originate from different transformers or bus systems. Using redundant supplies will minimize the possibility that power to both circuits will be lost simultaneously.
<u>Refuge Track:</u>	A dead end track, normally connected to a station track, primarily for the purpose of temporary storage of a disabled train.
<u>Regional Control Center (RCC):</u>	<i>See Control Center.</i>
<u>Regenerated Power:</u>	Electrical power generated by electric vehicles when they brake by using their electric motors as electric generators.
<u>Regulation:</u>	A rule and administrative code issued by governmental agencies at all levels – federal, state, county, and municipal that impose specific requirements and at times mandate permits or approvals by the agency (generally to ensure health and safety of the public). Although regulations are not laws, they have the force of law as they are adopted under authority granted by statutes.
<u>Relocations:</u>	The removal, rearrangement, and reinstallation of a utility facility required by a transportation improvement project.
<u>Response Spectrum:</u>	The response of damped single degree of freedom oscillators to an earthquake time history.
<u>Restricted Area:</u>	An area for which a railroad agency has responsibility and to which access is permitted only to authorized persons.

<u>Restricted Manual Mode:</u>	A mode of operation, enforced by the Automatic Train Control (ATC) system, which allows trains to proceed under the manual control of the locomotive engineer when there is an ATC malfunction. Permission to enter Restricted Manual mode must be granted by the train dispatcher. Movement must be at Restricted Speed subject to the prescribed operating rules and procedures.
<u>Restricted Speed:</u>	A speed, not exceeding 20 mph, at which it is possible to stop within one half the range of vision, short of the next signal, another train, obstruction, or derail, while looking out for broken rail or switch not properly lined.
<u>Retained Cut:</u>	Trackway section where tracks are placed uncovered, below existing ground level and where adjacent soil is supported with retaining walls above top of rail elevation.
<u>Retained Fill:</u>	Trackway section where tracks are placed on embankment material contained by retaining walls above existing ground.
<u>Return Circuit (Return System):</u>	<i>See Traction Power Return System.</i>
<u>Reverse Curve:</u>	Section of the horizontal alignment of the trackway in which a curve to the left or right is followed immediately by a curve in the opposite direction.
<u>Richter Scale:</u>	A logarithmic scale measuring the severity of earthquakes, based on the magnitude of ground motion.
<u>Ridership:</u>	Number of passengers using CHST System over a certain period of time.
<u>Right-of-Way:</u>	A legal right of passage over a defined area of real property used for highway, railway, public utility services, or other purposes. In transportation usage, refers to the corridor along a roadway or track alignment that is controlled by a transit or transportation agency/authority and is usually the access control line.
<u>Risk:</u>	In the consideration of hazards and vulnerabilities, a measure of the combined probability and severity of potential harm to one or more resources as a consequence of exposure to one or more hazards.

<u>Rolling Stock:</u>	Wheeled railway vehicles.
<u>Rules:</u>	Operating requirements found in the Code of Operating Rules, Special Instructions or other authorized CHST System publications.

## S

<u>Safe Braking:</u>	A set of design provisions and procedures which together ensure that a train's Automatic Train Protection stopping distance is safe in normal conditions and in all likely combinations of adverse factors and failure conditions.
<u>Safe Point Intercom (SPI):</u>	Intercom stations targeted for passengers to report emergencies only and have a single button to actuate this function. Part of the Telephone and Intercom System.
<u>Safety:</u>	The control of recognized hazards to achieve an acceptable level of risk.
<u>Scale:</u>	A graduated line representing a proportionate size.
<u>Sealing Characteristics:</u>	The capacity of the train to limit inside pressure change within given limits.
<u>Secondary:</u>	Secondary elements are those not designated as Primary Type 1 or Primary Type 2. Refer to the <i>Seismic</i> chapter, for detailed explanation of this general classification. <i>Also called Secondary Structures.</i>
<u>Section Insulator:</u>	A mechanical sectionalizing device installed in the overhead catenary providing electrical separation between two adjacent catenary sub-sections both energized by the same traction power supply source which permits the passage of pantographs under power from one energized electrical sub-section to the next.
<u>Security:</u>	A means, active or passive, that serves to protect and preserve an environment and allows for the conduct of activities within an organization or society without disruption.
<u>Seismic Hazards:</u>	Earthquake-induced conditions such as vibratory ground motion, liquefaction, lateral spreading, dynamic compaction, seismically-induced slope failures, and ground rupture.

<u>Seismic Source Model:</u>	The geographic distribution of potential seismic sources that could affect the seismic ground motion at a particular site.
<u>Service:</u>	The portion of the electric, gas, water, sewer, or communication system that connects a customer, usually at the meter location, to the utility distribution or supply system.
<u>Side Platforms:</u>	Station area adjacent to a single track for the purpose of passenger boarding and alighting.
<u>Shared Corridor:</u>	A portion of high-speed rail alignment where the high-speed trains operate on their own dedicated tracks parallel to and in the vicinity of other transportation systems such as highways, passenger railroads, or freight railroads.
<u>Shared Rail Corridor:</u>	A type of Shared Corridor in which the other transportation systems are other railroads which may include passengers and freight.
<u>Shared Track:</u>	A track designated in the operating rules for the operation of both the high-speed trains and other passenger or freight trains. Shared Track shall have time separation between the hours of operation of freight trains and the high-speed trains (temporal separation). <i>Sometimes referred to as Shared Use Track.</i>
<u>Shop Track:</u>	A designated track in a yard facility used for the maintenance or repair of rolling stock which is under the exclusive control of the Rolling Stock Maintenance employee in charge.
<u>Signal Aspect:</u>	<i>See Aspect.</i>
<u>Signal Block:</u>	<i>See Block.</i>
<u>Signal Indication:</u>	<i>See Indication.</i>
<u>Site Effects/Site Class:</u>	The effect of the subsurface soil/rock profile on the seismic ground motion and as classified in the California Building Code.
<u>Sleeve:</u>	A pipe in which a pipeline or conduit is inserted. <i>Also, called casing.</i>
<u>Slope Failures:</u>	Mapped or otherwise known slope failures such as rock falls, mud flows, debris flows, landslides, and other forms of slope failures.
<u>Slope Stability:</u>	The ability of slopes to resist movement.



**Design Terms**

<u>Slope Value:</u>	Slopes are defined as a fraction indicating the number of units of horizontal length required to achieve 1 unit of vertical distance, i.e., 2H:1V means the slope raises 1 unit vertically for 2 units of horizontal length.
<u>Sound Powered Telephone (SPT):</u>	A telephone system requiring no power, used for first responders in tunnels. Part of the Telephone and Intercom System.
<u>Span Length:</u>	In regard to Overhead Contact System (OCS), the distance between two consecutive OCS support points.
<u>Spiral:</u>	Curve of variable radius used to connect a straight section of track with the radius of the body of the curve. <i>Sometimes called a Transition or a Transition Spiral.</i>
<u>Stagger:</u>	Offset of the contact wire from the projected or super-elevated track centerline at each registration point that causes the contact wire to sweep side to side over the pantograph head during vehicle operation and which helps to distribute wear over the pantograph carbon collector strips.
<u>Standard:</u>	Uniform criteria, methods, processes and practices developed by a regulatory body, agency, industry association, or organizations such as trade unions and trade associations, or other professional affiliations, that represent accepted requirement or a benchmark to measure against.
<u>Static Contact Force:</u>	The mean vertical force exerted upward by the collector head on the overhead contact line, and caused by the pantograph-raising device, while the pantograph is raised and the vehicle is at standstill. <i>See also Contact Force.</i>
<u>Static Gauge:</u>	The maximum outline to which a vehicle may be fabricated. It will include only manufacturing tolerances.

<u>Static Wire:</u>	A wire, usually installed aerially adjacent to or above the catenary conductors and negative feeders, that connects Overhead Contact System (OCS) supports collectively to ground or to the grounded running rails to protect people and installations in case of an electrical fault. In an ac electrification system, the Static Wire forms part of the traction power return circuit and is connected to the running rails at periodic intervals and to the traction power facility ground grids. If mounted aerially, the static wire may also be used to protect the OCS against lightning strikes. <i>Sometimes termed Aerial Ground Wire.</i>
<u>Station:</u>	Areas within a station building envelope. Also, a place designated by name on the station pages of the current Timetable.
<u>Station Intercom:</u>	Intercoms which allow the station attendant to communicate with passengers at fare collection equipment, fare-barrier equipment, or through protective glass. Part of the Telephone and Intercom System.
<u>Station Track / Platform Track:</u>	A track for the purpose of bringing a train alongside a station platform for a stop to embark / disembark passengers.
<u>Steady Arm:</u>	A lightly loaded registration arm that serves to hold or steady the contact wire at its correct lateral displacement/stagger.
<u>Step Voltage:</u>	The difference in surface potential experienced by a person bridging a distance of 1 m (3'-3") with the feet without contacting any ground object.
<u>Structure Gauge:</u>	The outline defining the minimum distance from track centerline to various features.
<u>Stub End:</u>	A track that terminates at one end.
<u>Subballast Layer:</u>	An intermediate layer situated between the ballast and the subgrade layers. It protects the top of the embankment against erosion, ensures a better distribution of loads, and provides a leveled surface suitable for track laying. Subballast is made up of full crushed graduate gravel. This layer is referred to as the Blanket Layer in the International Union of Railways (Union Internationale des Chemins de Fer) (UIC) standards.

**Design Terms**

<u>Subgrade:</u>	The top layer of earthwork upon which the subballast layer rests. On an embankment, the subgrade will be formed of imported soil, whereas in a cut, it will be the naturally occurring soil.
<u>Subsidence:</u>	The gradual downward settlement or sinking of the ground surface.
<u>Subsoil:</u>	Soil of natural ground below subgrade.
<u>Substation (SS):</u>	An electrical installation where power is received at high voltage and transformed to the voltage and characteristics required at the catenary and negative feeders for the nominal 2x25 kV system, containing equipment such as transformers, circuit breakers and sectionalizing switches. It also includes the incoming high voltage lines from the power supply utility.
<u>Subsystem:</u>	<p>A grouping of items satisfying a logical group of functions within a particular system.</p> <p>An element of a system that, in itself may constitute a system.</p> <p>In regard to HST, refers to the major operational part of the high-speed rail system, i.e. infrastructure, rolling stock, train control, electrification, operations, and maintenance.</p>
<u>Superelevation:</u>	The difference in elevation between the outside rail of the curve and the inside rail of the curve measured between the highest point on each rail head. Normally called Cant in European publications.
<u>Supervisory Control and Data Acquisition (SCADA) System:</u>	System which provides centralized control and monitoring of multiple CHST systems.
<u>Switching Station (SWS):</u>	An installation at which electrical energy can be supplied to an adjacent, but normally separated electrical section during contingency power supply conditions. It also acts as a paralleling station.

<u>Switch (Turnout):</u>	<p>The component of a Turnout consisting of switch rails and connecting parts providing a means for making a path over which to transfer rolling stock from one track to another.</p> <ul style="list-style-type: none"><li>• Split Switch: Synonymous with Switch on modern railroads.</li><li>• Secant Point Switch: A switch point in which the arc of the radius of the switch rail or the turnout itself crosses the gauge line of the stock rail. American standard switch rails are Secant Point Switches.</li><li>• Tangent Point Switch: A switch point in which the arc of the radius of the switch rail or the turnout itself matches the gauge line of the stock rail. European and most other turnouts are designed to be Tangent Point Switches.</li></ul>
<u>System:</u>	<p>Grouping of items satisfying a logical group of functions.</p>
<u>System Height:</u>	<p>The vertical distance between the messenger and contact wires, at the support structure. <i>Also known as System Depth.</i></p>
<u>System Safety Engineering:</u>	<p>An engineering discipline that employs specialized professional knowledge and skills in applying scientific and engineering principles, criteria, and techniques to identify and eliminate hazards, in order to reduce the associated mishap risk.</p>
<b>T</b>	
<u>Telephone and Intercom System (TIS):</u>	<p>The system which provides mission critical voice communication functions for Authority personnel, Authority police personnel, third party emergency responders, and passengers.</p>
<u>Tension Length:</u>	<p>Length of a catenary section between its two termination points. <i>Also known as Tension Section.</i></p>
<u>Tensioning Device:</u>	<p>An assembly, typically placed at each end of a tension length, which comprises a balance weight arrangement that is used to maintain near-constant mechanical tension in one or more conductors of an auto-tensioned catenary.</p>

<u>Terminal Control Facility (TCF):</u>	A control center located at terminal stations that will have immediate supervisory oversight over train and passenger operations within each specific terminal. TCF personnel will ensure that appropriate information is relayed to passengers either automatically or manually, and directly manage the station facility and operations on a local level. Actual dispatching of trains will be controlled by mainline dispatchers at the Operations Control Center or Regional Control Centers who will interface closely with TCF personnel.
<u>Terminal Station:</u>	The first or last station of a passenger rail route.
<u>Terminal Storage and Maintenance Facility (TSMF):</u>	A yard facility located near a terminal that provides overnight and mid-day rolling stock storage and Level 1 to 3 maintenance capabilities.
<u>Ties or Sleepers:</u>	Beams placed horizontally and laid perpendicularly to the rail to hold the rails to gauge, distribute the load of the track and equipment to the underlying support, and hold the track in horizontal and vertical alignment. Ties are normally between 8 feet and 8.5 feet long, except those supporting turnouts may be up to 16 feet long. The material normally used in CHST track will be concrete, but ties may be of wood in yard turnouts and certain other special cases.
<u>Time History:</u>	The values of acceleration, velocity, or displacement with time for an earthquake.
<u>Top of Rail:</u>	Refers to the top of the rail on the track which defines the profile elevations of the track. On curves with superelevation, it is the top of the inside rail, also commonly called the top of low rail.
<u>Topographic Map:</u>	A map of the features of the surface of the earth considered collectively as to form.
<u>Touch Voltage:</u>	The potential difference between the ground potential rise (GPR) and the surface potential at the point where a person is standing while at the same time having a hand in contact with a grounded structure (Per IEEE-80).

<u>Trackbed Layers:</u>	<p>General term that includes the material imported for the foundation of the track. It includes the ballast and the following elements when present:</p> <ul style="list-style-type: none"><li>• Subballast layer</li><li>• Prepared subgrade</li><li>• Geosynthetics</li></ul>
<u>Track Centerline:</u>	<p>The line equidistant between the inside faces of the rail heads of a track.</p>
<u>Track Centers:</u>	<p>Distance between adjacent track centerlines.</p>
<u>Track Circuit:</u>	<p>A method of determining occupancy of a section of track and/or a broken rail by sending an electrical signal down the track from the transmit end to the receive end of the section of track, which indicates that the section of track is complete and not occupied by a train by detecting a minimum level of the proper signal at the receive end.</p>
<u>Track Formation Level:</u>	<p>Surface intended to receive the trackbed layers.</p>
<u>Track Foundation:</u>	<p>Constitutes ballast, blanket, and subgrade which is placed/exists below track structure to transmit load to subsoil.</p>
<u>Track Gauge:</u>	<p>Distance between the inner side of the rail heads.</p>
<u>Traction Electrification System (TES):</u>	<p>The combination of the Traction Power Supply System and the Overhead Contact System together with the traction power return system, a Supervisory Control and Data Acquisition system, which forms a fully functional system, and which provides the electrical energy to the electrically powered vehicles on the CHST railway line.</p>
<u>Traction Power Facilities (TPF):</u>	<p>A general term that encompasses substations, switching stations, and paralleling stations.</p>

Traction Power Return System:

All conductors, including the grounding system for the electrified railway tracks, which form the intended path for the traction return current from the wheel-sets of the traction units to the substations under normal operating conditions and the total current under fault conditions. The conductors may be of the following types:

- running rails
- impedance bonds
- static wires, and buried ground or return conductors
- rail and track bonds
- return cables, including all return circuit bonding and grounding interconnections
- earth

and, as a consequence of the configuration of the autotransformer connections, the negative feeders.

Traction Power Supply System (TPS):

The railway electrical distribution network used to provide energy to high-speed electric trains, which comprises three types of traction power facilities in addition to connections to the Overhead Contact System and the Traction Power Return System:

- Substations (SS),
- Switching Stations (SWS), and
- Paralleling Stations (PS)

Traffic Locking:

The enforcement of a single direction of operation in a track section.

Train Control and Communications Room (TCCR):

An equipment room that houses all electronics, power, and networking necessary for the Train Control and communications functions.

Trainset:

A minimum set of rolling stock that can operate in service.

Transition Rate (with distance):

The rate at which superelevation or unbalanced superelevation is placed in track, usually stated as feet per inch, or a ratio.

Transition Rate (with time):

The rate at which superelevation or unbalanced superelevation is experienced with time at a defined speed. Units are normally seconds per inch.

<u>Transition Track:</u>	A designated track connecting the main track to a yard facility designed to allow trains to safely reduce from and accelerate to main track speed. Automatic Train Control (ATC) rules are in effect on transition tracks. Movements will be governed by speed displayed on the Onboard Cab Display, unless the train is in ATC Bypass mode, in which case it will proceed at Restricted Speed.
<u>Transportation Demand Management:</u>	The operation and coordination of various transportation system policies and programs to manage travel demand to make the most efficient and effective use of existing transportation services and facilities.
<u>Transportation System Management:</u>	Actions that improve the operation and coordination transportation services and facilities to realize the most efficient use of the existing transportation system.
<u>Transverse:</u>	A facility passing from one side of the right-of-way to the other side of the right-of-way.
<u>Travel Time:</u>	The time spent on a train from a place of origin to a place of destination.
<u>Trolley Wire:</u>	Alternative term for contact wire used for single wire Overhead Contact System. <i>See Contact Wire.</i>
<u>Tsunamis:</u>	Waves that travel in the open ocean and are caused by an undersea earthquake, landslide, or volcanic activity.
<u>Turnout:</u>	Mechanical installation enabling trains to be guided from one track to another.

## U

<u>Unbalance, Unbalanced Superelevation:</u>	The difference between the Superelevation and Equilibrium Superelevation. In European publications, Unbalance is called Cant Deficiency (if the actual Superelevation is less than the Equilibrium Superelevation) and Excess Cant (if the actual Superelevation is greater than the Equilibrium Superelevation).
<u>Unbalanced Loads:</u>	Loads applied by a 3 phase transmission line that do not have the same load current across each of the 3 phases.



<u>Uninsulated Overlap (or Mechanical Overlap):</u>	A length of the overhead contact system where the contact and messenger wires of two adjoining tension sections overlap before terminating at opposite ends of the overlap section. The two catenaries are jumpered together, thus allowing pantographs under power to transition from one tension length to the next.
<u>Uplift:</u>	The vertical distance by which the overhead contact system is raised during the passage of a pantograph.
<u>Upper Part of Embankment:</u>	Top three feet of an embankment. It requires high quality design and construction in order to ensure the appropriate bearing-capacity to receive trackbed layers.
<u>Unstable Formation:</u>	It is yielding formation with non-terminating settlement including slope failure, which requires excessive maintenance efforts.
<b>V</b>	
<u>Variable Rate Spiral:</u>	<i>See half-sine spiral.</i>
<u>Variance:</u>	Approved deviation, or exception, from a Minimum design criteria or Minimum design standard.
<u>Vertical Curve:</u>	Transition between grades. Normally parabolic in the United States and Asian practices and circular arc radii in European practices.
<u>Virtual Transition:</u>	An imaginary transition imputed as being of the length of the truck centers (TC) of the passenger carrying vehicle operated into the curve or turnout.
<u>Virtual Transition Rate:</u>	The transition rate with time into a curve or turnout with no actual transition, or a transition only to a larger radius, not to infinity.
<u>Vital:</u>	A subsystem, element, component, or functional requirement in a safety critical system that is required to be implemented in a fail-safe manner.
<u>Volcanic:</u>	Mapped or otherwise known volcanic centers and/or hydrothermal activity associated with volcanic activity.

## W

<u>Watershed:</u>	The area that contributes water to a drainage system or stream.
<u>Wayside Drainage:</u>	Drainage system (enclosed pipes, ditches, precast channel) laid to collect and discharge surface water, seepage water, and ground water.
<u>Wayside Facilities:</u>	Facilities in close proximity to the trackway. It is inclusive of traction power, communications, and train control facilities and exclusive of tracks.
<u>Wayside Power:</u>	Electrical power provided from the utility grid to the electrified railroad right-of-way at convenient locations from the side of the rail tracks or corridor. Where utility feeds are not available, wayside power can be supplied by tapping the 25 kV ac parallel negative feeders with appropriate transformation.
<u>Wayside Power Control Cubicle (WPC):</u>	An enclosure for power supply equipment for operation of motorized disconnect switches and the associated Supervisory Control and Data Acquisition equipment located at the wayside.
<u>Wayside Signals:</u>	Devices located along the right-of-way for providing information to the locomotive engineers relative to train operations as opposed to the cab signal displays that are located within the control compartment of the rolling stock.
<u>Wide Area Network (WAN):</u>	Network which consists of the hardware and software required to switch, manage, process, control, monitor, and delivery data traffic between field locations and central control facilities via the fiber optic and copper cable infrastructure. The WAN's purpose is to deliver system data between any points on the wired network in a secure and reliable fashion.
<u>Wireless Local Area Network (WLAN):</u>	WLAN based on the IEEE 802.11 standards. <i>Also known as WiFi.</i>

## Y

<u>Yard:</u>	<p>Inclusive of the following:</p> <ul style="list-style-type: none"><li>• Rolling stock yard where revenue service vehicles are stored and maintained.</li><li>• MOI yard which supports maintenance of trackwork, structures, and other facilities.</li></ul>
--------------	---

---

**Design Terms**

<u>Yard Control Center (YCC):</u>	<i>See Control Center.</i>
<u>Yard Limits:</u>	The tracks governed by the Yard Control Center at a yard facility.
<u>Yardmaster:</u>	The employee responsible for ensuring the coordination and availability of the rolling stock fleet to meet daily service requirements and who has overall responsibility for all activities in the yard facility
<u>Yard Mode:</u>	A mode of operation within yard limits which allows trains to proceed under the manual control of the locomotive engineer at Restricted Speed not exceeding 15 mph. Speed and yard signal compliance will be automatically enforced.
<u>Yard Signal:</u>	A fixed signal within the designated limits of a yard facility that displays either a red or yellow aspect and governs movements within the limits of the yard facility. A yellow aspect indicates that a route is set and locked, and that the section of track between opposing yard signals within which the switches are located is unoccupied.
<u>Yard Signal System:</u>	A means of train control wherein trains are operated in Yard Mode under the control of the locomotive engineer, subject to yard signal indications, speed restrictions and special instructions.
<u>Yard Speed:</u>	Restricted Speed not exceeding 15 mph, within yard limits.
<u>Yard Track:</u>	A section of track used for storage of trains that is auxiliary to the main track and not used by trains that are carrying passengers. Refuge tracks at stations are yard tracks. Yards consist of more than one yard track used for storing trains, inspecting trains, and accessing maintenance facilities. Yard tracks may or may not have track circuits on them.
<u>Yoke Plate:</u>	A plate or casting typically proportioned to accommodate unequal tensions in two or more wires or cables that are terminated on one side and which are balanced by a single terminating cable on the other side, permitting the use of only one balance weight arrangement for multiple catenary conductors.